Honeywell



March 31, 2023

Tyler Abbott
Director, Land and Materials Administration
Maryland Department of the Environment
1800 Washington Boulevard, Suite 645
Baltimore, MD 21230-1719

Re: Performance Management Program, Dundalk Marine Terminal, Baltimore, Maryland

Dear Mr. Abbott:

Honeywell International Inc. (Honeywell) and the Maryland Port Administration (MPA) are pleased to submit the enclosed "Performance Management Program" (PMP), as a component of the Enhanced Isolation and Containment Remedy selected by the Maryland Department of the Environment (MDE) based on the Corrective Measures Alternatives Analysis (CMAA) submitted by MPA and Honeywell pursuant to the April 5, 2006, Consent Decree among MDE, MPA, and Honeywell. This PMP is intended to monitor the performance of the corrective measures implemented at the Dundalk Marine Terminal (DMT) to ensure that chromium ore processing residue is being contained and that the implemented remedy remains protective of human health and the environment.

The PMP consists of five components: (1) a Priority Storm Drain Inspection and Maintenance Plan; (2) a Surface Cover Inspection and Maintenance Plan; (3) a Sentinel Groundwater Monitoring Plan; (4) a Site Drinking Water Plan; and (5) a Health and Safety Plan. These five plans have been modified from interim plans or newly developed to identify actions to monitor and maintain the effectiveness of the implemented corrective measures, establish reporting mechanisms to document remedy component effectiveness, and establish criteria for any additional corrective measures that may be recommended based on monitoring data.

The Sentinel Groundwater Monitoring Plan includes a proposal by MPA to include seven additional wells. The existing groundwater monitoring well network is adequate for continued monitoring of all zones. However, MPA proposes to install six new, shallow aquifer wells focused on the southern portion of the site east of Area 1501/1602 to provide more data regarding groundwater flow in the area, and one new, shallow aquifer well in the northern portion of the site to provide groundwater flow and quality data. If MDE believes these additional wells are necessary, the parties will add them to the monitoring program.

We would like to meet with MDE at your earliest convenience to discuss the PMP.

We appreciate MDE's guidance and assistance in implementing the remedy at DMT. If you have any questions or require additional information, please do not hesitate to contact us.

Sincerely,

Benny Dehghi

Vice President, Global Remediation and Redevelopment

Honeywell

Copies: Mr. Matthew Zimmerman—MDE

Ms. Irena Rybak-MDE

Ms. Barbara Krupiarz—MDE

Mr. Michael Daneker-Arnold & Porter

Ms. Margaret Witherup-Witherup Allen Law

Mr. Chuck Anthony-Honeywell

Mr. Phil Whaling—MPA

Bill Richardson

Bill Richardson, Director Office of Environment Maryland Port Administration

Performance Management Program

Dundalk Marine Terminal Baltimore, Maryland

Prepared for



115 Tabor Road Morris Plains, New Jersey 07950

and



Maryland Port Administration 401 East Pratt St. Baltimore, Maryland

Prepared by

Jacobs

Jacobs Engineering 2411 Dulles Corner Park Suite #500 Herndon, VA 20171

March 2023



Contents

Acro	nyms a	nd Abbreviations	iii	
1.	Exec	utive Summary	1-1	
2.	Back	groundground	2-1	
3.	Cond	ceptual Site Model Update	3-1	
	3.1	Air Transport	3-1	
	3.2	Groundwater	3-1	
4.	PMP	Elements	4-1	
	4.1	Program Elements	4-1	
	4.2	Deliverables	4-1	
5.	PMP	Requirements	5-1	
	5.1	Storm Drain Inspection and Maintenance	5-1	
		5.1.1 Storm Drain Inspection	5-1	
		5.1.2 Storm Drain Maintenance	5-2	
	5.2	Surface Cover Inspection and Maintenance	5-2	
		5.2.1 Surface Cover Inspection	5-2	
		5.2.2 Surface Cover Maintenance	5-2	
		5.2.3 Strain Relief Measures Monitoring	5-2	
	5.3	Sentinel Groundwater and Sediment Monitoring and Sampling Plan	5-3	
	5.4	Site Drinking Water Plan		
	5.5	Master Health and Safety Plan	5-4	
6.	Wast	ewater Treatment Plant	6-1	
7.	Refe	rences	7-1	
Appe	endixes			
A B C D E F G	Surface Cover Inspection and Maintenance Plan Sentinel Groundwater Monitoring Plan Site Drinking Water Plan Maryland Port Administration Health and Safety Plan DMT NPDES Permit			

Figures

- 1 2 COPR Fill Map Priority Storm Drain Map



Acronyms and Abbreviations

CMAA Corrective Measures Alternatives Analysis

COPR chromium ore processing residue

CSM conceptual site model

DMT Dundalk Marine Terminal

HASP Health and safety plan

MDE Maryland Department of the Environment

mg/L milligram(s) per liter

MPA Maryland Port Administration

MS4 municipal separate storm sewer system; NPDES General Permit for Discharges from

State and Federal Small Separate Storm Sewer Systems

NPDES National Pollutant Discharge Elimination System

PMP Performance Management Program

SCMP Surface Cover and 14th and 15th Streets Storm Drain Inspection and Maintenance

Plan

SRT Strain relief trench
USZ Upper Saturated Zone

WWTP wastewater treatment plant



1. Executive Summary

This Performance Management Program (PMP) is intended to monitor the performance of the corrective measures implemented at the Dundalk Marine Terminal (DMT) to ensure that chromium ore processing residue (COPR) is being contained and that the implemented remedy remains protective of human health and the environment. The PMP is a component of the Enhanced Isolation and Containment Remedy selected by the Maryland Department of the Environment (MDE) on July 30, 2012, based on the Corrective Measures Alternatives Analysis (CMAA) submitted by CH2M on behalf of the Maryland Port Administration (MPA) and Honeywell International Inc. (Honeywell) in January 2011 (CH2M 2011).

The PMP builds on the remedial measures already implemented at the DMT and consists of five separate program elements:

- (1) A *Priority Storm Drain Inspection and Maintenance Plan* (Appendix A) to monitor the effectiveness of and maintain the lined storm drains, manholes, inlets, and associated infrastructure.
- (2) A Surface Cover Inspection and Maintenance Plan (Appendix B) to maintain the cover system over the COPR fill area and monitor the subsurface effects of COPR movement.
- (3) A Sentinel Groundwater Monitoring Plan (Appendix C) to monitor groundwater quality outside of the COPR fill area, monitor groundwater elevation, and flow directions, and confirm sediment and surface water reducing conditions.
- (4) A Site Drinking Water Sampling Plan (Appendix D).
- (5) A Master Health and Safety Plan (Appendix E).

Collectively, these five plans identify actions to monitor and maintain the effectiveness of the implemented corrective measures, establish reporting mechanisms to document remedy component effectiveness, establish policies and procedures, and set triggers and criteria for any additional corrective measures that may be recommended based on monitoring data.



2. Background

The DMT contains about 148 acres of COPR fill under pavement (Figure 1). MPA and Honeywell have been working in partnership to investigate and address environmental conditions caused by the presence of COPR at the DMT. In accordance with the 2006 Consent Decree, MPA and Honeywell completed thorough investigations of soil, groundwater, stormwater, and COPR at the DMT, as well as sediment and water in the Patapsco River. Completed work includes the following:

- Delineation of the extent and nature of COPR present within the 148-acre fill area at DMT.
- Evaluation of the mobility, fate, and transport of hexavalent chromium in environmental media to develop measures to quantify and reduce the flux of hexavalent chromium in stormwater.
- Determination of the presence and environmental significance of COPR-related constituents in the surface water storm drain system within the COPR fill area.
- Development of an understanding of the mechanisms associated with the expansion of COPR fill
 which was then utilized to mitigate the detrimental effects of COPR expansion on the existing surface
 cover and storm drain systems.

Based on the results of those investigations and pilot studies, MPA and Honeywell developed and submitted to MDE a CMAA dated January 2011 (CH2M 2011) evaluating five remedial alternatives for the DMT. These alternatives included a containment alternative, a full excavation alternative, as required by the Consent Decree, and a "No Further Action" alternative, as required under state law. The remedial alternatives were screened against the criteria specified in the Consent Decree, including protection of human health and the environment and potential impact on port operations. After accepting public comment on the CMAA, MDE selected Alternative 3, Enhanced Isolation and Containment, as the remedy for the DMT.

This PMP is a key element for ensuring attainment and management of remedial performance objectives. MPA's health and safety plan will continue to provide the overarching guidelines for safe implementation of the inspection and maintenance programs, including air monitoring during COPR excavations, excavation procedures and management of waste. This document presents the comprehensive framework for maintaining performance and imposes rigorous controls for long-term protection and maintenance of the COPR containment remedy. The PMP details the approach for performing priority storm drain and surface cover inspections, documenting inspection, and maintenance events, and routinely reporting information related to the containment remedy to the MDE. The PMP also includes a Sentinel Groundwater Monitoring Plan for the sampling and monitoring of groundwater quality. Lastly, a comprehensive Master Health and Safety Plan will remain in place for the protection of port users.



3. Conceptual Site Model Update

A key component of the PMP is fulfilling the requirement to update the conceptual site model (CSM) to identify changes that could potentially warrant evaluation of additional corrective measures and ensure the monitoring programs are aligned with current site conditions. The *Chromium Transport Study* (CH2M 2009b) documented the original CSM, which addressed various chromium migration pathways at the DMT. The goal of the PMP is to address the current site conditions as determined through observed changes in the CSM to date. The original CSM has remained relatively unchanged with a few exceptions, as noted below by media.

3.1 Air Transport

The 2009 CSM identified chromium transport through air as a potential incomplete pathway; however, MDE required perimeter air monitoring for hexavalent chromium (Cr(VI)) to document concentrations of Cr(VI) in airborne particulates at the DMT perimeter. In the 15 years of monitoring since, all Cr(VI) concentrations detected in the perimeter air have been significantly below the occupational exposure limits for Cr(VI) using the Occupational Safety and Health Administration Permissible Exposure Limit of 5,000 nanograms per cubic meter based on an 8-hour time-weighted average. The air monitoring program has confirmed the airborne migration of Cr(VI) from COPR is an incomplete pathway, and a request to discontinue the air monitoring program has been submitted to MDE.

3.2 Groundwater

An increase of groundwater elevations has been recorded in the COPR area from 2009 to present at locations near lined storm drains. The rise in groundwater elevation seems consistent with simulated rise conditions evaluated in the hydraulic model (CH2M 2009b) presented in the CMAA (CH2M 2011). This rise in groundwater is in response to the elimination of groundwater flux into the storm drains due to the lining program. The groundwater elevation conditions will continue to be monitored against the hydraulic model (CH2M 2009b) as part of the PMP.

The Area 1501/1602 COPR cell experienced elevated groundwater conditions before the current construction activities. In 2014, dewatering of the cell was initiated to relieve hydrostatic pressure within the confined cell by lowering the level of groundwater at the southern corner of Area 1501/1602. Additionally, a new geomembrane and asphalt cap was designed and is currently being installed to reduce recharge from surface water infiltration.

The evaluation of the site groundwater model in 2015 identified an additional water-bearing unit east of the 1501/1602 COPR cell and defined as the Upper Saturated Zone (USZ). The USZ occurs east of the 1501/1602 area where groundwater was encountered above the level of the piezometric surface in the underlying shallow aquifer. Due to the lack of lateral continuity, the zone is not considered an aquifer but acts to transmit infiltrated surface water downward to the shallow aquifer. The USZ is discussed further in the Sentinel Groundwater Monitoring Plan (Appendix C).

The rise in groundwater elevation has caused Cr(VI)-impacted groundwater to enter compromised sections of the subsurface utility system and transfer through electrical and communication conduits. Groundwater fluctuations, periodic dewatering operations, and surface water infiltration during rain events sometimes cause the Cr(VI)-impacted water to move into manhole vaults both inside and outside the COPR fill area. During rain events, a limited number of utility manholes experience overflow conditions. The manholes have been identified, and engineering controls are in place to prevent movement of Cr(VI)-impacted water beyond the immediate vicinity of the vaults. Remedial measures are currently being implemented to eliminate the movement of Cr(VI)-impacted water throughout the conduit system and reduce infiltration into the system. The parties will work with MDE to address these measures, which are outside the purview of the PMP.



4. PMP Elements

4.1 Program Elements

The PMP consists of five separate program elements:

- (1) A Storm Drain Inspection and Maintenance Plan (Appendix A) to monitor the effectiveness of and maintain the lined storm drains, manholes, inlets, and associated infrastructure.
- (2) A Surface Cover Inspection and Maintenance Plan (Appendix B) to maintain the cover system over the COPR fill area and monitor the subsurface effects of COPR movement.
- (3) A Sentinel Groundwater Monitoring Plan (Appendix C) to monitor groundwater quality outside of the COPR fill area, monitor groundwater elevation, and flow directions, and confirm sediment and surface water reducing conditions.
- (4) Site Drinking Water Sampling Plan (Appendix D).
- (5) Master Health and Safety Plan (Appendix E).

Collectively, these five plans identify actions to monitor and maintain the effectiveness of the implemented corrective measures, establish reporting mechanisms to document remedy component effectiveness, and set triggers and criteria for any additional corrective measures recommended based on monitoring data. Health and safety protocols continue to be subjected to the MPA's health and safety plan.

4.2 Deliverables

PMP deliverables to the MDE are listed below. The electronic databases for surface cover and storm drain maintenance, and groundwater, sediment, and surface water sample results are updated on an ongoing basis to document remedy maintenance and monitoring activities.

PMP Requirement	Report Name	Frequency
Storm drain inspection and maintenance	Storm Drain Inspection and Maintenance Report	Annual
Surface cover inspection and maintenance plan	Surface Cover Inspection Report	Annual
Surface cover inspection and maintenance plan	Surface Cover Repairs Summary Report	Annual
Sentinel groundwater sampling	Semiannual Groundwater Monitoring Report	Semiannual
Sediment sampling	Sediment Sampling Report	5-year intervals
Drinking water plan	Site Drinking Water Report	Quarterly



5. PMP Requirements

The Enhanced Isolation and Containment Remedy requires the repair and lining of priority storm drains within the COPR fill area to address the infiltration of groundwater and eliminate the discharge of chromium-impacted water to the Patapsco River. The priority storm drains include the 12th, 12.5th, 13th, 13.5th, 14th, and 15th Streets storm drains (Figure 2). After the drains are lined, routine inspection and repairs are necessary to confirm the effectiveness of the lining. Monitoring of these storm drains is performed pursuant to the National Pollutant Discharge Elimination (NPDES) permit, State Discharge No. 10-DP-3060, and NPDES Permit No. MD0066818, which was issued by the MDE to the MPA in August 2017. The permit renewal application was submitted to MDE on September 30, 2021. The NPDES permit is provided as Appendix F.

5.1 Storm Drain Inspection and Maintenance

Once a storm drain(s) successfully meets the NPDES monitoring requirements per the above permit, it may be removed from the NPDES permit (No. MD0066818) through a major modification. The storm drain then becomes part of the MPA's NPDES General Permit for Discharges from State and Federal Small Separate Storm Sewer Systems (MS4) program. The MS4 permit, General Discharge Permit No. 13-SF-5501, General NPDES No. MDR055501, was issued to MPA on October 31, 2018. A copy of the General Discharge Permit is included as Appendix G. Inspection and maintenance of the priority storm drains will continue to be performed pursuant to the PMP requirements described herein and the MS4 permit.

The six priority storm drains were lined to isolate the drains from COPR and prevent infiltration of chromium-impacted groundwater into the priority storm drain system. Four of the priority drains (12th, 12.5th, 13th, and 13.5th Streets storm drains) were also isolated from tidal influence by the installation of tidal exclusion vaults equipped with mechanical devices such as flap gates or duckbills. The tidal exclusion vaults provide safe and reliable access for conducting storm drain inspections and maintenance. The 14th and 15th Streets storm drains have enclosed outfall structures that were used to collect chromium-impacted dry weather flows prior to rehabilitation and lining, which was conveyed to the onsite wastewater treatment plant (WWTP). The WWTP is also regulated under the NPDES permit MD0066818.

5.1.1 Storm Drain Inspection

The first element of the PMP is the establishment of the *Priority Storm Drain Inspection and Maintenance Plan* (Appendix A). This plan includes the procedures for inspection, maintenance, documentation, and reporting for the priority storm drains. The lined storm drains will be subject to Tier I or Tier II inspections, as described below. Together, the tiered programs assess the condition of the liners and host structures and will result in the development and scheduling of storm drain maintenance activities.

The Tier I inspections use an observational approach, which includes a semiannual visual inspection at the storm drain trunk line structures to observe potential groundwater intrusion sources. If dry weather flow is observed during routine DMT operations or a Tier I inspection, then a partial Tier II inspection will be performed. Maintenance and repairs of the priority storm drain will be conducted, as required, in response to those inspection observations.

Tier II inspections will be completed on a 5-year rotational basis and include detailed visual inspections and CCTV (closed circuit television) footage of approximately 5,000 linear feet of storm drain per year, with the inspections including associated inlets, manholes, and lateral piping. Tier II inspections may also include pipe diameter measurements and or laser profiling to monitor pipe deformation due to COPR movement depending on visual observations made during the CCTV inspection. The inspection frequency for complete Tier II inspections may need to be adjusted in the future, depending on liner reliability as determined from the ongoing inspections of the storm drains. The basis for performing 5,000 linear feet each year is that the extent of such inspection represents approximately one-fifth of the total priority storm drain system. At a minimum, Tier II inspections will be performed on a 5-year rotational



basis but may be performed more frequently based on the findings of Tier I inspections and pipe deformation monitoring.

5.1.2 Storm Drain Maintenance

Upon discovery of a condition requiring repair to the liner system, liner repairs will be completed within 45 calendar days unless circumstances are identified that will require additional time for completion. Conditions requiring structural repair to the host pipe or inlet and manhole structures may require preparation of design drawings and specifications and may take longer than 45 days to complete. Reasonable means to expedite repairs will be made to effectively complete the repairs following the inspections.

5.2 Surface Cover Inspection and Maintenance

The development and implementation of a surface cover inspection and maintenance plan to maintain the existing pavement within the COPR fill area is the second component of the PMP. Current inspections are performed pursuant to the *Surface Cover and 14th and 15th Streets Storm Drain Inspection and Maintenance Plan* (CH2M 2007a) (SCMP), which was approved by MDE on August 28, 2007. The plan defined the methods and procedures for performing routine inspections, maintenance, and repair of surface cover materials and two of the priority storm drains affected by COPR fill at the DMT.

Inspection of the surface cover system within the COPR fill area is performed annually to identify and prioritize surface cover repairs to be performed during a calendar year. The *Surface Cover Inspection and Maintenance Plan* is provided as Appendix B and will replace the 2007 SCMP.

5.2.1 Surface Cover Inspection

The baseline surface cover inspection was conducted between March 5 and April 20, 2007. The baseline inspection included the visual inspection of existing pavements at DMT and mapping of cracks, holes, and surface manifestation of subsurface COPR expansion. The documentation of these areas using geographical information system—based maps and tables, was used to prioritize required maintenance measures. The *Surface Cover System Baseline Inspection Report* was submitted to MDE on September 12, 2007 (CH2M 2007b).

The annual surface cover system inspection reports identify specific areas for surface cover maintenance to be performed within the COPR fill area. Surface cover inspection data are mapped using a global positioning system (GPS)—enabled iPad with the ArcGIS Collector application and will be upgraded with technology advances. This electronic database will continue to be used for documenting surface cover inspections. Inspection findings and an overview of repair priorities will be documented in an annual report each calendar year.

5.2.2 Surface Cover Maintenance

Features identified for repair are located and marked in the field using a GPS-enabled iPad with the ArcGIS Collector application. A surface cover database is maintained to record inspection results and surface cover repairs. Surface cover repairs are prioritized based on the containment remedy and potential impact to port operations. Details of completed crack sealing and paving repairs (including pothole, full section mill and pave, crack area, heave, and miscellaneous repairs) are reported annually. The information in the database will be used to document completed surface cover inspection and maintenance activities, and to evaluate the condition of the existing surface cover at DMT. A summary of completed repairs and an overview of repair types will be documented in the annual report each calendar year.

5.2.3 Strain Relief Measures Monitoring

A heave investigation and minimization study was completed in 2009 to characterize the nature of manifestations of surface and subsurface heaving and expansion processes associated with the COPR



fill and to identify potential means to prevent or minimize surface pavement damage. The *Heave Investigation and Minimization Study Work Plan* (CH2M 2006) defined the scope of the investigation, provided a summary of background information and findings from previous studies, specified the methods and procedures to be used to analyze and characterize COPR heave processes, and specified the data quality objectives, quality assurance, quality control and reporting requirements, as appropriate. Additionally, inclinometers, Shape Accel Arrays, and surface survey pins (pk nails) were installed to monitor COPR horizontal and vertical movement.

Subsurface strain relief trenches (SRTs) consisting of a 2- to 3-foot-wide trenches backfilled with displaceable fill have been installed to minimize surface heave in Area 1800, protect the 15th Street storm drain piping from deformation, and protect the integrity of the clay containment dike in Area 1501/1602. COPR movement data collected from inclinometers, Shape Accel Arrays, and pk nails installed adjacent to the SRTs, along with visual observations of the displaceable SRT backfill and probing of the backfill for COPR impingement will be used to determine whether and where COPR may be laterally expanding into the trench. The data will also be used to determine the rate of COPR movement and estimate the remaining life of the trench. Repairs and maintenance will be designed and implemented to refurbish portions of the trench as needed. Storm drain pipe deformation monitoring data collected as part of the Tier II storm drain inspection described in the *Priority Storm Drain Inspection and Maintenance Plan* (Appendix A) will also be used to identify portions of the SRT that are no longer protecting storm drain piping from COPR expansion, or sections of piping where new SRTs are needed.

5.3 Sentinel Groundwater and Sediment Monitoring and Sampling Plan

Semiannual groundwater sampling has been performed under the *Interim Groundwater-Sampling Plan* (CH2M 2009a). This interim sampling plan was submitted to MDE on April 20, 2009, with subsequent revisions provided to MDE in June 2009. In February 2015, due to the abandonment of seven of the originally prescribed sampling monitoring well locations, MDE approved the sampling of seven replacement wells, chosen from among the existing well network. The interim sampling program superseded the previous monitoring program and was initiated to use the monitoring well network established during the chromium transport study investigation (CH2M 2009b).

The next element of the PMP is the establishment of a groundwater monitoring network to monitor groundwater quality outside of the COPR fill area. This plan will also confirm sediment and porewater reducing conditions beneath the river. A comprehensive groundwater monitoring plan, referred to as the *Sentinel Groundwater Monitoring Plan* (Appendix C), will include components of the interim groundwater monitoring plan, as well as several new elements for assuring site containment. The approach for establishing the sentinel monitoring system was based on an evaluation of the existing network of monitoring wells installed at the site.

Existing wells were evaluated to determine which locations were suitable for continued use as part of a sentinel monitoring program. The wells deemed unsuitable or duplicative for monitoring have been or will be abandoned. The *Sentinel Groundwater Monitoring Plan*, which contains the sitewide sentinel monitoring well network, and sediment and porewater sampling, is provided as Appendix C. This plan replaces the *Groundwater Sampling and Analysis Plan* (Maryland Environmental Service 1992a), which was prepared to comply with the 1992 *Corrective Measures Implementation Program Plan* (Maryland Environmental Service 1992b) as well as the *Interim Groundwater-Sampling Plan* (CH2M 2009a).

5.4 Site Drinking Water Plan

A site drinking water monitoring plan was submitted to MDE in 2006, and a revised plan was submitted in 2013. The purpose of this plan is to ensure that chromium-contaminated materials are not adversely impacting the drinking water at the DMT. The plan contains two sections. The first section describes a sampling plan to determine existing water system baseline conditions in relation to chromium's presence or absence and establishes a routine sampling program to test for chromium in the water distribution system. The second section provides a contingency plan for operation of the water system in the event of a pipe break, which includes the return of the system to normal operation after the break has been repaired. This revised *Site Drinking Water Plan* will replace previous versions and is provided in Appendix D.



5.5 Master Health and Safety Plan

MPA's health and safety plan (Appendix E) will continue to provide the overarching guidelines for safe implementation of the inspection and maintenance programs, inclusive of air monitoring during COPR excavations, excavation procedures and management of waste. MPA has developed the Master Health and Safety Plan (HASP) to provide both general and specific requirements regarding potential exposure to chromium and COPR at DMT. The plan outlines the responsibilities of all parties operating on this site and requires coordination of all health and safety plans to ensure compliance with state regulations and other applicable regulations. The Master HASP is intended to reduce the potential of health and safety hazards to personnel performing chromium-related or potentially chromium-related work projects at the DMT.



6. Wastewater Treatment Plant

Following completion of the 14th and 15th Streets storm drain lining and implementation of the action level monitoring in accordance with NPDES Permit Number MD0066818 (State Discharge Permit No. 10-DP-3060), the existing WWTP will no longer be needed to capture and treat dry weather flows from these priority storm drains. The WWTP may remain in service for a minimum period of 2 years, the period required to demonstrate action level monitoring requirements at the 12th,14th, and 15th Street storm drains have been met and a NPDES major permit modification has been issued. The WWTP may continue to operate beyond this time period under a revised operations plan to treat chromium-impacted water from other sources at DMT, or it may be decided that the water can be taken offsite and treated at an approved wastewater treatment disposal facility.

The other sources of chromium-impacted water being treated at the plant currently include water generated from site maintenance and construction projects and groundwater pumped from well EA-7S. Pumping of well EA-7S was initiated in 2014 as a temporary measure to lower the water level inside the COPR cell at the southern corner of Area 1501/1602. A shallow interceptor trench was installed to a maximum depth of 4 feet to collect groundwater moving along the top of the clay cap and between other gravel-like, porous layers below the original Area 1501/1602. The trench has several extraction points used for vacuum removal of any collected groundwater. Area 1501/1602 has since been capped, starting in 2021, to prevent surface water infiltration, and it is anticipated that pumping groundwater from within the COPR cell will no longer be required after the water level within the cell has stabilized. As part of the Area 1501/1602 cap work, well EW-1 (Extraction Well 1) was installed within the COPR cell as a highly conservative and precautionary measure to give Honeywell and MPA the option to control groundwater levels in the Area 1501/1602 COPR cell in the future, if needed.



7. References

CH2M. 2006. Heave Investigation and Minimization Study Work Plan. August.

CH2M. 2007a. Surface Cover and 14th and 15th Street Storm Drain Inspection and Maintenance Plan. August.

CH2M. 2007b. Surface Cover System Baseline Inspection Report. September.

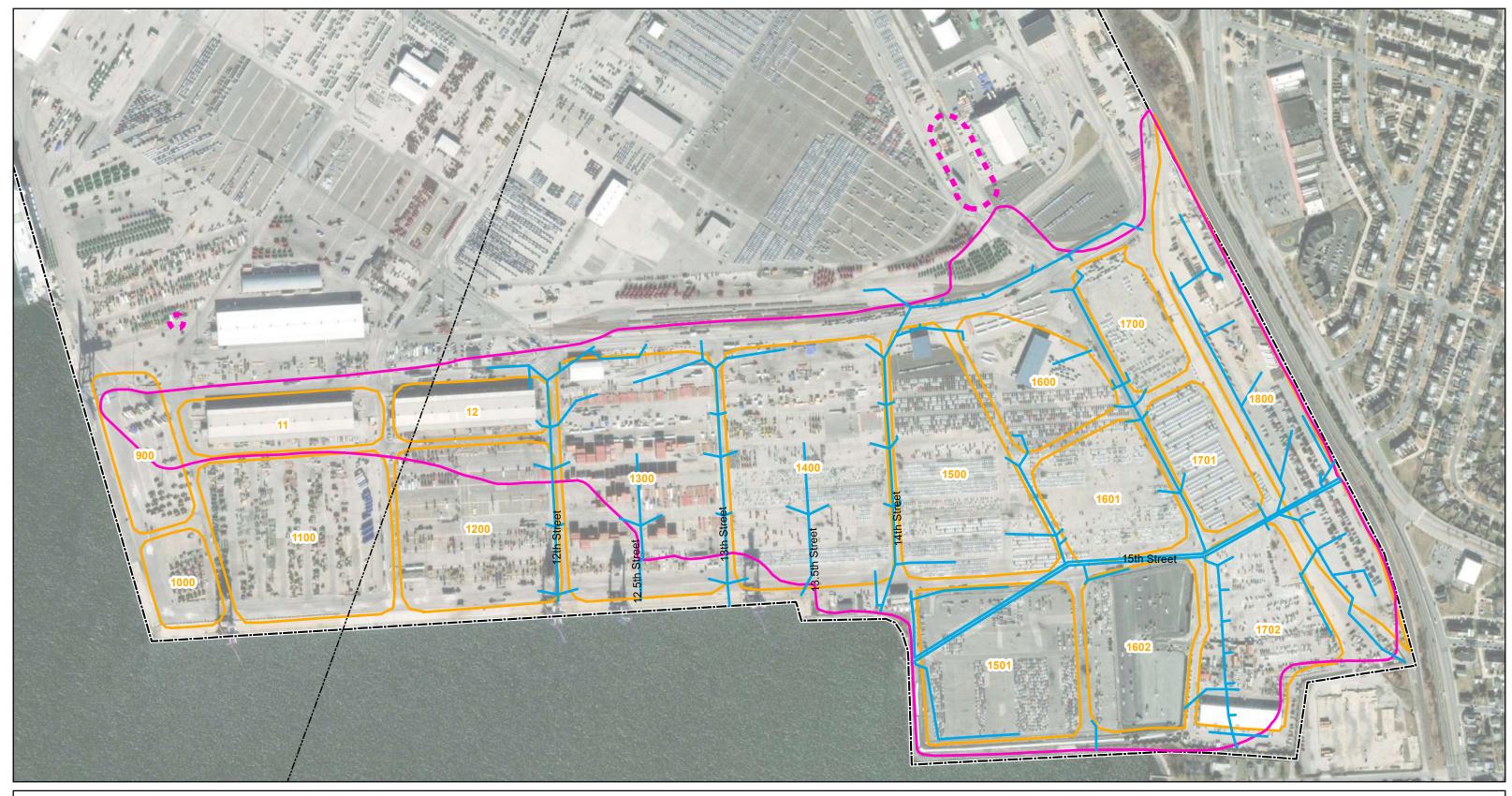
CH2M. 2011. Corrective Measures Alternatives Analysis, Dundalk Marine Terminal, Baltimore, Maryland. January.

CH2M. 2009a. Interim Groundwater-Sampling Plan, Dundalk Marine Terminal, Baltimore, Maryland. April.

CH2M. 2009b. Chromium Transport Study Report, Dundalk Marine Terminal, Baltimore, Maryland. October.

Maryland Environmental Service. 1992a. Groundwater Sampling and Analysis Plan.

Maryland Environmental Service. 1992b. Corrective Measures Implementation and Program Plan.

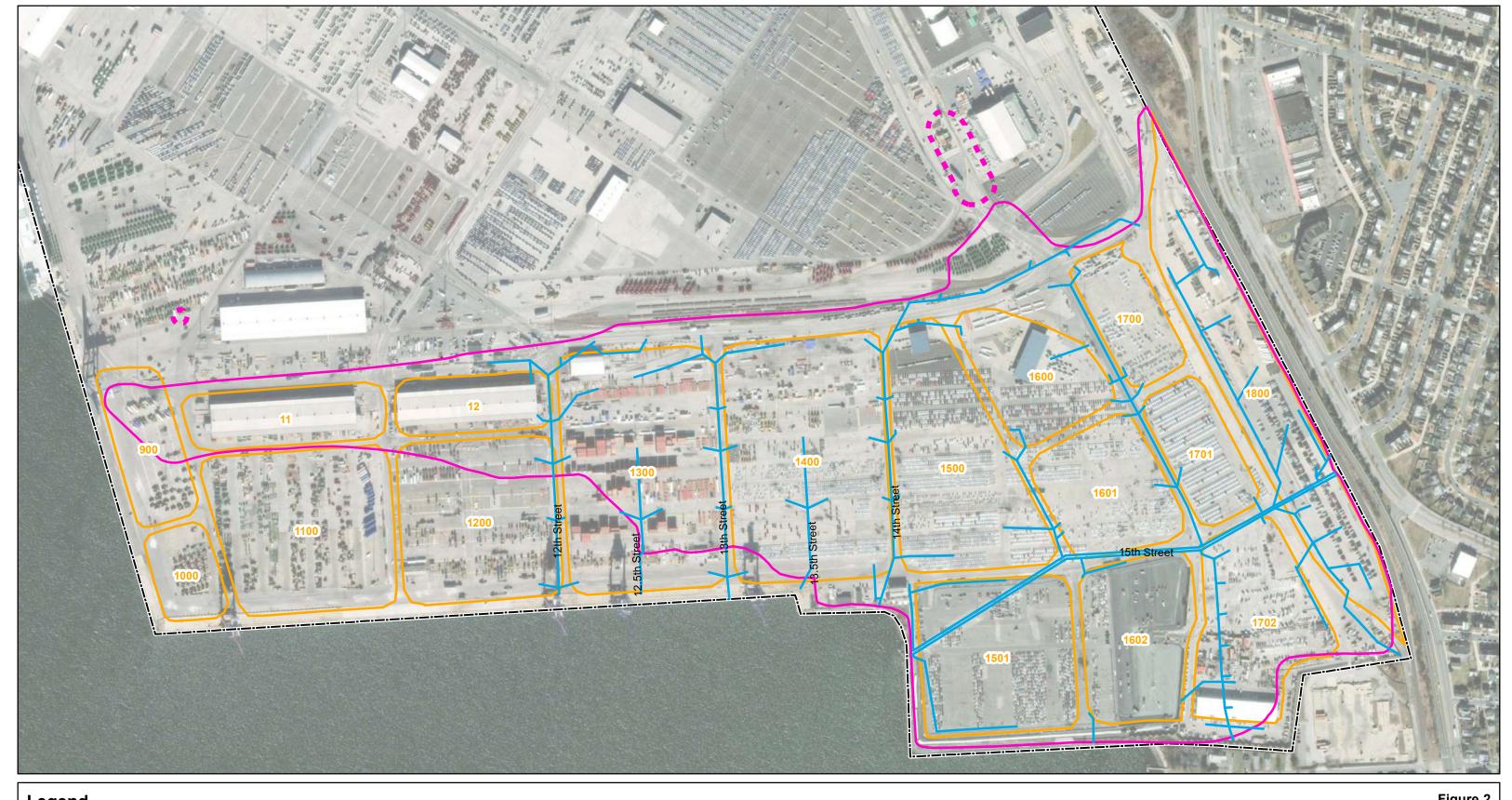


Legend

- Priority Storm Drain System
- ---- City/County Boundary
- Approximate COPR Extent
- COPR Extent
- Areas
- DMT Boundary

Figure 1 COPR Fill Area Dundalk Marine Terminal Baltimore, Maryland





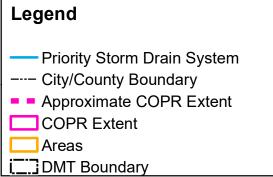
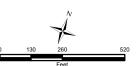


Figure 2 Priority Storm Drains Dundalk Marine Terminal Baltimore, Maryland



Appendix A
Priority Storm Drain Inspection and
Maintenance Plan

Performance Management Program Priority Storm Drain Inspection and Maintenance Plan

Dundalk Marine Terminal Baltimore, Maryland

Prepared for



115 Tabor Road Morris Plains, New Jersey 07950

and



Maryland Port Administration 401 East Pratt St. Baltimore, Maryland

Prepared by

Jacobs

Jacobs Engineering 2411 Dulles Corner Park Suite #500 Herndon, VA 20171

January 2023



Contents

1.	Intro	ductionduction	1
2.	Back	ground	2
3.	Storr	m Drain System Inspection Plan	3
	3.1	Scope	
	3.2	Tiered Inspections	3
	3.3	Dewatering and Cleaning	4
	3.4	Inspection Procedures	4
		3.4.1 Tier I Inspections	4
		3.4.2 Tier II Inspections	4
	3.5	Geographic Information System	6
	3.6	Field Documentation	6
4.	Storr	m Drain Maintenance	7
	4.1	Maintenance	
	4.2	Stormwater Management During Repairs	
	4.3	Field Documentation	7
5.	Storr	m Drain Deformation Monitoring	8
	5.1	Purpose	
	5.2	Laser Profiling in Storm Drains	
	5.3	Manual Measurement in Storm Drains	
	5.4	Monitored Pipe Sections	9
6.	Repo	orting	10

Appendixes

- Tier I Inspection Forms Α
- В Tier II Inspection Forms

Tables

Tentative Tier II Inspection Schedule

Figures

- 1 COPR Fill Area
- 2
- 12th Street Storm Drain System 12.5th Street Storm Drain System 3
- 4 13th Street Storm Drain System
- 13.5th Street Storm Drain System 5
- 6 14th Street Storm Drain System
- 7 15th Street Storm Drain System



1. Introduction

This Priority Storm Drain Inspection and Maintenance Plan is an integrated component of the overall Performance Management Program (PMP) for the long-term monitoring and maintenance of the remedy being implemented at Dundalk Marine Terminal (DMT).

Pursuant to the Consent Decree among the Maryland Department of the Environment (MDE), the Maryland Port Administration (MPA), and Honeywell International Inc. (Honeywell), and the Corrective Measures Alternatives Analysis required by the Consent Decree, MDE has approved an Enhanced Isolation and Containment remedy for the site. The Enhanced Isolation and Containment remedy includes a requirement to repair and line priority storm drains within the chromium ore processing residue (COPR) fill area to address the infiltration of and discharge of chromium-impacted water and a PMP to confirm and monitor the effectiveness of that relining program. This Priority Storm Drain Inspection and Maintenance Plan fulfills the PMP requirements with respect to the storm drain portion of the Enhanced Isolation and Containment remedy.

The priority storm drains are the 12th, 12.5th, 13th, 13.5th, 14th, and 15th Street storm drains. Monitoring of these storm drains is performed pursuant to the National Pollutant Discharge Elimination (NPDES) permit No. MD006818, which was issued by MDE for DMT in August 2017. Once a storm drain successfully meets the NPDES monitoring requirements, it may be removed from the NPDES permit through a major modification. Any storm drains removed from the NPDES permit will continue to be covered under the MPA's Municipal Separate Storm Sewer System (MS4) permit No. MDR 055501, which governs all other storm drains at DMT. For the priority storm drains, the MS4 inspections will be performed by MPA independent of the PMP inspections performed according to this plan.

This plan comprises the following sections:

- 1) **Background**—summarizes general background information related to the DMT.
- 2) **Storm Drain Inspections**—details the Tier I and Tier II inspections procedures, dewatering and cleaning requirements required prior inspection (if necessary), and the documentation and reporting of inspection findings.
- 3) Storm Drain Maintenance and Reporting—details the maintenance schedule for storm drain repair, requirements for stormwater management, and the documentation and reporting of completed repairs.
- 4) Storm Drain Deformation Monitoring—includes assessment of collected data to determine whether lateral COPR expansion may be impacting the pipe and causing deformation of the pipe wall. The data are further evaluated to assess the condition of the pipe and the effectiveness of existing strain relief, or need for new strain relief measures.

This plan describes the procedures for inspection, maintenance, documentation, and reporting with respect to the six priority storm drains. Information obtained from this program is considered, along with information from the program elements of the PMP, to evaluate the overall effectiveness of the Enhanced Isolation and Containment remedy. These storm drain inspections, as described in Section 3, are performed as either a Tier I (visual) or a Tier II (more detailed) inspection.



2. Background

The priority storm drain systems are constructed partially or fully in the COPR fill area at the DMT, as illustrated in Figure 1. Before being lined, the priority storm drains exhibited concentrations of hexavalent chromium in dry-weather flow resulting from groundwater intrusion into sections of the storm drains. The 12th Street, 12.5th Street, 13th Street, and 13.5th Street storm drains all have outfalls located at the steel bulkhead wharf that is covered by a 60-foot marine platform. Storm drains located along 14th and 15th Streets are also priority drains, with outfall basins located along the armored shoreline embankment, and are not obstructed by a marine platform. The 15th Street drain is unique in that it accepts both dryweather and wet-weather flow from the upgradient communities across Broening Highway.

The total length of the priority storm drains is approximately 23,400 feet. The pipe sizes for the priority drains range from 15 inches to 96 inches, with pipe shapes either round or elliptical. Generally, the catch basins farthest from the main trunk lines have the smallest pipe sizes leaving the basin. Pipe size generally increases in each basin until a main trunk line is intersected. The main trunk lines of the storm drains are accessible via manholes by personnel for visual inspection. Smaller-diameter storm drain piping less than 48 inches in diameter, particularly lateral piping and trunk line piping in the upper reaches of the storm drain, are generally inspected remotely using industry standard, robotic closed-circuit television (CCTV) equipment.

As originally designed and constructed, each priority storm drain outfall invert is located below the level of low tide of the Patapsco River. Accordingly, the priority storm drains were subject to tidal inundation that inhibited storm drain dewatering so that inspections, reliable sampling, and repairs could not be performed. As a result, tidal exclusion infrastructures (that is, vaults with backflow prevention) were installed at each of the priority drains with outfalls located beneath the marine platform to prevent or minimize tidal encroachment into the upper reaches of the drains. The 14th and 15th Street storm drains, which are not obstructed by the marine platform, have coffer-dam-type tidal exclusions, each of which is located outboard of the actual outfall location. Currently, dry-weather flow is collected in these coffer dams and pumped to a treatment plant for removal of hexavalent chromium and pH adjustment prior to discharge from Outfall 001 to the Patapsco River under the NPDES permit.

Inspection procedures for inlets, manholes, and tidal exclusion vaults, and pipe sections are described in detail in Section 3.



3. Storm Drain System Inspection Plan

3.1 Scope

Outfalls at the 12th, 12.5th, 13th, and 13.5th Street storm drains have means to exclude tidal influence using the tidal exclusion vaults. Both the 14th and 15th Street outfalls have outfall retention basins, which act as coffer dams, installed outboard of the outfall location. All tidal retention structures offer the capability of excluding the influence of the tide so that the bulk of the storm drain systems can be evacuated to prevent interference from residual rainfall or river water during inspection.

The following storm drain inspection and maintenance measures will be implemented at DMT:

- Perform Tier I inspections of each previously rehabilitated priority storm drain to determine the presence or absence of dry-weather flow at each trunk line manhole.
- Perform a Tier II inspection of each inlet, manhole, and pipe segment every 5 years to assess the
 condition of the liner and host structures. An anticipated Tier II inspection schedule is provided in
 Table 1. If groundwater intrusion is observed entering a manhole during Tier I inspections or routine
 site activities, a Tier II inspection will be performed on the lateral and upstream pipe sections
 discharging to the manhole.
- Document results of both the Tier I and Tier II inspections using digital forms that will be maintained in a geographic information system (GIS) database.
- Complete follow-up inspections to document the effectiveness of storm drain repairs.
- Monitor pipe wall deformation within the priority storm drain pipes, performed as part of the Tier II inspections.
- Prepare and submit to MDE an annual storm drain inspection and maintenance report in the first quarter of the following year.

3.2 Tiered Inspections

Tier I inspections use an observational approach that consists of performing visual inspections at the storm drain trunk line manholes to observe potential groundwater intrusion sources. Tier I inspections will initiate partial Tier II inspections to determine the source of dry-weather flow if such flow is observed during the visual inspection of a storm drain. If a partial Tier II inspection is required, it will be initiated within 14 days of the dry-weather flow observation. Maintenance and repairs will be conducted, as required, in response to inspection observations.

Tier II inspections are completed on a 5-year rotational basis and include detailed visual and CCTV inspections of on average 5,000 linear feet of storm drain system per year, including inlets, manholes, and lateral pipe sections (Table 1). A Tier II inspection may also be performed as needed if a Tier I inspection indicates a need for a more detailed inspection on a particular storm drain. The basis for performing 5,000 linear feet each year is that the extent of such inspection represents approximately one-fifth of the total priority storm drain system. The 5-year frequency for completing Tier II inspections is a minimum frequency, regardless of the observation of dry-weather flow, and considers weather, accessibility, standing water volume, and the occurrence of other independent factors. The inspection frequency for complete Tier II inspections can be increased in the future, depending on liner reliability as determined from the inspection record of the storm drains. Through the Tier I and Tier II inspections, the condition of the liners and host structures are assessed and will result in the development and scheduling of preventive maintenance activities.



Table 1. Tentative Tier II Inspection Schedule

Storm Drain	Year 1	Year 2	Year 3	Year 4	Year 5
12th St.			Х		
12.5th St.		Х			
13th St.	Х				
13.5th St.		Х			
14th St.				Х	
15th St.					Х

Inspection schedule to start over following year 5. Schedule is subject to Tier I findings.

3.3 Dewatering and Cleaning

Storm drains will typically be inspected in an as-found condition, without prior cleaning, so that signs of infiltration, such as calcification and staining, can be observed. Storm drain sections that contain significant amounts of debris or sediment that prohibit visual, or CCTV inspection will require cleaning prior to inspection.

Inspections will be conducted during dry weather, which is defined as a minimum period of 72 hours since the last rain event resulting in surface water entering the storm drain system. Dewatering of the storm drain will typically not be required for Tier I inspections; however, areas of the storm drain that hold standing water such as pipe sags and manhole floors will require dewatering prior to conducting Tier II inspections. Retained water will be removed from the storm drains by pumping the water downstream of the tidal exclusion vault or outfall basin for discharge to the river. As previously described, the 15th Street drain accepts flow from offsite sources; offsite flow will require diversion to the opposite twin storm drain trunk line while Tier II inspections are performed.

3.4 Inspection Procedures

3.4.1 Tier I Inspections

Tier I inspections will be performed semiannually to observe dry-weather flow from laterals and upstream trunk line piping at each trunk line manhole for the first 5 years, and then annually thereafter. Prior to an inspection, weather forecast information will be reviewed to verify that the inspection can be conducted during dry-weather conditions and that a minimum of 72 hours has passed since the last rain event. Structure covers along the storm drain trunk alignment will be removed to facilitate visual inspection. The Tier I Inspection Form provided in Appendix A will be used to document observations of flow/no-flow conditions at each trunk line manhole. If no flow is identified, the inspection will be considered complete. If chromium-impacted flow from lateral or upstream trunk line piping is observed, the upstream pipe sections, inlets, and manhole will be inspected as a Tier II inspection to locate the source of flow. Chromium-impacted dry-weather flow reported from an MS4 inspection will also prompt a partial Tier II inspection as described above. Repairs will be defined, scheduled, and implemented to restore containment. The estimated timeframe for completing repairs is discussed in Section 4.

3.4.2 Tier II Inspections

Tier II inspections comprise a visual inspection, either by CCTV or physical examination of each containment element, including lateral and trunk line piping, inlets, and manholes. Tier II inspection of the storm drains will include a physical inspection of the overall condition of each storm drain and the identification of compromised liners, structural damage, groundwater intrusion and other conditions suggesting that maintenance repair is necessary to maintain the integrity of the storm drains.



The entirety of the priority storm drain system will be inspected every 5 years. The 5-year inspections will be undertaken on a rotating basis, so that approximately one-fifth of the priority storm drain system undergoes a Tier II inspection every year. On average, approximately 5,000 linear feet of storm drain will be inspected annually. The Tier II Inspection Form provided in Appendix B will be used to document the portions of storm drains that may need corrective action so that preventive maintenance repairs can be scheduled and performed.

For Tier II inspections, planning activities will include the following:

- Secure and review the most recent inspection data (visual and CCTV) for the given drain
- Notify and coordinate with MPA operations to perform the storm drain inspections
- Reconcile the locations of inlets and manholes with the GIS storm drain map
- Assess current conditions within the pipes and arrange to dewater and clean the storm drains as needed
- Review the existing health and safety plan, including requirements for staffing, confined space entry, retrieval equipment, and air monitoring

3.4.2.1 Tier II Inlet, Manhole, and Tidal Exclusion Vault Inspection Procedures

The location of the inlets, manholes, and tidal exclusion vaults for each of the six priority storm drains are shown on Figures 2 through 7. Following are the procedures for inspection:

- 1) Record observations made during inspections on the Tier II Manhole/Inlet/Vault Inspection Digital Form (Appendix B). Ensure the nature of the damage and its location are described in sufficient detail and use standard nomenclature so that maintenance repairs can be developed and tracked. Take a photograph of the damage and append the photograph file in jpeg format to the corresponding digital entry.
- 2) With approved confined space entry procedures, enter structure, visibly inspect the surface of the high-density polyethylene liner and welds for structural damage, including holes, cracks, tears, and weld separations.
- Inspect all surfaces of the liner and field welds for active groundwater intrusion. Provide additional description of groundwater intrusion to assist in relocating area for repair (weep, pooling water, calcification, etc.).
- 4) Check liner for separation from the host manhole walls.
- 5) Check all lined surfaces for bulges or signs of water buildup behind the liner material.
- 6) Inspect the transition area between the lined structure and the pipe liner for active groundwater intrusion or separation.
- 7) Inspect the chimney parging for cracking, spalling, and active groundwater intrusion.
- 8) Inspect the transition strip mechanical seal for cracking or active groundwater intrusion.
- 9) Inspect the tidal exclusion vault flap gate or duck bill valve for cracking and proper operation.

3.4.2.2 Tier II Lined Pipe Inspection Procedures

The manhole and catch basins are typically connected by varying sizes of bell and socket reinforced concrete pipe ranging in size from 15 inches to 96 inches in diameter. The location and label of each pipe section for each of the six priority drains are shown on Figures 2 through 7. The following procedures should be followed:

1) Record observations made during inspections on the Tier II Pipe Section Inspection Form (Appendix B). If conducting a visual inspection of the pipe section, take a photograph of the damage and append the photograph file in jpeg format to the corresponding digital entry. If the pipe section is inspected via CCTV, append a screen capture of the video in jpeg format showing the damaged area.



- 2) Inspect pipe sections greater than or equal to 48 inches visually unless physical access is restricted, or conditions are not conducive to safe entry. Inspect smaller piping remotely by CCTV.
- 3) For visual inspection, upon entry into the manhole/inlet, set the measuring wheel to 0 feet at the downstream reference manhole/inlet. When a condition is observed, record the location using the distance from the reference manhole/inlet and the radial circumferential position measured in clock positions, looking upstream. For CCTV inspections, the camera will automatically log distance from the reference manhole/inlet. For both visual and CCTV inspections, the zero-reference point is typically the point where the pipe enters the manhole/inlet. The pipe inspector will always be present with the CCTV operator during the inspection to record data on the Tier II Pipe Section Inspection Digital Form.
- 4) Inspect the liner for signs of structural damage, including cracking, holes, tears, delamination (for cured-in-place pipe liners only), and seam or weld separation.
- 5) Inspect the liner for active groundwater intrusion. Provide additional description of groundwater intrusion to assist in relocating the area for repair (weep, pooling water, calcification, etc.).
- 6) Check liner for separation from the host pipe.
- 7) Check liner for bulges or signs of water buildup behind the liner.
- 8) Inspect spiral pipe rehabilitation (SPR) grout injection port locations for active groundwater intrusion and separation.
- 9) Inspect SPR and Expanda profile butt joints for groundwater intrusion and separation.
- 10) Inspect the liner for signs of radial compression, exhibited by reduction in inside diameter of the pipeline.
- 11) Monitor deformation of the host piping by laser profiling or manually measuring sections of piping where deformation has historically been observed and new areas where radial compression was observed during the current visual and CCTV inspection.

3.5 Geographic Information System

The existing, computer-based GIS model for DMT will be used to develop base maps, figures, and other data summaries showing, as required, the locations, limits, coordinates, and other pertinent data for operating areas, roadways, structures, utilities, and other points of interest. Inspection and repair data will be added to the GIS database using relative distances from named inlet and manhole structures. The DMT GIS model will be used as the basis for illustrating and documenting storm drain system inspection and maintenance information.

3.6 Field Documentation

Electronic digital inspection forms will be completed in the field using a tablet device for upload to a storm drain inspection database. Storm drain inspection results will be reviewed for preventive maintenance repair planning and inspection report preparation. When inspecting storm drains, the procedures listed below will be followed.

- 1) Complete the digital inspection forms by noting the storm drain designation, identified issue, and relative location.
- 2) Annotate the GIS base map to show the location of each identified issue.
- 3) Prepare an inspection summary for each storm drain system that includes all locations requiring repair.



4. Storm Drain Maintenance

4.1 Maintenance

Upon discovery of a condition requiring repair to the liner system, liner repairs will be completed within 45 calendar days unless circumstances are identified that will require additional time for completion. Conditions requiring structural repair to the host pipe or inlet and manhole structures may require preparation of design drawings and specifications and may take longer than 45 days to complete. Reasonable means to expedite repairs will be made to effectively complete the repairs following the inspections.

Parts and materials necessary to complete typical and minor maintenance repairs, such as small cracks, transition strip separation, pinhole leaks, or other items, will be maintained onsite so that these issues can be mitigated following the inspection. More significant types of maintenance repairs, such as transition strip damage that requires replacement, will be undertaken by specialty contractors. In such instances, development of an engineering design and specifications may be necessary.

4.2 Stormwater Management During Repairs

Retained, non-impacted water will be removed as needed by pumping the water downstream of the tidal exclusion vault or outfall basin for discharge to the river. Impacted stormwater will be collected during repair activities and transported to the onsite groundwater treatment plant or to an approved disposal facility.

4.3 Field Documentation

Electronic digital work completion will be completed in the field using a tablet device for upload to a storm drain database. Storm drain repairs will be documented on the appropriate inspection form. Following repairs, a follow-up inspection shall be performed no later than 6-months after completion of the repair.



5. Storm Drain Deformation Monitoring

5.1 Purpose

At the DMT, pipe deformation (particularly springline compression) is typically the result of lateral COPR movement. Collected data typically includes measurements of horizontal and vertical diameters that can be used to determine whether COPR expansion may be impinging on the outside of the pipe and the rate of pipe compression. Historically, these data have been used to assess the condition of the pipe, length of strain relief measures and pipe protection needed, and long-term effectiveness of strain relief trench measures that have been installed. As part of the Tier II inspections, deformation monitoring via laser profiling or manual measurements will be performed on sections of piping where deformation has historically been observed and new areas where radial compression was observed during the Tier II visual or CCTV inspections. Sections of piping where deformation has historically been observed are described in Section 5.4.

5.2 Laser Profiling in Storm Drains

Laser profiling consists of scanning the inside surface of a pipe using an array of remotely operated optical and acoustical sensors to produce a three-dimensional point cloud data set. The data is processed to derive radial dimensions. Each laser profiling scan represents a "snapshot" in time. The location and rate of pipe deformation is determined by comparing repeated scans over time.

The complete data set from each scan includes spreadsheets of pipe radii, ovality, and other parameters at frequent intervals along the pipe length. The laser profiling summary reports typically include descriptions of inspection methods, photographs, and plots of horizontal diameter, vertical diameter, ovality (percent deviation of horizontal and vertical diameters from one another), and deviation from the pipe's design diameter (sometimes termed "buildup" and "corrosion" to denote a decrease or increase, respectively from the design diameter).

To allow comparison of repeat laser profiling scans, the data provided are converted to a common reference (stationing) system based on the location of the farthest downstream structure, and scans performed on different dates are plotted on the same graph. Data are plotted in two ways: pipe dimensions and ovality with station, and deviation from the theoretical horizontal and vertical diameter with station.

The first scan performed in a length of pipe is the baseline scan for that length, performed in the first year of implementation of the storm drain inspection and maintenance plan of the PMP. To provide repeatability for subsequent scans, the following items are documented for each survey, including the baseline:

- Description of zero-reference point identifiable to 0.1 feet precision (typically, the point where the pipe enters the farthest downstream manhole/inlet)
- Frequency (spacing) of dimension measurements along the length and circumference of the pipe
- Precision of dimension measurements

5.3 Manual Measurement in Storm Drains

In pipes large enough for physical entry, manual measurements of pipe inside dimensions are performed on an as-needed basis. Crown-to-invert height is typically measured using a rigid bar of known length to penetrate bottom sediment combined with a stick ruler to the crown. A prefabricated steel "T" bar with a length equal to the pipe radius may be used to facilitate and improve repeatability of height and springline width measurements. The measuring bar also serves as a useful tool to dislodge silt cake / scale and to sound the pipe wall for delamination of the liner or concrete delamination in the underlying host pipe.



Manual measurements are typically performed at 25- to 50-foot intervals along the length of the pipe, with more frequent measurements at locations of known historic deformation or other conditions of concern. The magnitude and rate of pipe deformation is determined by comparing measurements taken over time. Consistency of location, method, and alignment (that is, plumbness of height measurements and perpendicularity of width measurements) of measurements is important to allow successive measurements to be compared.

5.4 Monitored Pipe Sections

At a minimum, the following pipe sections where deformation was observed prior to lining or rehabilitation will be monitored in conjunction with Tier II inspections.

- 12.5th Street storm drain—upstream of manhole MH-311, bumper pipe strain relief installed during rehabilitation
- 13th Street storm drain—trunkline section between tidal exclusion vault TEV-002 and manhole MH-314
- 13.5th Street storm drain—all piping upstream of tidal exclusion vault TEV-001, bumper pipe strain relief installed during rehabilitation
- 14th Street storm drain—trunkline section between manholes MH-319 and MH-320
- 15th Street storm drain—trunkline section between outfall basin EW-125 and manhole MH-339 (SRT protection installed), trunkline section between manholes MH-483 and MH-351, trunkline section between manholes MH-482 and MH-352



6. Reporting

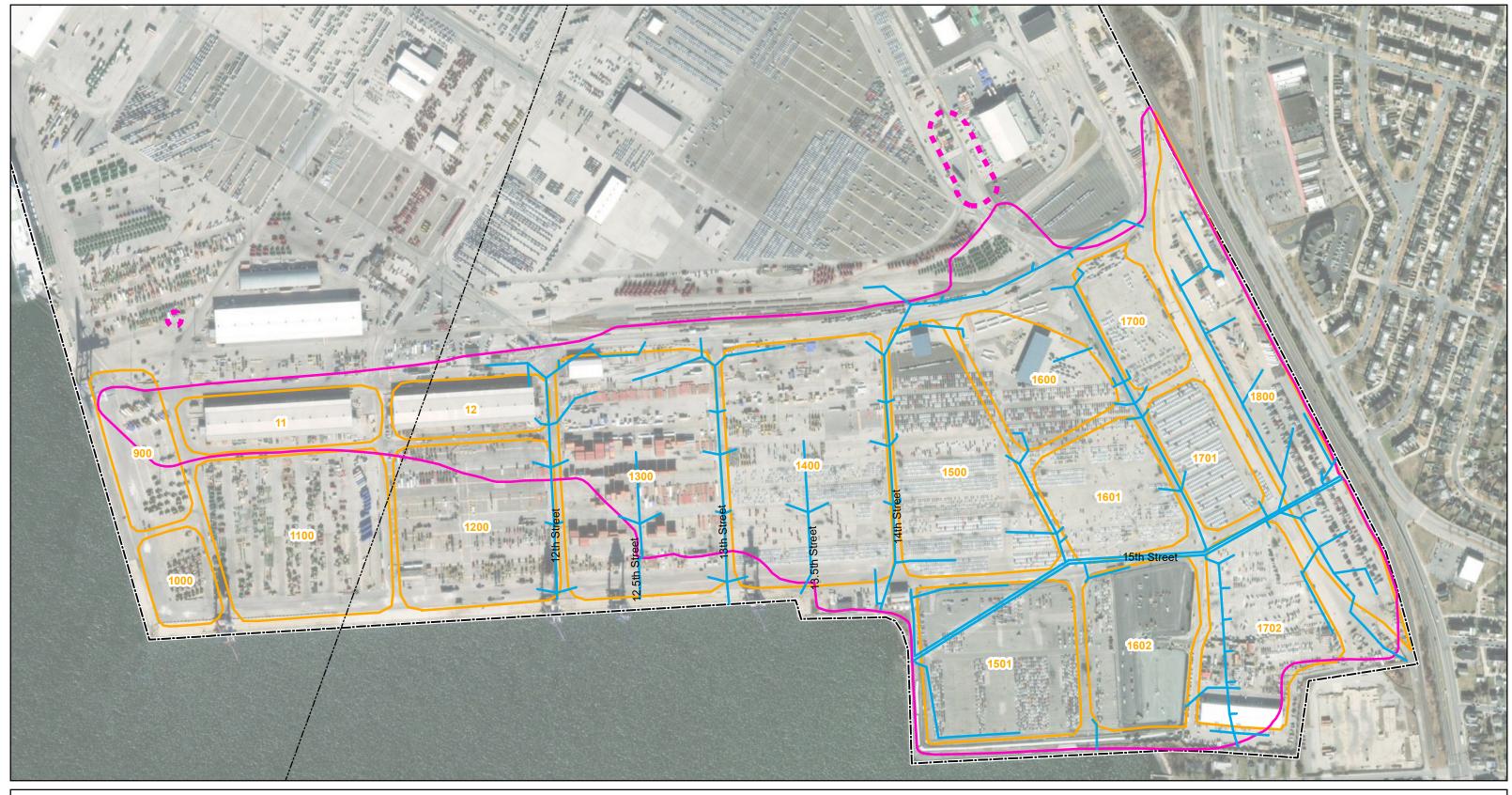
If chromium-impacted flow is observed during a PMP Tier I or Tier II inspection, MPA will be notified by Honeywell, and MPA will report the flow to the MDE under the MS4 program as applicable. If chromium-impacted dry-weather flow is observed by MPA during an MS4 inspection, Honeywell will be notified by MPA, and a partial Tier II inspection will be performed to determine and eliminate the source of the flow. Honeywell will provide notification to MPA of all planned Tier I and Tier II inspections.

An annual storm drain inspection and maintenance report documenting all storm drain inspection results, maintenance, and repairs will be prepared and submitted to MDE. This annual report will take the place of the Storm Drain Rehabilitation Schedule currently being provided to MDE on behalf of Honeywell and the MPA.

At a minimum, each annual report will include the following information:

- Dates of inspection(s) for each storm drain
- A summary of inspection procedures and results
- List of specific areas (table format) requiring maintenance or repair, including location, description, disposition status, and proposed repair method
- Completed repair location tables and figures for each storm drain including repair method used to complete the repair
- As-built drawings and specifications for more significant repairs, as appropriate
- Photos, if applicable
- Summary laser profiling reports and comparison plots, if applicable
- Appendix including the inspection data sheets

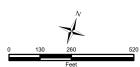
Figures

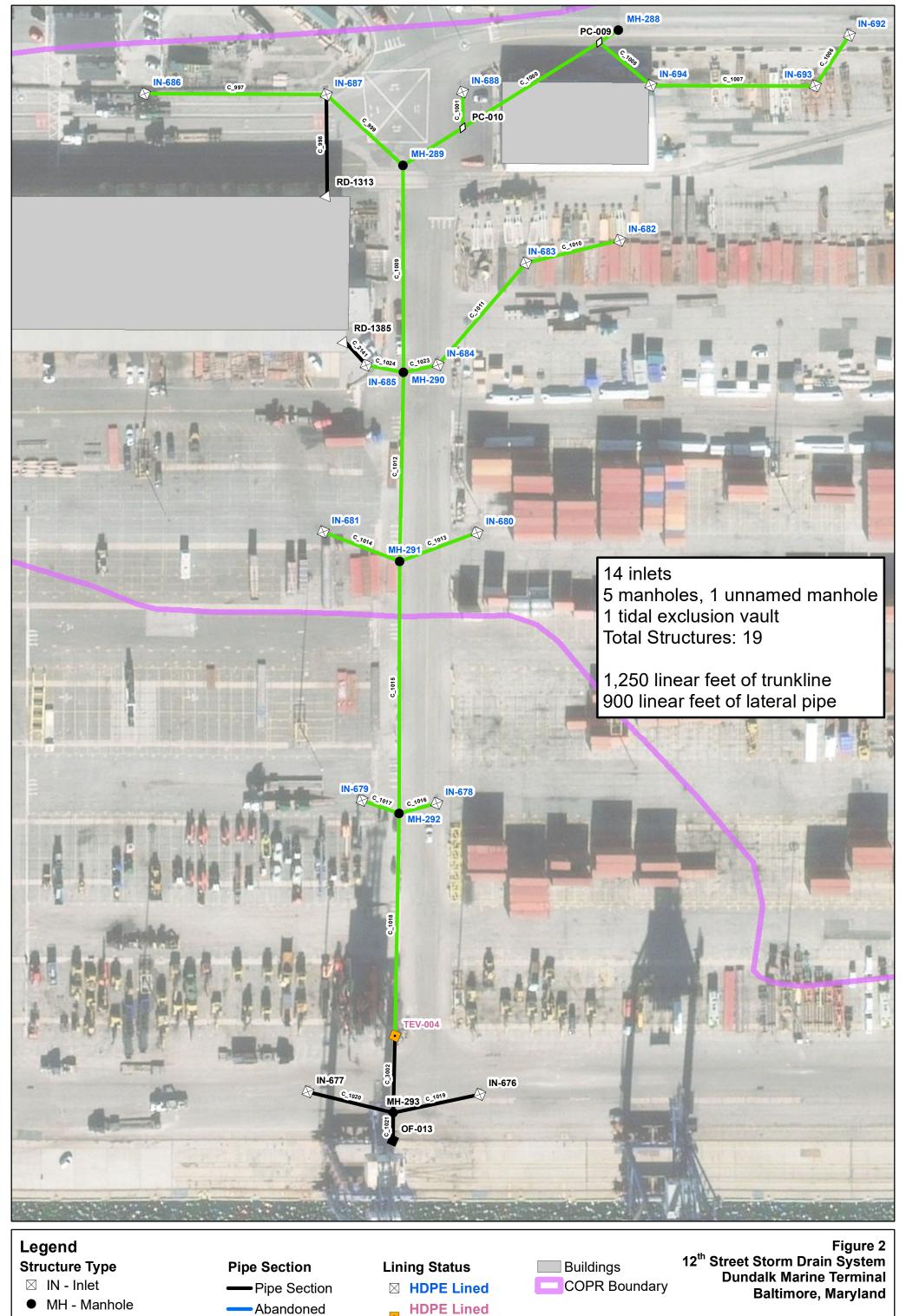


Legend

- Priority Storm Drain System
- ---- City/County Boundary
- Approximate COPR Extent
- COPR Extent
- Areas
- DMT Boundary

Figure 1 COPR Fill Area Dundalk Marine Terminal Baltimore, Maryland





■ OF - Outfall Replacment Rehabilitated PC - Pipe Connection Rehabilitated △ RD - Roof Drain Connection _ **Jacobs** Movement Monitoring

■ TEV - Tidal Exclusion Vault

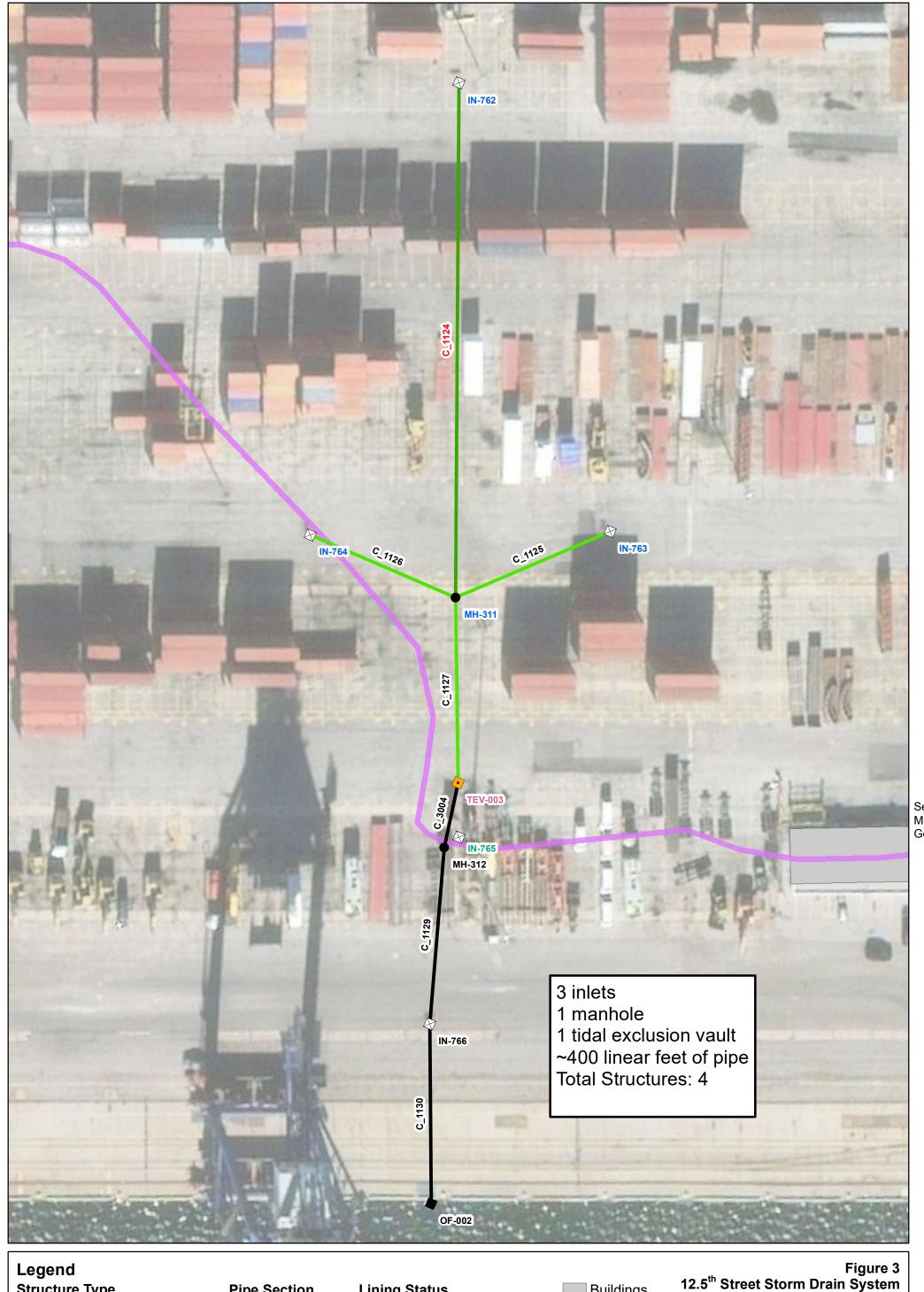
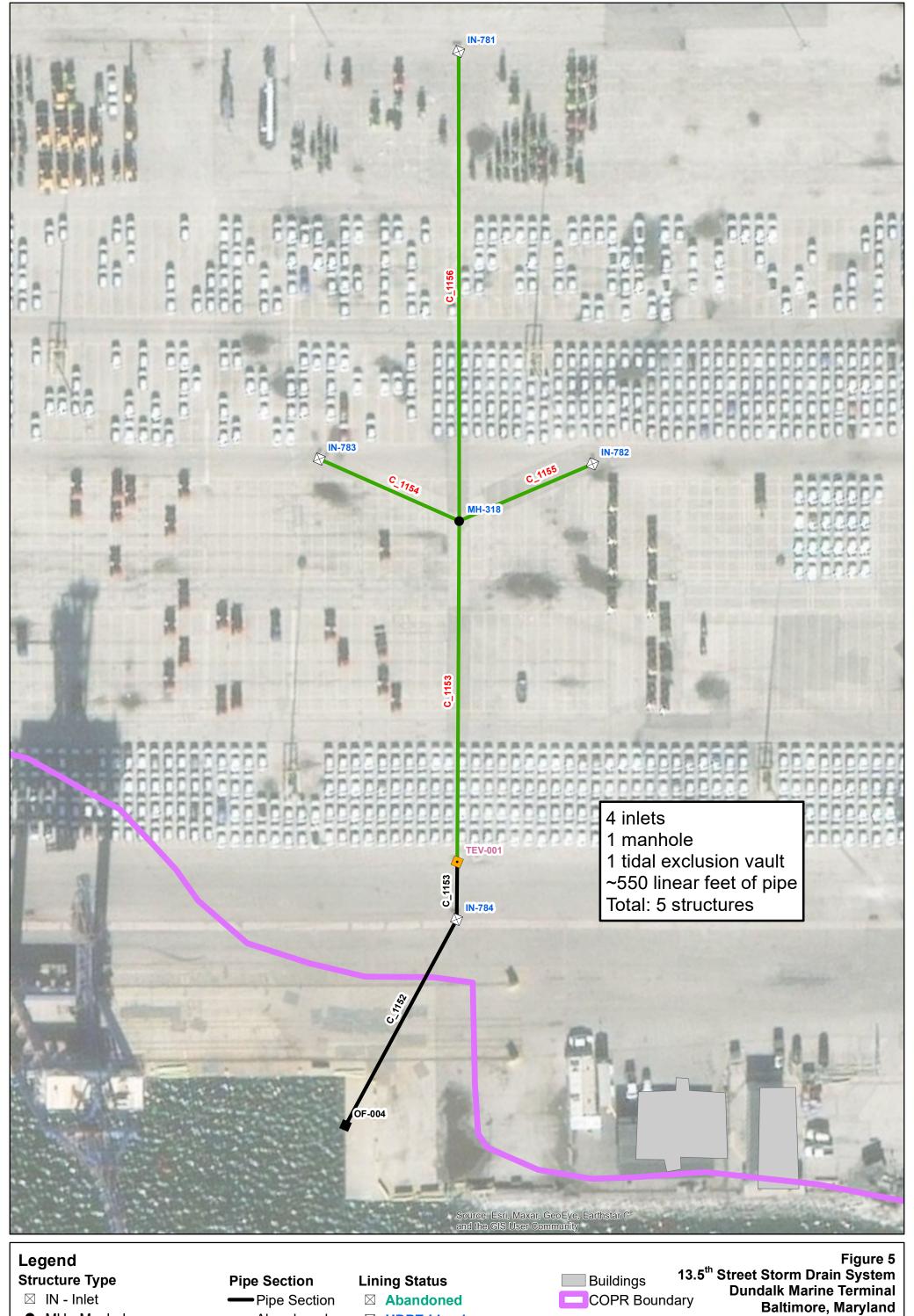


Figure 3 12.5th Street Storm Drain System Dundalk Marine Terminal **Structure Type** Buildings **Lining Status Pipe Section** ■Pipe Section COPR Boundary **⊠** Abandoned Baltimore, Maryland MH - Manhole -Abandoned **⋈** HDPE Lined **■ HDPE Lined Replacment** ■ OF - Outfall Rehabilitated ♦ PC - Pipe Connection Rehabilitated Movement Monitoring △ RD - Roof Drain Connection **Jacobs** • TEV - Tidal Exclusion Vault

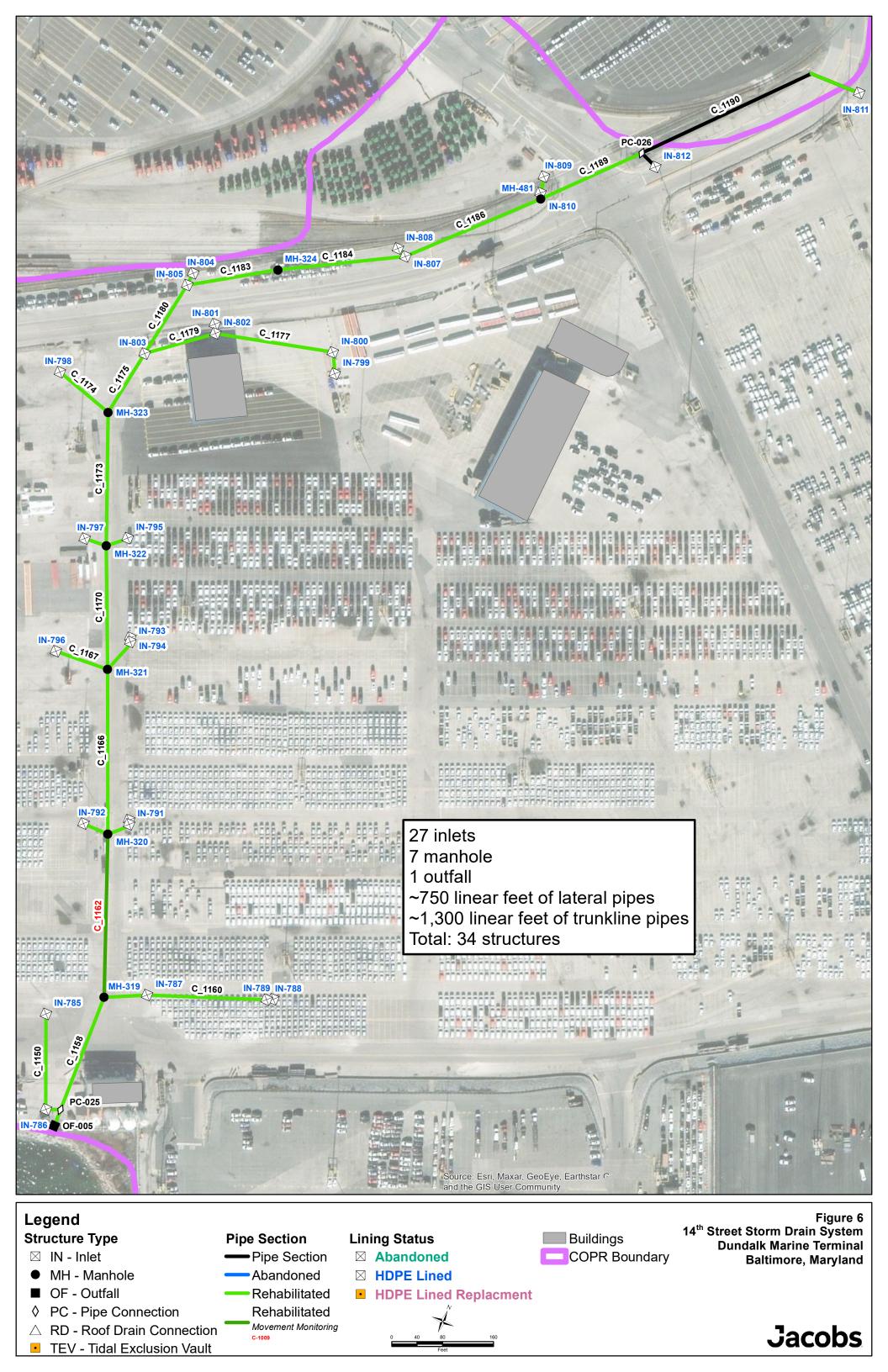


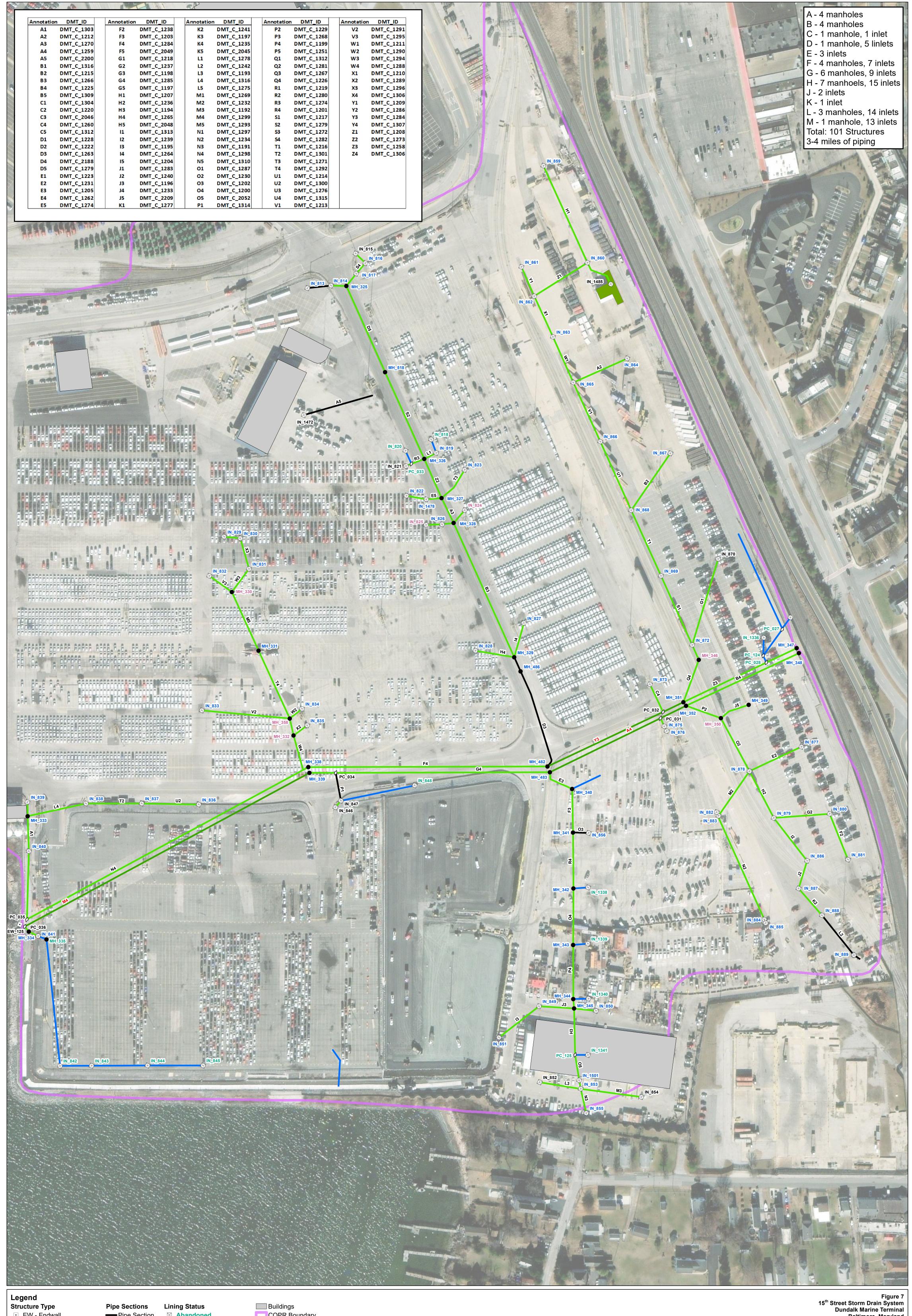
COPR Boundary Pipe Section **△** Abandoned Baltimore, Maryland MH - Manhole Abandoned **⋈** HDPE Lined ■ OF - Outfall Rehabilitated ■ HDPE Lined Replacment PC - Pipe Connection Rehabilitated Movement Monitoring △ RD - Roof Drain Connection **Jacobs** ■ TEV - Tidal Exclusion Vault



MH - Manhole **⋈** HDPE Lined -Abandoned ■ OF - Outfall Rehabilitated ■ HDPE Lined Replacment PC - Pipe Connection Rehabilitated △ RD - Roof Drain Connection Movement Monitoring **Jacobs**

■ TEV - Tidal Exclusion Vault





EW - Endwall

MH - Manhole ■ OF - Outfall ♦ PC - Pipe Connection

△ RD - Roof Drain Connection

Pipe Section

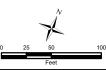
Abandoned ---Rehabilitated Rehabilitated

Movement Monitoring

⋈ HDPE Lined HDPE Lined Replacment

COPR Boundary Decon Pad

Baltimore, Maryland



Appendix A Tier I Inspection Forms

Tier I Storm Drain Groundwater Intrusion Inspection Digital Form

Storm Drain System (circle one)	12	12.5	13	13.5	14	15
Date				_		
Inspector's Initials				_		
Date of Last Rain Event						

		Left Lateral			Right Lateral			Upstream Pipe	!	
Component ID (Trunk Line Manholes)	Flow (Y/N) (Note 1)	Groundwater Intrusion (Notes 2,3,4)	CCTV Required (Y/N) (Note 5)	Flow (Y/N) (Note 1)	Groundwater Intrusion (Notes 2,3,4)	CCTV Required (Y/N) (Note 5)	Flow (Y/N) (Note 1)	Groundwater Intrusion (Notes 2,3,4)	CCTV Required (Y/N) (Note 5)	Comments (Note 6)

Notes -

- 1. Open each trunk line manhole and observe the presence or absence of dry weather flow entering the structure from the left and right laterals, and upstream piping.
- 2. If flow is observed, determine the upstream storm drain component (inlet, manhole, or pipe section) associated with the location of the groundwater intrusion.
- 3. A separate digital form will be used to further describe the groundwater intrusion including exact location, subcomponent (i.e., liner, weld, transition strip, mechanical seal), and type of damage (i.e., hole, tear, crack, separation)
- 4. Take a photograph or video screen capture of each groundwater intrusion location.
- 5. If the source of the groundwater intrusion is located in a small diameter pipe section, indicate that a CCTV inspection will be required to determine the exact location and type of damage.
- 6. Complete the comments column as needed.

Appendix B Tier II Inspection Forms

Tier II Storm Drain Inlet, Manhole, Vault Inspection Digital Form

Storm Drain System (circle one)	12	12.5	13	13.5	14	15
Storm Drain Component						
Date						
Inspector's Initials						
Date of Last Rain Event						
Structure Dewatered	Υ	N				
Structure Clean	Υ	N				

	1																				т —					1									I	
Item Number					Locatio										Subcom								W Intru							Structu					Photo	
(Note 1)	<u> </u>			. (Notes 2	,3)				_					(Note	2)					1		(Note	2)						(Note 2	2)				(Note 4)	Comment (Note 5)
	US	DS	Left	Right	Floor	Wall	CE	CH	0	H	is c	CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	Н	CR	SP	SE	WS	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	H	is c	CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	Н	CR	SP	SE	WS	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	Н	is c	CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	Н	CR	SP	SE	ws	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	Н	is (СР	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	Н	CR	SP	SE	ws	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	Н	is c	CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	н	CR	SP	SE	ws	BU	С	OP	0		
	US	DS			Floor	_	+	СН	0	Η.	is c	CP		LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	н	CR	SP	SE	ws	BU	С	OP	0		
	US	DS	_		Floor	_		CH	_	H	_	CP	1	LW	LG TS	_		MS	_		GI	_	+	_	0	н	CR	SP	SE	ws	BU		OP	0		
	_	DS			Floor			CH	_	i i	_	CP	-		LG TS	_	TSW			_	GI	_		_	0		CR	SP	SE	ws	BU		OP			
	US			_	_	_	+	_	_	-	_	-	-	LW			_	+	_	_	+	_	+	_	_	п	_	_	_	_	-	L	-	U		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	H	is c	CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	Н	CR	SP	SE	WS	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	CH	0	H	is c	CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	Н	CR	SP	SE	WS	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	Н	is (CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	Н	CR	SP	SE	WS	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	Н	is (CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	н	CR	SP	SE	WS	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	Н	is c	CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	Н	CR	SP	SE	ws	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	Н	is c	СР	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	н	CR	SP	SE	ws	BU	С	OP	0		
	US	DS	Left	Right	Floor	Wall	CE	СН	0	Н	is c	CP	L	LW	LG TS	TSA	TSW	MS	FG	0	GI	WP	PW	CA	0	Н	CR	SP	SE	ws	BU	С	OP	0		
	US	DS	_		Floor	_		СН	0	Н	is c	CP	L	LW	LG TS	TSA	_		FG	0	GI	WP	PW	CA	0	н	CR	SP	SE	ws	BU	С	OP	0		
	_	DS	_		Floor	_		_	_	-	_	CP	L	LW		TSA	_	_	_	_	GI	_	_		0	н	CR	SP	SE	_	BU	С	OP	0		
	US	DS	_	_	Floor	+		СН	0	Н	_	CP	1	LW	LG TS	_	_		_		GI	_	PW	CA	0	н	CR	SP	SE	ws	BU	С	OP	0		
	_		_	-	Floor	_		СН	_	-	_	CP		LW	LG TS	_	_	+	+	-	GI	_	+	_	0	н	CR	SP	SE	ws	<u> </u>	-	OP	1		
	_	DS			Floor	_	_	_	_	-	_	CP	-	LW		TSA	_	+	_	_	GI	_	_		0	н	CR	SP	SE	_	BU	-	OP	1		

Notes -

 ${\bf 1.} \ \ {\bf Enter \, sequencial \, item \, number \, for \, each \, location \, where \, groundwater \, instrusion \, or \, structural \, damage \, is \, observed. \\$

Subcomponent

HS - Host structure

- Describe the location, subcomponent, and observation by circling all applicable fields.
 Left and right location descriptions are referenced looking upstream.
- 4. Take a photograph for each issue.
- 5. Complete the comments column as needed.

Location

US - Upstream

Key -

DS - Downstream	CP - Chimney parging
CE - Ceiling	L - Liner
CH - Chimney	LW - Liner weld
O - Other	LG - Liner grout
	TS - Transition strip
	TSA - Transition strip adhesive
	TSW - Transition strip weld
	MS - Mechanical Seal

FG - Flap gate or duck bill valve O - Other

GW Intrusion GI - Active Groundwater Intrusion WP - Weep PW - Pooling Water CA - Calcification O - Other

Structural H - Hole CR - Crack or tear SP - Spalling SE - Seperation of liner WS - Weld separation BU - Bulge in liner C - Compression of host structure

OP - Operation O - Other

Tier II Storm Drain Pipe Section Inspection Digital Form

Storm Drain System (circle one)	12	12.5	13	13.5	14	1
Storm Drain Component						
Date						
Inspector's Initials						
CCTV Contractor (if applicable)						
Date of Last Rain Event						
Line Dewatered	Y	N				
Line Clean	Υ	N				

Location

	Start	End	Start	End																									
	Longitudinal Distance	Longitudinal Distance	Radial Position	Radial Position																									
n Number otes 1,2)	(feet) (Note 3)	(feet) (Note 3)	(clock) (Note 4)	(clock) (Note 4)				Subcom (Not							W Intrusi						Struc	ctural te 5)				Photo (Note 6)	Springline (Note 7)	Crown to Invert (Note 8)	Comment (Note 9)
,	,,	, ,	, ,		НР	L	J	JW	SE	GP	MS	0	GI	WP	PW	CA	0	Н	CR	DL	SE	ws	BU	С	0	,,	, , ,	,	
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	н	CR	DL	SE	ws	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	H CR DL SE WS BU C O											
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	н	CR	DL	SE	ws	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	н	CR	DL	SE	ws	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	WS	BU	С	0				
					НР	L	J	JW	SE	GP	MS	О	LK	WP	PW	CA	0	Н	CR	DL	SE	ws	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	WS	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	ws	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	ws	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	ws	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	ws	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	ws	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	WS	BU	С	0				
					НР	L	_	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	WS	BU	С	0				
					HP	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	WS	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	ws	BU	С	0				
					HP	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	WS	BU	С	0				
					HP	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	WS	BU	С	0				
					НР	L	J	JW	SE	GP	MS	0	LK	WP	PW	CA	0	Н	CR	DL	SE	WS	BU	С	0				

- Notes
 1. Enter sequencial item number for each location where groundwater instrusion or structural damage is observed.

 2. Inspections are conducted from downstream to upstream from the reference inlet, manhole, or vault structure.

 3. Longitudinal distances are measured from the downstream inlet, manhole, or vault structure.

 4. Radial positions should be identified using clock positions looking upstream.

 5. Describe the location, subcomponent, and observation by circling all applicable fields.

 6. Take a photograph or video screen capture of each observation.

- 7. Measure springline distance using "T" bar as described in Section 5.1.3.
 8. Measure crown to invert distance using "T" bar described in Section 5.1.3.
- 9. Complete the comments column as needed.

<i>i</i> -			
	Subcomponent	GW Intrusion	Structural
	HP - Host pipe	GI - Active Groundwater Intrusion	H - Hole
	L - Liner	WP - Weep	CR - Crack or tear
	J - Joint	PW - Pooling water	DL - Delamination (CIPP liner only)
	JW - Joint weld	CA - Calcification	SE - Seperation of liner
	S - Seam	O - Other	WS - Weld or seam seperation
	GP - Grout port		BU - Bulge in liner
	MS - Mechanical Seal		C - Compression of host structure
	O - Other		O - Other

Appendix B Surface Cover Inspection and Maintenance, and Movement Monitoring Plan

Performance Management Program Surface Cover Inspection and Maintenance Plan

Dundalk Marine Terminal Baltimore, Maryland

Prepared for



115 Tabor Road Morris Plains, New Jersey 07950

and



Maryland Port Administration 401 East Pratt St. Baltimore, Maryland

Prepared by

Jacobs

Jacobs Engineering 2411 Dulles Corner Park Suite #500 Herndon, VA 20171

March 2023



Contents

1.	Intro	ction	1
2.	Back	ound	2
3.	Surfa	Cover Inspection	3
	3.1	Scope	3
	3.2	Inspection	3
		3.2.1 Inspection Mapping and Forms	3
		3.2.2 Field Inspection and Recording Procedures	3
		3.2.3 Geographic Information System	4
	3.3	Reporting	4
4.	Surfa	e Cover Maintenance	5
	4.1	Scope	5
	4.2	Reporting	5
5.	Straii	Relief Measures Monitoring	6
	5.1	Objectives	
	5.2	Scope	
	5.3	Cased Inclinometers	
		5.3.1 Reading Cased Inclinometers	7
		5.3.2 Cased Inclinometer Data Presentation	7
	5.4	Shape Accel Array Inclinometers	7
		5.4.1 Reading SAA Inclinometers	8
		5.4.2 SAA Data Presentation	8
	5.5	Surface Point Monitoring	8
		5.5.1 Reading SPMs	8
		5.5.2 SPM Data Presentation	8
	5.6	Strain Relief Trench Backfill Monitoring	9
	5.7	Maintenance of Instrumentation and Strain Relief Measures	9
		5.7.1 Inclinometers and SAAs	9
		5.7.2 Surface Survey Points	9
		5.7.3 Strain Relief Measure Maintenance	9
	5.8	Reporting	9

Appendixes

- A Sample Inspection Form for Surface Cover Inspections
- B Sample Surface Cover Inspection Report
- C Sample Surface Cover Repairs Summary Report

Tables

1 Standard Cover System Description Codes

Figures

- 1 COPR Fill Area
- 2 Surface Cover Inspection Zones
- 3 Site Plan of Ground Surface and Subsurface Movement Monitoring Points



1. Introduction

This Surface Cover Inspection and Maintenance Plan is an integrated component of the overall Performance Management Program (PMP) for the long-term monitoring and maintenance of the remedy being implemented at the Dundalk Marine Terminal (DMT).

Pursuant to the Consent Decree among the Maryland Department of the Environment (MDE), the Maryland Port Administration (MPA), and Honeywell International (Honeywell), and the Corrective Measures Alternatives Analysis required by the Consent Decree, MDE has approved an Enhanced Isolation and Containment remedy for the Site. The Enhanced Isolation and Containment remedy includes a requirement to maintain and repair the asphalt pavement surface cover in the chromium ore processing residue (COPR) fill area and a PMP to confirm and monitor the long-term effectiveness of the surface cover. This Surface Cover Inspection and Maintenance Plan fulfills the PMP requirements regarding the Enhanced Isolation and Containment remedy's surface cover portion.

This plan outlines the ongoing surface cover inspection and maintenance that Honeywell and MPA will undertake to maintain protection of the surface cover system. Ongoing inspection is necessary to identify damage to the surface cover and forms the basis for repairs and maintenance that must be implemented to ensure the integrity of the cap and its continued effectiveness.

The approach outlined in this plan for performing surface cover inspection and maintenance follows the plan described in the August 2007 *Surface Cover and 14th and 15th Streets Drain Inspection and Maintenance Plan* (SCMP), which was submitted to the MDE pursuant to Section III.B.4 of the Consent Decree. Following the baseline surface cover inspection documented in the September 2007 *Surface Cover System Baseline Inspection Report, Dundalk Marine Terminal*, the inspection frequency was changed from semiannual to annual based on observations that the cover system at DMT did not erode or deteriorate as quickly as soil cover systems at typical landfill sites. This plan also includes procedures for monitoring and maintaining strain relief measures that have been installed to prevent surface heave and protect the Area 1501/1602 COPR cell clay containment dike and storm drains from lateral COPR movement.

This Surface Cover Inspection and Maintenance Plan includes the following:

- Annual inspection of the surface cover system within the COPR fill area.
- Documentation of the surface cover inspections and status of recommended repairs using digital forms, maintained in a geographic information system (GIS) database.
- Performance of annual surface cover repairs and maintenance based on the inspection results and on observations of heave and maintenance priorities during MPA port operations.
- Annual monitoring and maintenance of existing strain relief measures and implementation of the repairs, as needed.
- Preparation and submittal of an annual Surface Cover and Strain Relief Measures Inspection Report.
- Preparation and submittal of an annual Surface Cover and Strain Relief Measures Repairs Report.

This plan outlines inspection and maintenance procedures and documentation and reporting requirements. This document also details the methodologies associated with monitoring strain relief measures.



2. Background

The COPR fill area consists of active operational areas, with asphalt pavement being the primary cover system over the COPR fill. A map of the COPR fill area is shown on Figure 1. Except for a portion of Area 1800 and Area 1501/1602, pavement systems consist of conventional asphalt, roller-compacted concrete asphalt, or concrete "pavers." A 7.2-acre portion of Area 1800 was reconstructed in 2008 as a pilot area to evaluate the performance of a 4.4-acre modified conventional asphalt pavement section, a 1.8-acre low permeability asphalt ("Matcon") pavement section, and a 1-acre articulated block cover pavement section under heavy port loading conditions.

Area 1501/1602 has approximately 7 feet of non-COPR surcharge fill and 15.5 inches of stone base and asphalt surfacing, which were placed to mitigate heave conditions. A geomembrane liner, drainage layer, and new surface cover system were installed over approximately 21 acres in 2022 to prevent surface water infiltration. A previously unpaved railroad track area north of the East Service Road is scheduled to be paved in 2023 to minimize surface water infiltration. The switch areas located within this track area will remain unpaved; however, the area will be graded to minimize surface flow to these areas.

Since 2009, the inspection process has been conducted and documented annually using inspection and documentation procedures originally detailed in in the SCMP. The inspection, maintenance, and reporting process for surface cover maintenance is summarized in this plan, which supersedes the SCMP. Inspection findings and an overview of repair priorities are documented in the *Surface Cover System Inspection Report* for each respective calendar year. Repaired issues are summarized annually in a Summary Report for Repair Cycle, Surface Cover System. Repairs are completed and documented based on methods and procedures described in Section 5.

The 2009 *Heave Investigation and Minimization Study* (HIMS) report provided a detailed explanation of COPR mineralogy, chemical expansion mechanisms, physical manifestations of subsurface COPR expansion, and heave mitigation measures. After the issuance of the HIMS report, movement monitoring continued and has been reported in a series of HIMS Supplemental Data Reports issued annually since 2013. COPR movement rates at DMT are well understood and are constant based on over 12 years of data. The final HIMS Supplemental Data Report summarizing data collected in 2022 will be submitted to the MDE in the second quarter of 2023; future lateral movement monitoring associated with the containment remedy will continue to be reported as part of the Surface Cover Inspection and Maintenance program.

Subsurface strain relief measures implemented at DMT include strain relief trenches (SRTs) and storm drain bumper pipes. Inclinometers, Shape Accel Arrays (SAAs), and surface survey pins (pk nails) were installed as part of the HIMS Work Plan (CH2M 2006). The inclinometers, SAAs, and pk nails installed adjacent to the strain relief trenches and bumper pipe sections, along with visual observations of the displaceable SRT backfill and probing of the backfill for COPR impingement, will be used to determine whether and where COPR may be laterally expanding into the trench or bumper pipes. Inspection and maintenance of the SRTs and bumper pipes will be conducted as part of this plan.



3. Surface Cover Inspection

3.1 Scope

The annual surface cover system inspection will be conducted in early spring and shall include inspection of the entire COPR area the first year, followed by inspecting one of the three zones in subsequent years thereafter, as defined in Section 3.2, within the COPR fill area based on procedures detailed in Sections 4.2 and 4.3. Due to the sensitive nature of the shoreline of Area 1501/1602, the exterior slope of Area 1501/1602 will be inspected annually. The inspection includes observing the general condition of the surface cover system and documenting identified pavement surface repair features, such as cracks, potholes, and surface heave features that could affect the integrity of the surface cover.

3.2 Inspection

The annual surface cover inspection will include sections of DMT underlain by COPR fill. The fill area was divided into three zones of near equal acreage that will be inspected at least once every three years on a rotating basis: east, west, and central. A map showing the three surface cover inspection zones is shown as Figure 2. Boundary lines between inspection zones are positioned parallel to major northbound cross streets within the COPR area. Inspections involve walking each area in parallel transects approximately 50 feet apart, observing existing surface conditions on each side of the travel path. Observed surface-damage features requiring some form of repair, such as potholes, mechanical damage, surface heave features, crack features, and drainage system damage (indicating that the cover or drainage system may be compromised or may become compromised soon) are located, photographed, and documented.

The inspection of the Area 1501/1602 shoreline may be performed through visual observation from the shoreline or by drone survey techniques. Observations of potential erosion, sloughing conditions, failure of rip rap embankment or noticeable discharge from the shoreline (sediment) will be noted. Any of these conditions would warrant further review and evaluation depending on the nature and severity.

3.2.1 Inspection Mapping and Forms

Observations are noted using a form-based GIS application developed from the blank form included as Appendix A. Field maps with real time position information are available for reference through the GIS application and include aerial imagery, boundaries of each operational area, major roads, and the COPR fill boundary. The application was developed from lessons learned during the baseline inspection and resulting repair cycle. Inspection entries are exported from the application and converted into the Excel table included as Appendix B in the annual Surface Cover Inspection Report.

3.2.2 Field Inspection and Recording Procedures

A visual walking inspection is performed to assess the general condition of the pavement surface and to identify and record the locations of features such as surface cracks, broken or spalled pavement, potholes, open or unpaved areas, heaved areas, depressed areas, damaged surface drainage structures, COPR Blooms, areas around cap penetrations such as light poles and loading docks, and other conditions indicating that the integrity or function of the cover system may be compromised or may become compromised in the future.

Locations of identified pavement damage features will be determined using a Trimble R1 global positioning system (GPS) unit with a horizontal accuracy of approximately 1 meter (3.25 feet). GPS coordinates are taken at the approximate center of the features so that they can be accurately located for future maintenance, repair, and documentation. The order of inspection during an inspection cycle is controlled by accessibility to the individual areas due to ongoing port operations. Inspection paths will be recorded using GPS logging equipment to identify areas that were not inspected because of port operation restrictions, including cargo loading, however inspections will be coordinated with port operations to minimize inaccessible areas during the current inspection and access areas that were inaccessible during the previous inspection. For newly identified features, approximate lengths



and widths of cracks and approximate dimensions of potholes, areas of cracking, and other identified features are estimated or measured in the field.

Once a repair feature is identified and located, the following information is recorded:

- A point identification number (issue number), description code from Table 1, and photo of repair feature
- Coordinates identifying the approximate center of the feature, or center of irregular areas
- General type of feature (pothole, open/unpaved area, surface crack, crack area, surface heave area, inlet/manhole, unpaved gravel area) along with dimensions (width, length, height, diameter) for each location

Table 1. Standard Cover System Description Codes

Code	Description
1	3-inch mill-and-pave asphalt repair
2	Uncovered area: open or unpaved
3	Linear surface cracks or joints: between 0.25 and 1 inch wide and greater than 4 inches deep
4	Area requiring the replacement of the full 7-inch section of asphalt due to alligator or spider cracking
5	Surface heaving: areas that may affect the safe operation of port equipment (field determined)
6	Drainage inlet or manhole: damaged
7	Monitoring wellhead: damaged
8	Gravel cover areas: discoloration
9	Other (any issue not listed above that affects the barrier, safety, or integrity of the surface cover; description provided in the notes section of inspection form)

3.2.3 Geographic Information System

The existing, computer-based GIS model for DMT is used to develop base maps, figures, and other data summaries showing, as required, the locations, limits, coordinates, and other pertinent data for operating areas, roadways, structures, utilities, and other points of interest. Inspection and repair data will be added to the GIS database using coordinates, dimensions, repair type, and description recorded during the inspection. The GIS model is used as the basis for illustrating and documenting surface cover system inspections and maintenance information.

3.3 Reporting

Surface cover inspection findings and repair recommendations will be included in the annual Surface Cover and Strain Relief Measures Inspection Report prepared and submitted to MDE in the third quarter of each year. A sample of the surface cover inspection portion of the report is provided as Appendix B.

At a minimum, the report will include the following surface cover inspection information:

- A table including newly identified issues with the surface cover system and pending items from previous cycles
- A table with estimated quantities for each repair type by operational area
- A figure showing the location and repair type for each identified issue within the COPR fill area
- Repair recommendation for the current repair cycle



4. Surface Cover Maintenance

4.1 Scope

Features recommended for repair are located and marked in the field using a GPS-enabled iPad with the ArcGIS Field Maps application. Access to the areas and movement of select storage containers, equipment, or trailer chassis are coordinated with MPA port operations in preparation of repair activities.

Pavement and concrete cracks less than 1 inch wide are cleaned and sealed using a hot-rubber compound. Cracks greater than 1 inch wide, potholes, and surface-damage areas are milled to a depth of 3 inches, tack coated, and resurfaced using a hot-asphalt mix. Crack areas where the base asphalt material is not exposed are milled to a depth of 3 inches, tack coated, and resurfaced using a hot-asphalt mix. Crack areas where the base asphalt material is exposed are typically saw cut and repaired using a 6-inch milled tie-in, 4 inches of base course asphalt, and 3 inches of surface course asphalt.

Large area paving and heave repairs typically include coring to determine asphalt cover thickness and developing a milling design that: (1) minimizes exposure to the gravel base; (2) maintains the minimum surface cover profile typically consisting of 8 inches of dense graded aggregate, 4 inches of base course asphalt, and 3 inches of surface course asphalt per MPA specifications; and (3) restores surface water drainage to the nearest storm drain inlet to minimize standing water. Following the milling of the asphalt, excavation of the subgrade material to reach suitable depths to adhere each repair to the specified profile is completed, with placement of base course and surface course asphalt per the paving specifications.

Based on the surface cover inspection observations, areas that may potentially affect the port's operational or safety requirements will be addressed on a priority basis. Other features will be prioritized based on accessibility subject to port operations and cargo loading. Identified surface cover damage not repaired during the current repair cycle will be identified for repair during future maintenance cycles.

4.2 Reporting

Completed surface cover repairs will be summarized in the annual Surface Cover and Strain Relief Measures Repair Report prepared and submitted to MDE by the first quarter of the following year. A sample of the surface cover repairs portion of the report is provided as Appendix C. The report will include the following surface cover repair information:

- A table summarizing the amount of each type of repair completed in each operational area.
- A figure illustrating the locations of completed surface cover repairs and large area paving sections.
- A photolog highlighting typical repairs capturing before and after paving or sealing.
- An appendix including a repair disposition table indicating the status of each issue identified in the inspection report as either "completed" or "pending."



5. Strain Relief Measures Monitoring

5.1 Objectives

The primary objectives for monitoring the existing strain relief measures at DMT are to:

- 1) Ensure the continued protection of the Areas 1501/1602 COPR cell clay containment dike and storm drains from lateral COPR movement
- Understand the rate and direction of surface and subsurface lateral movement adjacent to the strain relief measures
- 3) Evaluate the effectiveness and estimate the remaining life of SRTs and bumper pipes
- 4) Design and plan for maintenance repairs

5.2 Scope

Subsurface strain relief measures implemented at DMT include SRTs and storm drain bumper pipes. SRTs consisting of a 2- to 3-foot-wide trench backfilled with displaceable fill have been installed to mitigate surface heave in Areas 1501/1602, and 1800; protect the 15th Street storm drain piping from lateral deformation; and protect the clay containment dike in Area 1501/1602 from lateral deformation. Bumper pipes consisting of sacrificial, small diameter high density polyethylene pipes placed outside a corrugated metal "shell" pipe, are placed radially along the exterior of sections of storm drain piping to accommodate COPR displacement. Bumper pipes have been installed along sections of the 12.5th and 13.5th Street storm drains, and along the perimeter of the trash interceptor structure installed in Area 1800. The locations of existing SRTs and bumper pipe sections are shown in Figure 3.

Movement data collected from inclinometers, SAAs, and pk nails installed adjacent to the SRTs and bumper pipe sections, along with visual observations of the displaceable SRT backfill and probing of the backfill for COPR impingement will be used to evaluate the performance of strain relief measures and estimate their remaining service life. Maintenance repairs will be designed and implemented to refurbish portions of the SRTs, and bumper pipes as needed.

Strain relief measures monitoring will be performed annually, as listed below. Components of the monitoring consist of the following:

- Measuring inclinometers and SAAs:
 - 10 conventional cased inclinometers, COPR fill area (outside Area 1501/1602)
 - 28 SAA inclinometers in Area 1501/1602
- Survey point (pk nail) monitoring in Areas 1300, 1400, 1501/1602, 1800
- Inspecting and probing the trench backfill in Area 1501/1602 at existing access ports

The locations of all surface and subsurface movement monitoring instrumentation described above are shown in Figure 3.

5.3 Cased Inclinometers

Cased inclinometers are specialized manufactured plastic casings with internal groves at the 90-degree points, drilled and grouted into the ground. By manually reading the casings with a special probe, changes in lateral profile of the casings from time of installation can be determined.

Inclinometer data are used to establish lateral subsurface movement rate and direction, which can be used to evaluate the performance of strain relief measures. For example, inclinometers installed on the protected side of



an SRT should show lower movement rates than inclinometers on the non-protected side, and lateral movement rates can be used to estimate the amount of displacement that has occurred into an SRT or bumper pipe section and hence its remaining service life.

5.3.1 Reading Cased Inclinometers

Inclinometer data are obtained by raising and lowering an inclinometer probe at specified locations, with the probe data captured by a datalogger, as described below. Inclinometer readings are currently performed using a digital inclinometer probe manufactured by Geokon.

Inclinometer casings have two sets of internal grooves oriented at right angles to one another, one set designated as the A-axis, and one set the B-axis. At DMT, the A-axis casing grooves are typically set to approximate magnetic north, and the B-axis casing grooves set to approximate the west–east direction of potential movement. Readings are taken by lowering a digital inclinometer probe in the A-axis grooves to the first (bottom most) depth reading point within the casing and raising the probe in 2-foot intervals, taking readings at each interval.

Because the inclinometer probe is equipped with a bi-axial tilt sensor, it records tilt readings in both the A-axis and B-axis directions at each interval. Each A-axis and B-axis reading point is multiple readings, which are averaged by software internal to the data logger. Upon completion of a set of readings with depth in a casing, the probe is then reversed 180 degrees and again lowered to the first (bottom) reading point, and a second set of readings is taken. The average of the two reading sets is reported as the reading for that depth. Averaging the reversed readings is intended to remove the effect of the inherent bias of the probe's tilt sensor. As a quality control check, the magnitude and stability of the probe's bias during each reading is set across multiple readings and is reviewed by generating "checksum plots."

5.3.2 Cased Inclinometer Data Presentation

Inclinometer data are reduced and plotted by GTilt (or equivalent) software. The software sums changes in casing inclination between reading points (bottom to top) to obtain the lateral deflected profile. The software averages the 0- and 180-degree reading sets to remove probe bias in accordance with manufacturer recommendations. Two types of inclinometer data plots are provided: cumulative displacement profile and time rate plots. Data from both the A-axis and B-axis are provided for each plot.

Cumulative displacement (CD) plots provide a graphical depiction of the laterally deflected profile of the inclinometer casing, as deviated from the original assumed perfectly vertical line (zero movement). Time rate (TR) plots are used to track displacements at specific depths within the inclinometer casings over calendar time. The depth selected for plotting varies depending upon inclinometer location. Typically, the plotted depth is the maximum recorded lateral displacement within or slightly above the COPR layer. The slope of that plotted line represents the rate of lateral displacement for the depth point plotted. TR plots provide movement rate at the analysis depth, typically expressed as annualized rates.

In addition to the CD and TR plots, a specialized inclinometer displacement direction (DD) plot plan is prepared to show the direction of lateral subsurface movement of all monitored inclinometers.

5.4 Shape Accel Array Inclinometers

SAA inclinometers are strings of micro-electro-mechanical tilt sensors spaced at 1-foot intervals connected in an array and installed vertically below the ground surface. SAA sensor strings are either placed in smooth PVC (polyvinyl chloride) casings drilled and grouted into the ground or are directly grouted into a borehole. SAAs record lateral movement with depth without the need to lower probes. The tilt of the individual sensors is integrated to produce a profile of lateral deflection in two perpendicular axes (*x* and *y*) with depth. Surface survey points are placed at the ground surface at the SAA to measure top movement for inclinometers not embedded sufficiently deep to attain bottom fixity. Like cased inclinometers, SAA inclinometer data are used to establish lateral subsurface movement rate and direction which can be used to evaluate the performance of strain relief measures.



SAA inclinometers are read by manual computer connection. Because there is no manual reading error, SAAs are considered more accurate and have a higher resolution than cased inclinometers.

5.4.1 Reading SAA Inclinometers

SAA readings are collected by connecting a laptop computer to the SAA using the Measurand SAA Field Power Unit. A "snapshot" of deflection (tilt) of each sensor in the SAA string is recorded using Measurand SAA Recorder software and the data is imported into Microsoft Excel for analysis.

5.4.2 SAA Data Presentation

For each SAA, three plots are provided. The first is a CD plot documenting the lateral displacement profile of the SAA inclinometer in *x*- and *y*-axes from the baseline reading. Positive *x*-axis displacement indicates movement to the north, and positive *y*-axis displacement indicates movement to the west.

Because SAA inclinometers installed in the Area 1501/1602 COPR containment cell typically do not extend to a known non-moving stratum, an assumption of zero movement at the bottom of the sensor array ("bottom fixity") is not necessarily applicable. To provide an absolute point of reference, survey points are established at the top of each SAA. The CD plots are adjusted to show the top movement as surveyed. The tilt deviations of each sensor element are then summed from the top to the bottom to produce the CD profile.

The second type of data plot provided for each SAA are TR plots used to track displacements at specific depths along the SAA string over time. Two analysis depths were selected for each SAA location based on observed movement trends. Typically, an analysis depth was assigned within the fill above the top clay cap of the COPR cell, and the other analysis depth was assigned to the depth where the greatest CD was observed, usually within the COPR fill. Resultant displacement magnitude and annualized rate are computed from the x-axis and y-axis displacements at each analysis depth. Annualized rate data allow movement comparison between SAA inclinometers installed on different dates and provide a convenient means to assess changes in movement rates over time.

The third type of plot for analyzing the inclinometer SAA data is a DD plot plan. The DD plan is prepared to show the lateral displacement direction of all the SAA inclinometers at the selected analysis depths in plan view.

5.5 Surface Point Monitoring

Surface point monitoring (SPM) consists of pk nails, survey points, or scribe marks embedded in the ground surface or on structures affixed to the ground surface. pk nails are standard survey nails embedded in the ground surface. Survey points were also established at the top of each SAA inclinometer for use in computing SAA displacement.

Ground surface movement is a valid proxy for determining subsurface lateral movement where block movement is occurring. Historic movement monitoring data (Appendix 4) have indicated block movement near previously installed SRTs, bumper pipes, and the shoreline.

5.5.1 Reading SPMs

SPM coordinates and elevations are surveyed by conventional survey methods. Data recorded from each reading includes spatial northing and easting coordinates referenced to horizontal datum Maryland State Plane NAD 83/91 and elevation referenced to vertical datum NAVD88. Horizontal data (northing and easting) are measured to Second-Order, Class 1 accuracy. Vertical (elevation) data are measured to First-Order, Class 1 accuracy.

5.5.2 SPM Data Presentation

SPM survey data provided by the surveyor are reduced by calculating the rate of lateral displacement in inches per year from the change in surveyed coordinates between initial baseline readings and the most recent survey.



Scaled vectors (movement rate and direction) are plotted on plan drawings (SPM plots) to show relative displacement trends. The total (vertical) change in elevation from the baseline readings to the most recent survey at each point are also shown on the plots. Positive elevation change represents an increase in ground surface elevation (heave). Negative elevation change represents a decrease in ground surface elevation (settlement).

5.6 Strain Relief Trench Backfill Monitoring

The strain relief trenches in Area 1501/1602 were installed with observation ports consisting of manholes opening to a large diameter pipe that extends into the SRT backfill. The backfill is inspected by inserting a rod into the backfill at the left, right and center of the manhole using a drill rig to determine whether COPR has expanded into the backfill. COPR is much harder than the displaceable backfill and rejection of the rod prior to reaching the bottom of the trench is indicative of the presence of COPR. The trench backfill, which consists of a mixture of ground COPR, peat, and bentonite is also inspected for consistency to confirm that the backfill remains in a displaceable condition.

5.7 Maintenance of Instrumentation and Strain Relief Measures

5.7.1 Inclinometers and SAAs

When cased inclinometers are too crimped for use, by either the standard probe or the special short probe (designed to pass minor crimping), they are abandoned by grouting in accordance with COMAR regulations. SAA inclinometers that were fully sealed in grout at the time of installation do not require further abandonment. SAA inclinometers installed in sleeve pipes (without grout) require removal and grouting of the sleeve according to COMAR regulations. Inclinometers and SAAs will be replaced or added as needed. Replaced inclinometers and SAAs must be "re-baselined" and the absolute deflection of the new instrument is not directly comparable to the deflection of the new instrument. However, movement rate and direction computed over successive readings of the new instrument may be compared with rate and direction from the replaced instrument.

5.7.2 Surface Survey Points

Surface survey points that are disturbed or destroyed by traffic, snow removal, construction, or other activities are periodically replaced. Replaced SPM points, like inclinometers and SAAs, must be "re-baselined" and the absolute position of a new point is not directly comparable to the position of the replaced point.

5.7.3 Strain Relief Measure Maintenance

Based on movement monitoring data and observations indicating that the strain relief measure is nearing the end of life, portions of the SRTs or bumper pipe sections will be rehabilitated or replaced. This maintenance requires design and planning, and in some instances permitting prior to implementation. Design plans and specifications for the repair will be provided to MDE for review.

5.8 Reporting

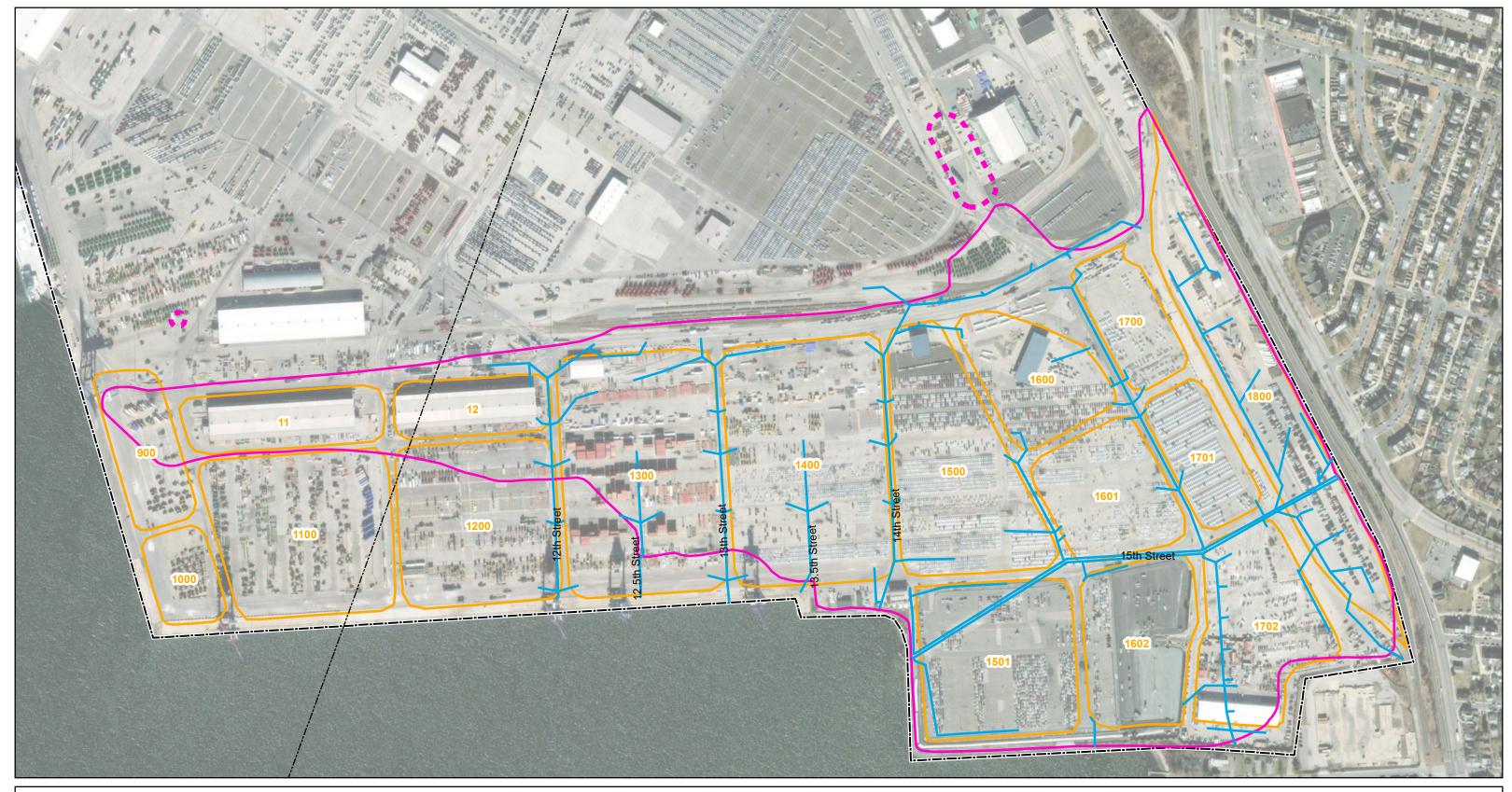
Results of the strain relief measures monitoring will be summarized in the annual Surface Cover and Strain Relief Measures Inspection Report. Maintenance repairs will be included in the annual Surface Cover and Strain Relief Measures Repairs Report. The reports will include the following strain relief measures inspection and repair information, as applicable:

- · Instrumentation description and location
- Data collection and reduction procedures
- A summary narrative and appendices containing the data for each instrument type collected during the prior year, plotted as described in the preceding sections
- Recommendations for instrumentation maintenance



- Recommendations for additional instrumentation or abandonment of existing instrumentation
- Conclusions regarding comparison of current movement rates to historical rates
- Conclusions regarding the effectiveness and remaining life of the strain relief measures
- Recommendations for strain relief measure maintenance repairs and addition of new strain relief measures as needed.

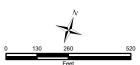
Figures

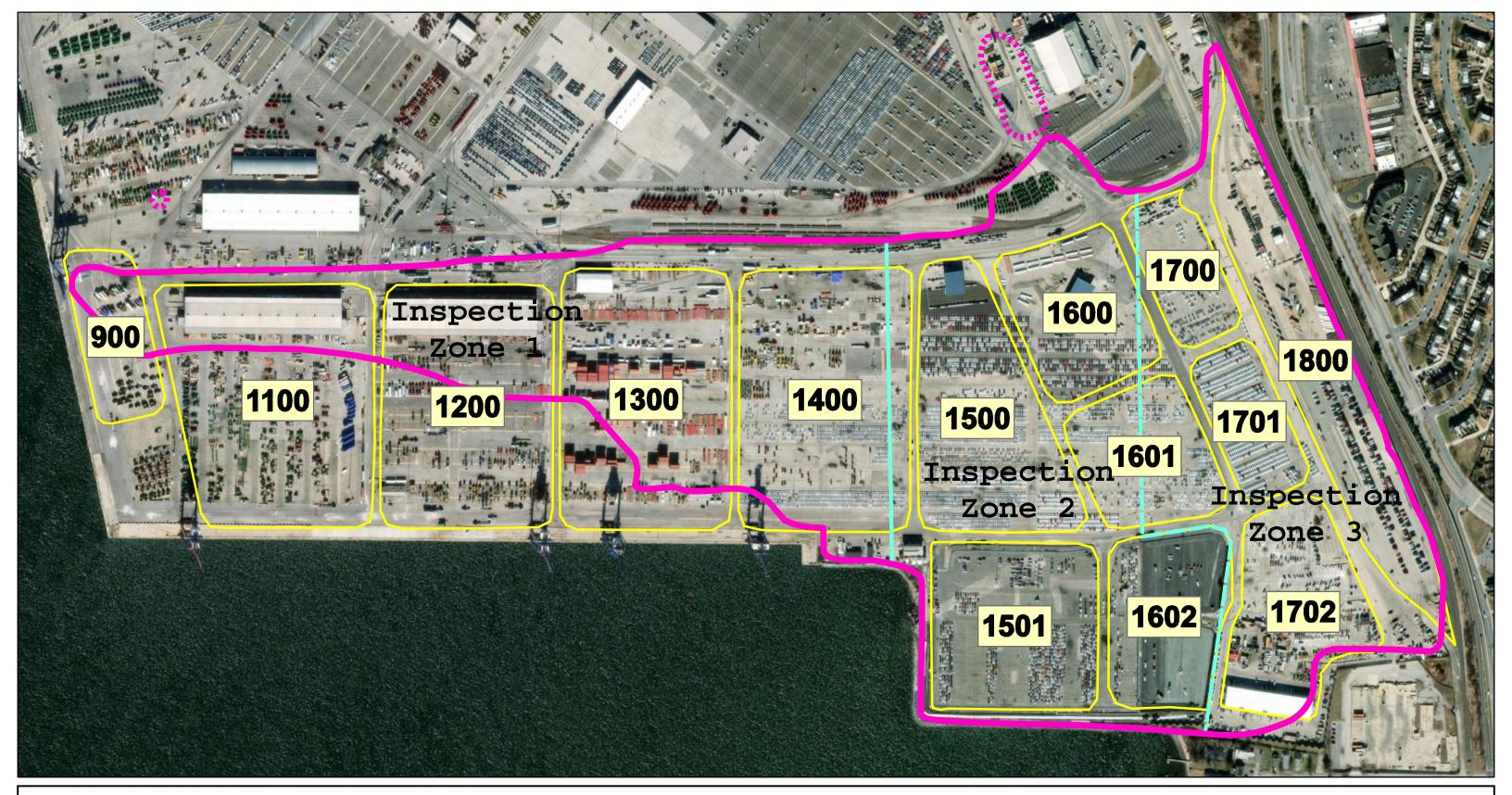


Legend

- Priority Storm Drain System
- ---- City/County Boundary
- Approximate COPR Extent
- COPR Extent
- Areas
- DMT Boundary

Figure 1 COPR Fill Area Dundalk Marine Terminal Baltimore, Maryland







Operational Area

COPR Extent

Approximent COPR Extent

Surface Cover Inspection Zone Boundary



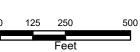
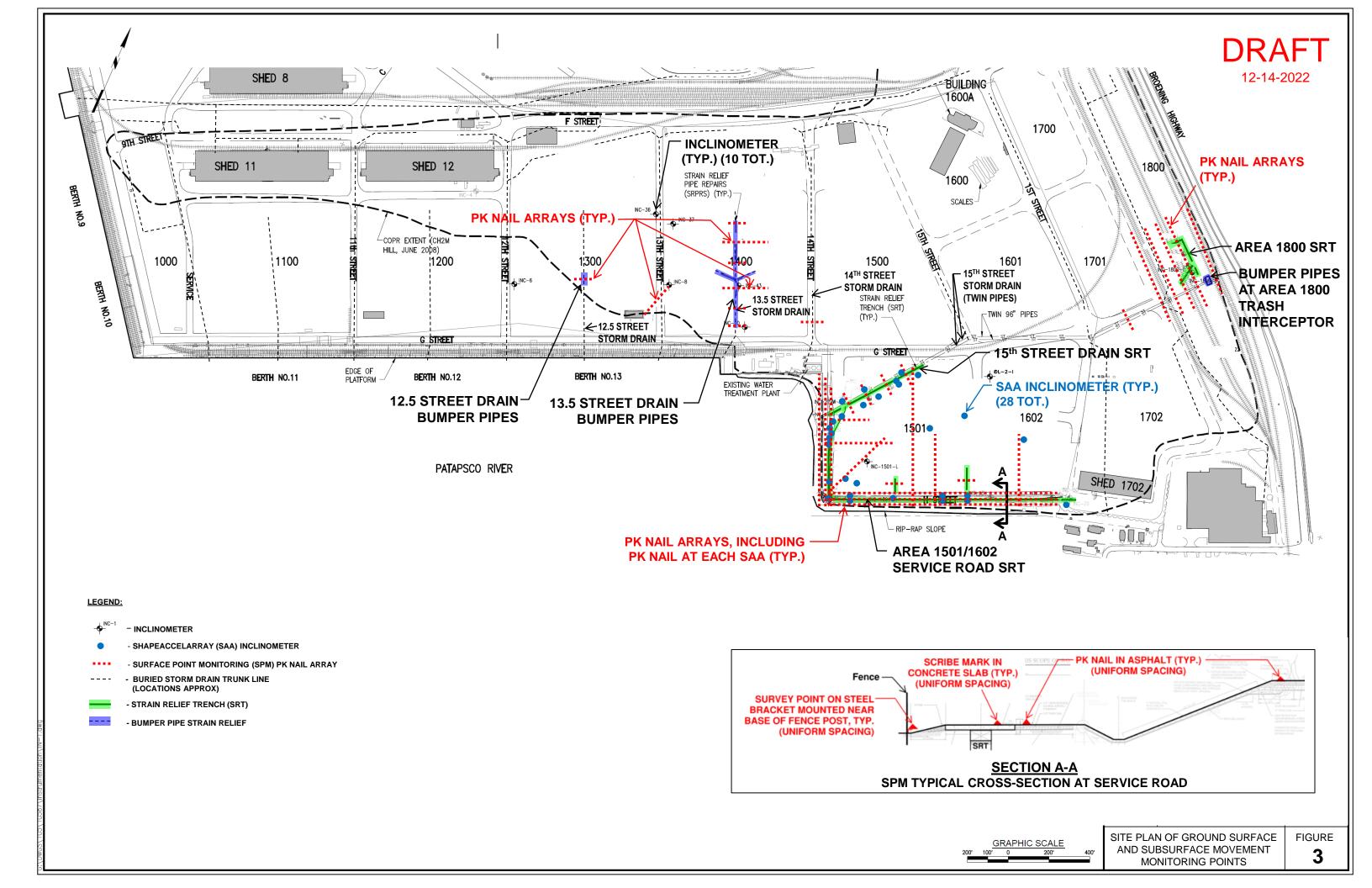


FIGURE 2 2023 Surface Cover Inspections, Inspection Zones Dundalk Marine Terminal Baltimore, Maryland



Appendix A Sample Surface Cover Inspection Form

CS	Pot Cor Dol	ncret	e Slat	b		LP GC MW	Grav	vel C	over	ell		MH RR	Inlet Man Hole Rail Road			Other									of _ a		
											1 - 3	" M&P	2 - Open/U	Inpaved Area	3 - Surfac	ce Crack		" Section	5 - Surface	Heaving	g Area	6 - Inlet/	7 - Monitor	8 - Gravel Cov	er Discoloration	tion	
Issu #	ie P	CS	BIDPI	LP	Cate GC	gory MW	ln	МН	RR	0	Length (feet)	Width (feet)	Length (feet)	Width (feet)	Avg. Width (inches)	Length (feet)	Length (feet)	Width (feet)	Max. Height (inches)	Length (feet)	Width (feet)	Manhole ID	Wellhead ID	Length (feet)	Width (feet)	Vegetation	Comments
		1									,	(/	, ,	(/	,	,	,	, ,	,	,	,			, ,	,		1

Appendix B Sample Surface Cover Inspection Report

Dundalk Marine 1	Terminal 2021 Surfa	ace Cover Ins	pection Data Tab	ole																							
																					_	C 1-1-4/	7	8 - Gravel		Ē	
						_						' M&P	+	Jnpaved Area	3 - Surfa		1	Repair		e Heaving		6 - Inlet/	7 - Monitor	Discolor		atio	
	-	Issue	Location	Location	Status			Categor			Length	Width	Length	Width	Avg. Width	Length	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	get	
Area	Date	Number	Longitude	Latitude		P CS	S DP L	P GC M	M In M⊦	H RR O		(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	\ Ve	Comment
97	5/1/2019	1	-76.52380	39.24798							50	2															25 abandoned fence posts
97	5/1/2019	2	-76.52390	39.24816											0.5	220											Seam and lateral cracking
97	5/1/2019	3	-76.52380	39.24819															6	115	8						
97	5/1/2019	4	-76.52380	39.24822											0.5	150											
97	5/1/2019	5	-76.52410	39.24850											0.5	162										Х	Asphalt seam
97	5/1/2019	6	-76.52400	39.24826							13	10															Pitting asphalt
97	5/1/2019	7	-76.52410	39.24823							26	11															Pitted and cracked asphalt
97	5/1/2019	8	-76.52400	39.24807		Х					60	12															Potholes in roadway
97	5/1/2019	9	-76.52370	39.24829						Х			205	4													Open gravel along railroad tracks
97	5/1/2019	10	-76.52360	39.24837											0.25	115											Asphalt seam cracking
97	5/1/2019	11	-76.52380	39.24844			+								0.5	90											Asphalt seam
97	5/1/2019	12	-76.52350	39.24814			+ +								0.25	525											Seal asphalt seam around tracks
97	5/1/2019	13	-76.52320	39.24801			+ +								0.25	50											Allicates associate
97	5/1/2019	14	-76.52310	39.24792			+ +				20	15			0.05	0.5											Alligator cracking
97	5/1/2019	15	-76.52300	39.24786			+ +								0.25	95											Dath also to you do you
97	5/1/2019	16	-76.52300	39.24756		х			+	+	27	23		+			400	-				-					Potholes in roadway Asphalt cracking in roadway next to tracks
97	5/1/2019	17	-76.52330	39.24758					+	+	22	_		+			100	5				-					,
97	5/1/2019	18	-76.52340	39.24772			++		+	+	22	8			0.5	225											Asphalt cracking on road shoulder
97 97	5/1/2019	19 20	-76.52350	39.24779						+ +	32	12			0.5	225											Seal asphalt seams
	5/1/2019		-76.52370	39.24788			++		+	+	32	12	70	-	-												Asphalt cracking
97	5/1/2019	21	-76.52340	39.24791					+	×	+		70	2								-					Open asphalt along rail
900	5/18/2019	3	-76.53450	39.24196		+	+	×	+	+	+			+					+			-	DMT 34M				Two damaged well pads.
900	5/18/2019	7	-76.53430	39.24191		+ +	+ +	X		+ +	1				0.5	235							DIVIT 34IVI				Various cracks.
900	5/18/2019		-76.53430	39.24191		+ +	+ +			+ +	1				0.5	140											Various Cracks.
900	5/18/2019	8	-76.53470	39.24169		+ +	+ +			+ +	16	2			0.25	140											-
		9		39.24169						+ +	10	2															Various cracks and conduit that runs parallel to dolly
900	5/18/2019	11	-76.53450	39.24141											0.25	250											nad.
900	5/18/2019	12	-76.53420	39.24146							32	13															pad.
900	5/18/2019	15	-76.53420	39.24150											0.25	270											
900	5/18/2019	20	-76.53350	39.24147											0.25	125											
900	5/18/2019	23	-76.53360	39.24170											0.75	38											Cracks around concrete manhole slab.
900	5/18/2019	24	-76.53350	39.24143											0.5	90											
900	5/18/2019	25	-76.53350	39.24133		х					3	7															Two potholes near dolly pad.
900	5/18/2019	26	-76.53360	39.24127			х								0.5	400											Both sides of dolly pad.
900	5/18/2019	28	-76.53390	39.24116											0.25	170											Conduit raceway. Unsealed on both sides.
900	6/12/2019	30	-76.53420	39.24289		х					25	4															
1100	6/11/2019	1	-76.53380	39.24240															8	350	10						
1100	6/11/2019	2	-76.53370	39.24225											0.25	500											Asphalt seams
1100	6/11/2019	3	-76.53320	39.24241					x		10	5															Repair asphalt around inlet
1100	6/11/2019	4	-76.53230	39.24259											0.5	100											
1100	6/11/2019	5	-76.53330	39.24234				х			ļ			1													
1100	6/11/2019	6	-76.53340	39.24209	<u> </u>	х			$\perp \perp \perp$	$\perp \perp \perp$	5	3	<u> </u>														
1100	6/11/2019	9	-76.53340	39.24192	<u> </u>	\perp			\bot	\bot			120	1												х	Gap on ramp
1100	6/12/2019	12	-76.53240	39.24186		x x		\bot	\bot	+	110	3		1					 								
1100	6/12/2019	13	-76.53230	39.24190	ļ	хх			1		30	4	-	1		ļ	1	1	 		1	ļ					
1100	6/12/2019	14	-76.53210	39.24211		\bot	+		+	+			8	6												х	
1100	6/12/2019	15	-76.53200	39.24198		хх		+	++	+	1			+			40	15	 								
1100	6/12/2019	16	-76.53190	39.24184	1	+	+	++	++	+		1	1			205	1		6	75	15						
1100	6/12/2019	17	-76.53220	39.24204	 	+	.	+ +	+	+	200	-	+	+	1	200	<u> </u>	1	 		<u> </u>	 					_
1100	6/12/2019	18	-76.53150	39.24223		хх	++	++	++	++	200	3		+	0.5	75	-	-	 		-						
1100 1100	6/12/2019 6/12/2019	19 20	-76.53160 -76.53160	39.24198 39.24188	1	++	+	+	++	++		1	1		0.5	75	1		8	90	15	-					-
1100			-76.53160 -76.53110	39.24188	+	+	+	+	++	+	1	1	+	+	0.25	70	1	-	0	90	13	-					Seal conduit saw cut
1100	6/12/2019 6/12/2019	21 22	-76.53110 -76.53060	39.24231		++-	+ +		+ +	+ + -	9	2		+	0.25	/0		1	+			 					Four abandoned fence post
1100	6/12/2019	23	-76.53060	39.24231		++	+	++	++	++	9		+	+					6	50	10	<u> </u>					Tour abandoned rence post
1100	6/12/2019	24	-76.53000	39.24223	1	++	+	++	++	++-	55	7	+	+		 	 			30	10	 					Cracked asphalt
1100	6/12/2019	25	-76.53110	39.24248	1	+	+++	+++	++	++-	35	10	1	+	<u> </u>	 	 	-	 		 						Cracking at bottom of ramp
1100	6/12/2019	26	-76.53100	39.24268		+			++	++	30	7		+				-	 			 					Cracks in asphalt
1100	6/12/2019	27	-76.53120	39.24285	†	+			++	+	40	4	1			1	1		 		1	†					
1100	6/12/2019	28	-76.53090	39.24288	†	+			++	+	8	4	1			1	1		 		1	†					_
1100	6/12/2019	29	-76.53110	39.24295		+			++	++-	"	7		+			10	10	 			 					Remove stop light foundation
1100	6/12/2019	30	-76.53110	39.24315		x			+ +	 ^	4	4		1			10		†			İ					
1100	6/12/2019	31	-76.53110	39.24310	<u> </u>	x	+ +	+ +	+ +	1 1	12	3	1						†			1					
	-,, -015			1	<u> </u>	+			1 1			<u> </u>	1						 								
<u> </u>	1	1	<u> </u>		1							1			<u> </u>	1	1	·	ıL		1		1	l.			_1

Dundalk Marine 1	Terminal 2021 Surfa	ce Cover Insp	ection Data Tab	ole																						
							4 0"	1 - 3" M&P			2.6.6		4 50		5.0.6			C Inlat/	7 Manitas	8 - Gravel Cover	Ē					
	Ī	lasus	Location	Location	Status	7	Category		1 - 3" Length	Width	2 - Open/C Length	Inpaved Area Width	3 - Surfa Avg. Width	Length	4 - FS Length	Repair Width	5 - Surfa Max. Height	ce Heaving / Length	Area Width	6 - Inlet/ Manhole	7 - Monitor Wellhead	Discoloration Length Width	tatio			
Area	Date	Issue Number	Longitude	Latitude	Status	P CS	DP IP	GC MW	In MH	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet) (feet)	ege	Comment
1200	6/13/2019	1	-76.53060	39.24277		. ss	+	GG IVIVV			(1001)	(1000)	50	1	(menes)	(1001)	(1001)	(1000)	(menes)	(1000)	(1001)	,,,	.5	(rect) (rect)	>	Gap in concrete slab
1200	6/13/2019	2	-76.53060	39.24277		 ^					35	13	30	1												Cracking asphalt
1200	6/13/2019	6	-76.53000	39.24266		хх					33	13					40	4								ordening dispirate
1200	6/13/2019	7	-76.52990	39.24273		x	+				55	1														
1200	6/13/2019	8	-76.52980	39.24274		хх											10	7								
1200	6/13/2019	9	-76.52960	39.24282		хх											15	15								
1200	6/13/2019	10	-76.52950	39.24292		хх					7	3														
1200	6/13/2019	11	-76.52930	39.24275						х																Damaged water valve manhole
1200	6/13/2019	12	-76.52920	39.24253					х		10	5														Loose inlet frame
1200	6/13/2019	13	-76.52900	39.24314											0.75	30										
1200	6/13/2019	14	-76.52870	39.24312		хх	+ +				30	1														
1200	6/13/2019	15	-76.52850	39.24305		+		+			10	6														Consiste a new holls
1200	6/13/2019	16	-76.52840	39.24259		+ +	+ +	+ + +			20	20														Cracking asphalt
1200 1200	6/13/2019 6/13/2019	17 18	-76.52830 -76.52790	39.24266 39.24268		- L		1			35 10	10 5														
1200	6/13/2019	22	-76.52790	39.24268		X	+ +	+ + +			10	5			0.5	50									×	
1200	6/13/2019	23	-76.52840	39.24333		+ +	1				11	15			0.5	30									^	Cracked asphalt
1200	6/13/2019	26	-76.52850	39.24362		+ +	+ +			×	22	2														11 abandoned fence posts
1200	6/13/2019	27	-76.52850	39.24357							25	20														Cracked asphalt
1200	6/13/2019	29	-76.52850	39.24380							20	15														Cracked asphalt
1200	6/13/2019	30	-76.52960	39.24349							315	53														Cracked asphalt
1200	6/13/2019	31	-76.53080	39.24304		х					364	33														
1200	6/13/2019	32	-76.53080	39.24291						х	10	2														Remove two guardrail posts
1200	6/13/2019	33	-76.53070	39.24288		х					30	30														Cracked asphalt
1200	6/13/2019	34	-76.53080	39.24305		х					30	15														Cracked asphalt
1200	6/13/2019	35	-76.53080	39.24326						х							30	10								Replace curb
1200	6/13/2019	36	-76.52970	39.24360		\perp		\perp		х							300	8								Cracked asphalt
1200	6/13/2019	37	-76.52920	39.24366				х										<u> </u>					DMT-30S			Damaged well pad
1200	6/14/2019	38	-76.52900	39.24385		+	+ +	+		Х							227	7							_	Remove guardrail, replace with jersey barriers
1200 1200	6/14/2019	39 40	-76.52940	39.24386		+ +	+ +	+ + +			90	45					90	45							х	Old trailer location Cracked asphalt
1200	6/14/2019 6/14/2019	41	-76.52880 -76.52880	39.24399 39.24389		+ +	+ +	+ + +			90	45			0.5	155										Cracked aspirare
1200	0/14/2013	71	70.32000	33.24303		1 1				1					0.5	155										
1300	5/1/2019	1	-76.52500	39.24271				х																		Damaged well pad
1300	5/1/2019	2	-76.52500	39.24267				х																		Damaged well pad
1300	5/29/2019	3	-76.52620	39.24224					х		30	15										TEV-004				Pave around vault manholes
1300	5/29/2019	4	-76.52570	39.24239													30	15								Settlement
1300	5/29/2019	5	-76.52600	39.24232							80	4														
1300	5/29/2019	6	-76.52540	39.24245															10	127	15					
1300	5/29/2019	7	-76.52520	39.24249		\perp		\perp							0.5	135									х	Seal crack below sidewalk
1300	5/29/2019	8	-76.52530	39.24264		+	x	+			60	35													_	Cracked asphalt
1300	5/29/2019	9	-76.52520	39.24271		х	х	+ + +			12	3														
1300 1300	5/29/2019	10	-76.52500 -76.52530	39.24259 39.24255		+ +	+ +	+	+		2 25	20													+	
1300	5/29/2019 5/29/2019	11 12	-76.52550	39.24263		x	x	+++			8	3														
1300	5/29/2019	13	-76.52600	39.24253		+^+	 ^ -	+ + +	+		3	,		+	0.25	35		+				<u> </u>			+	Seal patches
1300	5/29/2019	14	-76.52620	39.24243		+ +	+ +	 	\dashv				20	15	0.25	1 33		1	1							· ·
1300	5/29/2019	15	-76.52640	39.24253	1	1	х	1	1		7	3	1	1				1				1				
1300	5/29/2019	16	-76.52630	39.24255							32	7						L							L	
1300	5/29/2019	17	-76.52610	39.24266							20	8														
1300	5/29/2019	18	-76.52610	39.24274							21	10														
1300	5/29/2019	19	-76.52520	39.24295		$\bot \bot$		$\bot \bot \bot$	х		18	14										MH-314				
1300	5/29/2019	20	-76.52530	39.24312		+ +	$\bot \bot$	+					ļ			ļ			14	52	25					Heave in roadway
1300	5/29/2019	21	-76.52610	39.24283		х	+	+ $+$ $+$	\perp		35	4	<u> </u>	1		1		1								
1300	5/29/2019	22	-76.52660	39.24283		++	+	+			32	20				_		-								
1300	5/29/2019	23	-76.52630 76.52650	39.24262	-	++	+	+++			20	20		-				+	-			-			-	
1300	5/29/2019	24	-76.52650 -76.52680	39.24262	-	++	+	+++	+		35	35		+		1		+	-			-			-	Repair DMT-37M
1300 1300	5/29/2019 5/29/2019	25 26	-76.52680 -76.52670	39.24278 39.24289		+ +	+	+ + +	+				1	+		1		+	10	25	20	-			+	перан имп-элм
1300	5/29/2019	27	-76.52670	39.24289		++	+	x	+	v				+		1		+	10	43	20		DMT-6S		+	
1300	5/29/2019	28	-76.52540	39.24337		+ +	+ +	 ^ 	-	 ^	125	3		+				+					DIVIT-03			+
1300	5/29/2019	29	-76.52550	39.24356		х	+ +	 	\dashv		115	3	1	1				1	1							Seam separation
1300	5/29/2019	34	-76.52550	39.24371	1	1		1			60	3		1				1				1				Separated asphalt seams
1300	5/29/2019	35	-76.52550	39.24367	İ	1 1	1	1					Ì		0.5	65						İ				
1300	5/30/2019	36	-76.52570	39.24386											0.5	120										Cracks at asphalt seams
1300	5/30/2019	37	-76.52620	39.24350															6	100	15					

Dundalk Marine T	Terminal 2021 Surfa	ice Cover Insp	ection Data Tab	le																								
								1 - 3'	' N / Q. D	2 - Open/Unpaved Area		3 - Surfa	co Crack	/ EC	Repair	5 Surf	ace Heaving	Aroa	6 - Inlet/	7 - Monitor	8 - Grave Discolo		5					
		Issue	Location	Location	Status	Category			Length	Width	Length	Width	Avg. Width	Length	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	tatic				
Area	Date	Number	Longitude	Latitude	Status	P CS	DP LP	GC MW	In MH	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	ege/		Comment
1300	5/30/2019	38	-76.52690	39.24342			++	1 10.00			22	12	(,	(7	, ,	(,	(,	(,	, , , , , ,	(,	(,			,,	(,			comment
1300	5/30/2019	39	-76.52740	39.24328		x					5	6																
1300	5/30/2019	40	-76.52760	39.24305		х					22	6																
1300	5/30/2019	41	-76.52770	39.24319		х					25	4															Seam se	paration
1300	5/30/2019	42	-76.52780	39.24326											0.25	35												
1300	5/30/2019	49	-76.52580	39.24404							36	3																
1300	5/30/2019	50	-76.52580	39.24409											0.5	70											Seal saw	
1300	5/30/2019	51	-76.52580	39.24412			+ +						2	2				1									Hole in a	sphalt
1300	5/30/2019	52	-76.52680	39.24382			+ +	+ +		1					0.5	100			8	130	15							
1300 1300	5/30/2019 5/30/2019	54 62	-76.52700 -76.52810	39.24375 39.24370		x	+ +	+ +			25	3	1		0.5	100		1										
1300	5/30/2019	63	-76.52680	39.24416		1^	+ +				23						256	10									Damage	d bollards and charging stations
1300	5/30/2019	64	-76.52650	39.24405											0.5	70											1 101	0 0
1300	6/3/2019	65	-76.52610	39.24454											0.75	80											Seal sear	n
1300	6/3/2019	66	-76.52640	39.24446											0.5	100												
1300	6/3/2019	67	-76.52680	39.24448															6	50	10							
1300	6/3/2019	68	-76.52720	39.24426							32	15																
1300	6/3/2019	69	-76.52760	39.24402		х	+				15	15																
1300	6/3/2019	70	-76.52780	39.24395			+ +	+ +		1		40			0.25	150												
1300 1300	6/3/2019 6/3/2019	71 72	-76.52810 -76.52700	39.24387 39.24444		+	+	+ +		-	50 350	10 8																
1300	6/3/2019	73	-76.52670	39.24466		+ +	+++	+		*	330	٥							6	75	10							
1300	6/3/2019	74	-76.52660	39.24465			+ +				70	7								,,,	10						Remove	curb
1300	6/3/2019	75	-76.52650	39.24463											0.25	90		İ									Asphalt :	seam around electric manhole
1300	6/3/2019	76	-76.52630	39.24475															12	85	15							
1300	6/3/2019	77	-76.52630	39.24480							30	25															Cracking	asphalt
1300	6/3/2019	78	-76.52720	39.24437		х							30	20														
1300	6/3/2019	79	-76.52760	39.24424							225	3															Seam se	
1300	6/3/2019	80	-76.52800	39.24425		-	+	+			85	15																water adjacent to building
1300	6/3/2019	81	-76.52840	39.24406			+ +	+ +	Х	1	6	6																asphalt around IN-688 rebar in asphalt
1300 1300	6/3/2019 6/3/2019	83 84	-76.52740 -76.52620	39.24450 39.24496		+ +	+++	+							0.25	110											Cut back	Tebai iii aspiiait
1300	6/3/2019	85	-76.52680	39.24481			+ +								0.25	250												
1300	6/3/2019	86	-76.52730	39.24476									12	10				İ										
1300	6/3/2019	87	-76.52780	39.24444															6	130	8							
1300	6/3/2019	88	-76.52850	39.24420											0.25	150											Seal sear	ms
1300	6/3/2019	89	-76.52800	39.24438											0.5	130												
1100	5 /s /2010		76 50000	20.24240																	20							
1400 1400	5/1/2019	1	-76.52380 76.52300	39.24319		+	+	+ +		-					0.5	6E			8	145	20							
1400	5/1/2019 5/1/2019	3	-76.52390 -76.52480	39.24313 39.24284			++-	+			41	3			0.5	65											Δsnhalt :	seam separating under puddle
1400	5/1/2019	4	-76.52500	39.24286		х	+ +				8	4															Two pot	, ,
1400	5/1/2019	5	-76.52490	39.24293			+ +	х																				d well pad
1400	5/1/2019	6	-76.52490	39.24298		х					6	4															Pothole	at seam
1400	5/1/2019	7	-76.52430	39.24321											0.5	130											Unsealed	d asphalt patch and surrounding cracks
1400	5/1/2019	8	-76.52370	39.24328	1	+	+	++-	\vdash	 			1		0.5	170	 					-					Crack un	der ponding
1400	5/1/2019	9	-76.52370 -76.52330	39.24328 39.24348	1	+ +	+ +	+ +		 			+		0.5	100	1	1			1	 				х		ong asphalt seam
1400	5/1/2019	10	-76.52300	39.24351		+ +	+ +	1 1		 			1		3.3	100		1	10	110	15	<u> </u>					c. ack alc	
1400	5/1/2019	11	-76.52260	39.24364	1		1 1						1			1	1		10	70	20						Heave in	roadway
1400	5/1/2019	12	-76.52290	39.24375											0.5	100										х		
1400	5/1/2019	13	-76.52330	39.24362							20	20															Alligator	cracking
1400	5/1/2019	14	-76.52390	39.24348			\bot								0.5	85	<u> </u>											
1400	5/1/2019	15	-76.52510	39.24306	<u> </u>	+	+ +	+	х	\vdash	8	8	1				<u> </u>				1					-	Damage	d asphalt around inlet grate at IN-777
1400	5/1/2019	16	-76.52480	39.24264	1	+ +	+	1 1	\vdash	+			1		0.5	30		1		1	1	<u> </u>				1	Cracke	ong asphalt soams
1400 1400	5/1/2019 5/1/2019	17 19	-76.52440 -76.52320	39.24274 39.24322	1	+ +	+ +	+ + -	\vdash	 		1	+		0.5 0.25	300 63	1			1	1	-				 	_	ong asphalt seams d asphalt patch
1400	5/1/2019	20	-76.52320	39.24322	<u> </u>	+ +	+ +	1	\vdash				+		0.23	0.5	 		12	33	20						Jiisealet	- III - III
1400	5/1/2019	21	-76.52310	39.24326		1 1	+ +						1		0.25	37				- 33								
1400	5/1/2019	22	-76.52280	39.24330			1 1						İ		0.5	165	Ì					İ						
1400	5/1/2019	24	-76.52260	39.24347											0.5	100												
1400	5/1/2019	25	-76.52260	39.24335															12	155	10							
1400	5/1/2019	27	-76.52290	39.24345	ļ		$\bot \bot$			$\perp \perp$					0.5	222											Cracks a	ong asphalt seam
1400	6/4/2019	28	-76.52610	39.24498	<u> </u>	+	+	+		\vdash	116	6	1			<u> </u>	<u> </u>									-		
1400	6/4/2019	29	-76.52580	39.24498		+ +	+	+ +		 			+		0.35	420		1	6	6	6	 				-	Acabalt	saams
1400	6/4/2019	30	-76.52550	39.24515	I					1	l	1	1	1	0.25	420	<u> </u>	1	L		<u> </u>	<u> </u>	ı			1	Asphalt :	ocums .

Dundalk Marine Terminal 2021 Surface Cover Inspection Data Table																	ī	8 - Gravel Cover									
								1 - 3	" M&P	2 - Open/l	Unpaved Area	3 - Surfa	ce Crack	4 - FS F	Repair	5 - Surf	ace Heaving A	rea	6 - Inlet/	7 - Monitor		oration	ion				
		Issue	Location	Location	Status		Category		Length	Width	Length	Width	Avg. Width	Length	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	etat			
Area	Date	Number	Longitude	Latitude		P CS	S DP	LP GC	MW I	In MH RR	O (feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Veg	Comment
1400	6/4/2019	31	-76.52530	39.24513															12	35	12						
1400	6/4/2019	33	-76.52490	39.24529															10	44	8						
1400	6/4/2019	34	-76.52440	39.24572									23	5													
1400	6/4/2019	36	-76.52430	39.24553 39.24570									1		0.5	135			10	32	20						
1400 1400	6/4/2019 6/4/2019	39 40	-76.52390 -76.52380	39.24570											0.5 0.5	50											+
1400	6/4/2019	41	-76.52380	39.24558				+	+						0.5	90											+
1400	6/4/2019	42	-76.52380	39.24547											0.5	30			6	72	15						
1400	6/4/2019	43	-76.52430	39.24551							x 100	2							-								Remove 50 fence posts
1400	6/4/2019	44	-76.52410	39.24537															12	133	10						
1400	6/4/2019	45	-76.52400	39.24521		х					3	3															
1400	6/4/2019	46	-76.52370	39.24531				х							0.5	72										х	
1400	6/4/2019	47	-76.52360	39.24512											0.25	115											
1400	6/4/2019	49	-76.52390	39.24509					Х														P-6				Damaged well pad
1400	6/5/2019	50	-76.52260	39.24325		-			+ +						0.5	100											Cool watch
1400 1400	6/5/2019 6/5/2019	51 52	-76.52260 -76.52300	39.24322 39.24316		$\vdash\vdash$	+	x	++	+		1	+		0.5 0.5	92 72										v	Seal patch
1400	6/5/2019	52	-76.52300 -76.52280	39.24316		\vdash	+	^	++	++		1	+		0.5	12			12	55	10		1		1	х	+
1400	6/5/2019	54	-76.52280	39.24310			+		+			1	+		0.5	200			14	33	10					х	+
1400	6/5/2019	55	-76.52300	39.24307			+	\dashv	$\dagger \dagger$			1	+		0.25	185											Seal patches
1400	6/5/2019	59	-76.52350	39.24288					T		6	2	1										1		1		
1400	6/5/2019	60	-76.52400	39.24270																							Missing hydrant
1400	6/5/2019	61	-76.52400	39.24275				х							0.5	72										х	
1400	6/5/2019	62	-76.52420	39.24264											0.5	75											
1400	6/19/2019	64	-76.52310	39.24230									6	2													Puncture in asphalt
1400	6/19/2019	65	-76.52340	39.24239						+		-	2	2													Hole in asphalt
1400	6/19/2019	66	-76.52330	39.24246		-			+ +						0.75	22										Х	Seal around crane stop
1400 1400	6/19/2019 6/19/2019	67 68	-76.52260 -76.52270	39.24258 39.24244				-	+				24	3	0.5	240											Seal surface cracks around conduit
1400	6/19/2019	69	-76.52270	39.24244									24	3	0.5	240											Seal asphalt at edge of containment wall
1400	6/19/2019	70	-76.52230	39.24278				-					20	6	0.5	240											Scar aspirare at eage or contaminent wan
1400	6/19/2019	71	-76.52240	39.24273									2	2													Abandoned fence posts
1400	6/19/2019	72	-76.52250	39.24298							60	3															Cracking asphalt over conduit
1400	6/19/2019	75	-76.52320	39.24275			х										21	4									Repair dolly pad seam
1400	6/19/2019	78	-76.52310	39.24288															8	58	30						
1400	6/17/2019	80	-76.52580	39.24435		х					120	3															Repair asphalt seam
1400	6/17/2019	81	-76.52540	39.24456											0.5	200											
1400	6/17/2019	82	-76.52480	39.24459		-			+ +				47		0.25	375											
1400 1400	6/17/2019 6/20/2019	84 85	-76.52400 -76.52600	39.24497 39.24461				_		++-	35	4	17	3													
1400	6/20/2019	86	-76.52560	39.24454		х		-			35	4			0.5	125											-
1400	6/20/2019	87	-76.52580	39.24495				-			x				0.5	123	350	12									Remove guardrail replace with jersey barrier
1400	6/21/2019	88	-76.52570	39.24406							30	3					550										
1400	6/21/2019	89	-76.52450	39.24452			\top		t		 		1		0.5	520											Seal seams
1400	6/21/2019	90	-76.52460	39.24447															8	50	15						
1400	6/21/2019	92	-76.52330	39.24464							35	3															
1400	6/21/2019	94	-76.52480	39.24416				х									23	15									
1400	6/21/2019	95	-76.52480	39.24401			\perp		$\bot \bot$	\perp		1			0.25	20											Seal conduit paving
1400	6/21/2019	97	-76.52530	39.24394		$\vdash \vdash$	+		++	\bot		1	1		1	<u> </u>			6	70	10						
1400	6/21/2019	98	-76.52550	39.24374		$\vdash \vdash$	+	-	++	++	10	4	+		 	 			42	4.60	20						
1400	6/21/2019	99	-76.52470 76.52420	39.24369		\vdash	+	+	++	++	+ +	+	+		0.25	200			10	160	20		-		-		
1400 1400	6/21/2019 6/21/2019	100 101	-76.52430 -76.52330	39.24378 39.24434		\vdash	+		++	+	+ +	1	+		0.25	200			6	58	10		 		 		+
1400	6/21/2019	101	-76.52330	39.24432		\vdash	+	+	++	++		+	+		0.5	100			-	30	10						Seal seams
1400	6/21/2019	103	-76.52320	39.24411			+	\dashv	++		1 1		1		J.,J	100	25	13									
1400	6/21/2019	104	-76.52330	39.24417					t		1 1	1	1				18	9									
1400	6/21/2019	105	-76.52340	39.24395									L						8	60	8						
1400	6/21/2019	106	-76.52320	39.24394											0.5	470										х	
1400	6/21/2019	107	-76.52390	39.24376															12	100	25						
1400	6/21/2019	108	-76.52400	39.24373			х		$\bot \bot$			1					36	10									Remove dolly pad
1400	6/21/2019	109	-76.52440	39.24357		$\vdash \vdash$	+		++	\bot		1	1		0.5	300											Seal seams
1400	6/21/2019	110	-76.52490 -76.52450	39.24334		$\vdash \vdash$	+	х	++	+		1	+		1	1			16	23	16		-		-		
1400	6/21/2019	111	-76.52450 76.52300	39.24338			+	-	++	+	+ +	1	+		0.5	200			10	84	15		1		1		+
1400 1400	6/21/2019 6/21/2019	112 113	-76.52290 -76.52310	39.24391 39.24401		\vdash	х	-	++	++	1 1	+	+		0.5	300	32	128					 		 		Remove dolly pad
1400	0/21/2013	113	, 0.52510	33.24401		\vdash	1		++			1	+				52	120									
	I	i									1 1	1	1		1	1	1		1	1			1	1	<u> </u>	l	_1

Dundalk Marir	e Terminal 2021 Surfa	ce Cover Insp	ection Data Tabl	e											1									0 640	al Causa		
											1-3	3" M&P	2 - Open	Unpaved Area	3 - Surfa	ace Crack	4 - FS F	Repair	5 - Surf	ace Heaving Ar	rea	6 - Inlet/	7 - Monitor		el Cover oration	uo	
		Issue	Location	Location	Status	1		Cat	egory		Length	Width	Length		Avg. Width	_	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	etati	
Area	Date	Number	Longitude	Latitude		Р	CS DP		MW	In MH R	R O (feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Vege	Comment
1500	5/1/2019	1	-76.51960	39.24373							19	9															1
1500	5/1/2019	2	-76.52000	39.24388											0.25	38											
1500	5/1/2019	3	-76.51990	39.24381							10	10															Concrete patch with cracking
1500	5/1/2019	4	-76.51980	39.24368		х					4	4															
1500	5/1/2019	5	-76.52030	39.24371					-		8	3															Puncture in asphalt
1500	5/1/2019	7	-76.52390	39.24597				 		-			8	5										1			Open area at edge of asphalt
1500 1500	5/1/2019 5/1/2019	10	-76.52360 -76.52330	39.24608 39.24623							+ 1		8	5										0	6		Open area at edge of asphalt Discolored gravel adjacent to railroad switch
1500	5/1/2019	12	-76.52370	39.24576							x 12	2												8	0		Remove sic abandoned fence posts
1500	5/1/2019	13	-76.52330	39.24599						х	24	2															Asphalt damage around inlet grate at IN-801
1500	5/1/2019	14	-76.52330	39.24604											0.5	150											
1500	5/1/2019	16	-76.52310	39.24606							x 22	2															Remove 11 abandoned fence posts
1500	5/1/2019	17	-76.52300	39.24595							66	9															Cracked asphalt next to canopy
1500	5/1/2019	18	-76.52310	39.24592			x								0.75	360										x	Crack between concrete slab and asphalt around
1500	5/1/2019	19	-76.52340	39.24582											0.25	130											canopy Crack for conduit sawcut and adjacent crack
1500	5/1/2019	20	-76.52360	39.24574					1 1	х	28	4															Damaged asphalt around IN-803
1500	5/1/2019	21	-76.52360	39.24550							40	2											<u> </u>		<u> </u>		Severe cracks in roadway
1500	5/1/2019	22	-76.52350	39.24549															10	38	20						
1500	5/1/2019	23	-76.52340	39.24542		х					37	6															
1500	5/1/2019	24	-76.52330	39.24544		$\sqcup \bot$	\perp		\bot										12	114	12		ļ	ļ	ļ		
1500	5/1/2019	25	-76.52310	39.24556					+										12	57	10						May 20 2024 discouries and the different 45% 02% 40%
1500	5/1/2019	26	-76.52300	39.24562															18	116	10						May 20, 2021 dimensions updated from 16"x82'x10' to 18"x116'x10'
1500	5/1/2019	27	-76.52300	39.24563															18	112	10						May 20, 2021 dimensions updated from
1500	5/1/2019	28	-76.52280	39.24578					+ +										8	88	25						18"x112x10'.
1500	5/1/2019	29	-76.52280	39.24575				 	-						0.5	225			0	00	25					х	+
1500	5/1/2019	30	-76.52260	39.24576					1 1						0.5	223			14	76	15					^	
1500	5/1/2019	31	-76.52260	39.24565				1 1											16	164							5/20/2021 Dimensions updated from 14"x164'x20'
																			16	104	20						to 16"x164'x20'
1500	5/1/2019	32	-76.52280	39.24552					1-1		44	10															Cracked asphalt with ponding
1500 1500	5/1/2019 5/1/2019	33 34	-76.52290 -76.52310	39.24545 39.24542		-			-		+ +						24	23	6	42	10						Alligator cracking
1500	5/1/2019	35	-76.52320	39.24542				 	-		24	24							6	42	10						+
1500	5/1/2019	36	-76.52330	39.24529							24	24			0.5	180											Cracks along and adjacent to asphalt seams
1500	5/1/2019	37	-76.52340	39.24520					1 1						0.25	350											Seal asphalt seam
1500	5/1/2019	38	-76.52280	39.24480				х											8	16	16						Heave under light pole
1500	5/1/2019	39	-76.52280	39.24484			х								0.5	20											Seal transformer pad
1500	5/1/2019	41	-76.52170	39.24515											0.5	300											Seal asphalt seam
1500	5/1/2019	42	-76.52170	39.24514															10	55	12						<u> </u>
1500	5/1/2019	43	-76.52180	39.24494					1-1						0.25	250				40	20						Reseal asphalt seam
1500 1500	5/1/2019 5/1/2019	44 45	-76.52220 -76.52230	39.24490 39.24489		-			-		+ +				0.5	220			14	40	30					x	Asphalt seam
1500	5/1/2019	46	-76.52230	39.24483					1 1		30	5			0.5	220										Х	Pitted asphalt
1500	5/1/2019	47	-76.52240	39.24486		х			+		15	10											<u> </u>	<u> </u>	<u> </u>		
1500	5/1/2019	48	-76.52250	39.24483					\dagger		30	10											1	1	1		Alligator cracking
1500	5/1/2019	49	-76.52260	39.24481															14	70	20						
1500	5/1/2019	50	-76.52260	39.24476							16	10										-					Alligator cracking
1500	5/1/2019	51	-76.52280	39.24465							1 1								8	68	15						
1500	5/1/2019	52	-76.52280	39.24466		$\vdash \vdash$	\perp		++	\perp	+	1			0.5	100											Asphalt seam
1500	5/1/2019	53	-76.52280	39.24460		\vdash	+		+		+ + -	1					17	12					1	1	1		Alligator cracking
1500 1500	5/1/2019 5/1/2019	54 55	-76.52280 -76.52290	39.24459 39.24457		\vdash	-	\vdash	++		+ + -	+				1	19 20	10 14						-			Alligator cracking Alligator cracking
1500	5/1/2019	56	-76.52290 -76.52290	39.24457		+	+		+		+ +	+					20	13					 	 	 		Alligator cracking
1500	5/2/2019	57	-76.52290	39.24435		\vdash	+		++		1 1	1							6	50	10						<u> </u>
1500	5/2/2019	58	-76.52280	39.24432					1 1		28	20									-		1	1	1		Alligator cracking
1500	5/2/2019	59	-76.52260	39.24422															8	23	12						
1500	5/2/2019	60	-76.52260	39.24437		ШΤ									0.25	40						-					
1500	5/2/2019	61	-76.52240	39.24446															10	113	10						
1500	5/2/2019	62	-76.52230	39.24435		$\vdash \vdash$	\perp		++	\perp	1		2	2													Coring location
1500	5/2/2019	63	-76.52220	39.24437		$\vdash \vdash$	+		++	+	58	13											-	-	-		Cracked asphalt along seam
1500	5/2/2019	64 67	-76.52220 -76.52200	39.24437 39.24455		\vdash	+	\vdash	++	+	40	12							10	46	10						+
1500 1500	5/2/2019 5/2/2019	68	-76.52200 -76.52190	39.24453		++	+		++	++	+ +	+			0.25	370			10	40	10						Seal asphalt patches
1500	5/28/2019	69	-76.52180	39.24462					+		44	23			0.23	3,0							<u> </u>	<u> </u>	<u> </u>		Cracking asphalt
1500	5/28/2019	70	-76.52170	39.24466					1 1		24	12				1											Cracking asphalt
															•	•										•	·

Same Location Location Location Location Location Location Location Location Location Location Location Location Location Location Location Location Location Location Location P CS DP LP GC MW In MH RR O (feet)	(feet) 5 120 25 460 5 560	-	oair Width (feet)	5 - Surface Heaving Max. Height Length (inches) (feet)	Area Width (feet)	6 - Inlet/ Manhole ID	7 - Monitor Wellhead ID	8 - Grave Discolor Length (feet)		Vegetation	Comment
Same Location Location Location Location Location Location Location Location Location Location Location Location Location Location Location Location P CS DP LP GC Mw In MH RR O (feet) (fee	Vidth Length (feet) 5 120 25 460 5 560	Length	Width	Max. Height Length	Width	Manhole	Wellhead	Length	Width	Vegetatic	Comment
Area Date Number Longitude Latitude P CS DP LP GC MW In MH RR O (feet) (f	(feet) 5 120 25 460 5 560	-								Vege	Comment
1500 5/28/2019 72 -76.52130 39.24467 x 5 5 1500 5/28/2019 73 -76.52130 39.24469 0.5 1500 5/28/2019 74 -76.52150 39.24453 0.25 1500 5/28/2019 75 -76.52150 39.24456 0.5 1500 5/28/2019 76 -76.52270 39.24454	5 120 25 460 5 560	(icety	(icct)	(menes) (rect)	(rect)	15	15	(1000)	(icct)	>	Comment
1500 5/28/2019 73 -76.52130 39.24469 0.5 1500 5/28/2019 74 -76.52150 39.24453 0.25 1500 5/28/2019 75 -76.52150 39.24456 x 0.5 1500 5/28/2019 76 -76.52270 39.24454 x 0.5	25 460 5 560								I		
1500 5/28/2019 74 -76.52150 39.24453 0.25 1500 5/28/2019 75 -76.52150 39.24456 x 0.5 1500 5/28/2019 76 -76.52270 39.24454 x 0.5	25 460 5 560										Seal asphalt seams
1500 5/28/2019 75 -76.52150 39.24456 x 0.5 1500 5/28/2019 76 -76.52270 39.24454 0.5	5 560										Seal seams for asphalt patches
1500 5/28/2019 76 -76.52270 39.24454											Seal dolly pad
	25 255	45	16				1				
1500 5/28/2019 77 -76.52290 39.24447	255	20	14								
1500 5/28/2019 78 -76.52110 39.24412 0.25											Seal asphalt patches
1500 5/28/2019 79 -76.52120 39.24403 155 3											Replace asphalt over conduit
1500 5/28/2019 81 -76.52180 39.24385 X 0.5	5 480										
1500 5/28/2019 82 -76.52180 39.24382 115 3											Replace asphalt over conduit
1500 5/28/2019 85 -76.52210 39.24368 22 2											Mill and pave along damaged seam
1500 5/28/2019 86 -76.52230 39.24355		20	12								
1500 5/28/2019 88 -76.52150 39.24375				6 90	5						
1500 5/28/2019 93 -76.52150 39.24356 0.5										х	
1500 6/17/2019 93 -76.52090 39.24449 0.5	5 190									х	
1501/1602 5/7/2019 1 -76.51720 39.24232				10 36	6						Heave near wall
1501/1602 5/22/2019 2 -76.52020 39.24298 x				8 70	15		1				From LP 15-17
1501/1602 5/22/2019 3 -76.51970 39.24265				12 90	35		1				Creative and areas and makely
1501/1602 5/22/2019 4 -76.51980 39.24344 0.5	5 2			40 240	20		1				Crack seal around patch.
1501/1602 5/22/2019 5 -76.51960 39.24319			-	10 218	30						51 feet of crack sealing around 5 guard rail posts.
1501/1602 5/22/2019 6 -76.52090 39.24125 x 1	. 51										31 leet of crack sealing around 3 guard rail posts.
1501/1602 5/22/2019 7 -76.52090 39.24130 x 5 7											Two potholes from previous boring locations.
1501/1602 5/22/2019 8 -76.52090 39.24132 0.5	5 50										
1501/1602 5/22/2019 9 -76.52080 39.24130 X				8 6	6						Heave around inlet.
1501/1602 5/22/2019 10 -76.52090 39.24171 0.5	5 70										
1501/1602 5/22/2019 11 -76.52110 39.24198 0.5	5 85										
1501/1602 5/22/2019 12 -76.52110 39.24213 0.5											
1501/1602 5/22/2019 13 -76.52060 39.24157 0.5											Multiple cracks
1501/1602 5/22/2019 14 -76.52050 39.24155 0.25											
1501/1602 5/22/2019 15 -76.52040 39.24143 0.25											
1501/1602 5/22/2019 16 -76.52040 39.24142 0.5							1				Crack seal around old concrete well pad.
1501/1602 5/22/2019 17 -76.52040 39.24163 0.5 1501/1602 5/22/2019 18 -76.52060 39.24180 x 4 4	5 90						Halmann				Tue senerate well nade. One should nad
1501/1602 5/22/2019 18 -76.52060 39.24180 x 4 4 1501/1602 5/22/2019 19 -76.52080 39.24211 0.5	5 75		-		+		Unknown				Two separate well pads. One abandoned.
1501/1602 5/22/2019 20 -76.52050 39.24211 0.5											-
1501/1602 5/23/2019 21 -76.52020 39.24157 5.55	3 30			6 72	20						+
1501/1602 5/23/2019 22 -76.52020 39.24165 0.5	5 90										
1501/1602 5/23/2019 23 -76.52050 39.24219 0.5											
1501/1602 5/23/2019 24 -76.52060 39.24240 51 4											
1501/1602 5/23/2019 25 -76.52060 39.24242 24 6											
1501/1602 5/23/2019 26 -76.52040 39.24218 0.5	5 100										Seal four asphalt seams
1501/1602 5/23/2019 27 -76.52020 39.24212 0.5	5 50										
1501/1602 5/23/2019 28 -76.52000 39.24162 0.5	5 260										Cracks in ponding area
1501/1602 5/23/2019 29 -76.51960 39.24169 0.5					1						
1501/1602 5/23/2019 30 -76.51950 39.24181 0.5	5 70				1	1					
1501/1602 5/23/2019 31 -76.52000 39.24211 13 4		 			1	1	1				Puncture in asphalt
1501/1602 5/23/2019 32 -76.52000 39.24213 0.5	5 44					-		-			
1501/1602 5/23/2019 33 -76.52010 39.24237		 	-	8 75	20	+		-			
1501/1602 5/23/2019 34 -76.52030 39.24247 x 13 10 1501/1602 5/23/2019 35 76.52030 39.24247 x 13 10 10 1501/1602		 	-		1	+		+ +			
1501/1602 5/23/2019 35 -76.52030 39.24249 12 10 1501/1602 5/23/2019 36 76.52030 39.24249 12 10 142 1		 	-		1	+		+ +			
1501/1602 5/23/2019 36 -76.51990 39.24263 x 43 8 1501/1602 5/23/2019 37 -76.51970 39.24220 0.75	75 73				+	+		+			Seal asphalt seam
1501/1602 5/23/2019 37 -76.51970 39.24220 0.75 1501/1602 5/23/2019 38 -76.51970 39.24215 x 0.25		 			+	+		+ +			Crack around electric manhole
1501/1602 5/23/2019 39 -76.51900 39.24190	27	 		24 75	50	+	1	+			
1501/1602 5/23/2019 40 -76.51920 39.24225 x 2 2				/3	30	1					+
1501/1602 5/23/2019 41 -76.51950 39.24221 0.5	5 60	 			+	1	1	† †			
1501/1602 5/23/2019 42 -76.51880 39.24197 22 9					1	1	1	† †			
1501/1602 5/23/2019 43 -76.51870 39.24218 x 14 4											
1501/1602 5/23/2019 44 -76.51880 39.24235				14 118	20	L					
1501/1602 5/23/2019 45 -76.51890 39.24250 32 3											
1501/1602 5/23/2019 46 -76.51890 39.24266 0.5	5 150										Seal asphalt seams
1501/1602 5/23/2019 47 -76.51910 39.24254 0.5	5 225										
1501/1602 5/23/2019 48 -76.51900 39.24223 1	. 80										
1501/1602 6/21/2019 49 -76.51740 39.24244				16 148	20						

Dundalk Marine T	erminal 2021 Surfa	ce Cover Insp	ection Data Tab	le																							
											1 2	' M&P	2 Onon/I	Inpaved Area	3 - Surfa	so Crask	4 EC	Repair	E Curf.	ace Heaving	A ro. 2	6 - Inlet/	7 - Monitor	8 - Grave Discolo		uc	
		Issue	Location	Location	Status	7		Category	,		Length	Width	Length	Width	Avg. Width	Length	4 - F3 Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	tatic	
Area	Date	Number	Longitude	Latitude	Status	P CS	DP LE	GC MV		RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	ege	Comment
1501/1602	6/21/2019	50	-76.51750	39.24245				1 1010			(,	(,	(,	(7	0.5	190	(,	(,	, , , , ,	(,	(,			(,	(7		Comment
1501/1602	6/21/2019	51	-76.51810	39.24243		+ +	+		+		4	4			0.3	150											
1501/1602	6/21/2019	52	-76.51840	39.24372							1	7	8	4													Conduit open area
1501/1602	6/21/2019	53	-76.51860	39.24386											0.5	100											
1501/1602	6/21/2019	54	-76.51890	39.24314															10	47	2						
N1600	5/1/2019	1	-76.52230	39.24752						х	8	2															4 abandoned fence posts
N1600	5/1/2019	2	-76.52220	39.24750				х		х			67	20													Railroad ballast
N1600	5/1/2019	3	-76.52230	39.24746						х					0.5	75										х	Crack at top of curb and curb gutter
N1600	5/1/2019	4	-76.52230	39.24736													35	23									Alligator cracking
N1600	5/1/2019	5	-76.52220	39.24735											0.75	215											Long crack plus seams for traffic light signal wire
N1600	5/1/2019	6	-76.52200	39.24740		+ +			+ +	+ +	50	8															
N1600	5/1/2019	7	-76.52180	39.24739							65	4															Alligator cracking along asphalt seams
N1600	5/1/2019	8	-76.52150	39.24741				x		х			40	15													Railroad ballast at top of drainage swale
N1600	5/1/2019	9	-76.52190	39.24730							62	12															Cracking in western most north bound lane
N1600	5/1/2019	10	-76.52230	39.24766							12	3														х	Repair asphalt
N1600	5/1/2019	11	-76.52230	39.24755						х							10	5									Dig out asphalt and reset manhole ring/lid
N1600	5/1/2019	12	-76.52240	39.24757						x	36	11														x	Abandon fence posts. Damaged asphalt from curb to
N1600	5/1/2019	13	-76.52260	39.24764			+		+	1 1	3	2															bottom of slope Small hole adjacent to electric manhole
N1600	5/1/2019	14	-76.52270	39.24764			+	+ +			3				0.25	55											Small hole adjacent to electric mannole
N1600	5/1/2019	15	-76.52280	39.24757		 	+ +		+ +	+ +	22	5			0.23	33										×	Damaged asphalt on curb
N1600	5/1/2019	16	-76.52260	39.24760									9	4													
N1600	5/1/2019	17	-76.52280	39.24751		х					25	19															
N1600	6/11/2019	18	-76.52210	39.24673															10	126	20						
N1600	6/11/2019	19	-76.52220	39.24668									6	2													Hole in asphalt
N1600	6/11/2019	20	-76.52250	39.24646											0.25	300											Seal seam
N1600	6/11/2019	22	-76.52290	39.24625						х																	Damaged well or valve cover
N1600	6/11/2019	23	-76.52230	39.24671													67	15									
N1600	6/11/2019	24	-76.52220	39.24678													25	20									
N1600	6/11/2019	25	-76.52190	39.24701											0.75	180										х	
N1600	6/11/2019	26	-76.52190	39.24692			+		1	х				_			8	5									Remove stop light foundation
N1600	6/11/2019 6/11/2019	27 28	-76.52190 -76.52200	39.24716 39.24719			+	+ +					6	2	0.5	10											Sign post
N1600 N1600	6/11/2019	28	-76.52210	39.24719			+		+ +				10	5	0.5	10										Х	+
N1600	6/11/2019	30	-76.52220	39.24688									85	12													
N1600	6/11/2019	31	-76.52270	39.24668						x			10	10													Railroad switch
N1600	6/11/2019	32	-76.52280	39.24670							10	5															Puncture in asphalt
N1600	6/11/2019	33	-76.52260	39.24723							4	2															Puncture in asphalt
N1600	6/11/2019	34	-76.52310	39.24690											0.5	50											
N1600	6/11/2019	35	-76.52290	39.24723											0.5	100										х	
N1600	6/11/2019	36	-76.52270	39.24731											0.5	100										х	
N1600	6/11/2019	37	-76.52230	39.24730			\bot \bot		1 1		20	10															Asphalt cracking
N1600	6/11/2019	38	-76.52220	39.24731	1	+	++	+	+-	+-	30	10	_	-	-	_					1		_				Asphalt cracking
N1600	6/11/2019	39	-76.52170 76.52170	39.24716		Х	+	+	+ +	+						1	50	50		20	30	1	1				
N1600	6/11/2019	40	-76.52180	39.24703	-	+	++	+	+-	+-	1	 	 	-	-	1	-	 	8	30	30	-	1			-	
1600	5/1/2019	1	-76.52190	39.24570		+	++	++	+ +	++-		1	1		0.25	60		<u> </u>		1		 	1				Seal asphalt seam
1600	5/1/2019	2	-76.52180	39.24550		1 1	+ +			1 1					0.23	70						<u> </u>	1				Seal asphalt seam
1600	5/10/2019	5	-76.52140	39.24576		х	+ +	1 1		1			1		1	25											Crack in concrete slab
1600	5/10/2019	6	-76.52200	39.24596						1 1					0.5	260						1					
1600	5/10/2019	7	-76.52160	39.24596															16	120	20						5/20/2021; Heave extends into reinforced concrete under canopy. Dimensions updated from
1600	5/10/2019	8	-76.52160	39.24578	 	+	++	+	+	+	1	 	 	+	 	1	 	 	14	30	15	-	1				10"x115'x20' to 16"x120'x20'
						+ +	+ +	1 1		1			1		0.75	500					1						Seal seam at concrete and asphalt joint around
1600 1600	5/10/2019 5/10/2019	9	-76.52160 -76.52190	39.24611 39.24618		X				+					0.75 0.25	560 40											canopy
1600	5/10/2019	11	-76.52230	39.24595											0.5	165							1				Asphalt seam
1600	5/10/2019	12	-76.52260	39.24613						1 1					0.5	100						1					Asphalt seam for conduit
1600	5/10/2019	13	-76.52260	39.24627													75	6									Repair asphalt around guardrail
1600	5/10/2019	14	-76.52270	39.24620						х	38	2															19 abandoned fence posts
1600	5/10/2019	15	-76.52240	39.24630						$\bot \Box$					0.25	195											Seal asphalt seams
1600	5/10/2019	16	-76.52230	39.24615						\bot					0.25	200											
1600	5/10/2019	17	-76.52160	39.24632		\bot	\bot \bot		\bot \bot	\bot						ļ			6	40	10		ļ				
1600	5/10/2019	18	-76.52200	39.24655											0.25	115			l			<u> </u>					Seal asphalt seam

Dundalk Marine	Terminal 2021 Surfa	ce Cover Ins	ection Data Tab	ole																								
											1 - 3'	' M&P	2 - Onen/I	npaved Area	3 - Surfa	ce Crack	4 - FS	Repair	5 - Surf	ace Heaving	Area	6 - Inlet/	7 - Monitor	8 - Grave Discolo		uo		
		Issue	Location	Location	Status	7		Categor	У		Length	Width	Length	Width	Avg. Width	Length	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	etati		
Area	Date	Number	Longitude	Latitude		P CS	S DP LI	P GC M		RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Vege		Comment
1600	5/10/2019	19	-76.52110	39.24680			1								0.25	40												
1600	5/10/2019	20	-76.52070	39.24644											0.25	120											Seal conduit seam	
1600	5/10/2019	21	-76.52110	39.24650				х					43	25													Open area near towe	f
1600	6/14/2019	22	-76.52110	39.24613		+			+						0.5	260										х		
1600 1600	6/14/2019 6/14/2019	23 24	-76.52080 -76.52050	39.24614 39.24624		+ +			+ +	1					0.25	50			14	120	15							
1600	6/14/2019	25	-76.52060	39.24624		x			+++	1	8	6							14	120	15							
1600	6/14/2019	26	-76.52050	39.24637					1 1										8	45	15							
1600	6/14/2019	27	-76.52060	39.24631											0.5	245												
1600	6/14/2019	28	-76.52050	39.24611		х	:										16	8										
1600	6/14/2019	29	-76.52050	39.24617											0.5	145												1105 11 005 1
1600	6/14/2019	30	-76.52030	39.24613															10	140	20						5/20/2021. Partial he in summer 2020	140 feet to 86 feet on eave removal during MPA paving
1600	6/14/2019	31	-76.52030	39.24623		+			+	X	2	2			0.5	475											Abandon fence post	
1600 1600	6/14/2019 6/14/2019	32 33	-76.52000 -76.52030	39.24592 39.24583		+			++-						0.5 0.25	175 140											Seal parches and sear	m
1600	6/14/2019	34	-76.52090	39.24591					++-						0.25	130											Seal seam	-
1600	6/14/2019	35	-76.52050	39.24557			×	(+ +	1 1					0.5	72										х		
1600	5/20/2021	1	-76.52120	39.24634															6	82	4							
1600	5/20/2021	2	-76.52000	39.24602															14	128	15							
1600	5/20/2021	3	-76.52070	39.24603					\perp						0.25	55												
1600	5/20/2021	4	-76.52090	39.24570						-					0.5	90												
1600 1600	5/20/2021 5/20/2021	5 6	-76.52110 -76.52090	39.24599 39.24506		+			++-						0.75 0.25	47 25											Crack a newly forming	g heave
1600	5/20/2021	7	-76.52170	39.24616					++-						0.23	23			6	45	3						cruck a newly forming	, neave
1600	5/20/2021	8	-76.52180	39.24606					1 1						0.5	75				.5								
1601	5/1/2019	1	-76.51830	39.24483						х	36	2															18 abandon fence po	sts
1601	5/1/2019	2	-76.51820	39.24479					\perp	х			6	1													6 gaps in curb	
1601	5/1/2019	3	-76.51830	39.24466					+ +	-					0.5	125			10	100	12							
1601 1601	5/1/2019 5/1/2019	5	-76.51840 -76.51840	39.24465 39.24459		Y			+ +		3	3							10	100	12							
1601	5/1/2019	6	-76.51830	39.24453		^			+ +										10	60	20							
1601	5/1/2019	7	-76.51870	39.24442											0.5	72												
1601	5/1/2019	8	-76.51860	39.24425		х					16	7															Pothole adjacent to e	xisting patch.
1601	5/1/2019	9	-76.51880	39.24424					\perp		17	12															Weathered asphalt.	
1601	5/1/2019	10	-76.51910	39.24400 39.24420		+++			+		42	4			0.5	200											Lateral and seam crac	
1601 1601	5/1/2019 5/1/2019	11 12	-76.51920 -76.51920	39.24420					+ +		20	15			0.5	200											Lateral and Seam Crac	N.
1601	5/1/2019	13	-76.51930	39.24394					+ +		32	4																
1601	5/1/2019	14	-76.51950	39.24405											0.5	100												
1601	5/1/2019	15	-76.51950	39.24379		х					7	5																
1601	5/1/2019	16	-76.51970	39.24382					\perp		18	3																
1601	5/1/2019	17	-76.51960 76.51900	39.24409		 	, .		+	+ +	1	 			0.5	202	1			1	1	 					Lateral cracking and s	eams.
1601 1601	5/1/2019 5/1/2019	18 19	-76.51900 -76.51880	39.24444 39.24457		X	×		++	+ + -	1	-			0.5 0.5	64 87	1			1	-	-				Х	+	
1601	5/1/2019	20	-76.51840	39.24490	1	+ +	1 1		+ +	х	4	2		1	0.5	67	<u> </u>			<u> </u>	<u> </u>		1				Two abandon fence p	osts
1601	5/1/2019	21	-76.51870	39.24494	1				1					1	0.25	205												147 feet to 205 on June 17,
					1	+	\bot	+	+	+				1							ļ						2021	
1601 1601	5/1/2019	22	-76.51910 -76.51920	39.24475 39.24468	1	×	+	+	+	+	2	2		1	0.5	188	1			1	 	-					сгаск along asphalt se	eams. Some resealing
1601	5/1/2019 5/1/2019	23	-76.51920 -76.51910	39.24468 39.24467	+	x	+	++	+ +	+ + -	3	3		+		 	1			1	 						Pothole in puddle	
1601	5/1/2019	25	-76.51910	39.24445	1	1	1 1		+ +	1 1	<u> </u>			1	1	1	<u> </u>		8	175	15		1				. I I I Paddic	
1601	5/1/2019	26	-76.51920	39.24447		х									0.75	35											Crack resealing over a	asphalt covered dolly pad
1601	5/1/2019	27	-76.51960	39.24442											0.5	138											Asphalt seam and late	
1601	5/1/2019	28	-76.51960	39.24433	1	x					18	3		1]		T			Weathered asphalt ad	djacent to existing patches
1601	5/1/2019	29	-76.51980	39.24400	+	++	+	+	++	++-	24	4	-	+		 	-	-					 				+	
1601	5/1/2019	30	-76.51980	39.24447	1				1			<u> </u>		1	0.25	150											Seam and lateral crac	.ks
1601	5/1/2019	31	-76.51960	39.24470															14	90	15							
1601	5/1/2019	32	-76.51950	39.24477											0.5	100											Seam and lateral crac	ks
1601	5/1/2019	33	-76.51940	39.24484	1	\bot			+	+				1	0.75	170										х		
1601	5/1/2019	34	-76.51910	39.24501	1	+	+	+	+	+	-			1	0.5	120	-		14	93	10		1				Coal coares	
1601 1601	6/14/2019	35 36	-76.52000 -76.51980	39.24563 39.24566	-	+	+	+	++	+	-		-	-	0.5 0.5	120 60	-			-			 				Seal seams	
1001	6/14/2019	36	-76.51980	59.Z45bb	1					1 1	<u> </u>	<u> </u>	<u> </u>	1	0.5	DU	1	1	<u> </u>	<u> </u>	l	<u> </u>	<u> </u>	I		1		

Dundalk Marine T	Terminal 2021 Surfa	ce Cover Ins	ection Data Tab	ile																							
											1 ,	3" M&P	2 Onen/I	Innavad Araa	3 - Surfa	co Crack	4 50	Repair	E Curd	face Heaving	Aron	6 - Inlet/	7 - Monitor	8 - Grave Discolo		L.	
		Issue	Location	Location	Status	7		Catogo	nrv.		Length	Width	Length	Jnpaved Area Width	Avg. Width	Length	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	tatic	
Aroa	Date	Issue	Longitude	Latitude	Status	РС	S DP LE	Catego P GC N	nw In M	H RR ((feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	ege	Comment
Area	Date	Number	Longitude	Latitude		1 60	5 01 1.1	i de li	100 111 101	III KK	(icct)	(icct)	(icct)	(icct)	(menes)	(icct)	(icct)	(icct)	(inches)	(icct)	(reet)	10	10	(reet)	(icci)	>	Heave length changed from 130 feet to 105 feet
1601	6/14/2019	38	-76.51940	39.24540															12	105	15						when feature inspected on June 17, 2021. A portion of the heave was removed during MPA paving of First Street in August 2020.
1601	6/14/2019	39	-76.51960	39.24546											0.5	290											Triisi Siteet iii Adgusi 2020
1601	6/14/2019	40	-76.51960	39.24558															10	50	15						
1601	6/14/2019	41	-76.51990	39.24518)	34	2															Remove 17 abandoned fence posts
1601	6/14/2019	42	-76.52010	39.24533											0.5	550										х	
1601	6/14/2019	43	-76.51960	39.24538											0.5	160											
1601	6/14/2019	44	-76.51920	39.24534															12	28	15						
1601	6/14/2019	45	-76.51960	39.24509			+			\perp									16	145	15						
1601	6/14/2019	46	-76.51990	39.24490			+	_							0.5	220										Х	
1601	6/14/2019	47	-76.51890	39.24523			+			+					0.5	240											
1601 1601	6/14/2019 6/14/2019	48 49	-76.52000 -76.52000	39.24471 39.24452		+	++	+++		++					0.5	275			8	160	8					х	
1601	6/14/2019	50	-76.52000	39.24468		+ +	+			+					0.5	500			8	160	8					×	+
1601	6/14/2019	51	-76.52040	39.24455		+ +	+++			+++					0.5	300			12	90	10						+
1601	6/14/2019	53	-76.52050	39.24449			+ +			+ +	55	25								- 50	10						Cracking asphalt
1601	6/14/2019	54	-76.52020	39.24424		1 1									0.5	155											
1601	6/14/2019	55	-76.52010	39.24420			x	<							0.25	72											
1601	6/17/2021	1	-76.51940	39.24523															6	53	5						
1601	6/17/2021	2	-76.51850	39.24496		х					17	10															Pitting asphalt
1700/1701	5/1/2019	1	-76.51910	39.24666											0.5	200											Lateral and seam cracks
1700/1701	5/1/2019	2	-76.51920	39.24672			х	<		,	:																Repair 14' of curb around light
1700/1701	5/1/2019	3	-76.51920	39.24660															6	75	10						
1700/1701	5/1/2019	4	-76.51940	39.24647			\perp			\perp					0.5	250											Seal five asphalt seams
1700/1701	5/1/2019	5	-76.51970	39.24635		+	++												8	50	10						
1700/1701 1700/1701	5/1/2019 5/1/2019	6 7	-76.51970 -76.51990	39.24622 39.24619			+			+++					0.5 0.5	260 115											Crack at asphalt seam
1700/1701	5/1/2019	8	-76.51990	39.24610											0.5	113			6	65	15						Heave along roadway and shoulder. Shorten from 65 feet to 50 feet on 5/19/2021 because of paving of summer of 2020
1700/1701	5/1/2019	10	-76.51940	39.24597		1 1													6	95	10						Summer of 2020
1700/1701	5/1/2019	11	-76.51920	39.24626											0.5	300											
1700/1701	5/1/2019	12	-76.51880	39.24633															8	80	15						
1700/1701	5/1/2019	13	-76.51860	39.24614											0.25	200											Cracks along seams
1700/1701	5/1/2019	14	-76.51890	39.24601			$\perp \perp$			\perp					0.5	205											Crack along seam
1700/1701	5/1/2019	15	-76.51940	39.24580		х	х	(Install asphalt curb around light pole slab
1700/1701	5/1/2019	16	-76.51880	39.24566			+			+									8	92	10						
1700/1701	5/1/2019	17	-76.51860	39.24594			+			+++									6	27	10						
1700/1701 1700/1701	5/1/2019 5/1/2019	20 21	-76.51880 -76.51830	39.24541 39.24589		+ +	+			+					0.25	150			ь	53	10						+
1700/1701	5/1/2019	22	-76.51850	39.24561		+	++	+		++					0.23	130			8	60	10						
1700/1701	5/17/2019	23	-76.51970	39.24649		1 1									0.5	600					10						Seal seams
1700/1701	5/17/2019	24	-76.51930	39.24662															8	90	15						
1700/1701	5/17/2019	25	-76.51930	39.24682											0.5	500											Seam and lateral cracks
1700/1701	5/17/2019	26	-76.51990	39.24646											0.5	360											Crack at asphalt seam
1700/1701	5/17/2019	27	-76.51970	39.24678															8	165	15				-		
1700/1701	5/17/2019	28	-76.51960	39.24706															6	52	5						Heave at edge of concrete slab. Length changed from 52 feet to 96 feet on 5/19/2021. Updated to 85 feet from 70 feet in length on
1700/1701	5/17/2019	29	-76.52000	39.24687	<u></u>														6	85	10	<u></u>					5/19/2021
1700/1701	5/17/2019	30	-76.52020	39.24678											0.5	400											
1700/1701	5/17/2019	32	-76.52020	39.24700						$ ^{-}$	1								8	70	50]	[Length changed from 70 feet to 90 feet on
1700/1701	5/17/2019	33	-76.52000	39.24717	 	++	++	++	++	++	+	+	1	1	0.5	175			 	+							5/19/2021
1700/1701	5/17/2019	34	-76.52030	39.24717	<u> </u>	1 1	+ +	++		+ +	1	1	1	1	0.5	150				1							+
1700/1701	5/17/2019	35	-76.52050	39.24700		1	+	++		+		1	1		1				6	78	10						+
1700/1701	5/17/2019	36	-76.52050	39.24680															10	65	15						Length changed from 50 to 65 feet on May 19, 2021
1700/1701	5/17/2019	37	-76.52050	39.24679			+ +	$\bot \bot$	$\bot\bot$	+ +			1	ļ	0.25	47	ļ				ļ		ļ			х	Mill a d pave along seam
1700/1701	5/17/2019	39	-76.52090	39.24693	ļ		++	++	+	++		+	10	10						1							Hydrant repair. Repair curb
1700/1701	5/17/2019	40	-76.52060	39.24703		+	++	+	+	++	-	+	-		0.5	1100	ļ			-	ļ						
1700/1701	5/17/2019	41	-76.52030	39.24742	1	+	++	++	++	++	30	12	1	1	0.5	500	1		1	1	1	1					
1700/1701 1700/1701	5/17/2019 6/4/2019	42 43	-76.52030 -76.51710	39.24752 39.24530	-	+	+	++	++	+	26	12	1	-	0.25	180				1							+
1700/1701	6/4/2019	43	-76.51710	39.24534		+	++	+	++	++	+	+	1	1	0.23	100			14	235	20						
1/00/1/01	0/7/2013		, 0.31/30	33.27334	1	111							1	1	1	1	1	1	1 17	233		1	1	<u> </u>			

Dundalk Marine T	erminal 2021 Surfa	ace Cover Insp	ection Data Tab	le							1		1									<u> </u>	1	8 - Gravel	Cover	T	
											1 - 3"	M&P	2 - Open/U	Inpaved Area	3 - Surfa	ce Crack	4 - FS	Repair	5 - Surfa	ace Heaving	Area	6 - Inlet/	7 - Monitor	8 - Gravei Discolor		io	'
		Issue	Location	Location	Status	l		Category			Length	Width	Length	Width	Avg. Width	Length	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	etat	1
Area	Date	Number	Longitude	Latitude		P CS	DP LP	GC MW	In MH	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Veg	Comment
1700/1701	6/4/2019	45	-76.51740	39.24524											0.25	195											Seal patches
1700/1701	6/4/2019	46	-76.51760	39.24530							33	6															
1700/1701	6/4/2019	47	-76.51760	39.24527							19	18													· · · · · ·		
1700/1701	6/4/2019	48	-76.51770	39.24523		\bot		\bot \bot	\perp		24	13															Alligator cracking
1700/1701	6/4/2019	49	-76.51780	39.24520		\perp				1 1									10	30	15						
1700/1701	6/4/2019	50	-76.51760	39.24505		х	-				20	20							_							-	
1700/1701	6/4/2019	51	-76.51800	39.24506		+ +		+		1					0.5	00			8	82	10						
1700/1701 1700/1701	6/4/2019 6/4/2019	53 54	-76.51810 -76.51770	39.24486 39.24557		+++				+ + -	4	2			0.5	80											
1700/1701	6/5/2019	55	-76.52070	39.24725		+	+ +				4				0.75	150											Seal seam
1700/1701	6/5/2019	56	-76.52070	39.24722		х					25	10			0.75	150											
1700/1701	6/5/2019	57	-76.52100	39.24708											0.5	185											
1700/1701	6/5/2019	58	-76.52100	39.24698		х					12	9															
1700/1701	6/5/2019	59	-76.52110	39.24708							43	3															Separated seam
1700/1701	6/5/2019	60	-76.52080	39.24742			х								0.25	90											
1700/1701	6/5/2019	61	-76.52060	39.24778											0.75	19										х	
1700/1701	6/5/2019	62	-76.52110	39.24740						х	2	2				_											Sign post
1700/1701	6/5/2019	63	-76.52120	39.24711	-	+		+	-		1		1	1	0.5	425	-	1	10	40	45						Seal curb
1700/1701 1700/1701	6/5/2019 6/5/2019	64 65	-76.52120 -76.52080	39.24743 39.24773	-	++	+	++	+	++	-		1	+		-		-	10 6	40 42	15 10					 	
1700/1701	6/5/2019	66	-76.52080 -76.52090	39.24773 39.24784	 	+ +	+++	+ + +	+	 			+	+			310	16	0	42	10						+
1700/1701	6/5/2019	67	-76.52140	39.24735					×		10	8					310	10									Repave around inlet IN-812
1700/1701	6/5/2019	68	-76.52160	39.24730							- 10	-			0.5	140											
	, -,				1	1		1			1		1	1			1				1						
1702	5/7/2019	1	-76.51720	39.24233											1	27											
1702	5/7/2019	2	-76.51710	39.24226									2	2												х	Holes is asphalt
1702	5/7/2019	3	-76.51710	39.24226							46	6															Wide crack
1702	5/7/2019	4	-76.51690	39.24230											0.25	120											Seal top of curb and gutter pan
1702	5/7/2019	5	-76.51670	39.24233		\perp				1 1			2	2													Hole in asphalt
1702	5/7/2019	6	-76.51640	39.24237		+				+ + -			6	2													Hole in asphalt
1702 1702	5/7/2019 5/7/2019	7 8	-76.51600 -76.51580	39.24251 39.24254		+	-			+ +	6	4	8	2													Four holes in asphalt Puncture in asphalt
1702	5/7/2019	9	-76.51560	39.24260		+	+ +				26	5															Alligator cracking
1702	5/7/2019	10	-76.51550	39.24268							20	3			0.5	100										х	7 mgator crossing
1702	5/7/2019	11	-76.51570	39.24273															8	100	15						
1702	5/7/2019	12	-76.51570	39.24268				х															TPZ-30B				Damaged well pad
1702	5/7/2019	13	-76.51620	39.24255											0.5	92											
1702	5/7/2019	14	-76.51630	39.24249				х															DMT-39S				Damaged well pad
1702	5/7/2019	15	-76.51720	39.24243		+	-					_			0.5	40										-	All: 1
1702	5/7/2019	16	-76.51710	39.24244		+					25	5			0.5	100											Alligator cracking along seam
1702 1702	5/7/2019 5/7/2019	17 18	-76.51710 -76.51670	39.24246 39.24262		+ + -				+ + -					0.5 0.25	100 45										х	
1702	5/7/2019	19	-76.51640	39.24268			+ +								0.23	90										^	
1702	5/7/2019	20	-76.51580	39.24287		 ^									0.75	86										х	Crack between asphalt and building
1702	5/7/2019	21	-76.51590	39.24298	1	1		1			33	4	1	1		1	1				1					х	Gap between building and asphalt
1702	5/7/2019	22	-76.51570	39.24303											0.5	20										х	
1702	5/7/2019	23	-76.51560	39.24307							4	2															Hole on asphalt
1702	5/7/2019	24	-76.51580	39.24310		+		+			1		1	1	1	1		1	8	111	10						
1702	5/7/2019	25	-76.51580	39.24313		++	+	+			40	10		1	<u> </u>				10	0.5							Alligator cracking
1702	5/7/2019	26	-76.51590	39.24310	1	+		+	-+	+	40	10	+	+	1		1		10	80	6					+	Alligator cracking
1702 1702	5/7/2019 5/7/2019	27 28	-76.51610 -76.51640	39.24315 39.24306	1	+ +	+++	+ + +	+		48	16	1	1	0.5	367		1			1						Crack between asphalt and concrete slab
1702	5/15/2019	29	-76.51760	39.24488		++	+ +	+ + +	+				+	+	0.3	307			12	60	17						Middle of the road.
1702	5/15/2019	30	-76.51730	39.24465		+ +		+ + +	+				1	1	1	50				30	1,					†	
1702	5/15/2019	31	-76.51740	39.24486	1	1 1		1							0.5	55	1									1	
1702	5/15/2019	32	-76.51730	39.24490											0.5	51											
1702	5/15/2019	33	-76.51720	39.24483											0.5	33									·		
1702	5/15/2019	34	-76.51700	39.24469		$\bot \bot$	\bot	+					1	1	0.5	200										1	
1702	5/15/2019	35	-76.51700	39.24488		+		+			1		1	1	0.5	75		1		1	1						
1702	5/15/2019	36	-76.51730	39.24512		+ +	+	+	\perp				1	1	0.5	31		1									In hatungar inggride and a second
1702	5/15/2019	37	-76.51700	39.24510	-	++	+	+	-+	\vdash	1		1	+		1		1	12	65	15						In between jersey barriers and roadway. Retween light note and jersey barriers
1702 1702	5/15/2019 5/15/2019	38 39	-76.51710 -76.51690	39.24507 39.24503		++	x	+	-		1			+	0.5	65			14	90	10					x	
1702	5/15/2019	40	-76.51690 -76.51680	39.24503 39.24502	1	+ +	+ + ×	+ + +	+				1	1	0.5	60		1			1					×	
1702	5/15/2019	41	-76.51680	39.24510		+ +		+ + +	-				1		0.5	111											In roadway.
1702	5/15/2019	42	-76.51660	39.24501		11	1 1	1			20	5				<u> </u>											Roadway
												•	-		1									L.			

Dundalk Marine 1	Terminal 2021 Surfa	ace Cover Ins	pection Data Tab	le							_												ı	1 0 0	10		
											1 - 3"	M&D	2 - Open/I	Inpaved Area	3 - Surfa	ro Crack	1 - FS	Repair	5 - Surf	ace Heaving	Area	6 - Inlet/	7 - Monitor	8 - Grav Discolo		5	
		Issue	Location	Location	Status	7		Categor	,		Length	Width	Length	Width	Avg. Width	Length	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	tatic	
Area	Date	Number	Longitude	Latitude	Status	P CS	S DP LP			H RR O		(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	ege	Comment
1702	5/15/2019	43	-76.51670	39.24497		x			v		15	3	(,	(/	()	(1000)	(,	(1000)	()	(,	(1000)			(,	(,	>	Spalling in concrete
1702	5/15/2019	44	-76.51670	39.24498		<u> </u>			1	+ + -	110	10															Spanning in concrete
1702	5/15/2019	45	-76.51670	39.24497		+ +	+ +		+ +	+	110	10			0.5	300											Parallel cracks in the concrete.
1702	5/15/2019	46	-76.51620	39.24485		×	+ +				50	3			0.5	300											In roadway. Spalling.
1702	5/15/2019	47	-76.51630	39.24473		X					15	3															Spalling just west of roadway.
1702	5/15/2019	48	-76.51620	39.24470											1	120											Asphalt patch previously sealed.
1702	5/15/2019	49	-76.51630	39.24464															10	21	13						
1702	5/15/2019	50	-76.51630	39.24464											0.25	42											Various cracks near inlet.
1702	5/15/2019	51	-76.51640	39.24467											0.5	120										х	Dolly pad up to jersey barriers.
1702	5/15/2019	52	-76.51640	39.24464											0.5	60											Crack from dolly pad to jersey barrier.
1702	5/15/2019	53	-76.51710	39.24441							25	10															
1702	5/15/2019	54	-76.51680	39.24451															6	130	5						
1702	5/15/2019	55	-76.51730	39.24422					-										8	78	30						
1702	5/15/2019	56	-76.51770	39.24408		+ +			-	+ + -						400			10	30	15						Heave by light pole. In roadway.
1702 1702	5/15/2019 5/16/2019	57 58	-76.51770 -76.51770	39.24415 39.24401		+ +		+ +		+++					1	100 95											Various cracks in roadway.
							+ +		+ +						1												Cracking just east of jersey barriers by light pole 17-1
1702	5/16/2019	59	-76.51760	39.24402											0.5	25					<u></u>						2. 25mile just case of jersey partiers by light pole 17-1
1702	5/16/2019	61	-76.51740	39.24394											1	100											
1702	5/16/2019	62	-76.51730	39.24411															8	50	10						
1702	5/16/2019	63	-76.51670	39.24433			х								0.25	80										х	Crack around 17-4
1702	5/16/2019	64	-76.51660	39.24439		$\bot \bot$	$\bot \bot$	$\bot\bot$	$\bot \bot$	+ +			1		0.5	90	ļ	1									
1702	5/16/2019	65	-76.51600	39.24457				-	<u> </u>		50	2															Damaged concrete in roadway.
1702	5/16/2019	66	-76.51600	39.24450						+-			-		0.5	75											
1702	5/16/2019	67	-76.51630	39.24437				+ +	-	+ + -					0.5	240											Various cracking east of dolly pad.
1702	5/16/2019	68	-76.51680	39.24416		+ +	+ +	+++	+ +	+					0.5 0.5	45 60											
1702 1702	5/16/2019 5/16/2019	69 70	-76.51660 -76.51710	39.24424 39.24402		+ +	+	++	+ +	+ + -					0.5	150											Cracking on north side of heave.
1702	5/16/2019	70	-76.51710	39.24404		+ +	+ +		+ +	+					0.5	115											Cracks and asphalt patch seal missing.
1702	5/16/2019	74	-76.51740	39.24381		+ +		+ +		+ + -					0.5	35											eracks and aspirate patern sear missing.
1702	5/16/2019	75	-76.51730	39.24392			+ +				20	13			0.5	- 55											
1702	5/16/2019	78	-76.51600	39.24415											0.25	120											
1702	5/16/2019	79	-76.51600	39.24428			х								1	170											
1702	5/16/2019	80	-76.51580	39.24429															8	50	20						
1702	5/16/2019	81	-76.51570	39.24436											0.25	70											
1702	5/16/2019	82	-76.51580	39.24427											0.5	45											
1702	5/16/2019	83	-76.51620	39.24405											0.5	160											
1702	5/16/2019	84	-76.51660	39.24389						+-			-		0.5	160											
1702	5/16/2019	85	-76.51690	39.24382					-						0.5	60											
1702	5/16/2019	87	-76.51700	39.24379		+ +			-	+ + -					0.5	40											Cracking near intersection
1702 1702	5/21/2019 5/21/2019	89 90	-76.51760 -76.51680	39.24347 39.24357				+	1 1	+					0.5 0.5	80 30											Cracking near intersection.
1702	5/21/2019	91	-76.51680	39.24337		+ +			1	+ + -					0.5	85											+
1702	5/21/2019	92	-76.51670	39.24341	-	++	+	+	+	++			+		0.5	85	1	+					 	-		х	+
1702	5/22/2019	93	-76.51670	39.24327				+ +	1 1	+ + -			1		0.5	135		+					1			<u> </u>	+
1702	5/22/2019	94	-76.51710	39.24322			1 1	++	+ +	+ + -					1			1	8	25	10						1
1702	5/22/2019	95	-76.51730	39.24322					1									1	6	33	12						
1702	5/22/2019	96	-76.51720	39.24298											0.5	110						Electric					Asphalt pad around electric vault. Manhole ring is
					1	+	+	+	 ^	+			1	-	0.5	110	_			-		vault	1	1		-	cracked and needs to be sealed.
1702	5/22/2019	97	-76.51730	39.24282	1	+	+	+	+	+ +	100	15	1	-	1	1	1	1		1				1		1	Area of road between the two patches.
1702 1702	5/22/2019 5/22/2019	98 99	-76.51720 -76.51610	39.24282 39.24314	-	++	+	++	+	++	50	15	2	2	1	-	1	+		-	 	-	 	-		-	NW corner of Shed1702A
1702	5/22/2019	100	-76.51610 -76.51630	39.24314 39.24326		++	+	+	+ +	+ +	50	15	1		0.25	210	1	+			1		1			х	Multiple cracks within jersey barrier area.
						+	++	+	+ +	++			+				1	+								×	Various cracks including seam between asphalt
1702	5/22/2019	101	-76.51640	39.24346		<u> </u>				<u> </u>					0.5	75		<u> </u>									patches.
1702	5/22/2019	102	-76.51630	39.24373											0.5	220											
1702	5/22/2019	103	-76.51620	39.24342						\bot					0.5	75											
1702	5/22/2019	104	-76.51590	39.24343		\bot				$\bot \bot$			1			ļ			10	50	8						Heave inside of jersey barrier.
1702	5/22/2019	105	-76.51590	39.24365	1	\bot		+	+	+			1		0.5	280	1	1		1	1	1	1	1		1	Both joints around dolly pad.
1702	5/22/2019	106	-76.51570	39.24373		\bot		+	+	+			1		0.5	280	1	1			ļ		ļ				Construct and and an inchination of the construction of the constr
1702	5/22/2019	107	-76.51540	39.24384		++	++	+	+	++	1		1		0.5	88	 	1								-	Cracks at entrance to lot.
1702	5/22/2019	108	-76.51540	39.24394	1	+	X		+	+ +	1		1		0.5	66	1	1		1	1	1	1	1		1	LP-A
1702	5/22/2019	109	-76.51550	39.24414		х													10	85	15						Heave on both concrete and asphalt. Offshoot to the east at the seam of concrete and asphalt.
1702	5/22/2019	110	-76.51550	39.24413					1								25	0.5									case at the seam of concrete and aspirate.
1702	5/22/2019	111	-76.51560	39.24412					х										8	10	10						Volcano type heaves three total. Around inlet.

Dunualk Warine	Terminal 2021 Surf	ace Cover Insp	ection Data Tabl	le																							
												"	2 0 //		2 6 6							C Inlas/	7 Manitan		el Cover	⊊	1
		г. т	Lagation	Lacation	Chahua	1						" M&P		Inpaved Area		•	4 - FS I			ace Heaving A		6 - Inlet/	7 - Monitor		oration	atic	
		Issue	Location	Location Latitude	Status	В	CC DD		egory	la MU D	Length (feet)	Width (foot)	Length (foot)	Width (foot)	Avg. Width	-	Length (feet)	Width (feet)	Max. Height (inches)	Length (feet)	Width (foot)	Manhole ID	Wellhead ID	Length (foot)	Width (foot)	eget	
Area	Date	Number	Longitude			Р	C3 DP	LP GC	IVIW	In MH R	K O (leet)	(feet)	(feet)	(feet)	(inches)	(feet)	(reet)	(leet)			(feet)	IU	IU	(feet)	(feet)	Š	Comment
1800	5/1/2019	1	-76.51860	39.24758			_		+		 								8	80	9						Debte de la cataloga di la cataloga de la cataloga
1800	5/1/2019	2	-76.51880	39.24741		х	х				15	2															Potholes at spalling concrete in slab
1800	5/1/2019	3	-76.51860	39.24763					х	,	110	9											DMT-79M				Severe cracking in asphalt between railroad tracks. Includes well
1800	5/1/2019	4	-76.51840	39.24740				,			23	15															Low point between railroad tracks with sediment and
1800	3/1/2019	4	-70.51640	39.24740				*			23	15														×	vegetation growth
1800	5/1/2019	5	-76.51860	39.24764									40	3													Wide crack under fence. Fill with asphalt and
1800	5/1/2019	6	-76.51830	39.24734							×								10	18	15					х	compact Heave around abandon hydrant
1800	5/1/2019	7	-76.51880	39.24720											0.5	19											
1800	5/1/2019	8	-76.51870	39.24732			x			x					0.5	42										х	Seal crack between inlet and concrete slabs
1800	5/1/2019	9	-76.51880	39.24715			х								0.5	400										х	Crack between concrete slab and asphalt
1800	5/1/2019	10	-76.51900	39.24702			_			,					0.5	163											Crack around inlet and between concrete slab
							^			^					0.5	103											sections
1800	5/1/2019	11	-76.51880	39.24688							4	2															Two holes through asphalt
1800	5/1/2019	12	-76.51860	39.24687					+						0.25	60											
1800	5/1/2019	13	-76.51850	39.24669		-					.		-		0.5	100			12	100	12						Heave at edge of concrete slab
1800	5/1/2019	14	-76.51830	39.24692		++	х	 	+	,	`	1	+	1	1	+		1	12	100	12	1	 	1	 	1	Replace damaged asphalt over light pole foundation
1800	5/1/2019	15	-76.51870	39.24709				×			16	14														x	The place damaged aspirate over light pole foundation
1800	5/1/2019	16	-76.51810	39.24725		\Box \dagger				,	(0.5	106											Cracks between railroad tracks
1800	5/1/2019	17	-76.51820	39.24719		LI	х												10	65	10						Heave pushing up edge of concrete slab
1800	5/1/2019	18	-76.51800	39.24714							75	16									-						Cracked asphalt. Ponding water
1800	5/1/2019	19	-76.51780	39.24694			х			x									12	70	15						Heave under concrete slab extending towards inlet
									+																		Heavy at odes of consucts slab
1800 1800	5/1/2019	20	-76.51810 -76.51800	39.24675		-	х						-		0.5	28			24	115	15						Heave at edge of concrete slab
1800	5/1/2019 5/1/2019	21	-76.51800	39.24685 39.24674			-						+		0.5 0.5	35											Crack around inlet
1800	5/1/2019	22	-76.51820	39.24674			х		+						0.3	154											Crack in concrete slab and along slab edge
							^								0.23	134											Ponding area adjacent to light pole with sediment
1800	5/1/2019	24	-76.51770	39.24685							20	14														х	and vegetation
1800	5/1/2019	25	-76.51790	39.24649															8	43	10						
1800	5/1/2019	26	-76.51810	39.24638						х					0.5	30											
1800	5/1/2019	27	-76.51750	39.24683											0.25	150										х	
1800	5/1/2019	28	-76.51750	39.24681									2	2													
1800	5/1/2019	29	-76.51740	39.24671											0.25	103											<u></u>
1800	5/1/2019	30	-76.51760	39.24606						х					0.25	33											Seal crack around inlet
1800	5/1/2019	31	-76.51750	39.24635							X				0.5												Monitoring point missing lid and frame
1800 1800	5/1/2019 5/1/2019	32 33	-76.51720 -76.51720	39.24651 39.24660					+						0.5 0.25	50 162										x	Crack between jersey barrier and asphalt
1800	5/1/2019	34	-76.51720	39.24600											0.25	117										X	Crack between jersey barrier and aspirare
1800	5/1/2019	35	-76.51740	39.24586											0.25	83											Seal asphalt seam crack again
1800	5/1/2019	36	-76.51710	39.24615				1	x						0.23	03											Cracked well frame
1800	5/1/2019	37	-76.51680	39.24632					^				39	9													Stone cover
1800	5/1/2019	38	-76.51690	39.24579		+	-		+	++	+ +	1		 	0.5	270											Crack between toe drain and asphalt
1800	5/1/2019	39	-76.51680	39.24547						+	1 1	1	1	1	0.5	45											
1800	5/1/2019	40	-76.51610	39.24569											0.5	128											
1800	5/1/2019	41	-76.51620	39.24560		Lİ									0.25	197											
1800	5/1/2019	42	-76.51640	39.24549				х							0.5	17											
1800	5/1/2019	43	-76.51640	39.24545			х								0.5	50											Crack around manhole slab
1800	5/1/2019	44	-76.51650	39.24523											0.25	125											
1800	5/1/2019	45	-76.51610	39.24534		$\sqcup \bot$			\bot	$\perp \perp \perp$	1	<u> </u>	1	<u> </u>	0.5	108											
1800	5/1/2019	46	-76.51570	39.24526		\sqcup			+	\perp	+	1	1	1	0.25	88			ļ				ļ	1	ļ		4
1800	5/1/2019	47	-76.51610	39.24493		\vdash			+	++			1	1	0.5	55											-
1800	5/1/2019	48	-76.51590	39.24480		\vdash		$\vdash \vdash$	++	+	+	 	1	1		 			10	15	15			1		-	<u> </u>
1800	5/1/2019	49	-76.51550	39.24466		\vdash	+	\vdash	++	++	+ +	1	+	1	0.25	75 150							1	1	1	1	
1800	5/1/2019	50	-76.51510	39.24460		\vdash		 	+		+ + -	-	1	-	0.5	156			 				 		 		
1800	5/1/2019	51	-76.51490	39.24490		\vdash		\vdash	++	++	9	3	+	-	1	+			-	1			-		-		Cracked asphalt
1800 1800	5/1/2019 5/1/2019	52 53	-76.51590 -76.51620	39.24574 39.24594		++	+		++	++	80	3	+	1	1	+	10	5									Holes through asphalt
1800	5/1/2019	53	-76.51620 -76.51640	39.24594 39.24609		++		 	+	+	+ +	1	+	1	1	+	130	12	<u> </u>			1	 	1	 	1	noics through asphalt
						++		 	+	+	+ +	1	+	1		+			1				 	1	 		Separation and heaves between rails and bollards
1800	5/1/2019	55	-76.51670	39.24634		\perp)			<u> </u>				116	4									Separation and neaves between rails and bollarus
1800	5/1/2019	56	-76.51710	39.24661													27	4									Separating asphalt under fence
1800	5/1/2019	57	-76.51580	39.24566									414	4													
1800	5/2/2019	58	-76.51440	39.24412									7	7												х	
1800	5/2/2019	59	-76.51450	39.24406					х	$\perp \perp \perp$			1		0.25	10			1				TPZ-28			х	Two well pads
1800	5/2/2019	60	-76.51470	39.24411											0.5	100											

Part	Dundalk Marine 1	Terminal 2021 Surfa	ice Cover Ins	pection Data Tab	le																							
Part												1 - 3"	M&D	2 - Open/I	Innaved Area	3 - Surfa	ico Crack	1 - FS	Ronair	5 - Surf	ace Heaving /	۸roa	6 - Inlet/	7 - Monitor			5	
Section Sect			Issue	Location	Location	Status	7		Category								T .										tati	
March Marc	Aroa	Date				Status	P CS	S DP LE			H RR O			-		_	_			-	-						ege	Commont
							. 65	, J. L.	00 1010			(1000)	(1000)	(1000)	(1000)		1	(1001)	(1000)	(menes)	(1000)	(1000)	,,,		(1001)	(1000)	>	
30 30 30 30 31 32 32 32 33 34 34 34 34		1					+ +		+ +		+ +						1										1	Seath and lateral Cracking
1.50							+ +		1																			
1975 1976							+ +		++			24	4			0.5	200											Remove six 4x4 concrete pads
March Marc							1 1					27	-			0.25	206											·
Second S							1 1					6	2			0.23	200											
1969 1975	1800															0.5	350											Asphalt seam
Secondary Seco	1800	5/2/2019	68	-76.51490	39.24433			х								0.5	66											
1986 1976	1800	5/2/2019	69	-76.51510	39.24418															6	89	12						
Section Sect	1800	5/2/2019	70	-76.51530	39.24411											0.5	95											Crack at asphalt seam
March Marc	1800	5/2/2019	71	-76.51520	39.24448						х			24	25													Open area at railroad switch
1906 1908 1909	1800				39.24445											0.5	120											
March Marc	1800	5/2/2019	73	-76.51530	39.24429							30	14															·
1965 1968 1969	1800	5/2/2019	74	-76.51540	39.24426													10	2									
1909 1909	1800	5/2/2019	75	-76 51570	39 24455		+ + ^					78	3					10	3									replace
1979 1979		1	1		1		×					,,,	3			0.5	54											
1402 12022							† † ^									0.0	<u> </u>			12	40	18						Heave under fence
1982 1982 1982 1984			1		1											0.5	76											Asphalt seam
March Marc	1800			-76.51620	39.24492						х									8	20	10						
March Marc	1800	5/2/2019	80	-76.51630	39.24498									20	5													Open at railroad switch
180	1800	5/2/2019	81	-76.51640	39.24508							8	2															Four abandoned fence posts
## 14 Part	1800	5/2/2019	82	-76.51660	39.24506							28	5															
1400	1800	5/2/2019	83	-76.51930	39.24707																							31 holes in asphalt along yellow line between tracks
1900							+ +		+		Х	42	2	62	2												-	Wide crack pear fence
389 3-6/2019 18							+ +		+ +		+ +	43	3			0.25	20										X	
100							+ +		1							0.25	30											
1900	1800	5/6/2019	86	-76.52100	39.24917			x								0.5	100											bony pad and small crack to the histae of feffee line.
1930 1930	1800	5/6/2019	87	-76.52080	39.24910															8	10	10						Heave by fence.
1-10 1-10	1800	5/6/2019	88	-76.52080	39.24903									10	33													To the NW of tracks. Under poly.
1906	1800	5/6/2019	89	-76.52070	39.24913									12														Three separate open unpaved areas near train gate.
180 My/2014 91							+ +		+ +		×			12	ь		+			12	25	10					1	Potygon PP and houndary fonce
1900 56/2019 92 -76.5000 32.4982 8 8 8 8 8 8 8 8 8					1		+ +		+ +							0.5	50			12	33	10					-	
1500 51/2019 92 75/2019 92 75/2019 93 75/2019 94 75/2019 95 75/2019 95 75/2019 97					1		+ +		1 1		Y			6	10	0.5	30											
1500 15/2019 94									++		+^-				10	0.5	110											•
1800 5/h/2019 98											х									10	15	15						
1800	1800	5/6/2019	95	-76.52060	39.24877															10	40	3						
1800 5/6/2019 98	1800	5/6/2019	96	-76.52060	39.24884											0.5	40											
1800 5/6/2019 99	1800	5/6/2019	97	-76.52050	39.24880											0.5	150											Seam and cracks in between RR.
1800 5/6/2019 100 -76-52010 39-24865	1800	5/6/2019	98	-76.52050	39.24862						х			15	9													Three separate unpaved areas.
1800 5/6/2019 101 -76.52010 39.24865 X X X X X X X X X	1800		99						$\perp \perp$		х			20	8													RR switch
1800 5/6/2019 102 7-5/2000 39 2-4854 X X X X X X X X X	1800	5/6/2019	100	-76.52030	39.24861											0.5	102											
1800 5/6/2019 102 76,5200 39,24854	1800	5/6/2019	101	-76.52010	39.24865											0.5	120											
1800 5/6/2019 103 -76.51900 39.24864 0 0 0 0 0 0 0 0 0	1800	5/6/2019	102	-76 52010	39 24854		1 v									1											Y	
3800 5/6/2019 105 -76.51970 39.24844						<u> </u>	 ^	1	+ +		+ + -					0.5			1				<u> </u>				^	
1800 5/6/2019 105 -76.51200 39.24837			1				1	x	1		 			3	2		<u> </u>										1	
1800 5/6/2019 106 -76.52030 39.24829						İ	1 1		1 1		1 1					1	45						İ					
1800	1800	5/6/2019		-76.52030	39.24829		х									0.5	330											Portion north of light pole.
1800 5/6/2019 110 -76.52050 39.24813	1800	5/6/2019	107	-76.52020	39.24838		х													10	25	15						Partial concrete slab and asphalt by tracks.
1800 5/6/2019 110 -76.5205 39.24806	1800	5/6/2019	108	-76.52040	39.24831						х			15	25													Six separate sections.
1800 S/6/2019 111 -76,52000 39,24813									$\perp \perp$		$\perp \perp \perp$			13	6													
1800	1800	5/6/2019	110	-76.52050	39.24806		$\bot \bot$	\bot	$\bot \bot$	$oxed{oxed}$	х					0.5	20		<u> </u>								ļ	· -
1800 5/6/2019 112 -76.51990 39.24819	1800	5/6/2019	111	-76.52000	39.24813							200	2															
1800 5/6/2019 113 -76.51990 39.24824							X	++	++		++-							1	1				 				+	
180 5/6/2019 114 -76.51990 39.24828 X I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 70 3 I 10									+ +		+ +	1/0				1	240	1	1				<u> </u>				 	
1800 5/6/2019 115 -76.51980 39.24831 X I 40 3 I August I Chipping by MES storage sheds. 1800 5/6/2019 116 -76.51980 39.24832 X I			1			†			+ +		+ + -	70	3			-	270						†				1	
1800 5/6/2019 116 -76.51980 39.24832 X I I 0.5 122 I I Seam between asphalt and concrete by MES 1800 5/6/2019 117 -76.51950 39.24830 X Cracking in between tracks. 1800 5/6/2019 118 -76.51940 39.24823 I					1		- · · ·	+	+ +		+ + -						<u> </u>	1	1				<u> </u>				1	
1800 5/6/2019 117 -76.51950 39.24830 Image: square							+ +	+	+ +		++-	1				0.5	122										1	
1800 5/6/2019 118 -76.51940 39.24823 Image: square of the control of the contr							 ^		+		+																х	
1800 5/6/2019 119 -76.51930 39.24811 1 1 1 35 1 80.24811 1 1 1 1 1 35 1 <t< td=""><td></td><td></td><td></td><td></td><td></td><td>1</td><td>1 </td><td></td><td>1 </td><td></td><td> </td><td></td><td></td><td></td><td></td><td></td><td>1</td><td></td><td></td><td></td><td></td><td></td><td>1</td><td></td><td></td><td></td><td><u> </u></td><td></td></t<>						1	1		1		 						1						1				<u> </u>	
1800 5/6/2019 120 -76.51900 39.24790			1			1	1 1		1 1		1					1							İ				1	
	1800				1											1											х	
	1800	5/6/2019	121	-76.51870																6	20	8						Heave by the fence.

																							8 - Grav	el Cover	_	1
						_					1 - 3'	' M&P	2 - Open/U	npaved Area	3 - Surfac	e Crack	4 - FS Repair	5 - Surf	face Heaving A		6 - Inlet/	7 - Monitor	Discol	oration	ţio	
		Issue	Location	Location	Status				egory		Length	Width	Length	Width	Avg. Width	Length	Length Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	geta	
Area	Date	Number	Longitude	Latitude		P C	CS DP	LP GC	MW In	MH RR	O (feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	\ Ve	Comment
1800	5/6/2019	122	-76.51870	39.24755)	х											10	50	15						Just inside fence of MES compound.
1800	5/6/2019	123	-76.51880	39.24754)	х				20	3														
1800	5/6/2019	124	-76.51880	39.24747)	х								1	55										By fence inside MES compound.
1800	5/6/2019	125	-76.51900	39.24741		,	x				130	1														Inside fence of MES compound. Concrete spalling.
1800	5/6/2019	126	-76.51930	39.24771)	х								1	33										
1800	5/6/2019	127	-76.51940	39.24767)	x								1	375									х	Seam between concrete and asphalt inside MES compound. Runs entire length of fence.
1800	5/9/2019	128	-76.52040	39.24795														6	25	10						
1800	5/9/2019	129	-76.52020	39.24775														8	26	20						
1800	5/9/2019	130	-76.52060	39.24778											0.5	50									Х	Reseal crack
1800	5/9/2019	131	-76.52060	39.24788									2	2												Open area near guardrail post
1800	5/9/2019	132	-76.52010	39.24750														14	78	10						
1800	5/9/2019	133	-76.52010	39.24743							55	3														Wide crack at seam with concrete slab
1800	5/9/2019	134	-76.52000	39.24768		x >	х				47	2														
1800	5/9/2019	135	-76.52000	39.24778			х								0.25	52										
1800	5/9/2019	136	-76.51950	39.24764)	х								0.5	26										
1800	5/9/2019	137	-76.52010	39.24812)	х								0.5	430									Х	
1800	5/9/2019	138	-76.51950	39.24756		x >	х				10	2														
1800	5/9/2019	139	-76.51960	39.24749											0.25	30									х	Seal seam between asphalt and CS around inlet
1800	5/9/2019	140	-76.51970	39.24742						х			18	5												Open area at railroad switch
1800	5/9/2019	141	-76.51980	39.24730)	х								1	18										
1800	5/9/2019	142	-76.51960	39.24739			х								0.5	105										
1800	5/9/2019	143	-76.51950	39.24747)	х								0.5	80									Х	
1800	5/9/2019	144	-76.51940	39.24741		x >	х				15	2														
1800	5/9/2019	145	-76.51910	39.24742						х								6	59	10						
1800	5/9/2019	146	-76.51910	39.24725)	х			х								14	75	13						Under fence
1800	5/9/2019	147	-76.51910	39.24742											1	75										
1800	5/9/2019	148	-76.51950	39.24718						х	88	3														Cracked asphalt
1800	5/9/2019	149	-76.51960	39.24712)	х											6	40	10						
1800	5/9/2019	150	-76.51930	39.24711			_			х			5	3				1								
1800	5/9/2019	151	-76.51910	39.24695						х			40	7												
1800	5/9/2019	152	-76.51910	39.24695			_ _			х					0.25	50		1								
1800	5/9/2019	153	-76.51900	39.24664)	х											14	61	18						
1800	5/9/2019	154	-76.51880	39.24674						х			6	5												
1800	5/9/2019	155	-76.51870	39.24668		$\bot\bot$		$\perp \perp$		х	32	3						1								
1800	5/9/2019	156	-76.51850	39.24658				$oxed{oxed}$		х					0.25	50										
1800	5/9/2019	157	-76.51850	39.24653		$\bot\bot$				х								6	37	7						
1800	5/9/2019	158	-76.51860	39.24647		x >	х				40	2						1								
1800	5/9/2019	159	-76.51820	39.24634			_			х	33	8						1								Cracked asphalt
1800	5/9/2019	160	-76.51710	39.24550											0.5	145										
1800	5/9/2019	161	-76.51750	39.24559				1 1]		14	35	20		1	l			

Appendix C Sample Surface Cover Repairs Summary Report

Summary Report for 2021 Repair Cycle, Surface Cover System

Dundalk Marine Terminal Baltimore, Maryland

Prepared for



115 Tabor Road Morris Plains, New Jersey 07950

Prepared by

Jacobs

2411 Dulles Corner Park Suite 500 Herndon, VA 20171

April 2022



Contents

Acro	onyms a	nd Abbreviations	iii
1.	Intro	duction and Background	1-1
	1.1	Introduction	1-1
	1.2	Background	1-1
2.	Sum	mary of 2021 Pavement Repairs	2-1
	2.1	General	2-1
	2.2	Project Team	2-1
	2.3	Summary of Completed Surface Cover Maintenance Activities	2-1
	2.4	Remaining Maintenance Activities	2-2
Α	Dunc	lalk Marine Terminal Surface Cover Repair Disposition	
Tabl	es		
1	DMT	Surface Cover Repairs, Annual Summary	1-2
2	,	ect Team	
3	Sumi	mary of Completed Repairs by Operational Area	2-1
Figu	ires		
1	• • •	R Fill Area	
2	2021	Surface Cover Repairs	



Acronyms and Abbreviations

COPR chromium ore processing residue

DMT Dundalk Marine Terminal

ft² square foot

MDE Maryland Department of the Environment

MPA Maryland Port Administration

SCMP Surface Cover and 14th and 15th Streets Drain Inspection and Maintenance Plan

(CH2M HILL Engineers, Inc., 2007)

PPS0823211121WDC iii



1. Introduction and Background

1.1 Introduction

This report documents surface cover repairs conducted by Jacobs Engineering Group Inc. (Jacobs), during the 2021 surface cover repair cycle within the area of Dundalk Marine Terminal (DMT) that is underlain by chromium ore processing residue (COPR) fill. These repairs were conducted in accordance with the August 2007 Surface Cover and 14th and 15th Streets Drain Inspection and Maintenance Plan (SCMP), which was submitted to the Maryland Department of the Environment (MDE) pursuant to Section III.B.4 of the April 5, 2006, Consent Decree entered into by MDE, the Maryland Port Administration (MPA), and Honeywell International Inc. (Honeywell).

1.2 Background

DMT is a major maritime terminal located in Baltimore, Maryland, and owned and operated by the MPA. The site covers approximately 580 acres, approximately 148 acres of which were constructed on land reclaimed from prior marshlands and the Patapsco River using fill materials composed partially of COPR. The configuration of DMT, including the delineation of current operational areas and the approximate limits of the 148-acre portion of DMT underlain by the COPR fill (e.g., COPR fill area), is illustrated in Figure 1.

The COPR fill area generally consists of active operational areas, with pavements being the primary cover system over the COPR fill. Except for a portion of Area 1800 and Areas 1501 and 1602, pavement systems generally consist of conventional asphalt, roller-compacted concrete asphalt or concrete "pavers." A 7.2-acre portion of Area 1800 was reconstructed in 2008 as a pilot area to evaluate the performance of a 4.4-acre modified conventional asphalt pavement section, a 1.8-acre low permeability asphalt pavement section, and a 1-acre articulated block cover pavement section under heavy port loading conditions. Areas 1501 and 1602 have approximately 7 feet of non-COPR surcharge fill and 15.5 inches of stone base and asphalt surfacing, which were placed to mitigate heave conditions. A small unpaved railroad track area, approximately 0.31 acre, is located to the north of the East Service Road and operational areas. Another unpaved railroad track area, approximately 0.24 acre, is north of Area 1800. These areas are surfaced with gravel and/or railroad ballast.

For DMT to continue to operate as an active marine terminal, the existing pavement surface must remain in a functional condition, both as a structural pavement capable of supporting active port operations and as a surface cover system to minimize the potential for exposure to and release of COPR-related constituents to the environment and to reduce surface water infiltration. These functions are being maintained through a proactive cover system inspection and maintenance program detailed in the approved SCMP.

In March and April 2007, CH2M performed on behalf of MPA and Honeywell a baseline surface cover inspection pursuant to the September 2006 draft of the SCMP. This baseline surface cover inspection was documented in the September 2007 *Surface Cover System Baseline Inspection Report, Dundalk Marine Terminal* (baseline inspection report). Repairs recommended in the baseline inspection report, as well as repairs to features noted during subsequent site observations, were performed between July 2008 and May 2009. Repairs from that repair cycle were documented in the June 2009 *Summary Report for 2008 and 2009 Repairs, Surface Cover System, Dundalk Marine Terminal* (2008–2009 cover repair summary report). A change from semiannual inspection frequency originally proposed in the SCMP to annual inspections was recommended based on observations made after the baseline cover system inspection indicated that the pavements that serve as cover systems at DMT do not erode or deteriorate as quickly as soil cover systems at typical landfill sites and, therefore, require only an annual inspection frequency.

The first annual inspection under the proposed annual inspection and repair cycle was conducted in June 2009 by CH2M representatives who had been part of the baseline inspection and initial cover system repair cycle pursuant to the SCMP. The inspection, hereinafter referred to as the spring 2009 surface

PPS0823211121WDC 1-1



cover inspection, was documented in the *2009 Surface Cover System, and 14th and 15th Street Storm Drain Inspection Report, Dundalk Marine Terminal (July 2009).* This report contained recommendations for the repair of approximately 24,971 linear feet of surface cracks, 26,661 square feet (ft²) of alligator cracking, 123,545 ft² of heave areas, and 8,174 ft² of potholes and cracks greater than 1-inch wide. Honeywell and MPA retained a paving contractor to perform the recommended repairs. Also included in the 2009 repair cycle scope of work was the repair of the Area 1501/1602 surcharge area side slope and berm, capping of light-pole areas with asphalt, abandoning fence post stubs, and replacing concrete slab sections associated with consolidation sheds 11 and 12. The 2009 surface cover repair cycle began in October and concluded in mid-December with the start of winter. Repairs from that repair cycle were documented in the report *Summary Report for Fall 2009 Repair Cycle, Surface Cover System, Dundalk Marine Terminal* (May 2010).

Since 2009, the surface cover inspection has been conducted annually. Inspection findings and an overview of repair priorities are documented in the *Surface Cover System Inspection Report* for each respective calendar year. Table 1 details completed crack sealing and paving repairs (including pothole, full-section, mill and pave, crack area, heave, and miscellaneous repairs) for all repair cycles.

The 2021 annual surface cover inspection began in May and concluded in June. Results of the inspection were detailed in the 2021 Surface Cover System and 14th and 15th Street Storm Drain Inspection Report, Dundalk Marine Terminal (October 2021). The report identified repair features including about 51,000 linear feet of surface cracks; approximately 89,760 ft² of pavement mill and pave repairs; approximately 38,000 ft² of full depth pavement repairs; and about 11,800 ft² of miscellaneous surface cover repairs.

Surface cover repairs completed during the period 2005 to 2021 are summarized in Table 1.

Table 1. DMT Surface Cover Repairs, Annual Summary

Period	Paving Repair (Acres)	Crack Sealing (Linear Feet)
2005–2006	2.00	_
2007–2008	7.20	_
Fall 2008 through spring 2009	1.67	25,485
Summer and fall 2009	3.87	11,270
2010	4.43	14,105
2011	5.89	7,000
2012	2.04	7,245
2013 (through June 2014)	4.45	15,995
2014	1.43	8,665
2015	0.50	13,460
2016	2.56	17,185
2017	4.29	17,605
2018	1.41	18,250
2019	0.99	9,610
2020	1.43	0
2021	1.24	20,280
Totals	45.40	186,155

The 2021 surface cover repair cycle began in September 2021 and concluded in December 2021. The remainder of this report summarizes repairs that were made during the 2021 repair cycle.

1-2 PPS0823211121WDC



Approximent COPR Extent

COPR Extent

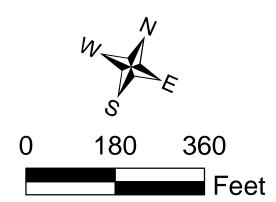


FIGURE 1 **COPR Fill Area** Dundalk Marine Terminal Baltimore, Maryland



2. Summary of 2021 Pavement Repairs

2.1 General

Features identified for repair are located and marked in the field using a global positioning system—enabled iPad with the ArcGIS Collector application. Access to the areas and movement of select storage containers, equipment, or trailer chassis are coordinated with port operations in preparation of repair activities.

Pavement and concrete cracks less than 1 inch wide are cleaned and sealed using a hot-rubber compound. Cracks greater than 1 inch wide, potholes, and surface-damage areas are milled to a depth of 3 inches, tack coated, and resurfaced using a hot-asphalt mix. Crack areas where the base asphalt material is not exposed are milled to a depth of 3 inches, tack coated, and resurfaced using a hot-asphalt mix. Crack areas where the base asphalt material is exposed are typically saw cut and repaired using a 6-inch milled tie-in, 4 inches of base course asphalt, and 3 inches of surface course asphalt.

Large area paving and heave repairs typically consist of coring to determine asphalt cover thickness; developing a milling design that: (1) minimizes exposure to the gravel base; (2) maintains the minimum asphalt thickness specification; and (3) restores surface water drainage to the nearest storm drain inlet to minimize standing water; milling, and installing base and surface asphalt as required.

P. Flanigan and Sons, Inc. performs the milling and paving repairs, while their lower-tiered subcontractor Teniseal Corporation conducts the crack sealing. Direction, observation and reporting of the inspections and completed work are performed by Jacobs on behalf of Honeywell and the MPA.

2.2 Project Team

The individuals and organizations with the 2020/21 surface cover repair cycle are summarized in Table 2.

Table 2. Project Team

Organization	Key Representative	Function
MPA	Bill Richardson	Owner representative and Party to Consent Decree
Honeywell	George Pfeiffer Eric Christodoulatos	Party to Consent Decree Party to Consent Decree
Jacobs	Kevin Wittmeyer Lisa Carter Bill Morris	Construction Project Manager Field Oversight Field Oversight

2.3 Summary of Completed Surface Cover Maintenance Activities

A summary of completed surface cover maintenance activities by operational area is presented in Table 3. The locations of completed repairs across the COPR fill area are illustrated in Figure 2. Detailed information for the surface cover repairs is provided in the Surface Cover Repair Disposition table (Appendix A). Heave repair areas and potholes, cracks, and crack repair areas that were included within the boundary of a large-area paving repair were given a disposition status of "completed."

PPS0823211121WDC 2-1



Table 3. Summary of Completed Repairs by Operational Area

Operational Area	Linear Crack Sealing (ft)	Mill and Pave, Open Area, Full-section, and Other Maintenance-type Repairs (ft²)	Large Area Paving (ft²)
97	0	0	0
800/900	0	0	0
1100	0	0	0
1200	0	0	0
1300	4,845	6,965	0
1400	8,580	5,545	0
1500	2,070	2,410	37,280
1501/1602	0	0	0
1600	0	0	0
1601	720	1,665	0
1700/1701	2,795	0	0
1702	0	0	0
1800	0	250	0
North of 1600	1,270	0	0
Total	20,280	16,835	37,280

Factors limiting repairs at various locations during the repair cycle included the following:

- Cargo and vehicle obstructions on or adjacent to surface cover repair features
- Requests by port operations to limit repairs in cargo loading or offloading areas

2.4 Remaining Maintenance Activities

Repairs that were not completed due to limited access or other operational causes during the 2021 repair cycle will be included in the scope of work for the 2022 cover system repair cycle, along with other repairs identified during the 2022 surface cover inspection. Repairs that were not completed during the 2021 repair cycle were given a disposition status of "pending" in the Surface Cover Repair Disposition table (Appendix A).

2-2 PPS0823211121WDC



Completed Surface Cover Feature
 Completed Large Area Repair
 Approximent COPR Extent
 COPR Extent



FIGURE 2 2021 Surface Cover Features Dundalk Marine Terminal Baltimore, Maryland Appendix A Dundalk Marine Terminal Surface Cover Repair Disposition

Dundalk Marine	Terminal 2021	Surface Cover R	epair Dispositio	on													_					
				_	1						M&P	+	npaved Area	3 - Surfa		4 - FS Repair	5 - Surface Heaving		nlet/ 7 - Monito		er Discoloration	ation
	Issue	Location	Location	Status			Category			Length	Width	Length	Width	Avg. Width	Length	Length Width	Max. Height Length		nhole Wellhead		Width	geta
Area	Number	Longitude	Latitude		P CS	DP LP	GC MW	In MH	RR O		(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (feet)	(inches) (feet)	(feet)	D ID	(feet)	(feet)	S Comment
97	2019-1	-76.523834	39.247978	Pending						50	2											25 abandoned fence posts
97	2019-2	-76.523900	39.248159	Pending			<u> </u>					1		0.5	220			_				Seam and lateral cracking
97	2019-3	-76.523795	39.248189	Pending			-										6 115	8				
97	2019-4	-76.523751	39.248222	Pending										0.5	150							A b - b
97	2019-5	-76.524058	39.248500	Pending						42	40	1		0.5	162							x Asphalt seam
97 97	2019-6	-76.524018	39.248257	Pending			+ +			13	10	 										Pitting asphalt
97	2019-7 2019-8	-76.524098 -76.524041	39.248226 39.248074	Pending Pending	Y					26 60	11											Pitted and cracked asphalt Potholes in roadway
97	2019-8	-76.523704	39.248290	Pending	^		+ +			00	12	205	4									Open gravel along railroad tracks
97	2019-10	-76.523584	39.248369	Pending			+ +		^			203	-	0.25	115							Asphalt seam cracking
97	2019-11	-76.523819	39.248435	Pending										0.5	90							Asphalt seam
97	2019-12	-76.523464	39.248144	Pending										0.25	525							Seal asphalt seam around tracks
97	2019-13	-76.523168	39.248014	Pending										0.25	50							· ·
97	2019-14	-76.523053	39.247915	Pending						20	15											Alligator cracking
97	2019-15	-76.522966	39.247862	Pending										0.25	95							
97	2019-16	-76.523028	39.247559	Pending	х					27	23											Potholes in roadway
97	2019-17	-76.523259	39.247583	Pending												100 5						Asphalt cracking in roadway next to tracks
97	2019-18	-76.523360	39.247719	Pending						22	8											Asphalt cracking on road shoulder
97	2019-19	-76.523509	39.247789	Pending										0.5	225							Seal asphalt seams
97	2019-20	-76.523685	39.247877	Pending						32	12											Asphalt cracking
97	2019-21	-76.523367	39.247914	Pending					х			70	2									Open asphalt along rail
													<u> </u>					ļļ		<u> </u>	<u> </u>	
900	2019-3	-76.534498	39.241963	Pending			х	$\sqcup \sqcup$		1			1					 	DMT 34M	1	1	Two damaged well pads.
900	2019-7	-76.534324	39.241907	Pending										0.5	235							Various cracks.
900	2019-8	-76.534953	39.241838	Pending										0.25	140							
900	2019-9	-76.534739	39.241693	Pending						16	2	 										
900	2019-11	-76.534474	39.241413	Pending										0.25	250							Various cracks and conduit that runs parallel to dolly pad.
900	2019-12	-76.534235	39.241460	Pending						32	13											dolly pau.
900	2019-15	-76.534184	39.241504	Pending										0.25	270							
900	2019-20	-76.533497	39.241470	Pending										0.25	125							
900	2019-23	-76.533577	39.241697	Pending										0.75	38							Cracks around concrete manhole slab.
900	2019-24	-76.533535	39.241425	Pending										0.5	90							
900	2019-25	-76.533547	39.241334	Pending	х					3	7											Two potholes near dolly pad.
900	2019-26	-76.533550	39.241272	Pending		х								0.5	400							Both sides of dolly pad.
900	2019-28	-76.533879	39.241161	Pending										0.25	170							Conduit raceway. Uneealed on both sides.
900	2019-30	-76.534248	39.242893	Pending	х					25	4											
1100	2019-1	-76.533837	39.242397	Pending													8 350	10				
1100	2019-2	-76.533689	39.242249	Pending										0.25	500							Asphalt seams
1100	2019-3	-76.533232	39.242406	Pending				х		10	5	 										Repair asphalt around inlet
1100	2019-4	-76.532349	39.242588	Pending			+	++	_	1	-	1	1	0.5	100			 		1	1	
1100	2019-5	-76.533253	39.242335	Pending		_	X	++			2	1	1	1				 		1	1	
1100 1100	2019-6 2019-9	-76.533406 -76.533399	39.242087 39.241922	Pending Pending	х		+	++-		5	3	120	1	-				 		+	1	x Gap on ramp
1100	2019-9	-76.533399 -76.532383	39.241922	Pending	x x		+ + -	+		110	3	120	1					+ +	-		-	x Gap on ramp
1100	2019-12	-76.532383	39.241861	Pending	x x		+ +	+ + -	+	30	4		1					+ +		+	1	
1100	2019-13	-76.532283	39.242108	Pending	<u> </u>					30		8	6									x
1100	2019-15	-76.532047	39.241976	Pending	x x		1				<u> </u>	 	†			40 15		† †		1	1	
1100	2019-16	-76.531926	39.241839	Pending								1					6 75	15				
1100	2019-17	-76.532160	39.242036	Pending			1				1			1	200					1		
1100	2019-18	-76.531541	39.242228	Pending	х х					200	3											
1100	2019-19	-76.531572	39.241980	Pending										0.5	75							
1100	2019-20	-76.531634	39.241877	Pending													8 90	15				
1100	2019-21	-76.531097	39.242313	Pending										0.25	70							Seal conduit saw cut
1100	2019-22	-76.530613	39.242308	Pending						9	2											Four abandoned fence post
1100	2019-23	-76.530582	39.242251	Pending													6 50	10				
1100	2019-24	-76.531092	39.242484	Pending						55	7											Cracked asphalt
1100	2019-25	-76.530989	39.242703	Pending						35	10		ļ					<u> </u>		1	<u> </u>	Cracking at bottom of ramp
1100	2019-26	-76.531194	39.242678	Pending			\bot	\Box		30	7		ļ							<u> </u>	ļ	Cracks in asphalt
1100	2019-27	-76.530988	39.242848	Pending			\bot \bot	$\sqcup \sqcup$		40	4		ļ					<u> </u>		1	ļ	
1100	2019-28	-76.530917	39.242880	Pending			+	\vdash		8	4		<u> </u>			1.0				 	<u> </u>	
1100	2019-29	-76.531120	39.242946	Pending			+	$\sqcup \sqcup$	х				1			10 10		 		1	1	Remove stop light foundation
1100	2019-30	-76.531226	39.243150	Pending	Х					4	4	1	ļ]	L					1	<u> </u>	

Dundalk Marine	Terminal 2021	Surface Cover R	epair Dispositio	on																					1			
		1			_						<u> </u>	1 - 3" N			Jnpaved Are	_	Surface Crack		Repair		face Heaving		6 - Inlet/	7 - Monitor		er Discoloration	ation	
	Issue	Location	Location	Status		oc D	0 10	GC MW		H RR		ength	Width	Length	Width	Avg. W	_	Length	Width	Max. Height	Length		Manhole	Wellhead	Length	Width	egeta	Commont
Area	Number 2019-31	Longitude -76.531124	Latitude 39.243096	Pendin		-2 DI	P LP	GC IVIW	/ in ivi	н кк	_	feet)	(feet)	(feet)	(feet)	(inch	es) (feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Š	Comment
1100	2019-31	-70.551124	39.243090	Penum	g x				++	++		12	3			+												+
1200	2019-1	-76.530641	39.242766	Pendin	g	х				+++				50	1													Gap in concrete slab
1200	2019-2	-76.530648	39.242708	Pendin								35	13															Cracking asphalt
1200	2019-6	-76.530006	39.242659	Pendin	g x	х												40	4									
1200	2019-7	-76.529911	39.242731	Pendin	g	х						55	1															
1200	2019-8	-76.529775	39.242738	Pendin	g x	х												10	7									
1200	2019-9	-76.529621	39.242816	Pendin		х												15	15									
1200	2019-10	-76.529488	39.242917	Pendin		х				1 1		7	3			+												
1200	2019-11	-76.529270	39.242751	Pendin	-		-		1	1	х	10	-															Damaged water valve manhole
1200 1200	2019-12 2019-13	-76.529234 -76.528971	39.242530 39.243138	Pendin Pendin					х	+++		10	5			0.7	30											Loose inlet frame
1200	2019-13	-76.528698	39.243130	Pendin	-	x			+ +	+++		30	1			0.7	30											
1200	2019-15	-76.528530	39.243047	Pendin	-				1 1	1 1	_	10	6															
1200	2019-16	-76.528424	39.242587	Pendin	-							20	20															Cracking asphalt
1200	2019-17	-76.528340	39.242658	Pendin	g							35	10															
1200	2019-18	-76.527891	39.242675	Pendin	g x							10	5															
1200	2019-22	-76.528402	39.243224	Pendin	g											0.5	50										х	
1200	2019-23	-76.528437	39.243333	Pendin	-							11	15															Cracked asphalt
1200	2019-26	-76.528485	39.243623	Pendin		_	_		-	+++		22	2									.						11 abandoned fence posts
1200 1200	2019-27 2019-29	-76.528483 -76.528519	39.243565 39.243801	Pendin Pendin						+		25	20 15			+												Cracked asphalt Cracked asphalt
1200	2019-29	-76.529631	39.243487	Pendin	-					++		315	53			+												Cracked asphalt
1200	2019-31	-76.530755	39.243038	Pendin	-							364	33															Cracked aspirate
1200	2019-32	-76.530756	39.242905	Pendin	_					+ +		10	2															Remove two guardrail posts
1200	2019-33	-76.530697	39.242876	Pendin	-						_	30	30															Cracked asphalt
1200	2019-34	-76.530818	39.243051	Pendin	g x							30	15															Cracked asphalt
1200	2019-35	-76.530767	39.243263	Pendin	g						х							30	10									Replace curb
1200	2019-36	-76.529724	39.243604	Pendin							х					1		300	8									Cracked asphalt
1200	2019-37	-76.529210	39.243656	Pendin	-			х		1 1									_					DMT-30S				Damaged well pad
1200	2019-38	-76.528968	39.243853	Pendin					<u> </u>	+ +	х							227	7									Remove guardrail, replace with jersey barriers
1200 1200	2019-39 2019-40	-76.529421 -76.528816	39.243856 39.243989	Pendin Pendin	-	-	+			++	_	90	45			+		90	45			+					Х	Old trailer location Cracked asphalt
1200	2019-40	-76.528818	39.243894	Pendin					+ +	+++		90	43			0.5	155											Cracked aspirate
1200	2025 12	70.020010	33.2.303.	, c.i.a.i.						+ +						0.5	133											
1300	2019-1	-76.524984	39.242710	Pendin	g			х																				Damaded well pad
1300	2019-2	-76.525049	39.242670	Pendin	g			х																				Damaged well pad
1300	2019-3	-76.526177	39.242239	Pendin	g				х			30	15									-	TEV-004					Pave around vault manholes
1300	2019-5	-76.526014	39.242315	Comple	te							80	4															
1300	2019-4	-76.525722	39.242385	Pendin						1 1								30	15									Settlement
1300	2019-6	-76.525393	39.242454	Pendin		_			1 1	+++	_					0.5	125			10	127	15						Cool group halous sidous lik
1300 1300	2019-7 2019-8	-76.525171 -76.525255	39.242486 39.242640	Pendin Pendin		-	+			++	_	60	35			0.5	135					+					Х	Seal crack below sidewalk Cracked asphalt
1300	2019-8	-76.525233	39.242714	Pendin					++	+	_	12	3		+	+		+	1							1		o. doned dopridit
1300	2019-10	-76.524973	39.242594	Pendin		T			+ +	+	_	2	2		+	+					1							+
1300	2019-11	-76.525292	39.242550	Comple								25	20															
1300	2019-12	-76.525469	39.242627	Pendin	g x	х						8	3															
1300	2019-13	-76.525983	39.242526	Pendin					$\perp \perp$	$\bot \bot$						0.2	35											Seal patches
1300	2019-14	-76.526240	39.242430	Pendin					++	+	_			20	15	\perp		1										
1300	2019-15	-76.526384	39.242528	Pendin	-	х			++	++		7	3		+	+		1			1					1		<u> </u>
1300 1300	2019-16 2019-17	-76.526303 -76.526098	39.242553 39.242655	Pendin Comple	_	+			++	++		32 20	7 8	-	+	+		+	1		+					-		+
1300	2019-17	-76.526098 -76.526136	39.242655	Comple		+			++	++	_	21	10		+	+	+	+	1									+
1300	2019-18	-76.525169	39.242743	Pendin					x	++		18	14		+	+		+	1				MH-314					+
1300	2019-20	-76.525260	39.243120	Pendin		1			 ^	+	1	<u> </u>			1	1		1		14	52	25						Heave in roadway
1300	2019-21	-76.526120	39.242831	Comple	-							35	4															
1300	2019-22	-76.526590	39.242829	Pendin	g							32	20															
1300	2019-24	-76.526477	39.242624	Pendin					$\perp \Gamma$			35	35															
1300	2019-23	-76.526334	39.242618	Pendin					+	\perp		20	20															
1300	2019-25	-76.526764	39.242776	Pendin	_	\perp			++	++					1													Repair DMT-37M
1300	2019-26	-76.526709 -76.526709	39.242888	Pendin		-			++	++	.,			1	1	+		1	1	10	25	20		DMT CC		1		<u> </u>
1300 1300	2019-27 2019-28	-76.526520 -76.525353	39.242976 39.243367	Pendin Pendin		+		Х	++	++	х	125	3		+	+	_	+	1					DMT-6S				+
1300	2019-28	-76.525475	39.243556	Pendin		+			++	+	_	115	3		+	+		+	1									Seam separation
1300	2020 20	, 5.5254, 5	33.2.3330	· criuiii	0 ^													L	1	I.	1	L L			<u> </u>	I		

Dundalk Marine	Terminal 2021	Surface Cover R	epair Disposition	on																				1		
	Issue	Location	Location	Status	7		Cato	aon.		-	1 - 3"			Inpaved Area	3 - Surface Avg. Width		4 - FS F			ace Heaving A		6 - Inlet/	7 - Monitor		er Discoloration	ation
Area	Issue Number	Location Longitude	Location Latitude	Status	P CS	DΡ	Categ LP GC		n MH R	R O	Length (feet)	Width (feet)	Length (feet)	Width (feet)	(inches)	Length (feet)	Length (feet)	Width (feet)	Max. Height (inches)	Length (feet)	Width (feet)	Manhole ID	Wellhead ID	Length (feet)	Width (feet)	e Comment
1300	2019-33	-76.526044	39.243468	Complete	+	Dr	LF GC	10100 11	I IVIII K	N O	50	10	(ieet)	(leet)	(inches)	(ieet)	(leet)	(ieet)	(ilicites)	(reet)	(ieet)	ID	ID	(ieet)	(reet)	Cracking asphalt
1300	2019-34	-76.525531	39.243714	Complete		+ +					60	3														Seaparated asphalt seams
1300	2019-35	-76.525512	39.243671	Pending											0.5	65										
1300	2019-36	-76.525654	39.243862	Pending											0.5	120										Cracks at asphalt seams
1300	2019-37	-76.526237	39.243501	Pending															6	100	15					
1300	2019-38	-76.526855	39.243419	Pending		1					22	12														
1300	2019-39 2019-40	-76.527422 76.527612	39.243275 39.243054	Pending	X						5 22	6														
1300 1300	2019-40	-76.527613 -76.527733	39.243054	Pending Pending	×	+ +					25	4														Seqm separation
1300	2019-42	-76.527821	39.243263	Pending	1	+ +					23				0.25	35										эсціп эсрагалогі
1300	2019-49	-76.525751	39.244037	Pending							36	3														
1300	2019-50	-76.525786	39.244090	Pending											0.5	70										Seal sawcut
1300	2019-51	-76.525768	39.244116	Pending									2	2												Hole in asphalt
1300	2019-52	-76.526782	39.243818	Pending															8	130	15					
1300	2019-53	-76.526859 -76.527014	39.243846	Complete	+ +	+ +					20	20			0.5	100										
1300 1300	2019-54 2019-62	-76.527014 -76.528114	39.243750 39.243702	Pending Pending	×	+		-		+++	25	3			0.5	100										
1300	2019-62	-76.526769	39.244164	Pending	*	+ +					25	3					256	10								Damaged bollzrds and charging stations
1300	2019-64	-76.526466	39.244051	Pending	+ +										0.5	70	250	10								Sumages some as and energing seatons
1300	2019-65	-76.526082	39.244536	Pending											0.75	80										Seal seam
1300	2019-66	-76.526355	39.244455	Pending											0.5	100										
1300	2019-67	-76.526820	39.244478	Pending															6	50	10					
1300	2019-68	-76.527186	39.244261	Complete							32	15														
1300	2019-69	-76.527647	39.244015	Complete	х						15	15														
1300	2019-70	-76.527816	39.243951	Pending		1									0.25	150										
1300	2019-71	-76.528104	39.243865	Pending	+ +	+ +			+ + .	. —	50	10														
1300 1300	2019-72 2019-73	-76.526970 -76.526675	39.244437 39.244662	Pending Pending	+ +	+		-	++'		350	8							6	75	10					
1300	2019-73	-76.526558	39.244654	Pending		++					70	7							0	/3	10					Remove curb
1300	2019-75	-76.526492	39.244630	Pending	+ +						70				0.25	90										Asphalt seam around electric manhole
1300	2019-76	-76.526347	39.244747	Pending															12	85	15					
1300	2019-77	-76.526276	39.244801	Complete							30	25														Cracking asphalt
1300	2019-78	-76.527156	39.244372	Pending	х								30	20												
1300	2019-79	-76.527584	39.244236	Complete							225	3														Seam separation
1300	2019-80	-76.528000	39.244247	Pending		1					85	15														Ponding water adjacent to building
1300	2019-81	-76.528413	39.244058	Pending	+ +	+ +		Х	(6	6														Cracked asphalt around IN-688
1300 1300	2019-83 2019-84	-76.527402 -76.526224	39.244503 39.244962	Pending Complete	+ +	+		-		+ +					0.25	110										Cut back rebar in asphalt
1300	2019-85	-76.526826	39.244810	Complete	+ +	1 1									0.25	250										
1300	2019-86	-76.527309	39.244761	Pending		1 1	-						12	10	0.25	250										
1300	2019-87	-76.527844	39.244443	Pending															6	130	8					
1300	2019-88	-76.528468	39.244200	Complete											0.25	150										Seal seams
1300	2019-89	-76.527990	39.244380	Complete											0.5	130							_	_		
	1				+	$\perp \perp$				\perp			1	1						1						
1400	2019-1	-76.523778	39.243190	Pending	+	++	\dashv			+			1	1	0.5	65			8	145	20					
1400	2019-2 2019-3	-76.523881 -76.524754	39.243133	Complete	+	+ +	-		+	+	<i>/</i> 11	3	1		0.5	65										Asphalt seam separating under puddle
1400 1400	2019-3	-76.524754 -76.524953	39.242843 39.242856	Complete Complete	+	++	+			+	41 8	4	+	+												Two potholes
1400	2019-4	-76.524947	39.242939	Pending	1 1	+		х		+	U	-	+													Damaged well pad
1400	2019-6	-76.524859	39.242984	Pending	х	1 1				+ +	6	4														Pothole at seam
1400	2019-7	-76.524265	39.243213	Complete											0.5	130										Unsealed asphalt patch and surrounding cracks
					+-	+	+		+	+			-							ļ					ļ	Construe des son disc
1400	2019-8 2019-9	-76.523699 -76.523286	39.243279 39.243475	Pending	+ +	+				+			1	1	0.5	170										Crack under ponding x Crack along asphalt seam
1400 1400	2019-9	-76.523286 -76.522979	39.243475	Complete Pending	+ + -	+			+++	+			1	1	0.5	100	 		10	110	15					x Crack along asphalt seam
1400	2019-10	-76.522979	39.243513	Pending	++-	++	+	-	+	+			+			 			10	70	20				-	Heave in roadway
1400	2019-12	-76.522913	39.243749	Pending		† †				+ +					0.5	100				<u> </u>						x
1400	2019-13	-76.523256	39.243616	Pending							20	20	1	1			†									Alligator cracking
1400	2019-14	-76.523861	39.243481	Pending											0.5	85										
1400	2019-15	-76.525061	39.243056	Complete				х	(8	8						-						-		Damaged asphalt around inlet grate at IN-777
1400	2019-16	-76.524822	39.242636	Complete			\Box			\perp					0.5	30										
1400	2019-17	-76.524445	39.242743	Complete	+	$\perp \perp$				\perp			1		0.5	300										Cracks along asphalt seams
1400	2019-19	-76.523221	39.243223	Complete	+	++				+			1	1	0.25	63	-				2.2					Unsealed asphalt patch
1400	2019-20	-76.523240	39.243199	Pending	1 1		1 1						1	1	<u> </u>	1			12	33	20				1	

Dundalk Marine	Terminal 2021	Surface Cover R	epair Dispositio	on																					
					ı						' M&P		npaved Area	3 - Surfac		4 - FS Repai			ce Heaving A		6 - Inlet/	7 - Monitor	8 - Gravel Cove	er Discoloration	ition
	Issue	Location	Location	Status			Category			Length	Width	Length	Width	Avg. Width	Length	-	/idth	Max. Height	Length		Manhole	Wellhead	Length	Width	geta
Area	Number	Longitude	Latitude		P CS	DP LP	GC MW	In MH	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (f	eet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	S Comment
1400	2019-21	-76.523088	39.243256	Complete										0.25	37										
1400	2019-22	-76.522827	39.243299	Complete										0.5	165										
1400	2019-24	-76.522608	39.243471	Complete				1 1						0.5	100										
1400	2019-25	-76.522564	39.243346	Pending				+ + +					-	0.5	222			12	155	10					Cracks along apphalt soons
1400 1400	2019-27 2019-48	-76.522932 -76.523914	39.243451 39.245025	Complete	,		+ +	+ + +		23	5			0.5	222										Cracks along asphalt seam
1400	2019-48	-76.526072	39.243023	Complete Pending	*		+ +	+ + +		116	6		1												
1400	2019-28	-76.525777	39.244977	Pending			+ +	+ + +		110	0		1					6	6	6					
1400	2019-30	-76.525455	39.245150	Complete										0.25	420			Ů							Asphalt seams
1400	2019-31	-76.525263	39.245134	Pending										0.23				12	35	12					
1400	2019-33	-76.524854	39.245291	Pending														10	44	8					
1400	2019-34	-76.524429	39.245720	Pending								23	5												
1400	2019-36	-76.524286	39.245527	Pending														10	32	20					
1400	2019-39	-76.523931	39.245704	Complete										0.5	135										
1400	2019-40	-76.523836	39.245655	Pending										0.5	50										
1400	2019-41	-76.523753	39.245581	Pending										0.5	90										
1400	2019-42	-76.523822	39.245468	Pending														6	72	15					
1400	2019-43	-76.524272	39.245514	Pending					х	100	2														Remove 50 fence posts
1400	2019-44	-76.524077	39.245367	Pending														12	133	10					
1400	2019-45	-76.524012	39.245214	Pending	х			1 1		3	3		1												
1400	2019-46	-76.523733	39.245308	Pending		Х	+-+	1 1						0.5	72										X
1400	2019-47	-76.523563	39.245121	Pending				1 1						0.25	115							D. C.			Deve-sed well sed
1400 1400	2019-49 2019-50	-76.523886 -76.522584	39.245094	Pending			х	+ + +						0.5	100							P-6			Damaged well pad
1400	2019-50	-76.522631	39.243245 39.243223	Complete Complete			+ +	+ + +						0.5	92										Seal patch
1400	2019-51	-76.522814	39.243223	Pending			+ +	+ + +					1	0.5	92			12	55	10					Sear pateri
1400	2019-52	-76.522989	39.243155	Pending		×	+	+ + +						0.5	72			12	33	10					x
1400	2019-54	-76.522847	39.243086	Complete		^								0.5	200										x
1400	2019-55	-76.522995	39.243066	Complete										0.25	185										Seal patches
1400	2019-59	-76.523512	39.242877	Pending						6	2														
1400	2019-61	-76.523998	39.242753	Pending		х								0.5	72										x
1400	2019-60	-76.523993	39.242698	Pending																					Missing hydrant
1400	2019-62	-76.524238	39.242635	Complete										0.5	75										
1400	2019-80	-76.525801	39.244353	Complete	х					120	3														Repair asphalt seam
1400	2019-81	-76.525367	39.244561	Complete										0.5	200										
1400	2019-82	-76.524849	39.244586	Pending										0.25	375										
1400	2019-84	-76.524017	39.244972	Pending				1 1				17	3												
1400	2019-83	-76.524135	39.244995	Complete				1 1		30	12		_												Dunatura in annhalt
1400	2019-64	-76.523098	39.242299 39.242393	Pending			+	+ + +				6 2	2												Puncture in asphalt Hole in asphalt
1400 1400	2019-65 2019-66	-76.523356 -76.523278	39.242393	Pending Pending			+ +	+ + +				2		0.75	22										x Seal around crane stop
1400	2019-67	-76.522601	39.242582	Pending			+	+ + +						0.73	240										Seal surface cracks around conduit
1400	2019-68	-76.522663	39.242444	Pending								24	3	0.5											
1400	2019-69	-76.522189	39.242593	Pending				† † †					1	0.5	240										Seal asphalt at edge of containment wall
1400	2019-70	-76.522326	39.242776	Pending				1 1		İ		20	6												-
1400	2019-71	-76.522399	39.242731	Pending								2	2												Abandoned fence posts
1400	2019-72	-76.522542	39.242983	Pending						60	3														Cracking asphalt over conduit
1400	2019-75	-76.523221	39.242750	Pending		х										21	4								Repair dolly pad zeam
1400	2019-78	-76.523115	39.242884	Pending			\bot	\bot \downarrow \downarrow										8	58	30					
1400	2019-85	-76.525988	39.244605	Complete	х		+	$\downarrow \downarrow \downarrow$		35	4	1	1												
1400	2019-86	-76.525634	39.244544	Pending	\vdash	_	+	1 1					1	0.5	125										
1400	2019-87	-76.525823	39.244949	Pending	\vdash		++	+	х			1	1			350	12							-	Remove guardrail replace with jersey barrier
1400	2019-88	-76.525685	39.244060	Pending	\vdash	_	+ +	+ + +	-	30	3	1	1	0.5	F20	 									Soal soams
1400 1400	2019-89 2019-90	-76.524514 -76.524574	39.244518 39.244466	Pending Pending	\vdash		++-	+ + +	+			+	1	0.5	520			8	50	15					Seal seams
1400	2019-90	-76.524574 -76.523791	39.244746	Complete	 		+ + -	+ + +		30	10					 		0	30	13					Asphalt cracking
1400	2019-91	-76.523258	39.244635	Complete			+ + -	† † †		35	3	1													
1400	2019-94	-76.524757	39.244160	Pending		х	1 1	1 1		- 33		1	1			23	15								
1400	2019-95	-76.524778	39.244013	Pending		<u> </u>		† † †	\dashv					0.25	20										Seal conduit paving
1400	2019-97	-76.525294	39.243937	Pending				1 1							-			6	70	10					
1400	2019-98	-76.525472	39.243735	Complete				1 1		10	4														
1400	2019-99	-76.524677	39.243690	Pending														10	160	20					
1400	2019-100	-76.524310	39.243783	Complete										0.25	200										
1400	2019-101	-76.523319	39.244343	Pending														6	58	10					

Dundalk Marine	Terminal 2021	Surface Cover R	Repair Dispositio	n																				
					_					1 - 3'	" M&P	2 - Open/L	Inpaved Area	3 - Surfa	ace Crack	4 - FS Repair	5 - Suri	face Heaving	Area	6 - Inlet/ 7	- Monitor	8 - Gravel Cove	er Discoloration	ion
	Issue	Location	Location	Status			Category	,		Length	Width	Length	Width	Avg. Width	Length	Length Width	Max. Height	Length	Width	Manhole \	Wellhead	Length	Width	etat
Area	Number	Longitude	Latitude		P CS	DP LP	GC MW	In MH	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	S Comment
1400	2019-102	-76.523106	39.244317	Pending										0.5	100								, ,	Seal seams
1400	2019-103	-76.523195	39.244112	Pending												25 13								
1400	2019-104	-76.523310	39.244168	Pending												18 9								
1400	2019-105	-76.523419	39.243950	Pending	+++		++	+++	_							10 3	8	60	8					
1400	2019-103	-76.523151	39.243939	Complete	+ + +			 						0.5	470		8	- 00	8	+				
	2019-106	-76.523131			++++		1							0.5	470		12	100	25					X
1400			39.243763	Pending	++++		1									26 10	12	100	25					Domove delly ned
1400	2019-108	-76.523978	39.243729	Pending	++++	X	+ +	+++	-					0.5	200	36 10		+						Remove dolly pad
1400	2019-109	-76.524406	39.243568	Complete	+		1	+ + +						0.5	300		16	22	16					Seal seams
1400	2019-110	-76.524930	39.243342	Pending	+++	Х	+	+++	_								16	23	16	-				
1400	2019-111	-76.524511	39.243378	Pending	+++												10	84	15					
1400	2019-112	-76.522933	39.243911	Pending	+++									0.5	300									
1400	2019-113	-76.523115	39.244013	Pending	\bot	х		\perp								32 128								Remove dolly pad
					\bot			\perp																
1500	2019-1	-76.519629	39.243732	Complete						19	9													
1500	2019-2	-76.519953	39.243883	Pending										0.25	38									
1500	2019-3	-76.519908	39.243809	Complete						10	10													Concrete patch with cracking
1500	2019-4	-76.519768	39.243677	Complete	х					4	4													
1500	2019-5	-76.520326	39.243711	Complete						8	3													Puncture in asphalt
1500	2019-7	-76.523882	39.245971	Pending								8	5											Open area at edge of asphalt
1500	2019-9	-76.523629	39.246076	Pending								8	5											Open area at edge of asphalt
1500	2019-10	-76.523284	39.246231	Pending																		8	6	Discolored gravel adjacent to railroad switch
1500	2019-12	-76.523726	39.245759	Pending					х	12	2													Remove sic abandoned fence posts
1500	2019-13	-76.523279	39.245986	Pending				х		24	2													Asphalt damage around inlet grate at IN-801
1500	2019-14	-76.523250	39.246044	Complete										0.5	150				ĺ					
1500	2019-16	-76.523051	39.246060	Pending					х	22	2													Remove 11 abandoned fence posts
1500	2019-17	-76.523024	39.245951	Pending						66	9													Cracked asphalt next to canopy
														<u> </u>	<u> </u>				1					Crack between concrete slab and asphalt around
1500	2019-18	-76.523115	39.245920	Pending	X									0.75	360									canopy
1500	2019-19	-76.523380	39.245816	Pending										0.25	130									Crack for conduit sawcut and adjacent crack
1500	2019-20	-76.523581	39.245740	Pending				х		28	4													Damaged asphalt around IN-803
1500	2019-21	-76.523578	39.245500	Pending						40	2													Severe cracks in roadway
1500	2019-22	-76.523452	39.245494	Complete													10	38	20					
1500	2019-23	-76.523383	39.245416	Complete	х					37	6													
1500	2019-24	-76.523345	39.245439	Complete		-											12	114	12					
1500	2019-25	-76.523117	39.245558	Complete		-											12	57	10					
				·										1					1.0					May 20, 2021 dimensions updated from 16"x82'x10'
1500	2019-26	-76.523038	39.245622	Complete													18	116	10					to 18"x116'x10'
1500	2019-27	-76.522981	39.245625	Complete													18	112	10					May 20, 2021 dimensions updated from
				· ·																				18"x112x10'.
1500	2019-28	-76.522776	39.245784	Complete													8	88	25					
1500	2019-29	-76.522821	39.245746	Complete										0.5	225									х
1500	2019-30	-76.522580	39.245755	Complete													14	76	15					
1500	2019-31	-76.522591	39.245653	Complete													16	164	20					5/20/2021 Dimensions updated from 14"x164'x20'
					+++				_								-	_						to 16"x164'x20'
1500	2019-32	-76.522831	39.245521	Complete	+++				_	44	10													Cracked asphalt with ponding
1500	2019-33	-76.522910	39.245445	Complete												24 23								Alligator cracking
1500	2019-34	-76.523089	39.245421	Complete	+	\perp	 - - - - - - - - -	+			1			1			6	42	10					
1500	2019-35	-76.523249	39.245339	Complete	+ + +		+	+++		24	24						ļ	1	ļļ					
1500	2019-36	-76.523333	39.245293	Complete	+			+++						0.5	180			1						Cracks along and adjacent to asphalt seams
1500	2019-37	-76.523392	39.245199	Complete	+		\bot	$\sqcup \sqcup$						0.25	350									Seal asphalt seam
1500	2019-38	-76.522827	39.244804	Pending		х											8	16	16					Heave under light pole
1500	2019-39	-76.522788	39.244844	Pending	х			$\sqcup \sqcup$						0.5	20									Seal transformer pad
1500	2019-41	-76.521651	39.245152	Pending										0.5	300									Seal asphalt seam
1500	2019-42	-76.521678	39.245137	Pending													10	55	12					
1500	2019-43	-76.521841	39.244937	Pending										0.25	250									Reseal asphalt seam
1500	2019-44	-76.522216	39.244900	Pending													14	40	30					
1500	2019-45	-76.522285	39.244886	Pending										0.5	220									x Asphalt seam
1500	2019-46	-76.522177	39.244833	Pending						30	5										ĺ			Pitted asphalt
1500	2019-47	-76.522429	39.244859	Pending	х					15	10		1					1	1					
1500	2019-48	-76.522496	39.244827	Pending	1 1 1			1 1		30	10							1						Alligator cracking
1500	2019-49	-76.522608	39.244811	Pending	1 1 1			1 1 1					1				14	70	20					
1500	2019-50	-76.522565	39.244760	Pending	+ + +	\dashv		 		16	10						1	T						Alligator cracking
1500	2019-51	-76.522767	39.244654	Pending	+ + +	1	1 1	 	-				†	1	1		8	68	15					5 ·
1500	2019-52	-76.522751	39.244660	Pending	1 1 1	-	+ + -	 	1					0.5	100		1 -	1						Asphalt seam
1500	2019-52	-76.522751	39.244604	Pending	+++	+	+ + -	 	_		 	<u> </u>	+	5.5	100	17 12		+	+ + +		+			Alligator cracking
1300	2013 33	10.322131	33.277007	i chung	\bot		1 1					1	<u> </u>	<u> </u>	1	1, 14	ļ	1					l	,ga.co. o. aog

Dundalk Marine	Terminal 2021	Surface Cover R	epair Dispositio	n																_				
					Ī					1 - 3	" M&P	2 - Open/l	Jnpaved Area	3 - Surfac	e Crack	4 - FS Repair		rface Heaving		6 - Inlet/	7 - Monitor	8 - Gravel Cove		tion
	Issue	Location	Location	Status			Category			Length	Width	Length	Width	Avg. Width	Length	Length Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width] Seta
Area	Number	Longitude	Latitude		P CS [DP LP	GC MV	V In MH	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	ভূঁ Comment
1500	2019-54	-76.522807	39.244594	Pending												19 10								Alligator cracking
1500	2019-55	-76.522853	39.244570	Pending												20 14								Alligator cracking
1500	2019-56	-76.522924	39.244550	Pending												20 13								Alligator cracking
1500	2019-57	-76.522934	39.244345	Pending													6	50	10					
1500	2019-58	-76.522765	39.244321	Pending						28	20													Alligator cracking
1500	2019-59	-76.522623	39.244221	Pending													8	23	12					
1500	2019-60	-76.522603	39.244369	Pending										0.25	40									
1500	2019-61	-76.522380	39.244464	Pending													10	113	10					
1500	2019-62	-76.522313	39.244349	Pending								2	2											Coring location
1500	2019-63	-76.522226	39.244366	Pending						58	13													Cracked asphalt along seam
1500	2019-64	-76.522198	39.244366	Pending						40	12													
1500	2019-67	-76.522026	39.244546	Pending													10	46	10					
1500	2019-68	-76.521920	39.244532	Pending			 	+ + -						0.25	370			_						Seal asphalt patches
1500	2019-69	-76.521792	39.244619	Pending				+		44	23	-						_						Cracking asphalt
1500	2019-70	-76.521710	39.244660	Pending						24	12							+						Cracking asphalt
1500	2019-72	-76.521267	39.244669	Pending	х					5	5				400			+						
1500	2019-73	-76.521282	39.244688	Pending				+				-		0.5	120			_						Seal asphalt seams
1500	2019-74	-76.521523	39.244534	Pending				+	1					0.25	460			+						Seal seams for asphalt patches
1500	2019-75	-76.521471	39.244559	Pending		Х		+	1					0.5	560	45 46		+						Seal dolly pad
1500	2019-76	-76.522718	39.244535	Pending				+	1							45 16		+						
1500	2019-77	-76.522858	39.244466	Pending			+ +	+ + -						0.25	255	20 14		_						Coal asphalt natabas
1500	2019-78	-76.521122	39.244118	Pending			+ +	+ + -		455				0.25	255			_						Seal asphalt patches
1500	2019-79	-76.521177	39.244028	Complete			+ +	+ + -		155	3							_						Replace asphalt over conduit
1500	2019-80	-76.521363	39.243995	Complete		.,	+ +	++-	-	25	12	-		0.5	400									
1500 1500	2019-81 2019-82	-76.521827	39.243853	Pending		Х	+	+	\vdash	115	2	-		0.5	480									Ponaco asphalt over conduit
	2019-82	-76.521800 -76.522138	39.243820	Complete			+ +	+		115	3	22	2											Repace asphalt over conduit
1500	2019-85	-76.522138	39.243678 39.243553	Pending			+ +	+				22	2			20 12		+						Mill and pave along damaged seam
1500 1500	2019-86	-76.522314	39.243553	Pending Pending			+ +	+				+				20 12	6	90	5					
1500	2019-88	-76.521523	39.243755	Pending			+	++-						0.5	70		0	30	,					x
1500	2019-93	-76.521523	39.244489	Pending			+	++-						0.5	190			+						x
1500	2019-101	-76.520207	39.243967	Complete				+ + -		8	3			0.5	150									^
1500	2013 101	70.520207	33.243307	complete			+ +	+			,													
1501/1602	2019-1	-76.517246	39.242317	Pending				+ + -									10	36	6					Heave near wall
1501/1602	2019-2	-76.520201	39.242984	Pending		x		++-									8	70	15					From LP 15-17
1501/1602	2019-3	-76.519745	39.242654	Pending													12	90	35					
1501/1602	2019-4	-76.519820	39.243438	Pending										0.5	2									Crack eeal around patch.
1501/1602	2019-5	-76.519564	39.243187	Pending													10	218	30					
	2010.6			Danding										1	Г1									51 feet of crack sealing around 5 gaurd rail posts.
1501/1602	2019-6	-76.520914	39.241245	Pending					х					1	51									
1501/1602	2019-7	-76.520856	39.241303	Pending	х					5	7													Two poyholes from previous boring locations.
1501/1602	2019-8	-76.520893	39.241321	Pending										0.5	50									
1501/1602	2019-9	-76.520821	39.241303	Pending				х									8	6	6					Heave around inlet.
1501/1602	2019-10	-76.520911	39.241714	Pending								1	1	0.5	70			+						
1501/1602	2019-11	-76.521051	39.241982	Pending			+	+	\vdash	1	_	-	1	0.5	85				1	-				
1501/1602	2019-12	-76.521139	39.242130	Pending		_	1		+			1	+	0.5	30			+		-				Multiple are -1:-
1501/1602	2019-13	-76.520613	39.241570	Pending			+ +	+	+ +	+	-	1	+	0.5	90			+	1	1			1	Multiple cracks
1501/1602 1501/1602	2019-14	-76.520493 -76.520403	39.241549	Pending		_	+	+ +	+			1	1	0.25	60			+					-	
1501/1602	2019-15 2019-16	-76.520403 -76.520372	39.241431 39.241419	Pending Pending		_	+-	++	\vdash	+	 	-	+	0.25	65 2			+	1	-				Crack seal around old voncrete well pad.
				_			+ +							0.5										Crack seal around old voliciete well pad.
1501/1602	2019-17 2019-18	-76.520440 -76.520620	39.241630	Pending			×	+	\vdash	4	4	-		0.5	90						Unknown			Two separate well pads. One abandoned.
1501/1602			39.241804	Pending			<u> </u>	+ + -		4	4			0.5	75		+	+			Ulikilowii			Two separate well paus. One abandoneu.
1501/1602 1501/1602	2019-19 2019-20	-76.520757 -76.520541	39.242109 39.242073	Pending Pending			1 1	+ +	+ +			+	+	0.5 0.5	75 50			+		1				
1501/1602	2019-20	-76.520541 -76.520152	39.242073	Pending			1	+ + -	+ +	1	1	+	+	0.5	30		6	72	20	1			 	
1501/1602	2019-21	-76.520152	39.241574	Pending			1 1	+ +	1 1			+	+	0.5	90		0	12	20				 	
1501/1602	2019-22	-76.520200	39.242192	Pending		-	++-	++-		+	 		+	0.5	175			+	 	-				
1501/1602	2019-23	-76.520541	39.242192	Pending			+++	++-	+ +	51	4	+	+	0.5	1/3			+	1					
1501/1602	2019-24	-76.520608	39.242423	Pending			+	++-	+	24	6	+	+					+		-				
1501/1602	2019-25	-76.520008	39.242423	Pending		-	++-	++-			 		+	0.5	100			+	 	-				Seal four asphalt seams
1501/1602	2019-27	-76.520224	39.242116	Pending			+	++-	+	+		+	+	0.5	50			+		-				Sear rour aspirare searns
1501/1602	2019-27	-76.520224	39.242110	Pending		-	++-	++-		+	 		+	0.5	260			+	 	-				Cracks in ponding area
1501/1602	2019-28	-76.519618	39.241692	Pending			1	1 1	1 1			+	+	0.5	125			+	1	1				or some in portaining at each
1501/1602	2019-30	-76.519517	39.241814	Pending			1	1	1 1	1	1	1	1	0.5	70		1	+						
1301/1002	-010 00	, 5.51551	33.271017	· chang		I	11	1 1	<u> </u>	1	ı	1	1		,,,	1		1	1	İ	1		1	

Mary Mary	Dundalk Marine	Terminal 2021	L Surface Cover R	epair Dispositio	n																				
The column The																	4 - FS Repair				6 - Inlet/	7 - Monitor			tion
March Marc		-									_		_		_	-	•	_	_				Length	Width	geta
	Area		Longitude	Latitude		P CS I	DP LP	GC MW	In MH	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	S Comment
1.50 1.50	1501/1602		-76.519967	39.242109							13	4													Puncture in asphalt
		+			Pending	\perp									0.5	44									
					Pending													8	75	20					
Property 1968 20-2006 20-200		-				х						1													
Section Sect					-						+														
Decompose Control Co						х					43	8		1											
						\perp																			·
130,000 200 3,150,000 10,000 2,000 10,000 1					-	Х									0.25	24									Crack around electric mannole
1.500.125 1.50		_			-						2	2						24	/5	50					
					-	×						2		-	٥٢	60									
		_			-		_				22	0			0.5	60									
BACAGO 20 40 78-100 10 10 10 10 10 10 10		_			-	v																			
1800/160 200-04 Assessed 204-05 Assess		+				^					14	- 4						1/1	110	20					
March Marc											32	3						17	110	20					
Section Sect		+				+					32				0.5	150									Seal asphalt seams
					-									1											
System S					-																				
1907/100 2019 20		-																16	148	20					
Sympton Select Sympton Select Sympton Sympto		_			-										0.5	190									
SSAT_STATE STATES	1501/1602	2019-51	-76.518087	39.243113	Pending						4	4													
	1501/1602	2019-52	-76.518402	39.243724	Pending								8	4											Conduit open area
Miscon 2019 79-22294 28-29734 Pedrig	1501/1602	2019-53	-76.518579	39.243864	Pending										0.5	100									
Name	1501/1602	2019-54	-76.518928	39.243135	Pending													10	47	2					
Name																									
Miss 2019 79-52227 89-54755 77-52219 89-54755 77-5219 89-54755 77-5219 89-54755 77-5219 89-54755 79-5219 8	N1600	2019-1	-76.522291	39.247518	Pending					х	8	2													4 abandoned fence posts
Miles Mile	N1600	2019-2	-76.522232	39.247504	Pending			х		х			67	20											Railroad ballast
Milicio 2019-5 -76,5212-9 De227910 Penting De227910 Penting De227910 Pe				39.247455	Pending					х					0.5	75									x Crack at top of curb and curb gutter
Miss Miss	N1600	2019-4	-76.522287	39.247364	Pending	\perp											35 23								
Mission 20397 75-521456 39-247730 Persiding	N1600	2019-5	-76.522159	39.247347	Pending										0.75	215									Long crack plus seams for traffic light signal wire
Mission 20397 75-521456 39-247730 Persiding	N1600	2019-6	-76 522042	20 247400	Pending						50	0													
Matheway Matheway		_			-	+																			Aligator cracking along asphalt seams
Missor 2019-0 -7-8521873 92-8795 Pending		+			-			×		x		•	40	15											
No. 20 19 19 19 19 19 19 19 1		_			-	+		<u> </u>			62	12													
N1500 2019-11 76-522345 39-247569 Pending	N1600				-																				
National Color Nati	N1600	2019-11	-76.522345	39.247546	Pending					х							10 5								Dig out asphalt and reset manhole ring/lid
N1600 2013-13 -76-522567 39.247639 Pending 3 2	N1600	2010-12			Pending					v	26	11													Abandon fence posts. Damaged asphalt from curb
N1500 2019-14 -75-52284 39 24769 Pending N1500 2019-15 -75-52284 39 24769 Pending N1500 2019-16 -75-52284 39 24764 Pending N1500 2019-16 -75-52284 S9 24764 Pending N1500 2019-16 -75-52284 S9 24764 Pending N1500 2019-17 -75-52284 S9 24764 Pending N1500 2019-18 -75-52216 S9 24764 Pending N1500 2019-18 -75-52216 S9 24764 Pending N1500 2019-18 -75-52216 S9 24764 Pending N1500 S9 249-52 S9 24764 Pending N1500 S9 249-52 S9 2486										^															
N1500 2019-15 -76,522294 39,24756 Pending		_			-						3	2													Small hole adjacent to electric manhole
N1500 2019-18 76.522369 39.247504 Pending X X X X X X X X X					ŭ	\rightarrow								1	0.25	55									
N1500 2019-17 7-65-22759 33-247514 Pending X X X X X X X X X					_						22	5	_	.											x Damaged asphalt on curb
Nation 2019-18 -76.522233 39.246581 Pending		_				+					25	40	9	4											
N1600 2019-19 -76-522244 39.246456 Pending N1600 2019-20 -76-522509 39.246456 Pending N1600 2019-22 -76-522501 39.24677 Pending N1600					-	х	-	 	+		25	19		1			 	10	120	20				1	
N1600 2019-20 -76.522901 39.246545 Complete		-			-	+	-		+	-+	+	1	-	2				10	126	20				-	Hole in asphalt
N1600 2019-22 -76.5221901 39.246474 Pending N1600 2019-23 -76.5221901 39.246714 Pending N1600 N1600 2019-24 -76.522190 39.246701 Pending N1600 N16		_			-	+	+	 	+ + +	-			0		n 25	300									
N1600 2019-23 -76.522196 39.246776 Pending						++			\dagger	y	1				0.23	300			1						
N1600 2019-24 -76.52196 39.246776 Pending						++	_			^	1						67 15		1						1 1011 11111111111111111111111111111111
N1600 2019-25 7-6.521986 39.247013 Pending					_	+					1														
N1600 2019-26 -76.521923 39.246924 Pending N1600 2019-27 -76.521866 39.247162 Pending N1600 N1600 2019-28 -76.5220052 39.247045 Pending N1600 N160						++								1	0.75	180			1						x
N1600 2019-27 -76.521886 39.247162 Pending N1600 2019-28 -76.522013 39.247186 Pending N1600 2019-29 -76.522023 39.247386 Pending N1600										х							8 5								Remove stop light foundation
N1600 2019-28 -76.522013 39.247186 Pending		+									İ	Ì	6	2					1						Sign post
N1600 2019-29 -76.522052 39.247045 Pending		_			-										0.5	10									
N1600 2019-31 -76.522685 39.246678 Pending x 10	N1600	2019-29	-76.522052		Pending								10	5											
N1600 2019-32 -76.522795 39.246703 Pending Puncture in asphalt N1600 2019-33 -76.522642 39.247228 Pending Puncture in asphalt N1600 2019-34 -76.523075 39.246902 Pending Puncture in asphalt N1600 2019-35 -76.523075 39.246902 Pending N	N1600	2019-30	-76.522249	39.246876	Pending								85	12											
N1600 2019-33 -76.522642 39.247228 Pending Puncture in asphalt N1600 2019-34 -76.523075 39.246902 Pending Description	N1600	2019-31			Pending					х			10	10											Railroad switch
N1600 2019-34 -76.523075 39.246902 Pending	N1600				Pending						10	5											· · · · · · · · · · · · · · · · · · ·		Puncture in asphalt
N1600 2019-35 -76.522882 39.247232 Pending x N1600 2019-36 -76.522738 39.247314 Pending x N1600 2019-37 -76.522738 39.247314 Pending x N1600 2019-37 -76.522343 39.247301 Pending x	N1600	2019-33			Pending						4	2													Puncture in asphalt
N1600 2019-36 -76.522738 39.247314 Pending x x N1600 2019-37 -76.522343 39.247301 Pending x 0.5 100 x Asphalt cracking	N1600	2019-34	-76.523075	39.246902	Pending										0.5	50									
N1600 2019-37 -76.522343 39.247301 Pending 20 10 Asphalt cracking Asphalt cracking					Pending	\perp								ļ		1									х
		-				$\perp \perp \perp$					1	ļ			0.5	100			1						
N1600 2019-38 -76.522156 39.247309 Pending 30 10 Asphalt cracking		+			-	+						1		1					1					1	
	N1600	2019-38	-76.522156	39.247309	Pending						30	10			1	1			1						Asphalt cracking

Dundalk Marine	Terminal 2021	Surface Cover R	epair Dispositio	on																							
											1 - 3	3" M&P	2 - Open/L	Inpaved Area	3 - Surfac	ce Crack	4 - FS R	•	5 - Surfa	ce Heaving	Area	6 - Inlet/	7 - Monitor	8 - Gravel Cove	er Discoloration	tion	
	Issue	Location	Location	Status				Category	/		Length	Width	Length	Width	Avg. Width	Length	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	geta	
Area	Number	Longitude	Latitude		Р	CS D	P LP	GC MW	/ In Mi	H RR ((feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Veg	Comment
N1600	2019-39	-76.521688	39.247164	Pendin						\perp							50	50									
N1600	2019-40	-76.521799	39.247034	Pendin	g					\perp									8	30	30						
										\perp																	
1600	2019-1	-76.521931	39.245701	Pendin	~ -										0.25	60											Seal asphalt seam
1600	2019-2	-76.521783	39.245503	Pendin	~ -				+ +	+ +					0.5	70											Seal asphalt seam
1600	2019-5	-76.521428	39.245764	Pendin		Х				+					1	25											Crack in concrete slab
1600	2019-6	-76.522047	39.245958	Pendin	g				+ +	+					0.5	260											5/20/2021; Heave extends into reinforced concrete
1600	2019-7	-76.521602	39.245963	Pendin	g														16	120	20						under canopy. Dimensions updated from
																											10"x115'x20' to 16"x120'x20'
1600	2019-8	-76.521600	39.245775	Pendin	g														14	30	15						
1600	2019-9	-76.521561	39.246114	Pendin	g	х									0.75	560											Seal seam at concrete and asphalt joint around
1600	2019-10	-76.521871	39.246175	Pendin	<u> </u>				+ +	+ +					0.25	40											canopy
1600	2019-10	-76.522342	39.245947	Pendin						++					0.25	165											Asphalt seam
1600	2019-12	-76.522593	39.246126	Pendin	~ -					+ +					0.5	100											Asphalt seam for conduit
1600	2019-14	-76.522728	39.246203	Pendin	~ -					+ + ,	38	2			0.5	100											19 abandoned fence posts
1600	2019-13	-76.522587	39.246272	Pendin								<u> </u>					75	6									Repair asphalt around guardrail
1600	2019-15	-76.522394	39.246300	Pendin						1 1					0.25	195											Seal asphalt seams
1600	2019-16	-76.522293	39.246147	Pendin											0.25	200											
1600	2019-17	-76.521639	39.246319	Pendin	g														6	40	10						
1600	2019-18	-76.521977	39.246545	Pendin	g										0.25	115											Seal asphalt seam
1600	2019-19	-76.521098	39.246804	Pendin	g										0.25	40											
1600	2019-20	-76.520694	39.246442	Pendin	g										0.25	120											Seal conduit seam
1600	2019-21	-76.521057	39.246500	Pendin	g			х					43	25													Open area near toer
1600	2019-22	-76.521137	39.246131	Pendin	g										0.5	260										х	
1600	2019-23	-76.520797	39.246138	Pendin	g										0.25	50											
1600	2019-24	-76.520547	39.246244	Pendin	-														14	120	15						
1600	2019-25	-76.520602	39.246357	Pendin							8	6															
1600	2019-26	-76.520517	39.246369	Pendin	~ -					\perp									8	45	15						
1600	2019-27	-76.520583	39.246306	Pendin	~ -										0.5	245											
1600	2019-29	-76.520505	39.246174	Pendin					+ +	+ +					0.5	145	16	8									
1600	2019-28	-76.520477	39.246110	Pendin	g	х			+ +	+ +							16	8									Length changed from 140 feet to 86 feet on
1600	2019-30	-76.520261	39.246129	Pendin	g														10	140	20						5/20/2021. Partial heave removal during MPA
																											paving in summer 2020
1600	2019-31	-76.520270	39.246232	Pendin	g						2	2															Abandon fence post
1600	2019-32	-76.520047	39.245915	Pendin						\perp					0.5	175											
1600	2019-33	-76.520349	39.245830	Pendin											0.25	140											Seal parches and seam
1600	2019-34	-76.520858	39.245910	Pendin	~ -					\perp					0.25	130											Seal seam
1600	2019-35	-76.520511	39.245574	Pendin		_	Х			+ +					0.5	72			_							Х	
1600	2021-1	-76.521190	39.246337	Pendin					+ +	+ +					1				6	82	4						
1600	2021-2	-76.520035	39.246019	Pendin		_			+ +	+ +					0.25				14	128	15						
1600 1600	2021-3 2021-4	-76.520701 -76.520850	39.246031 39.245703	Pendin		+	+	 	+ +	+	+	-	-		0.25 0.5	55 90										1	+
1600	2021-4	-76.520850 -76.521132	39.245703	Pendin Pendin		+		 	+	++	+	1	1	1	0.5	47					1					1	+
1600	2021-6	-76.520858	39.245060	Pendin		+			+	++	+	1	1	+	0.75	25					<u> </u>					1	Crack a newly forming heave
1600	2021-7	-76.521690	39.246163	Pendin	~ -	\dashv			† †	++	1	1	1	1	1 3.23				6	45	3						, , , , , , , , , , , , , , , , , , , ,
1600	2021-8	-76.521793	39.246057	Pendin	-						1	1			0.5	75				-							1
						\neg							İ	İ	İ						Ì	İ					1
1601	2019-1	-76.518297	39.244828	Pendin	g					1	36	2															18 abandon fence posts
1601	2019-2	-76.518237	39.244788	Pendin	g						1		6	1													6 gaps in curb
1601	2019-3	-76.518279	39.244663	Pendin	g										0.5	125											
1601	2019-4	-76.518392	39.244645	Pendin	g														10	100	12						
1601	2019-5	-76.518395	39.244590	Pendin	-	$\perp \!\!\! \perp$			$\perp \perp$		3	3	<u> </u>	<u> </u>												<u> </u>	
1601	2019-6	-76.518335	39.244527	Pendin				$oxed{oxed}$	+	++		 	ļ	1					10	60	20					ļ	
1601	2019-7	-76.518689	39.244418	Pendin	-	\perp	\perp		+	\bot	1	 	-	<u> </u>	0.5	72					1					1	
1601	2019-8	-76.518628	39.244252	Comple		\perp		\vdash	+	+	16	7	<u> </u>	1							<u> </u>					<u> </u>	Pothole adjacent to existing patch.
1601	2019-9	-76.518836	39.244236	Comple					+	++	17	12	-		1											ļ	Weathered asphalt.
1601	2019-10	-76.519106	39.244001	Comple		+	-		++	++	42	4	1	1	2.5	200					1					1	Lateral and soom are th
1601	2019-11	-76.519198 -76.519330	39.244196	Pendin	~ -	+	-		+	++	30	15	-	-	0.5	200					1	1				1	Lateral and seam crack.
1601	2019-12 2019-13	-76.519229 -76.519262	39.243904 39.243943	Comple		+	+	 	+ +	+	20 32	15 4	-		+											1	+
1601 1601	2019-13	-76.519262 -76.519494	39.243943	Comple Pendin		+		 	+	++	32	4	1	1	0.5	100			+		1	1				1	+
1601	2019-14	-76.519494	39.244046	Comple		+		 	+ +	+ +	7	5	1	1	0.3	100			1		1	 		1		1	+
1001	2013 13	, 0.313370	33.273700	Comple	Λ						1 '	,	1	1	1		1		1		1	L	1	l	1	1	_1

Dundalk Marine	Terminal 2021	. Surface Cover R	epair Dispositio	on										1					•	. •	1			
	lanca I	1	l ti	Chahira	7		C-+				" M&P		npaved Area	3 - Surfac		4 - FS Repair		ace Heaving A				er Discoloration	atior	
Area	Issue Number	Location Longitude	Location Latitude	Status	D CS	DR I	Catego P GC M		H RR O	Length (feet)	Width (feet)	Length (feet)	Width (foot)	Avg. Width	Length (feet)	Length Width (feet) (feet)	Max. Height (inches)	Length (feet)	Width Manho (feet) ID	le Wellhead ID	Length (foot)	Width (foot)	egeti	Comment
Area 1601	2019-16	-76.519688	39.243816	Complete	+	DP L	P GC IV	VV III IV	n kk U	18	3	(leet)	(feet)	(inches)	(reet)	(leet) (leet)	(inches)	(reet)	(feet) ID	טו	(feet)	(feet)	>	Comment
1601	2019-10	-76.519563	39.244087	Pending	+ +			++		10	3			0.5	202									Lateral cracking and seams.
1601	2019-18	-76.518969	39.244442	Pending	х	1	x							0.5	64								х	
1601	2019-19	-76.518776	39.244571	Pending										0.5	87									
1601	2019-20	-76.518403	39.244900	Pending					х	4	2													Two abandon fence posts
1601	2019-21	-76.518735	39.244936	Pending										0.25	205									Blank increased from 147 feet to 205 on June 17,
1601	2019-22	-76.519064	39.244748	Pending				+++						0.5	188									2021 Crack along asphalt seams. Some resealing
1601	2019-23	-76.519186	39.244681	Pending	x					2	2			0.5	100									
1601	2019-24	-76.519120	39.244670	Pending	х					3	3													Pothole in puddle
1601	2019-25	-76.519036	39.244451	Pending													8	175	15					
1601	2019-26	-76.519220	39.244473	Pending	х									0.75	35									Crack resealing over asphalt covered dolly pad
1601	2019-27	-76.519619	39.244423	Pending	1	+++					1			0.5	138									Asphalt seam and lateral cracking
1601	2019-28	-76.519625	39.244328	Complete	x					18	3													Weathered asphalt adjacent to existing patches
1601	2019-29	-76.519794	39.243997	Complete						24	4													
1601	2019-30	-76.519760	39.244474	Pending										0.25	150									Seam and lateral cracks
1601	2019-31	-76.519595	39.244699	Pending													14	90	15					
1601	2019-32	-76.519504	39.244767	Pending										0.5	100									Seam and lateral cracks
1601	2019-33 2019-34	-76.519369	39.244837 39.245005	Pending		\vdash						-		0.75	170		14	93	10				Х	
1601 1601	2019-34	-76.519075 -76.520011	39.245633	Pending Pending	+ +	\vdash			++-					0.5	120		14	93	10					Seal seams
1601	2019-36	-76.519786	39.245659	Pending	1 1									0.5	60									Sea seams
1601	2019-37	-76.519412	39.245650	Pending										0.5	120									Repaired by MPA during August 2020 1st St. paving
1001	2019-37	-70.319412	39.243030	rending								-		0.5	120									H
1601	2019-38	-76.519390	39.245402	Pending													12	105	15					Heave length changed from 130 feet to 105 feet when feature inspected on June 17, 2021. A portion of the heave was removed during MPA paving of First Street in August 2020.
1601	2019-39	-76.519588	39.245458	Pending										0.5	290									
1601	2019-40	-76.519624	39.245576	Pending													10	50	15					
1601	2019-41	-76.519949	39.245178	Pending		-			X	34	2			0.5										Remove 17 abandoned fence posts
1601 1601	2019-42 2019-43	-76.520079 -76.519617	39.245332 39.245375	Pending	+ +				++		1	+		0.5 0.5	550 160								Х	
1601	2019-43	-76.519017	39.245336	Pending Pending				+++						0.5	100		12	28	15					+
1601	2019-45	-76.519555	39.245086	Pending	1 1												16	145	15					
1601	2019-46	-76.519919	39.244899	Pending										0.5	220								х	
1601	2019-47	-76.518919	39.245226	Pending										0.5	240									
1601	2019-48	-76.519987	39.244714	Pending										0.5	275								х	
1601	2019-49	-76.520009	39.244520	Pending													8	160	8					
1601	2019-50	-76.520340	39.244680	Pending										0.5	500		- 10						Х	
1601	2019-51	-76.520403	39.244553	Pending					-		25						12	90	10					Cracking apphalt
1601 1601	2019-53 2019-54	-76.520455 -76.520231	39.244485 39.244241	Pending Pending	1 1					55	25			0.5	155									Cracking asphalt
1601	2019-55	-76.520066	39.244199	Pending		١,	x		++					0.25	72									+
1601	2021-1	-76.519388	39.245232	Pending													6	53	5					
1601	2021-2	-76.518534	39.244960	Pending	х					17	10													Pitting asphalt
					\bot	\bot \bot																		
1700/1701	2019-1	-76.519131	39.246660	Pending	+	+	+	+	\bot		1			0.5	200									Lateral and seam cracks
1700/1701 1700/1701	2019-2 2019-3	-76.519225 -76.519228	39.246719 39.246604	Pending Pending	+		x	+	X	1	1	+	1				6	75	10		1	1		Repair 14' of curb around light
1700/1701	2019-3	-76.519228 -76.519363	39.246466	Pending	+ + -	++	++		+ +	1	1	+	1	0.5	250		Ü	13	10		1	1		Seal five asphalt seams
1700/1701	2019-5	-76.519689	39.246345	Pending	+ +		++	++	+ +		†	†	1	3.3	250		8	50	10			1		
1700/1701	2019-6	-76.519717	39.246222	Pending										0.5	260									
1700/1701	2019-7	-76.519852	39.246192	Pending										0.5	115									Crack at asphalt seam
1700/1701	2019-8	-76.519842	39.246095	Pending													6	65	15					Heave along roadway and shoulder. Shorten from 65 feet to 50 feet on 5/19/2021 because of paving of summer of 2020
1700/1701	2019-9	-76.519613	39.245934	Pending												64 24								Regrade around electric and stormdrain manholes
1700/1701	2019-10	-76.519417	39.245972	Pending	+	+	+	+	\bot		1			2 -	227		6	95	10					
1700/1701	2019-11	-76.519156 -76.519910	39.246260	Pending	+ +	++	+	+	+	1	1	1	-	0.5	300		0	00	15			-		+
1700/1701 1700/1701	2019-12 2019-13	-76.518810 -76.518590	39.246325 39.246142	Pending Pending	++-	++	+	++	++	+		+	1	0.25	200		8	80	15			1		Cracks along seams
1700/1701	2019-13	-76.518590 -76.518855	39.246142	Pending	++-	++	+	+	+	+	+	+		0.25	205						-			Crack along seam
1700/1701	2019-15	-76.519355	39.245799	Pending	x		x	+++	+ +		1	1		5.5	233									Install asphalt curb around light pole slab
· · · · · · · · · · · · · · · · · · ·										-						1						1		

Dundalk Marine	Terminal 2021	Surface Cover R	epair Dispositio	n										_											
					_					1 - 3	" M&P	2 - Open/L	Inpaved Area	3 - Surfac	ce Crack	4 - FS Repair	5 - Sur	face Heaving	Area	6 - Inlet/	7 - Monitor	8 - Gravel Cove	er Discoloration	tion	
	Issue	Location	Location	Status			Category			Length	Width	Length	Width	Avg. Width	Length	Length Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	geta	
Area	Number	Longitude	Latitude		P CS	DP LP	GC MV	V In MH	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Veg	Comment
1700/1701	2019-16	-76.518849	39.245660	Pending													8	92	10						
1700/1701	2019-17	-76.518568	39.245938	Pending													6	27	10						
1700/1701	2019-18	-76.518927	39.245432	Pending										0.5	150										Seal crack around asphalt patch
1700/1701	2019-19	-76.518833	39.245352	Pending													14	80	48						Heave in the roadway
1700/1701	2019-20	-76.518772	39.245406	Pending													6	53	10						
1700/1701	2019-21	-76.518311	39.245887	Pending										0.25	150										
1700/1701	2019-22	-76.518499	39.245608	Pending													8	60	10						
1700/1701	2019-23	-76.519702	39.246492	Pending										0.5	600										Seal seams
1700/1701	2019-24	-76.519326	39.246622	Pending													8	90	15						
1700/1701	2019-25	-76.519318	39.246816	Pending										0.5	500										Seam and lateral cracks
1700/1701	2019-26	-76.519872	39.246463	Pending										0.5	360										Crack at asphalt seam
1700/1701	2019-27	-76.519683	39.246782	Pending													8	165	15						
1700/1701	2019-28	-76.519618	39.247064	Pending													6	52	5						Heave at edge of concrete slab. Length changed
					+		+ +	+ + -	-																from 52 feet to 96 feet on 5/19/2021. Updated to 85 feet from 70 feet in length on
1700/1701	2019-29	-76.520009	39.246867	Pending													6	85	10						5/19/2021
1700/1701	2019-30	-76.520208	39.246779	Pending										0.5	400										3/13/2021
1700/1701	2010 21	-76.520575	20.246586	Donding										0.5	50										Marked as complete on May 19, 2021. Completed
1700/1701	2019-31	-70.520575	39.246586	Pending										0.5	50										during summer 2020 paving
1700/1701	2019-32	-76.520227	39.246997	Pending													8	70	50						Length changed from 70 feet to 90 feet on
				_	+ + +		+ +	+ +		1			1	0.5	475										5/19/2021
1700/1701	2019-33	-76.520028	39.247173	Pending	+++		+	-		-				0.5	175										
1700/1701	2019-34	-76.520285	39.247277	Pending	+ + +		+ +	+ +		1			1	0.5	150			70	10						
1700/1701	2019-35	-76.520514	39.246996	Pending	+ + +		+ +	+ +		1			1				6	78	10						Longth shanged from E0 to CE foot on May 10, 2021
1700/1701	2019-36	-76.520547	39.246800	Pending													10	65	15						Length changed from 50 to 65 feet on May 19, 2021
1700/1701	2019-37	-76.520513	39.246793	Pending	+ + +		+ +	+ +		1				0.25	47									х	Mill a d pave along seam
1700/1701	2019-38	-76.520938	39.246867	Complete										0.25	100										Seal conduit seam
1700/1701	2019-39	-76.520931	39.246932	Pending								10	10												Hydrant repair. Repzir curb
1700/1701	2019-40	-76.520639	39.247026	Pending										0.5	1100										+ ' ' '
1700/1701	2019-41	-76.520315	39.247424	Pending										0.5	500										
1700/1701	2019-42	-76.520261	39.247520	Pending						26	12														
1700/1701	2019-43	-76.517065	39.245303	Pending										0.25	180										
1700/1701	2019-44	-76.517344	39.245340	Pending													14	235	20						
1700/1701	2019-45	-76.517432	39.245240	Pending										0.25	195										Seal patches
1700/1701	2019-46	-76.517579	39.245304	Pending						33	6														
1700/1701	2019-47	-76.517568	39.245270	Pending						19	18														
1700/1701	2019-48	-76.517693	39.245233	Pending						24	13														Alligator cracking
1700/1701	2019-49	-76.517760	39.245195	Pending													10	30	15						
1700/1701	2019-50	-76.517570	39.245049	Pending	х					20	20														
1700/1701	2019-51	-76.517951	39.245059	Pending													8	82	10						
1700/1701	2019-52	-76.517915	39.244732	Pending													12	114	12						Completed during August 2020 paving by MPA
1700/1701	2019-53	-76.518110	39.244856	Pending										0.5	80										
1700/1701	2019-54	-76.517694	39.245570	Pending						4	2														
1700/1701	2019-55	-76.520702	39.247249	Pending										0.75	150										Seal seam
1700/1701	2019-56	-76.520698	39.247219	Pending	х					25	10														
1700/1701	2019-57	-76.521046	39.247080	Pending										0.5	185										
1700/1701	2019-58	-76.521014	39.246976	Pending	х					12	9														
1700/1701	2019-59	-76.521128	39.247077	Pending						43	3														Separated seam
1700/1701	2019-60	-76.520805	39.247418	Pending		х								0.25	90										
1700/1701	2019-61	-76.520634	39.247775	Pending										0.75	19									х	
1700/1701	2019-62	-76.521059	39.247398	Pending					х	2	2														Sign post
1700/1701	2019-63	-76.521178	39.247111	Pending										0.5	425										Seal curb
1700/1701	2019-64	-76.521178	39.247432	Pending													10	40	15						
1700/1701	2019-65	-76.520824	39.247728	Pending													6	42	10						
1700/1701	2019-66	-76.520898	39.247838	Pending												310 16									
1700/1701	2019-67	-76.521377	39.247349	Pending	\bot \bot \bot			х		10	8														Repave around inlet IN-812
1700/1701	2019-68	-76.521569	39.247296	Pending										0.5	140										
																						-			
1702	2019-1	-76.517164	39.242325	Pending										1	27										
1702	2019-2	-76.517073	39.242261	Pending								2	2									-		х	Holes is asphalt
1702	2019-3	-76.517108	39.242257	Pending						46	6														Wide crack
1702	2019-4	-76.516906	39.242300	Pending										0.25	120							-			Seal top of curb and gutter pan
1702	2019-5	-76.516704	39.242329	Pending								2	2												Hole in asphalt
1702	2019-6	-76.516433	39.242374	Pending								6	2									-			Hole in asphalt

Dundalk Marine	Terminal 2021	Surface Cover R	epair Dispositio	n																					
					-				L	1 - 3"	M&P	2 - Open/U	Inpaved Area	3 - Surfac	e Crack	4 - FS Repair	5 - Sur	face Heaving <i>i</i>	Area	6 - Inlet/	7 - Monitor	8 - Gravel Cove	er Discoloration	tion	
	Issue	Location	Location	Status			Category			Length	Width	Length	Width	Avg. Width	Length	Length Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	geta	
Area	Number	Longitude	Latitude		P CS	DP LP	GC MW	In MH I	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Veg	Comment
1702	2019-7	-76.515985	39.242512	Pending								8	2												Four holes in asphalt
1702	2019-8	-76.515838	39.242542	Pending						6	4														Puncture in asphalt
1702	2019-9	-76.515577	39.242603	Pending						26	5														Alligator cracking
1702	2019-10	-76.515532	39.242681	Pending										0.5	100									Х	
1702	2019-11	-76.515730	39.242729	Pending													8	100	15						
1702	2019-12	-76.515691	39.242683	Pending			х														TPZ-30B				Damaged well pad
1702	2019-13	-76.516235	39.242553	Pending					_ _					0.5	92										
1702	2019-14	-76.516323	39.242486	Pending			х						-								DMT-39S				Damaged well pad
1702	2019-15	-76.517229	39.242426	Pending			1	1 1 1		25				0.5	40										All:
1702	2019-16	-76.517122	39.242444	Pending			 	 		25	5			0.5	100										Alligator cracking along seam
1702	2019-17 2019-18	-76.517051 -76.516660	39.242458 39.242620	Pending			+	1 1 1						0.5	100									.,	+
1702 1702	2019-18	-76.516660	39.242620	Pending Pending	X		+	1 1 1						0.25 0.5	45 90									Х	+
1702	2019-19	-76.515808	39.242865	Pending	 			1 1						0.75	86									x	Crack between asphalt and building
1702	2019-20	-76.515877	39.242983	Pending			+			33	4			0.73	80									X	Gap between building and asphalt
1702	2019-22	-76.515656	39.243027	Pending						- 33	-			0.5	20									X	dup between building and aspirate
1702	2019-23	-76.515643	39.243070	Pending						4	2			0.3	20										Hole on asphalt
1702	2019-24	-76.515767	39.243095	Pending						•				 			8	111	10						
1702	2019-25	-76.515822	39.243131	Pending						40	10														Alligator cracking
1702	2019-26	-76.515890	39.243098	Pending													10	80	6						
1702	2019-27	-76.516072	39.243146	Pending						48	16														Alligator cracking
1702	2019-28	-76.516418	39.243058	Pending										0.5	367										Crack between asphalt and concrete slab
1702	2019-29	-76.517572	39.244875	Pending													12	60	17						Middle of the road.
1702	2019-30	-76.517263	39.244650	Pending										1	50										
1702	2019-31	-76.517372	39.244860	Pending										0.5	55										
1702	2019-32	-76.517324	39.244897	Pending										0.5	51										
1702	2019-33	-76.517175	39.244828	Pending										0.5	33										
1702	2019-34	-76.516990	39.244689	Pending										0.5	200										
1702	2019-35	-76.517014	39.244883	Pending										0.5	75										
1702	2019-36	-76.517265	39.245117	Pending										0.5	31										
1702	2019-37	-76.516998	39.245096	Pending					_ _								12	65	15						In between jersey barriers and roadway.
1702	2019-38	-76.517054	39.245065	Pending									-				14	90	10					Х	Between light pole and jersey barriers.
1702	2019-39	-76.516853	39.245027	Pending		Х								0.5	65									Х	LP-C
1702	2019-40 2019-41	-76.516798	39.245021	Pending				+++						0.5 0.5	60										In readurar
1702 1702	2019-41	-76.516773 -76.516640	39.245096 39.245008	Pending Pending	 			+++		20	5			0.5	111										In roadway. Roadway
1702	2019-42	-76.516673	39.244970	Pending	×			1 1		15	3			1											Spalling in concrete
1702	2019-43	-76.516766	39.244981	Pending	^		+			110	10														Spanning in concrete
1702	2019-45	-76.516670	39.244965	Pending						110	10			0.5	300										Parallel cracks in the concrete.
1702	2019-46	-76.516235	39.244849	Pending	x					50	3			0.3	300										In roadwwy. Spalling.
1702	2019-47	-76.516257	39.244731	Pending	х					15	3														Spalling just west of roadway.
1702	2019-48	-76.516239	39.244698	Pending										1	120										Asphalt patch previously sealed.
1702	2019-49	-76.516310	39.244644	Pending													10	21	13						
1702	2019-50	-76.516272	39.244636	Pending										0.25	42										Various cracks near inlet.
1702	2019-51	-76.516427	39.244666	Pending										0.5	120									х	Dolly pad up to jersey barriers.
1702	2019-52	-76.516439	39.244642	Pending						-				0.5	60							· · · · · · · · · · · · · · · · · · ·			Crack from dolly pad to jersey barrier.
1702	2019-53	-76.517147	39.244413	Pending	$oxed{oxed}$			$oxed{\Box}$	\Box	25	10														
1702	2019-54	-76.516750	39.244509	Pending			$\bot \bot$					ļ					6	130	5						
1702	2019-55	-76.517294	39.244216	Pending				$\sqcup \sqcup \sqcup$	\perp								8	78	30						
1702	2019-56	-76.517651	39.244082	Pending			+	\Box				1	1				10	30	15						Heave by lightpole.
1702	2019-57	-76.517694	39.244146	Pending			+	+	\dashv				1	1	100								1		In roadway.
1702	2019-58	-76.517675	39.244008	Pending	+++		+	+++	+				-	1	95			-							Various cracks in roadway.
1702	2019-59	-76.517599	39.244018	Pending										0.5	25										Cracking just east of jersey barriers by light pole 17-
1702	2019-61	-76.517406	39.243944	Pending	1 1 1		1	1 1 1	\dashv				1	1	100								<u> </u>		+
1702	2019-62	-76.517321	39.244109	Pending				 					†				8	50	10						+
1702	2019-63	-76.516745	39.244334	Pending		х	1		\dashv				1	0.25	80			1						х	Crack around 17-4
1702	2019-64	-76.516637	39.244387	Pending				1					1	0.5	90										
1702	2019-65	-76.515997	39.244571	Pending						50	2	Ì	1										İ		Damaged concrete in roadway.
1702	2019-66	-76.515972	39.244500	Pending										0.5	75										
1702	2019-67	-76.516258	39.244366	Pending										0.5	240										Various cracking east of dolly pad.
1702	2019-69	-76.516623	39.244242	Pending										0.5	60										
1702	2019-68	-76.516780	39.244164	Pending										0.5	45										
1702	2019-70	-76.517078	39.244022	Pending										0.5	150										Cracking on north side of heave.

Dundalk Marine	Terminal 2021	Surface Cover R	epair Dispositio	on																				
					_		Catalana			1 - 3	3" M&P	2 - Open/l	Jnpaved Area	3 - Surface (Crack	4 - FS Repair	5 - Su	face Heaving	Area	6 - Inlet/	7 - Monitor	8 - Gravel Cove	er Discoloration	tion
	Issue	Location	Location	Status			Catego			Length	Width	Length	Width	Avg. Width	Length	Length Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	getai
Area	Number	Longitude	Latitude		P CS	DP LF	GC M	W In M	H RR (O (feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	S Comment
1702	2019-72	-76.517321	39.244041	Pending										0.5	115									Cracks and asphalt patch seal missing.
1702	2019-74	-76.517403	39.243810	Pending		-								0.5	35									
1702	2019-75	-76.517289	39.243917	Pending						20	13			0.25	420									
1702	2019-78 2019-79	-76.516035 -76.515985	39.244152	Pending	+									0.25	120 170			-						
1702 1702	2019-79	-76.515985 -76.515774	39.244282 39.244294	Pending Pending		X	+ +							1	170		8	50	20					
1702	2019-80	-76.515743	39.244355	Pending	+++	+++								0.25	70		•	30	20					
1702	2019-82	-76.515821	39.244269	Pending										0.5	45									
1702	2019-83	-76.516174	39.244050	Pending					1 1					0.5	160									
1702	2019-84	-76.516600	39.243891	Pending										0.5	160									
1702	2019-85	-76.516894	39.243818	Pending										0.5	60									
1702	2019-87	-76.517021	39.243790	Pending										0.5	40									
1702	2019-89	-76.517616	39.243467	Pending										0.5	80									Cracking near intersection.
1702	2019-90	-76.516816	39.243566	Pending										0.5	30									
1702	2019-91	-76.516703	39.243412	Pending										0.5	85									
1702	2019-92	-76.516703	39.243471	Pending		-								0.5	85									Х
1702	2019-93	-76.516655	39.243272	Pending		-								0.5	135			25	10					
1702	2019-94 2019-95	-76.517149 -76.517289	39.243224	Pending	+	-											8	25 33	10					
1702			39.243222	Pending		+ +	+ +										ь	33	12					Asphalt pad around electric vault. Manhole ring is
1702	2019-96	-76.517246	39.242984	Pending				×						0.5	110					lectric vault				cracked and needs to be sealed.
1702	2019-97	-76.517271	39.242821	Pending						100	15													Area of road between the two patches.
1702	2019-98	-76.517157	39.242819	Pending								2	2											NW corner of Shed1702A
1702	2019-99	-76.516135	39.243139	Pending						50	15													
1702	2019-100	-76.516265	39.243264	Pending										0.25	210									x Multiple cracks within jersey barrier area.
1702	2019-101	-76.516401	39.243464	Pending										0.5	75									Various cracks including seam between asphalt patches.
1702	2019-102	-76.516278	39.243725	Pending					1 1					0.5	220									patches.
1702	2019-103	-76.516188	39.243418	Pending										0.5	75									
1702	2019-104	-76.515949	39.243430	Pending													10	50	8					Heave inside of jersey barrier.
1702	2019-105	-76.515873	39.243649	Pending										0.5	280									Both joints around dolly pad.
1702	2019-106	-76.515677	39.243725	Pending										0.5	280									
1702	2019-107	-76.515396	39.243844	Pending										0.5	88									Cracks at entrance to lot.
1702	2019-108	-76.515407	39.243941	Pending		х								0.5	66									LP-A
1702	2019-109	-76.515469	39.244135	Pending	х												10	85	15					Heave on both concrete and asphalt. Offshoot to the east at the seam of concrete and asphalt.
1702	2019-110	-76.515541	39.244134	Pending					1 1							25 0.5								the east at the seam of concrete and asphalt.
1702	2019-111	-76.515595	39.244122	Pending				х									8	10	10					Volcano type heaves three total. Around inlet.
1800	2019-1	-76.518648	39.247584	Pending													8	80	9					
1800	2019-2	-76.518757	39.247412	Pending	x x					15	2													Potholes at spalling concrete in slab
1800	2019-3	-76.518591	39.247627	Pending			×	(x	110	9										DMT-79M			Severe cracking in asphalt between railroad tracks.
				_		+ +																		Includes well Low point between railroad tracks with sediment
1800	2019-4	-76.518365	39.247401	Pending		х			x	23	15													and vegitation growth
1800	2019-5	-76.518569	39.247644	Pending								40	3											Wide crack under fence. Fill with asphalt and
				_	+	++	+	+	++			1	+ -	 			10	10	15				-	compact x Heave around abandon hydrant
1800 1800	2019-6 2019-7	-76.518289 -76.518769	39.247344 39.247200	Pending Pending	+	++	+ +	+	++	x			+	0.5	19		10	18	15					x Heave around abandon hydrant
1800	2019-7	-76.518769	39.247200	Pending	x		+ +	х	++			1	+	0.5	42		+							x Seal crack between inlet and concrete slabs
1800	2019-9	-76.518836	39.247319	Pending	X	+ +	+ +	1 ^	+			1	1	0.5	400								1	x Crack between concrete slab ans asphalt
				_	 			+ +	++								1							Crack around inlet and between concrete slab
1800	2019-10	-76.519009	39.247024	Pending	×			×	$\perp \perp$					0.5	163									sections
1800	2019-11	-76.518795	39.246881	Pending	+		+	\bot	+	4	2		1	<u> </u>									1	Two holes through asphalt
1800	2019-12	-76.518640	39.246865	Pending	+		+		++			1	<u> </u>	0.25	60									
1800	2019-13	-76.518464	39.246687	Pending	+		+	+	+			1	1	0.5	100		12	400	12				1	House at odge of operate date
1800	2019-14	-76.518318	39.246917	Pending	X	++	+	+	X					 			12	100	12				-	Heave at edge of concrete slab Replsce damaged asphalt over lightpole foundation
1800	2019-15	-76.518654	39.247091	Pending		x				16	14													x Repisce damaged aspirant over lightpole foundation
1800	2019-16	-76.518066	39.247247	Pending					х					0.5	106									Cracks between railroad tracks
1800	2019-17	-76.518194	39.247191	Pending	х												10	65	10					Heave pushing up edge of concrete slab
1800	2019-18	-76.518028	39.247144	Pending						75	16													Cracked asphalt. Ponding water
1800	2019-19	-76.517848	39.246938	Pending	x			x									12	70	15					Heave under concrete slab extending towards inlet
1800	2019-20	-76.518089	39.246750	Pending	×		+ +	++	++			1	+	+			24	115	15				1	Heave at edge of concrete slab
1800	2019-20	-76.517965	39.246851	Pending	++^		++	+	++	+	1	1	+	0.5	28		24	113	13					Crack around inlet
1800	2019-21	-76.518171	39.246737	Pending	+ + -		+ +		++			1	1	0.5	35								1	
						11					<u> </u>	1	1			l l			1				1	

Dundalk Marine	Terminal 2021	L Surface Cover R	Repair Dispositio	on																						
				6	_		6.				' M&P	+	Inpaved Area	3 - Surfac	_	4 - FS Re	•		face Heaving /		6 - Inlet/	7 - Monitor		er Discoloration	ation	
Araa	Issue Number	Location	Location Latitude	Status	D CS	DD II	Category GC MW		DD C	Length (foot)	Width (feet)	Length (foot)	Width (foot)	Avg. Width	Length (foot)	Length (foot)	Width (foot)	Max. Height (inches)	Length (foot)	Width (foot)	Manhole ID	Wellhead ID	Length (foot)	Width (foot)	egeti	Comment
Area 1800	2019-23	Longitude -76.518278	39.246650	Pending	P CS	DP LI	GC IVIV	V III IVIN	KK C	(feet)	(leet)	(feet)	(feet)	(inches) 0.25	(feet) 154	(feet)	(feet)	(inches)	(feet)	(feet)	טו	ID	(feet)	(feet)	>	Crack in concrete slab and along slab edge
				_	+ + ^			 						0.23	134											Ponding area adjacent to lightpole with sediment
1800	2019-24	-76.517660	39.246853	Pending						20	14														Х	and vegetation
1800	2019-25	-76.517947	39.246491	Pending														8	43	10						
1800	2019-26 2019-27	-76.518103	39.246382	Pending				х						0.5	30											
1800 1800	2019-27	-76.517489 -76.517452	39.246829 39.246813	Pending Pending				1				2	2	0.25	150										Х	
1800	2019-29	-76.517416	39.246713	Pending				1 1						0.25	103											
1800	2019-30	-76.517630	39.246056	Pending				х						0.25	33											Seal crack around inlet
1800	2019-31	-76.517469	39.246346	Pending					Х																	Monitoring point missing lid and frame
1800	2019-32	-76.517242	39.246513	Pending										0.5	50										Х	
1800	2019-33 2019-34	-76.517192	39.246604	Pending										0.25	162										Х	Crack between jersey barrier and asphalt
1800 1800	2019-34	-76.517183 -76.517361	39.246278 39.245858	Pending Pending										0.5 0.25	117 83											Seal asphalt seam crack again
1800	2019-36	-76.517137	39.246151	Pending			x	1 1						0.23	03											Cracked well frame
1800	2019-37	-76.516786	39.246315	Pending								39	9													Stone cover
1800	2019-38	-76.516948	39.245786	Pending										0.5	270											Crack between toe drain and asphalt
1800	2019-39	-76.516836	39.245470	Pending										0.5	45											
1800	2019-40	-76.516096 -76.516346	39.245685	Pending										0.5	128											
1800 1800	2019-41 2019-42	-76.516246 -76.516386	39.245597 39.245485	Pending Pending										0.25	197 17											
1800	2019-42	-76.516420	39.245451	Pending	х	 ^								0.5	50											Crack around manhole slab
1800	2019-44	-76.516451	39.245232	Pending										0.25	125											
1800	2019-45	-76.516064	39.245336	Pending										0.5	108											
1800	2019-46	-76.515732	39.245260	Pending										0.25	88											
1800	2019-47	-76.516052	39.244934	Pending										0.5	55											
1800	2019-48	-76.515911	39.244797	Pending	++-			1 1				1	1	0.25	75			10	15	15						
1800 1800	2019-49 2019-50	-76.515496 -76.515081	39.244661 39.244595	Pending Pending	+++			1 1				1	1	0.25	75 156				1							
1800	2019-51	-76.514935	39.244898	Pending						9	3			0.5	150											
1800	2019-52	-76.515932	39.245736	Pending						80	3															Cracked asphalt
1800	2019-53	-76.516158	39.245935	Pending												10	5									Holes through asphalt
1800	2019-54	-76.516418	39.246088	Pending												130	12									
1800	2019-55	-76.516729	39.246335	Pending					х							116	4									Separation and heaves between rails and bollards
1800	2019-56	-76.517110	39.246612	Pending												27	4									Separating asphalt under fence
1800	2019-57	-76.515833	39.245656	Pending								414	4													
1800	2019-58	-76.514372	39.244120	Pending								7	7												Х	
1800	2019-59	-76.514480	39.244056	Pending			х							0.25	10							TPZ-28			Х	Two well pads
1800 1800	2019-61 2019-60	-76.514557 -76.514691	39.244190 39.244109	Pending Pending										0.5	110 100			+								Seam and lateral cracking
1800	2019-62	-76.514955	39.244136	Pending				1 1						0.5	350											
1800	2019-63	-76.514948	39.243892	Pending										0.5	280											
1800	2019-64	-76.515052	39.243987	Pending						24	4															Remove six 4x4 concrete pads
1800	2019-65	-76.514643	39.244297	Pending										0.25	206											Seal around conduit cut
1800	2019-66 2019-67	-76.514524 76.514529	39.244387	Pending	+		+ +	+ +		6	2			0.5	350			1	1	-						Remove three abandoned fence posts
1800 1800	2019-67	-76.514828 -76.514919	39.244384 39.244331	Pending Pending	+ +	×	+ +	+ + -			1			0.5	350 66	+ +			+	<u> </u>						Asphalt seam
1800	2019-69	-76.514919	39.244331	Pending	+ + -	 ^		1 1			1			0.5	- 00	+ +		6	89	12						
1800	2019-70	-76.515259	39.244113	Pending	<u> </u>									0.5	95											Crack at asphalt seam
1800	2019-71	-76.515163	39.244479	Pending					х			24	25													Open area at railroad switch
1800	2019-72	-76.515301	39.244447	Pending										0.5	120											
1800	2019-73	-76.515276	39.244289	Pending				 		30	14															Cracked asphalt
1800	2019-74	-76.515414	39.244262	Pending	х											18	3									Wide crack in concrete slab. Saw cut, remove, replace
1800	2019-75	-76.515656	39.244547	Pending						78	3															
1800	2019-76	-76.515769	39.244521	Pending	х			$\bot \bot \Box$						0.5	54		-								-	
1800	2019-77	-76.515927	39.244779	Pending				+										12	40	18						Heave under fence
1800	2019-78 2019-79	-76.516052 -76.516233	39.244809 39.244917	Pending	+		+ +	+				+	+	0.5	76			8	20	10	1					Asphalt seam
1800 1800	2019-79	-76.516233 -76.516307	39.244917	Pending Pending	+ +		+ +	+ + -	х	+	1	20	5			+ +		8	20	10	-					Open at railroad switch
1800	2019-80	-76.516388	39.245075	Pending	+ +			1 1		8	2	20	1 -						1							Four abandoned fence posts
1800	2019-82	-76.516556	39.245058	Pending						28	5															·
1800	2019-83	-76.519261	39.247072	Pending					, x			62	2													31 holes in asphalt along yellow line between tracks
		10202							<u> </u>			J														

Dundalk Marine	Terminal 2021	Surface Cover R	epair Dispositio	on																						_	
		T.	,	r	_							'M&P		Inpaved Area		ace Crack	4 - FS Repa			ace Heaving A		6 - Inlet/	7 - Monitor		er Discoloration	ation	
	Issue	Location	Location	Status		. 1 1		egory	. 1		Length	Width	Length	Width	Avg. Width		_	/idth	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	geta	
Area	Number	Longitude	Latitude	- "		S DP	LP GC	MW	In MH	RR O		(feet)	(feet)	(feet)	(inches)	(feet)	(feet) (1	feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Ve	Comment
1800	2019-84	-76.520894	39.249135	Pending		+	_	1			43	3			0.25	20										Х	Wide crack near fence
1800	2019-85	-76.521049	39.249319	Pending		+ +									0.25	30	+										Crack near fence line. Dolly pad and small crack to the inside of fence line.
1800	2019-86	-76.520980	39.249167	Pending		х									0.5	100											bolly pad and small crack to the inside of ferice line.
1800	2019-87	-76.520846	39.249103	Pending															8	10	10						Heave by fence.
1800	2019-88	-76.520827	39.249029	Pending									10	33													To the NW of tracks. Under poly.
1800	2019-89	-76.520713	39.249133	Pending	:					х			12	6													Three separate open unpaved areas near train gate.
1800	2019-90	-76.520604	39.249069	Pending	,	+	-	1 1											12	35	10						Between RR and boundary fence.
1800	2019-91	-76.520767	39.248885	Pending				1 1							0.5	50					10						Cracks to the west of train tracks.
1800	2019-92	-76.520583	39.248921	Pending						х			6	10													Two open area where rail joints are located.
1800	2019-93	-76.520467	39.248848	Pending											0.5	110											Multiple tracks in between RR tracks.
1800	2019-94	-76.520463	39.248951	Pending						х									10	15	15						Under property fence.
1800	2019-95	-76.520583	39.248774	Pending															10	40	3						
1800	2019-96	-76.520593	39.248838	Pending											0.5	40											
1800	2019-97	-76.520492 76.520496	39.248795	Pending		+ +							15	9	0.5	150											Seam and cracks in between RR. Three congrete uppayed areas
1800 1800	2019-98 2019-99	-76.520486 -76.520445	39.248623 39.248488	Pending Pending		+	-	+ +	-	X			15 20	8													Three separate unpaved areas. RR switch
1800	2019-100	-76.520321	39.248608	Pending						^			20		0.5	102											Recessed areain between RRs.
				_		+ +		1 1																			Concrete slab for electrical utility near fence. And
1800	2019-101	-76.520062	39.248646	Pending	×										0.5	120											associated cracks.
1800	2019-102	-76.520084	39.248535	Pending											1	445										х	Concrete equipment pad.
1800	2019-103	-76.520028	39.248557	Pending									-		0.5	40											Lateral cracking between tracks.
1800	2019-104	-76.519842	39.248437	Pending		+ +	х						3	2	1	45											By lightpole base. Area around bollards previously sealed.
1800 1800	2019-105 2019-106	-76.519685 -76.520272	39.248474 39.248285	Pending Pending											0.5	330											Portion north of light pole.
1800	2019-107	-76.520214	39.248378	Pending		+ +									0.5	330			10	25	15						Partial cocrete slaband asphalt by tracks.
1800	2019-108	-76.520367	39.248309	Pending						х			15	25							13						Six separate sections.
1800	2019-109	-76.520432	39.248127	Pending									13	6													Under poly.
1800	2019-110	-76.520507	39.248063	Pending						х					0.5	20											Curb area by guardrail.
1800	2019-111	-76.519988	39.248128	Pending	. x						200	2															Large cracking in concrete by entrance of MES
1800	2019-112	-76.519903	39.248193	Pending	· ×						170	2															compound. Spalling in multiple sections. Concrete cracking inside MES compound.
1800	2019-113	-76.519867	39.248244	Pending		+++					2.0	_			1	240											Concrete cracking and spalling.
1800	2019-114	-76.519861	39.248278	Pending							70	3															Concrete cracking and spalling.
1800	2019-115	-76.519777	39.248310	Pending	x						40	3															Chipping by MES storage sheds.
1800	2019-116	-76.519782	39.248324	Pending	x										0.5	122											Seam between asphalt and concrete by MES
1800	2019-117	-76.519531	39.248297	Pending											0.5	52										х	Cracking in between tracks.
1800	2019-118	-76.519388	39.248233	Pending		+ +									1	60											Crack along fence line. Previously sealed.
1800 1800	2019-119	-76.519261 -76.518952	39.248113 39.247897	Pending Pending		+++		+							1	35 60										x	Reseal.
1800	2019-120	-76.518952 -76.518650	39.247596	Pending											1	60			6	20	8					Х	Heave by the fence.
1800	2019-122	-76.518715	39.247554	Pending															10	50	15						Just inside fence of MES compound.
1800	2019-123	-76.518774	39.247535	Pending							20	3															<u> </u>
1800	2019-124	-76.518805	39.247468	Pending	X										1	55											By fence inside MES compound.
1800	2019-125	-76.518997	39.247407	Pending	. x						130	1															Inside fence of MES compound. Concrete spalling.
1800	2019-126	-76.519260	39.247707	Pending		++		++			+	1	1	1	1	33	+			1					1		+
				_		+		1 1			+		†		<u> </u>												Seam between concrete and asphalt inside MES
1800	2019-127	-76.519370	39.247672	Pending		$\perp \perp$					1			1	1	375										х	compound. Runs entire length of fence.
1800	2019-128	-76.520368	39.247950	Pending		\perp		\sqcup			1		1	1		1			6	25	10						
1800	2019-129	-76.520220	39.247749	Pending															8	26	20						
1800	2019-130 2019-131	-76.520630 -76.520583	39.247778 39.247881	Pending		+ +							2	2	0.5	50										Х	Reseal crack
1800 1800	2019-131	-76.520583 -76.520147	39.247881	Pending Pending									2	2					14	78	10						Open area near guarsrail post
1800	2019-132	-76.520147	39.247432	Pending		+		+ +			55	3	†			1			14	,,,	10						Wide crack at seam with concrete slab
1800	2019-134	-76.519979	39.247681	Pending		1 1		\dagger			47	2		1		1											
1800	2019-135	-76.519954	39.247779	Pending		1 1		1 1			1		1		0.25	52				1							
1800	2019-137	-76.520125	39.248115	Pending	х										0.5	430										х	
1800	2019-136	-76.519504	39.247640	Pending				$oxed{\Box}$							0.5	26											
1800	2019-138	-76.519524	39.247560	Pending		++	_	1-1	_		10	2		1		1				ļ							
1800	2019-139	-76.519633	39.247488	Pending		+		++					10	+	0.25	30									-	х	Seal seam between asphalt and CS around inlet
1800	2019-140	-76.519742 -76.519750	39.247423	Pending		++	-	++	-	х	+	-	18	5	1	10				-					-		Open area at railroad switch
1800 1800	2019-141 2019-142	-76.519750 -76.519623	39.247297 39.247394	Pending Pending				++			+	1	1	1	0.5	18 105	+			1					1		+
1800	2019-142	-76.519527	39.247468	Pending				+			+	1	1	1	0.5	80									1	х	+
				8	<u> </u>					<u> </u>		1					1 1		1	l	1				1		

					_					1 - 3"	M&P	2 - Open/U	npaved Area	3 - Surfa	ce Crack	4 - FS	Repair	5 - Surfa	ce Heaving A	Area	6 - Inlet/	7 - Monitor	8 - Gravel Cove	r Discoloration	ion	
	Issue	Location	Location	Status			Category			Length	Width	Length	Width	Avg. Width	Length	Length	Width	Max. Height	Length	Width	Manhole	Wellhead	Length	Width	etat	
Area	Number	Longitude	Latitude		P CS	DP LP	GC MW	In MH	RR O	(feet)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	(feet)	(inches)	(feet)	(feet)	ID	ID	(feet)	(feet)	Veg	Comment
1800	2019-144	-76.519444	39.247412	Pending	хх					15	2															
1800	2019-145	-76.519098	39.247415	Pending					х									6	59	10						
1800	2019-146	-76.519096	39.247246	Pending	х				х									14	75	13		·		•		Under fence
1800	2019-147	-76.519095	39.247415	Pending										1	75											

Appendix C Sentinel Groundwater Monitoring Plan

Performance Management Program Sentinel Groundwater Monitoring Plan

Dundalk Marine Terminal Baltimore, Maryland

Prepared for



115 Tabor Road Morris Plains, NJ 07950

and



Maryland Port Administration 401 East Pratt St. Baltimore, MD 21202

Prepared by



Jacobs Engineering 2411 Dulles Corner Park Suite #500 Herndon, VA 20171

April 2023

Contents

Acrony	ms and	d Abbreviations	V
1.	Introdu	uction	1-1
	1.1	Background	1-1
	1.2	Plan Objectives	1-1
2.	Site Ba	ackground	2-1
	2.1	Site Geology—COPR Fill Area	2-1
	2.2	Site Geology—Area 1501/1602	2-1
3.	Conce	ptual Site Model	3-1
	3.1	Hydrogeologic Framework	
	3.2	Chromium Fate and Transport	3-3
	3.3	Groundwater–Surface Water Pathway	3-3
	3.4	Potential Human and Ecological Exposures	
		3.4.1 Human Health Risk Assessment (CH2M 2009c)	3-4
		3.4.2 Ecological Risk Assessment (CH2M 2009d)	
	3.5	Basis for Sentinel Monitoring Program	3-5
4.	Monito	ring Well Network	4-1
	4.1	Upper Saturated Zone	4-1
	4.2	Shallow Fill Unit	
		4.2.1 COPR Fill Area	
		4.2.2 Area 1501/1602	
	4.3	Upper Sand Unit	1-1 1-1 2-1 3-3 3-3 3-4 4-1 4-1 4-2 2-3 5-1 5-1 5-1 5-2 5-3 5-3 6-1 6-1
	4.4	Patapsco Aquifer	
	4.5	Patuxent Aquifer	4-3
5.		oring and Assessment	
	5.1	Groundwater Elevation Monitoring	
		5.1.1 Upper Saturated Zone	
		5.1.2 Shallow Fill—COPR Fill Area	
		5.1.3 Shallow Fill—Area 1501/1602	
		5.1.4 Upper Sand Unit	
		5.1.5 Patapsco Aquifer	
	5.2	5.1.6 Patuxent Aquifer	
	5.2	5.2.1 Shallow Fill and Upper Saturated Zone	
		5.2.2 Upper Sand Unit	
		5.2.3 Patapsco Aquifer	
		5.2.4 Patuxent Aquifer	
	5.3	Data Evaluation	
		5.3.1 Well Integrity	
		5.3.2 Groundwater Elevations	
		5.3.3 Chromium Concentrations in Groundwater	5-5
		5.3.4 Monitoring Plan Modifications	5-8
	5.4	Sediment and Porewater Sampling	5-8
6.	Report	ting	6-1
7.	Refere	nces	7-1



Appendixes

- A Sentinel Well Location Plan for Area 1501/1602
- B Sediment and Pore Water Monitoring Work Plan
- C Groundwater Action Levels

Tables

- 1 Chromium Transport Study, Tidal Study Results
- 2 Sentinel Well Network/Well Gauging
- 3 Sentinel Well Network/Well Sampling
- 4 Sentinel Well Network/Well Abandonment

Figures

- 1 Monitoring Well Location Map (Current)
- 2 Geologic Cross Section
- 3 Typical Well Configurations
- 4 Shallow Fill Monitoring Wells in Network
- 5 Proposed Wells for Abandonment
- 6 Upper Sand Monitoring Wells in Network
- 7 Patapsco Aquifer Monitoring Wells in Network
- 8 Patuxent Aquifer Monitoring Wells in Network
- 9 Groundwater Migration Pathways

Acronyms and Abbreviations

μg/L microgram(s) per liter

cm/s centimeter(s) per second

CMAA Corrective Measures Alternatives Analysis

COPR chromium ore processing residue

Cr (VI) hexavalent chromium
CSM conceptual site model

DMT Dundalk Marine Terminal

ERA Ecological Risk Assessment

HHRA Human Health Risk Assessment

IGSP Interim Groundwater Sampling Plan

MDE Maryland Department of the Environment

mg/L milligram(s) per liter

MPA Maryland Port Administration

PMP performance management program

1. Introduction

1.1 Background

This Sentinel Groundwater Monitoring Plan (Sentinel Plan) is an integrated component of the overall Performance Management Program (PMP) for the long-term monitoring and maintenance of the remedy being implemented at the Dundalk Marine Terminal (DMT).

Pursuant to the Consent Decree among the Maryland Department of the Environment (MDE), the Maryland Port Administration (MPA), and Honeywell International Inc. (Honeywell), as well as the *Corrective Measures Alternatives Analysis* (CMAA) (CH2M 2011) required by the Consent Decree, MDE selected Alternative 3, Enhanced Isolation and Containment Remedy, for the site. The Enhanced Isolation and Containment Remedy includes a requirement to establish a sentinel groundwater monitoring network inland of the shoreline perimeter to monitor groundwater flow and quality at the property boundary. The approach for establishing the sentinel monitoring system, based on an evaluation of the existing network of monitoring wells is described herein. This Sentinel Plan fulfills the PMP requirements of the Enhanced Isolation and Containment Remedy.

The Sentinel Plan will be the final groundwater monitoring plan, including components of and site understanding gained from monitoring results utilizing the *Interim Groundwater Sampling Plan* (IGSP) (CH2M 2009a). The IGSP was submitted to MDE on April 20, 2009, and revised in June 2009, and the *Conceptual Groundwater Monitoring Plan* was submitted in accordance with requirements set forth in a July 30, 2012, letter from MDE to Honeywell (MDE 2012). Honeywell and MPA have performed semiannual groundwater monitoring at the site since 2009 in accordance with the IGSP. Data from the semiannual groundwater monitoring events have been evaluated to assess changes in the groundwater flow regime and to validate the original 2009 groundwater model. The information from these events was incorporated into this Sentinel Plan, with consideration of potential changes in groundwater flow and analytical trends.

The Sentinel Plan is intended to:

- Define the objectives of the groundwater monitoring program
- Document the conceptual site model (CSM) for groundwater flow beneath DMT
- Describe how groundwater monitoring will be conducted
- Provide procedures to evaluate the data and to modify the monitoring program based on the data

1.2 Plan Objectives

The plan objectives are to collect and evaluate site-related data to confirm that the remedial components remain protective of human health and the environment. Conceptually, the data showing that this objective is being met would be collected from monitoring wells that are in water-bearing units horizontally contiguous to and downgradient of the chromium ore processing residue (COPR) fill area (**Figure 1**) or from wells that are screened below the COPR fill in the anticipated downgradient flow direction for the vertically lower aquifer unit in question.

2. Site Background

2.1 Site Geology—COPR Fill Area

The southern portion of DMT, the "COPR fill area," was constructed on lands reclaimed from prior marshlands and the Patapsco River by placement of COPR and non-COPR fill materials. The extent of COPR at DMT is defined because of data collected from over 400 investigation locations and a review of historical documents, aerial photography, and drawings detailing the facility's construction (CH2M 2009b) and is shown in Figure 1. There are approximately 2.5 million cubic yards of COPR located within approximately 148 acres of the COPR fill area. Vertically, COPR extends to a maximum depth of approximately 38.5 feet and ranges in thickness from 1 foot to 32 feet. The thickness of the non-COPR fill that overlies the COPR materials typically ranges between 2 and 22 feet. The southern and western edges of the DMT fill area end at a sheet pile wall with a pile-supported concrete platform, referred to as the "marine platform." To the southeast, the DMT fill area terminates at a riprap embankment, sloping from the terminal area to the Patapsco River along Area 1501/1602, which is discussed in Section 2.2. Farther to the east, the southern boundary abuts the adjacent neighborhood of Carnegie Plat and the adjacent Former Mestek property (owned by MPA). The northern boundary aligns with a former bulkhead along East Service Road and extends farther north to Dunmar Building 97. The eastern boundary aligns with the property boundary along Broening Highway (**Figure 1**).

Relatively thick deposits of alluvial sediments underlie the DMT fill area. The alluvial sediments are composed of three distinct soil lithologies: upper silt, alluvial sand, and lower silt. The alluvial sediments are believed to represent Quaternary low-land sediments that were deposited in an estuarine environment within the Patapsco River basin. The alluvial sediments were deposited within an erosional channel that was carved into the underlying Potomac Group sediments by the ancient Patapsco River. Beneath DMT, the surface of the erosional channel corresponds to the base of the alluvial sediments. The erosional channel occurs within the western two-thirds of the fill area and was filled with fine to very fine-grained sediments (primarily silts) rich in organic materials, which indicates that these sediments are of low permeability and are highly reducing.

The Cretaceous-age Potomac Group sediments underlie the alluvial sediments. In the Baltimore City area, the Potomac Group comprises unconsolidated clay, silt, sand, and gravel beds of the Patapsco, Arundel, and Patuxent Formations (Chapelle 1985; Bennett and Meyer 1952). On a regional scale, the stratigraphic formations within the Potomac Group are distinguishable based on lithologic, fossil, and geophysical evidence, and regional data suggest that the Patapsco, Arundel, and Patuxent Formations are continuous beneath DMT (Chapelle 1985; Bennett and Meyer 1952; Hansen 1968). Investigation results from DMT suggest that the stratigraphic contacts between the formations cannot be easily distinguished based on lithologic characteristics alone (CH2M 2009b). Therefore, the soil units are referred to only as the Undifferentiated Potomac Group (Potomac Group) sediments. The aquifer within the upper portion of the Potomac Group sediments at DMT is designated the Patapsco Aquifer. Clay horizons tend to thicken below a depth interval of approximately 140 to 160 feet below grade. Permeable units encountered beneath these clay horizons are designated the Patuxent Aquifer.

A geologic cross section across the DMT site is shown as **Figure 2**. The cross section extends vertically down to the Potomac Group and shows the fill area from Broening Highway to the western bulkhead.

2.2 Site Geology—Area 1501/1602

Area 1501/1602 encompasses approximately 21 acres in the southeast corner of DMT. The western and southern boundaries of Area 1501/1602 abut the Patapsco River. Much of this area was reclaimed from the Patapsco River channel in two phases of containment cell construction, for which COPR was used as fill, that occurred from about 1974 through 1982. First, select borrow material consisting of a mixture of sand, silt, and clay was placed over the native silt and sand at the river's edge to an elevation of about 2 feet above the mean low-water mark and then was capped with a 1- to 2-foot layer of impervious borrow material to create a clay liner. Above the clay liner, a clay containment dike was placed and graded to a 2:1 slope at the shoreline edges of Area 1501/1602, and COPR material was placed within the dike limits. Following placement in the containment cell, the COPR material was covered with a 1-foot-thick



impervious layer (clay cap). The clay cap extends over the clay containment dike, hydraulically isolating the COPR material along the current shoreline.

Containment measures were included in the design and construction of the cell to mitigate stormwater infiltration and to completely contain the COPR on the southern and western banks. The northern and eastern sides of the cell were confirmed to be mostly contained with a gap between the clay liner and the clay cap on the northeast side of the cell. An asphalt cap was placed over the clay cap to minimize stormwater intrusion into the cell, thereby reducing the potential for the transport of dissolved COPR constituents. The clay liner is intact, although portions of the clay cap are absent from the top of the cell in various, isolated locations.

Additional improvements occurred across Area 1501/1602 in 1999–2002. During this final phase, the grade of Area 1501/1602 was raised by 7 feet through the placement of surcharge material; this was intended to suppress COPR heave that had previously rendered the area unusable for port cargo storage operations. The surcharge fill was then covered with a 1-foot-thick layer of graded aggregate and 3 inches of surface course asphalt. Along the sloped edge of the western and southern shorelines, gabion baskets filled with coarse stone were placed. Additionally, in 2016, riprap and jersey barriers were placed along portions of both the western and southern shorelines to reduce the potential for shoreline erosion.

3. Conceptual Site Model

3.1 Hydrogeologic Framework

The hydrogeologic regime of DMT is one characterized by a series of interbedded water-bearing units that transmit flow and confining units, or aquitards, that retard groundwater flow. These characteristics have bearing upon the design of a comprehensive long-term monitoring program. In interbedded aquifer/aquitard systems, the laws of groundwater hydraulics dictate that groundwater flow paths in the higher-permeability strata will tend to be parallel to the bedding (horizontal in flat-lying strata) and nearly vertical in the intervening aquitards. At hydraulic conductivity contrasts of 100 or greater, which are more similar to the flow regime beneath DMT, flow paths in confining units become essentially vertical and flow paths in aquifers become essentially horizontal.

A schematic representation of the conceptual hydrogeologic framework is shown on **Figure 3**. Also shown are generalized groundwater flow lines illustrating the likely patterns of groundwater flow within this interbedded aquifer/aquitard hydrogeologic regime. As depicted on **Figure 3**, flow paths in more permeable units are generally horizontal, paralleling the stratigraphy. These more permeable units include the COPR and non-COPR fill and the Upper Saturated Zone, the Shallow Fill Unit, the Upper Sand, the Patapsco Aquifer, and the Patuxent Aquifer. In contrast, flow paths in the aquitard units are nearly vertical, or perpendicular to the stratigraphy. These units include the Upper Confining Unit, the river sediments, the Lower Confining Unit, and the Arundel Clay.

Several characteristics of DMT's hydrostratigraphy strongly influence potential contaminant transport and warrant consideration for the design of groundwater monitoring systems:

- The hydrogeology underlying the site is composed of multiple layers of more permeable water-bearing zones separated by low-permeability units that limit vertical groundwater flow between the water-bearing zones. Groundwater monitoring should be focused on the higher-permeability strata, given the propensity for lateral groundwater flow to be restricted to these aquifer units. It is typically only through these higher-conductivity strata that groundwater and any groundwater-borne contaminants can migrate laterally from beneath DMT and downgradient to receptors.
- In contrast, groundwater in the low-permeability confining units migrates typically vertically downward until higher-permeability strata are encountered. Once a higher-conductivity stratum is encountered, groundwater flow and any groundwater-borne contamination are then conveyed laterally.
- The vertical flow of groundwater though the confining units depends upon the units' vertical hydraulic conductivity and head differential between the overlying and underlying water-bearing zones. Correspondingly, the mass flow rate (mass per unit of time) of any groundwater-borne contaminants through the confining units is also low. As a result, even if the vertical migration of contaminant mass through the confining unit reaches the underlying, more permeable aquifer, concentrations often decline by one or more orders of magnitude due to mixing with the considerably higher lateral groundwater flow occurring within the aquifer.
- Transport of groundwater through confining units is slow, depending upon their hydraulic conductivities and ambient vertical hydraulic gradients.
- The combined impact of groundwater's relatively higher lateral flow in the aquifer units and the typically long transport time through the confining units dictates that in most circumstances, any groundwater contamination entering an aquifer unit beneath a waste disposal site will migrate laterally rapidly enough to be detected in the downgradient groundwater monitoring system long before it can simultaneously migrate appreciably downward through the underlying confining unit toward the next aquifer in the hydrogeologic sequence.

The implications of groundwater flow and contaminant transport in layered systems lead, in turn, to the following specific conclusions for designing a long-term monitoring program for the DMT hydrogeologic regime (additional details regarding each of the water-bearing zones and aquifers to be monitored are provided in Section 4):



- Groundwater monitoring should focus on the laterally continuous upper water-bearing units in the
 geologic sequence adjacent to and beneath the COPR fill area, the Shallow Fill Unit. As the Upper
 Saturated Zone is water bearing and is stratigraphically above the COPR, this zone will only be
 monitored for groundwater elevation.
- The second water-bearing zone, separated from the COPR fill by the Upper Confining Unit, is the
 Upper Sand, which also warrants monitoring. Groundwater could flow vertically downward from the
 COPR deposit through the confining unit to the Upper Sand, the second water-bearing zone, and
 then move laterally out from beneath the site. The Upper Sand should be monitored as close as
 practicable to the edge of the COPR boundary.
- Each successively deeper aquifer unit (below the Shallow Fill Unit and the Upper Sand) warrants progressively less or no monitoring attention since Cr(VI) migration and detection in the monitoring network should occur in the surficial or second water-bearing zones long before contaminants could migrate through the intervening confining unit(s) to deeper aquifers. This is especially true as the hydraulic conductivity contrast between the aquifer and confining units becomes greater. While it is prudent to have some level of monitoring in the underlying Patapsco Aquifer, the likelihood of Cr(VI) reaching that aquifer through natural migration is remote.



Given the unlikelihood of Cr(VI) ever reaching the Patapsco Aquifer and the fact that the Patapsco
Aquifer is separated from the underlying Patuxent Aquifer by yet another substantial aquitard, the
Arundel Clay, only limited monitoring of the Patuxent Aquifer is warranted. The three existing wells in
the Patuxent Aquifer at the DMT will remain to monitor potentiometric levels in that aquifer. In
addition, groundwater samples from these wells will also be collected to confirm the well integrity.

Confining units need not to be monitored as part of the long-term monitoring program since any significant lateral groundwater flow and potential Cr(VI) migration will be confined to the aquifer units. Aquitard units are also difficult to sample due to their low hydraulic conductivity and yield.

3.2 Chromium Fate and Transport

To quantify the fate and transport of chromium from the COPR fill area, Honeywell and MPA completed a Chromium Transport Study (CH2M 2009b). This study evaluated the quantities and valence states of chromium potentially being transported via storm drain flow, groundwater, air and tidal exchange with groundwater and storm drain flow in the storm drain system to the Patapsco River and Colgate Creek. Analytical results from the COPR investigation (CH2M 2009a) demonstrated that there is limited horizontal and vertical transport of Cr(VI) from the COPR fill area; specifically, that Cr(VI) concentrations in the alluvial soils immediately underlying the COPR fill are typically decreased by two to three orders of magnitude within a few feet of the COPR mass. The observed trends can be explained by the fact that the soil units and, more importantly, the groundwater underlying DMT offer a reductive environment for the reduction of Cr(VI) to Cr(III), typically Cr(OH)₃, which is insoluble. Further, the organic-rich sediments that underlie the COPR fill area act as a natural barrier to the migration of Cr(VI) due to their reducing environment. The following findings of this study constituted a fundamental basis for evaluating and developing remedial alternatives in the CMAA, and provided a basis for designing the sentinel groundwater monitoring program:

- Except for groundwater leakage into the storm drain system, direct groundwater transport of Cr(VI)
 does not constitute a significant transport pathway to the river or deeper groundwater systems
- Reductive mechanisms and physical barriers to groundwater movement limit the offsite transport of Cr(VI) through groundwater
- Impacts to groundwater have not been observed outside a horizontal distance of approximately 100 to 200 feet from the COPR limits
- Vertically, Cr(VI) impacts are not identified outside of the shallow water-bearing zones

3.3 Groundwater-Surface Water Pathway

To further evaluate and confirm the fate and transport of chromium from the COPR fill, a sediment and surface water study was conducted in the Patapsco River immediately adjacent to the site. The results of this study were presented in the *Sediment and Surface Water Study Report* (CH2M and ENVIRON, 2009). The findings of the surface water and sediment study supported the remedy selection in the CMAA and provide a basis for designing the sentinel groundwater monitoring program and porewater/surface water monitoring program, as follows:

- Cr(VI) was not detected in sediment pore water in any of the samples collected in any of the four quarterly sampling events.
- Cr(VI) was not detected in 97 percent of the surface water samples analyzed, and in those limited locations where it was detected, concentrations were well below the U.S. Environmental Protection Agency's water quality criteria.
- Measurements of geochemical parameters in pore water, surface water, and sediment demonstrate
 that conditions are favorable for the presence of chromium in the nontoxic trivalent chromium species
 (Cr(III)) rather than Cr(VI).



- Upwelling from shallow groundwater to surface water occurs primarily in the area southeast of COPR cell (1501 and 1602).
- Migration of Cr(VI) in groundwater is attenuated prior to upwelling into surface water.

Based on the results of this study and other related studies with respect to chromium geochemistry, total chromium in sediment is unlikely to oxidize to Cr(VI) in the future because the geochemical conditions necessary for this process do not naturally occur in the estuarine environment.

3.4 Potential Human and Ecological Exposures

Human health and ecological risk assessments were conducted by Honeywell and MPA to support the development of the CMAA and remedy selection. These risk assessments included the evaluation of potentially complete pathways for human and ecological exposures to COPR-related constituents of potential concern.

3.4.1 Human Health Risk Assessment (CH2M 2009c)

The shallow and deep groundwater units under the site have been investigated. Barriers (clay and organic silt units) are present that impede vertical migration of constituents of concern in the shallow groundwater unit to the deeper underlying potable groundwater unit (the Patuxent Aquifer). Therefore, the shallow groundwater unit was assessed further in the Human Health Risk Assessment (HHRA).

Site shallow groundwater is not a potable supply. In the State of Maryland, wherever the local jurisdiction provides municipal water, private potable wells cannot be installed. Potable water is supplied to DMT by the City of Baltimore, and the local regulations prohibiting the installation of private wells are reasonably expected to remain in place. Therefore, the potable water exposure pathway is considered incomplete for current and future site conditions. For the HHRA, potentially complete exposure pathways to site groundwater were assessed for incidental contact for onsite receptors (DMT workers and visitors, utility workers, and construction workers) and offsite receptors (residents near the adjacent cove, recreational users in the cove, and anglers in the Patapsco River and Colgate Creek).

The HHRA results indicated no unacceptable risks for onsite receptors (DMT workers, visitors, construction workers, and utility workers) or recreational users exposed to surface water and sediment in the cove adjacent to the site.

3.4.2 Ecological Risk Assessment (CH2M 2009d)

Areas of groundwater upwelling were identified in the near shore environment near Area 1501/1602 in the Trident probe groundwater upwelling survey; however, analytical results for groundwater samples from riverfront perimeter monitoring wells in Area 1501/1602 indicate that Cr(VI) is not detected at concentrations above the relevant surface water quality criteria. Therefore, groundwater does not appear to be a significant pathway for transport of Cr(VI) from DMT to the river.

Based on ecological survey observations within the study area (i.e., pore water, surface water, and sediment samples), the qualitative benthic study conducted in the study area, and other studies of fish, wildlife, and benthic communities conducted in the vicinity of the study area, the following specific receptors of potential concern were identified for the screening level ecological risk assessment:

- Benthic invertebrates: amphipods, polychaetes, marine clams, and mysid shrimp
- Pelagic fish: northern pipefish, white perch
- Omnivorous birds: mallard, hooded merganser
- Piscivorous birds: osprey



As COPR constituents do not biomagnify significantly within the food chain, it was determined that food chain risks did not require further evaluation, and the focus of the Ecological Risk Assessment (ERA) was on benthic and water column community exposure.

The ERA data was determined to be sufficient to establish that chromium and other COPR constituents do not pose an unacceptable risk to ecological receptors near DMT, and as such, it was concluded that no further action was required to assess the environmental impacts of COPR constituents from the site.

The results of the HHRA and ERA supported the selection of an Enhanced Isolation and Containment Remedy.

3.5 Basis for Sentinel Monitoring Program

The long-term groundwater monitoring program defined in this Sentinel Monitoring Plan is based on the current CSM for the site and details the monitoring network and the sampling, analysis, data evaluation, and reporting procedures. As detailed in this plan, the monitoring program incorporates a long-term optimization strategy to allow for annual review of the program to reaffirm that the components of the program remain current and protective. The strategy will also allow for review of the program and the defined sampling frequency to make sure that the methods for well sampling, sample analysis, data validation, data management, and reporting remain up to date. Annual reviews will also consider incorporation of relevant and applicable innovative technologies into the program as they are developed and become available for use. In addition to the extensive well network, there are new wells proposed that are detailed in Figure 4 that may be installed in the Shallow Fill to enhance the sentinel monitoring program.

4. Monitoring Well Network

4.1 Upper Saturated Zone

The Upper Saturated Zone occurs in a limited area east of the 1501/1602 area where groundwater was encountered above the level of the piezometric surface in the underlying shallow aquifer. Due to the potential lack of lateral continuity, the zone is not considered an aquifer but acts to transmit infiltrated surface water downward to the shallow aquifer The existing Upper Saturated Zone wells and piezometers will continue to be monitored for water elevations. Further assessment of the zone may be evaluated during installation of new monitoring wells targeting the Shallow Fill Unit in this area.

4.2 Shallow Fill Unit

4.2.1 COPR Fill Area

The Shallow Fill Unit, the uppermost laterally continuous hydrogeologic unit (shallow aquifer) beneath the site, is composed partly of the approximately 2.5 million cubic yards of COPR that underlie DMT. Groundwater flow in the unit is generally to the southwest, but local variations are observed where flow is affected by the heterogeneity of the fill material or by subsurface features, including storm drains, buried historic bulkhead features, and the sheet pile bulkhead that bounds the terminal to the south and west.

Excluding Area 1501/1602 (discussed below) and the immediate area between the 13.5th and 15th Streets' outfalls, the DMT fill area is bounded by vertical sheet pile bulkheads that impede the discharge of shallow groundwater to the Patapsco River. Evidence for the impeded discharge is based on groundwater levels along the immediate inboard face of the bulkhead that are approximately 2 feet above the river level and on the lack of tidal influence observed in shallow wells close to the bulkhead. The area between the 13.5th and 15th Street outfalls consists of low permeable materials based on boring logs, groundwater elevation data, and calibrated computer modeling, which also acts to impede the flow of groundwater to the river.

The Chromium Transport Study provided evidence that the extent of Cr(VI) groundwater contamination is limited to the Shallow Fill Unit (CH2M 2009b) and that the lateral extent within the Shallow Fill Unit is limited to within about 100 to 200 feet of the COPR fill boundary. Geochemical processes that reduce Cr(VI) to Cr(III) along the groundwater flow path appear to limit the lateral migration of Cr(VI). There are also physical and geochemical barriers to the vertical migration of groundwater and the Cr(VI) groundwater contamination to deeper hydrogeologic units. The Upper Confining Layer (Figure 2) has an average permeability of 1.62×10^{-7} centimeters per second (cm/s) (CM2M 2009b) and acts as a semiconfining layer beneath the Shallow Fill Unit.

Currently, there are 88 monitoring wells and 43 temporary piezometers installed within the Upper Saturated Zone and Shallow Fill units at the site and adjacent Former Mestek property (owned by MPA) (Figure 4), not including Area 1501/1602, discussed below. The monitoring wells were installed to monitor the groundwater elevations, with 15 wells being sampled semiannually, whereas most of the temporary piezometers were installed to monitor groundwater elevations for specific investigation tasks. The groundwater monitoring network for the Shallow Fill Unit includes 59 existing monitoring wells, 14 temporary piezometers, 9 shoreline wells, and 7 new monitoring wells, as discussed below, to provide an adequate coverage of wells across the site to generate groundwater potentiometric surface. A subset of 29 monitoring well locations was selected for semiannual groundwater sampling, with wells located along the site perimeter (perimeter wells), along the upgradient area (upgradient wells), and along the downgradient edge of the COPR boundary (interior wells). With this configuration, 19 existing monitoring wells and 29 temporary piezometers will be abandoned (Figure 5 and Table 4).



4.2.2 Area 1501/1602

The characteristics of the Shallow Fill Unit near Area 1501/1602 are discussed separately because they differ slightly owing to the way this area was constructed. The land that underlies Areas 1501/1602 was reclaimed by construction of an engineered containment cell where the COPR is encapsulated within a low-permeability liner and cover (CH2M 2009b). The COPR cell was constructed above the water table, and the cell is hydraulically separated from the Shallow Fill Unit on all sides excluding the northeast side (**Appendix A**), based on the substantial difference between water levels measured inside the cell and water levels measured outside (below) the cell in the Shallow Fill Unit. Below the COPR cell the Shallow Fill Unit is composed of non-COPR fill that was used to raise the grade of the river bottom to construct the cell with the exception of the northeast portion of Area 1501/1602 where COPR is not encapsulated within the COPR cell. A new cap featuring a geomembrane cover system constructed with asphalt pavement over the full extent of the area has been implemented.

The Area 1501/1602, as part of construction of the new cap, had a separate plan developed and submitted to the MDE in March 2021 detailing an already established well network to hydraulically monitor the groundwater conditions in Area 1501/1602 (**Appendix A**). Under the plan, 68 wells will be used to monitor groundwater elevations to confirm the lateral and vertical hydraulic containment of the COPR cell. The nine shoreline wells identified for water level monitoring under the Area 1501/1602 Sentinel Plan will also be sampled as part of this Sentinel Groundwater Monitoring Plan.

4.3 Upper Sand Unit

The Upper Sand is defined as the first unit of sand encountered beneath the Upper (silt) Confining Unit, which underlies the Shallow Fill Unit. The sands screened by the Upper Sand monitoring wells are not laterally extensive or continuous; therefore, the groundwater elevations measured in them are not contoured. Though upper sand units do not promote the horizontal flow of groundwater beneath DMT due to their discontinuous nature, they do represent an important monitoring interval to assess potential vertical migration of COPR-impacted groundwater as the sands occur within a few feet below the bottom of the COPR boundary.

Currently, there are 15 monitoring wells (DMT-US series) installed within the Upper Sands at the site (**Figure 6**). The monitoring wells were installed with the screen mid-point at an elevation of about 10 feet below mean sea level and are intended to monitor the groundwater elevations at the site, with 11 wells being sampled semiannually per the IGSP. The groundwater monitoring network for the Upper Sand Unit includes the existing 15 monitoring wells to provide an adequate coverage of wells across the site to generate vertical groundwater gradients, with 11 wells selected for groundwater sampling. Of the wells selected for sampling, six are located along the perimeter, three are at interior locations, and two are upgradient. No existing Upper Sand wells are to be abandoned (**Table 2**).

4.4 Patapsco Aquifer

Regional geologic data suggest that the upper portions of the Potomac Group sediments beneath DMT are classified as the Patapsco Formation, which occurs above the Arundel Clay. Medium-depth (M-series) wells (**Figure 7**) have well screen midpoints set to an elevation of about 95 feet below grade, which is in the Patapsco Formation where the aquifer matrix is composed mainly of sand. Based on groundwater flow measurements collected from the M-series wells, the groundwater flow is consistently south-southwest with hydraulic gradients ranging up to approximately 0.002. Groundwater levels in the M-series wells are consistently lower than water elevations in the Shallow Fill Unit and Upper Sand units. The difference in water elevation is further evidence that the confining units serve as effective physical barriers to hydraulic communication between the Shallow Fill Unit, the Upper Sand, and the Patapsco Aquifer.



Soil and groundwater sample results collected from the M-series wells suggest that the Patapsco Aquifer below DMT is not impacted by chromium constituents (CH2M 2009a). The absence of chromium-related impacts in the aquifer is explained by the thickness of the overlying Upper and Lower Confining Units (upper and lower silt layers), which lie between the Shallow Fill Unit and the Patapsco Aquifer. Both silt units have low permeability, which allows them to function as confining units, and organic material that would facilitate the reduction of Cr(VI) to the relatively immobile Cr(III) species, preventing the chromium constituents from reaching the deeper hydrogeologic units. The permeability of the lower silt, which ranges up to 50 feet thick below DMT, has been determined to be 9.77×10^{-8} cm/sec (CH2M 2009b).

The aquifer outcrops directly beneath the brackish Patapsco River, and historical pumping of the aquifer caused chloride contamination within the aquifer. Use of the Patapsco Aquifer as a water resource in the vicinity of Baltimore and the Patapsco River estuary had mostly ceased by 1950 due to chloride intrusion, and at present there is no major use of the aquifer in this region (Chapelle 1985). Significant pumping of the Patapsco Aquifer does, however, occur in northern Anne Arundel County, and the nearest pumping centers are approximately 7 miles from DMT. For this reason, it is prudent to have some level of monitoring in the underlying Patapsco Aquifer, although the likelihood of a Cr(VI) ever reaching that aquifer is remote.

Currently, there are 26 monitoring wells (M-series) installed within the Patapsco Aquifer at the site. The monitoring wells are intended to monitor the groundwater elevations at the site, with nine wells being sampled semiannually. The groundwater monitoring network for the Patapsco Aquifer includes the existing 23 monitoring wells to provide an adequate coverage of wells across the site to generate groundwater potentiometric surface, with 9 wells selected for groundwater sampling (**Table 3**). Of the wells for sampling, four are along the perimeter, four are at interior locations, and one is upgradient. Three existing M-series wells are to be abandoned (**Table 4**).

4.5 Patuxent Aquifer

D-series wells are screened in what regional geologic data suggest is the Patuxent Aquifer (CH2M 2009c). Several thick sequences of clay strata exist between the M-series and D-series wells. The clay strata is considered to be the Arundel Formation, which is a regional aquitard that separates the Patapsco and Patuxent Aquifers. The function of the clay strata as an aquitard beneath the DMT is supported by the low average permeability (9.20 × 10⁻⁸ cm/sec) of the clay strata and by a substantial difference in water elevations measured in co-located D- and M-series well pairs. The Patuxent Aquifer is unaffected by conditions at the site because the Patuxent is separated from the Patapsco by the Arundel Formation; sampling of M-series wells has indicated that the overlying Patapsco Aquifer has not been impacted.

There are three D-series installed within the Patuxent Aquifer at the site and the adjacent Former Mestek facility owned by MPA (**Figure 8**). The monitoring wells are intended to monitor the groundwater elevations at the site. Although site-related contamination would not be found in this aquifer unless detections occur in the M-series wells, the D-series wells are included in the groundwater sampling plan. There are no changes to the existing Patuxent Aquifer monitoring. No existing D-series wells are to be abandoned (**Table 3**).



5. Monitoring and Assessment

Monitoring will include periodic groundwater elevation monitoring and groundwater sampling from the monitoring well network to demonstrate the containment remedy selected for the site. The groundwater monitoring approach is like the one followed under the IGSP (CH2M 2009a), with the monitoring frequency set to semiannually for the groundwater elevation monitoring and groundwater sampling. The wells to be monitored are summarized in Table 2, which includes the rationale for the well use. In addition, at 5-year intervals the sediment and porewater under the Patapsco River will be sampled to confirm reducing conditions where groundwater may upwell into the river.

As described in Section 5.2, the results of chromium sampling will be evaluated for the detection of Cr(VI) outside the COPR fill area, and if confirmed, the monitoring results will be assessed for statistically significant increasing trends. Based on these findings, increased frequency of monitoring may be implemented to further assess changes in site groundwater conditions.

5.1 Groundwater Elevation Monitoring

The groundwater elevations measured from the monitoring well network (Table 2) will be used to assess the lateral and vertical hydraulic gradients at the site, and evaluate potential changes and their possible causes. During the groundwater elevation monitoring, the integrity of the well will be evaluated and will include well depth measurements to assess redevelopment needs, a well head condition assessment to correct observed damage, and a well seal evaluation to replace well seals.

5.1.1 Upper Saturated Zone

The Upper Saturated Zone piezometers and wells will be monitored for groundwater elevations and select proposed well locations outside the COPR boundary will be sampled if a zone is present.

5.1.2 Shallow Fill—COPR Fill Area

Hydraulic monitoring in the Shallow Fill Unit will include 63 wells located both inside and outside the COPR fill limits. Within the COPR fill, the hydraulic monitoring will be used to verify that groundwater is not infiltrating the relined storm drains, to compare vertical hydraulic gradients with those of deeper units, and to establish the groundwater elevations within this area for comparison of the wells in Area 1501/1602. Hydraulic measurements outside the COPR fill would be used to confirm that the sheet pile bulkhead remains an impediment to the direct discharge of shallow groundwater to the Patapsco River.

5.1.3 Shallow Fill—Area 1501/1602

The list of 68 monitoring wells in Area 1501/1602 that will be hydraulically monitored are included in Appendix A. The groundwater elevations in the Area 1501/1602 wells will be compared to the groundwater elevations in the adjacent Shallow Fill wells and to those in the underlying and adjacent Upper Sand wells to demonstrate hydraulic containment. The groundwater elevations will also be monitored to assess the effectiveness of the overlying cap and asphalt in reducing the amount of infiltration into the COPR cell.

5.1.4 Upper Sand Unit

Groundwater elevations in the 24 Upper Sand wells will be used to assess vertical hydraulic gradients at well locations adjacent to shallower or deeper wells, and to confirm hydraulic containment in the Shallow Fill wells. As eight of the Upper Sand wells are located adjacent to the Area 1501/1602, the groundwater elevations in these wells will also assess hydraulic containment of the COPR cell.



5.1.5 Patapsco Aquifer

Hydraulic monitoring in the Patapsco Aquifer will comprise 23 wells across the site, including wells installed within the footprint of the COPR fill limits. Groundwater elevations measured from these wells will be used to confirm groundwater flow conditions in the Patapsco Aquifer, to evaluate vertical hydraulic gradients, and to potentially assess well integrity issues related to potential detections during groundwater sampling.

5.1.6 Patuxent Aquifer

Although there are three wells in the Patuxent Aquifer, the wells are included in the network for completeness. Groundwater elevations measured in these wells will be used to confirm groundwater flow conditions in the aquifer and to evaluate vertical hydraulic gradients.

5.2 Groundwater Sampling

Monitoring wells included in the semiannual groundwater sampling are identified as a subset of wells in the network (Table 3). The well network includes wells used during the IGSP as previous analytical data from these wells form a robust database for comparison of future groundwater quality. Potential changes to the well network are discussed in Section 5.4. Low-flow groundwater sampling methods will be used to purge and sample the wells, including the collection of stabilization parameters such as pH, temperature, conductivity, and turbidity, and the groundwater samples will be submitted for analysis of total and dissolved chromium by SW-846 Method 6010B and hexavalent chromium (filtered) by SW-846 Method 7199A. Quality assurance and quality control will be performed in accordance with the DMT *Quality Assurance Program Plan* (CH2M 2007). Purge water will be processed through the onsite treatment plant or at an approved disposal facility.

5.2.1 Shallow Fill and Upper Saturated Zone

Groundwater samples will be collected from the following 31 groundwater monitoring wells screened in the Shallow Fill Unit and the Upper Saturated Zone:

- DMT-01S DMT-12S DMT-14S DMT-17S DMT-27S DMT-28S DMT-31S DMT-32S DMT-39S DMT-41S DMT-42S DMT-47S **DMT-56S EA-10S EA-12S EA-11S** EAC-01S S-1 S-2 S-31 S-3D S-41 S-4D S-5 S-611 S-6D S-7 **DMT-90S**² **DMT-91S**² DMT-92S²
- DMT-96S²

For distribution, wells are along the perimeter of the site, along the edge of the Area 1501/1602, outside the COPR fill limits in the site interior, below the COPR cell at Area 1501/1602, and along the eastern property boundary, considered upgradient wells. Analytical data from the perimeter wells will be used to confirm containment within the site boundary, whereas the data from the wells outside the COPR fill limits will be used to confirm the limited lateral migration potential from the COPR fill area. The well located within the COPR cell at Area 1501/1602 is screened below the clay liner and is used to assess if there is a potential for vertical migration of chromium from the COPR cell. The upgradient wells are used to assess the background groundwater quality.

.

Additional shallow well.

² Proposed well. An upper saturated zone well will be nested and sampled if the zone is present.



5.2.2 Upper Sand Unit

As the Upper Sand wells represent the first groundwater zone below the Shallow Fill Unit, these wells are critical to assess potential vertical migration of chromium from the COPR fill. There are 11 Upper Sand wells that will be sampled:

DMT-49US

DMT-52US

DMT-65US

DMT-72US

DMT-50US

DMT-53US

DMT-67US

DMT-73US

DMT-51US

DMT-64US

DMT-70US

Seven of the wells are located along the site perimeter to monitor for groundwater quality if chromium has migrated vertically down to this zone and then moved laterally toward the Patapsco River. 2 Upper Sand wells are located within the footprint of the COPR limits and will be used as early warning wells if chromium has migrated vertically down below the Shallow Fill Unit. One well is set as the upgradient location to assess background conditions. One well is located outside the footprint of the COPR limits at a downgradient location to evaluate potential detections before the groundwater migrated to the perimeter.

5.2.3 Patapsco Aquifer

The Patapsco Aquifer is hydraulically separated from the Shallow Fill Unit by more than 50 feet of low-permeability material based on geological evidence. Vertical migration of chromium though this thick lower confining layer is not likely before detections of chromium is found in the Upper Sand Unit wells or in the perimeter Shallow Fill Unit wells. A total of 23 wells are screened in the Patapsco Aquifer, 9 of the 23 wells are included in the semiannual sampling program, with four perimeter locations, one upgradient location, and four locations within the footprint of the COPR fill. All remaining M-series wells (following abandonment of EA-3M, EAC-4M, and EA-5M), will be sampled during the first year of the PMP and every 5 years after to evaluate overall integrity of the well, while the nine wells footnoted will be sampled semiannually as part of this program. Groundwater samples collected from these wells will be used to evaluate potential chromium detections, and to assess potential well integrity issues related to chromium detections during groundwater sampling.

DMT-02M

EAC-02M

EA-15M

• EA-07M³

DMT-37M³

EA-10M

EA-13M

EA-02M

EA-11M³

DMT-60M³

DMT-35M

DMT-78M

DMT-79M

DMT-01M-R³

EA-06M³

EA-08M

EA-14M

EAC-03M

DMT-34M³

DMT-80M

DMT-38M

DMT-77M³

DMT-36M³

5.2.4 Patuxent Aquifer

The Patuxent Aquifer is hydraulically separated from the Patapsco Aquifer by the Arundel Clay confining unit. Vertical migration of chromium through this thick lower confining layer and then through the Arundel Clay is not likely before detections of chromium is found in the shallower units. However, the three wells screened in the Patuxent Aquifer will be sampled under this plan to continue monitoring the groundwater quality at depth:

- DMT-81D
- DMT-82D
- DMT-83D

5-3

³ Sampled semiannually.



5.3 Data Evaluation

Data collected during the sampling and monitoring will be evaluated to confirm containment of chromium on the site. This evaluation will include the following:

- Well integrity
- Groundwater elevations
- Chromium concentrations in groundwater
- Statistical evaluation of data to determine if statistically significant increases are identified
- Reducing conditions in the Patapsco River
- Modifications to the wells, sampling frequency, or well network

5.3.1 Well Integrity

The well integrity from the monitoring event will be summarized in a table to confirm that the groundwater information collected from the well is not affected by potential well damage or accumulation of excessive amounts of sediment at the well bottom. Well damage, including improper well seals and broken surface casings, will be corrected between monitoring events. Excessive sediment accumulation will trigger redevelopment of the well prior to the next event. Potentially damaged wells will be taken into consideration during evaluation of the groundwater elevation and sampling results as potentially impacting the data results. A downhole video camera may be used to assess well blockages or suspected breaks in the well casing.

5.3.2 Groundwater Elevations

The groundwater depths measured during the monitoring event will be converted to groundwater elevations and corrected if the well is tidally influenced (Table 1). This procedure is the same as followed for the IGSP. Groundwater elevations are used to generate contoured piezometric maps for the Shallow Fill Unit, the Patapsco Aquifer, and Patuxent Aquifer. The elevations from the Upper Saturated Zone and Upper Sand wells will be plotted as well, but not contoured based on the limited lateral extent of the water-bearing zones and the various sand lenses that occur in the Upper Sand wells between the Upper and Lower Confining Units. The Shallow Fill Unit groundwater elevations will be evaluated and compared to the Model Simulations of Storm Drain Relining Report as presented in Appendix B of the CMAA to confirm effects of relining are consistent with predictions. The contoured maps will be used to confirm groundwater flow direction and hydraulic gradient, and will be compared to previous maps to assess general patterns of groundwater elevation changes over time. Time-series graphs of groundwater elevations may be plotted if long- or short-term changes are noted. Changes in the groundwater flow pattern or impact of changes to the groundwater elevations will be evaluated to assess if modifications to the well network or sampling program are needed.

Groundwater elevations from wells within Area 1501/1602 will also be contoured to document groundwater flow and gradients within the COPR cell. These elevations will be compared to the elevations in the well points (S-series) along the shoreline and the adjacent Shallow Fill Unit wells outside Area 1501/1602 to confirm hydraulic containment of the COPR cell. The contoured maps will also be compared to previous maps to assess general patterns of groundwater elevation changes over time. Time-series graphs will be plotted to monitor the effect of the efforts to reduce infiltration into the COPR cell. Issues related to potential breach of hydraulic containment or increased infiltration will be investigated in detail to confirm data integrity and then integrity of the containment cell will be evaluated to correct the issue.

The vertical hydraulic gradients between the Upper Saturated Zone, Shallow Fill Unit, the Upper Sands, the Patapsco Aquifer, and the Patuxent Aquifer will be calculated for nearby well screened in different units. Based on the previous semiannual sampling events, the vertical gradients fluctuate with the time of year due to natural changes in recharge, but are generally within a historical range. Vertical gradients



calculated outside the historical range will be reviewed to assess whether there is a potential vertical hydraulic connection. If the vertical hydraulic gradient indicates that there is a significant change, an assessment of the monitoring well integrity will be critically reviewed to make corrections. This may be necessary to prevent groundwater migration along the well casing or other integrity problems, which required monitoring well replacement.

5.3.3 Chromium Concentrations in Groundwater

Analytical results from the groundwater sampling will be validated by a third-party validator and then presented in summary tables. Level 4 validation will be completed in accordance with the Quality Assurance Project Plan (CH2M, 2007). Detections of Cr(VI) in a well will prompt a review of the field collection information and laboratory analytical process to assess whether issues related to well integrity, potential cross-contamination, or other sources of error could result in a false positive result. A Cr(VI) detection in a well that previously had no detections will prompt resampling to reduce the potential for a false positive detection. Well integrity issues, if identified, will be corrected prior to resampling. Resampled data will be used for in the evaluation process. If a Cr(VI) detection is confirmed, then the potential for a statistically significant increasing trend will be evaluated as the monitoring program continues. In addition, the confirmed Cr(VI) concentration(s) will be evaluated on the risk for potential adverse impacts to potential receptors, including Port Users, the adjacent residents, Patapsco River and the Patapsco Aquifer.

The development of this framework on evaluating chromium groundwater monitoring data is based on the objective of the sentinel groundwater monitoring program – that is, to allow sufficient time to implement contingent remedial measures, as appropriate, if Cr(VI) is detected above levels that may pose an adverse risk to human health and the environment at the site boundary. This is accomplished by establishing threshold Action Levels that would be an indicator of a potential adverse impact on surface water or groundwater outside of COPR fill areas. The general evaluation process of the analytical data will be based on the well location and groundwater-bearing zone where the well is screened, as follows:

- 1. Detection of Cr(VI) in wells not previously detecting Cr(VI) will trigger further assessment, e.g.:
 - a. Confirmation of detection
 - b. Assessment of temporal trends (i.e., are concentrations increasing)
 - c. Comparison with the risk-based Action Level protective of reasonably anticipated potential receptors
- 2. Action Levels are developed based on reasonably anticipated potential receptors:
 - a. Lateral flow to adjacent Patapsco River
 - b. Vertical migration to Patapsco Aquifer
- 3. For groundwater-bearing units that flow laterally towards the Patapsco River and/or offsite, detection of Cr(VI) concentrations above the Action Level may warrant increased monitoring frequency, sampling of surface water in the vicinity of well(s) with concentrations exceeding the Action Level, and/or consideration of modifications and/or additions to existing corrective measures.
- 4. For groundwater units that have a downward gradient toward the Patapsco Aquifer, detection of total chromium and/or Cr(VI) above Action Levels for protection of Patapsco Aquifer may warrant increased monitoring frequency, and/or consideration of modifications and/or additions to existing corrective measures.



Shallow Fill Unit

For evaluating groundwater monitoring data for lateral migration within the Shallow Fill Unit, the site was divided into two zones.

- Zone 1 wells are outside the footprint of the COPR fill limit in the portion of DMT that is contained within the perimeter bulkhead, which reduces the potential for groundwater discharges to the river. If Cr(VI) detections are confirmed in a Zone 1 well, the data from the well and in downgradient wells (if available) will be assessed for a statistically increasing concentration trends for evidence of Cr(VI) migration from the COPR fill area. If the well has confirmed Cr(VI) detections and an increasing trend is noted based on subsequent sampling events, the groundwater conditions at the site will be reviewed for changes that may have affected the stability of the groundwater system, including an assessment of upgradient conditions. A change in the well network will be evaluated to use an existing downgradient monitoring well or to drill a new downgradient monitoring well as a replacement monitoring location. Any new well installed will need to be assessed for tidal influence to generate an appropriate correction to the groundwater elevation. If there is no suitable downgradient location (i.e., perimeter monitoring well location), then evaluations will be conducted to confirm hydraulic containment or additional remedial actions considered to protect human health and the environment.
- Zone 2 wells are in areas where lateral groundwater flow may discharge to the river. Zone 2 is defined as the discharge area (1) between the site bulkhead and the COPR cell, (2) the western and southern sides of the COPR cell, and (3) the area south of the COPR cell (i.e., under the adjacent Mestek Property and Carnegie Plat neighborhood). For Zone 2, an Action Level was developed based on protection of ecological receptors in the Patapsco River where groundwater upwelling from this lateral groundwater flow may occur. The chronic saline surface water quality standard for Cr(VI) of 0.05 milligram per liter (mg/L) was selected as the basis for this Action Level. The Action Level calculated for the groundwater monitoring program is intentionally conservative. For example, it is assumed that all groundwater discharging from Zone 2 contains Cr(VI), mixing in surface water only occurs within first few inches of the water depth, and no exchange of surface water occurs over a 24hour period (i.e., tidal exchange is ignored). This Action Level thus provide a conservative threshold to trigger an investigation of potential changes in groundwater conditions and potential impacts to the Patapsco River, Based on the surface water quality standard as the target concentration, and estimation of groundwater discharge using the three-dimensional flow model developed for the site (CH2M 2009b), and the area of groundwater upwelling identified in the 2008 study (CH2M and ENVIRON 2009), an Action Level calculated for these areas of the site where groundwater discharges into the Patapsco River and divided by a safety factor of 2 for an Action Level of 25 mg/L Cr(VI). Supporting information for the computation of the Action Level is provided in Appendix C.

For these zones, any Cr(VI) detection will be confirmed, and if confirmed, a statistical evaluation of concentration trends will be conducted as additional data are collected. If a statistically significant increasing trend is identified, the groundwater conditions at the site will be reviewed for changes that may have affected the stability of the groundwater system, including an assessment of upgradient conditions. If appropriate, a change in the well network will be evaluated, specifically to consider incorporating an existing downgradient monitoring well or installing a new downgradient (offsite) monitoring well to assess lateral migration. If a new well is installed, it will be assessed for tidal influence to evaluate an appropriate correction to the groundwater elevation. Hydraulic containment will be confirmed, or additional remedial actions will be considered to protect human health and the environment. If concentration(s) exceed the Action Level for this lateral migration pathway, sampling frequency may be increased to support a more expedient trend evaluation. In addition, surface water sampling may be conducted to confirm that groundwater migration is not adversely impacting surface water quality in groundwater upwelling.



Upper Sand Unit

As detailed in Appendix C, groundwater Action Levels were developed for the Upper Sand Unit based on protection of the Patapsco Aquifer associated with a vertical migration pathway. These Action Levels are based on meeting the drinking water standard (i.e., federal and Maryland maximum contaminant level [MCL]) of 0.1 mg/L (100 micrograms per liter [μ g/L]) for total chromium and the Maryland groundwater cleanup standard of 0.000035 mg/L (0.035 μ g/L) for Cr(VI) (MDE 2018). The Action Levels calculated for the groundwater monitoring program are intentionally conservative and provide a threshold to trigger an investigation of potential changes in groundwater conditions or impacts to the Patapsco Aquifer rather than an indication of actual impacts.

Based on the drinking water and groundwater cleanup standards and the groundwater model developed for the site (CH2M 2009), Action Levels for vertical migration of 25.7 mg/L total chromium and 0.009 mg/L Cr(VI) were calculated for the Upper Sand Unit monitoring program. The details of this calculation are provided in Appendix C.

Detections of total chromium and Cr(VI) will be confirmed through resampling, as discussed above. If a statistically significant increasing trend for total chromium and Cr(VI) is identified as additional data are collected, the groundwater conditions at the site will be reviewed for changes that may have affected the stability of the groundwater system, including an assessment of upgradient conditions. If appropriate, a change in the well network will be evaluated, specifically to consider incorporating an existing downgradient monitoring well or installing a new downgradient (offsite) monitoring well to track lateral migration. If a new well is installed, it will be assessed for tidal influence to evaluate an appropriate correction to the groundwater elevation. Hydraulic containment will be confirmed, or additional remedial actions will be considered to protect human health and the environment.

If the average total chromium or Cr(VI) concentrations in the Upper Sand Unit wells for each monitoring event are above the respective Action Levels for vertical migration of 25.7 mg/L or 0.009 mg/L, respectively, the monitoring frequency may be increased to support the evaluation of concentration trends. In addition, monitoring of the Patapsco Aquifer wells will be increased to assess potential impacts to groundwater quality in this lower aquifer.

Patapsco and Patuxent Aquifers

Confirmed total chromium or Cr(VI) concentrations in the Patapsco and Patuxent Aquifer wells will be compared to the drinking water standard and groundwater cleanup level of 0.1 mg/L and 0.000035 mg/L, respectively. If an increasing trend is noted or if the concentrations are above the standard for total chromium or Cr(VI) during the past four sampling events, the groundwater conditions at the site will be reviewed for changes that may have affected the stability of the groundwater system, including an assessment of upgradient conditions. If appropriate, a change in the well network will be evaluated, specifically to consider incorporating an existing downgradient monitoring well or installing a new downgradient (offsite) monitoring well to track lateral migration. If a new well is installed, it will be assessed for tidal influence to evaluate an appropriate correction to the groundwater elevation. If there is no suitable downgradient location (i.e., perimeter monitoring well location), hydraulic containment will be confirmed through additional monitoring, or additional remedial actions will be considered to protect the receptors of this groundwater resource.



5.3.4 Monitoring Plan Modifications

This monitoring plan will be evaluated annually to make adjustments to specific wells, to the monitoring well network, and to the sampling frequency. Well integrity evaluations may require repairs to the well heads and redevelopment actions to remove accumulated sediment at the well bottom. If damaged beyond repair, a well may need to be replaced. Based on assessment of the groundwater sample analytical results, replacement wells or supplemental wells may be added to the well network. The well network may require adjustments if changing groundwater conditions are noted to the site due to changing site conditions. The monitoring frequency may be adjusted based on site conditions. The sediment and porewater sampling frequency may be increased if changing geochemical conditions that necessitate additional evaluation are noted. Modifications to this Sentinel Plan will be documented in the monitoring reports and will be implemented based on MDE approval.

5.4 Sediment and Porewater Sampling

As part of the *Corrective Measures Alternative Analysis* (CH2M 2011), the hydraulic containment system monitoring has a component to confirm chemically reducing conditions in the Patapsco River adjacent to DMT. Based on previous work completed at the site in 2007 and 2008, the reducing conditions will be assessed with the collection of sediment and porewater samples beneath the Patapsco River adjacent to the site, consistent with these prior studies. A detailed work plan for the sediment and porewater sampling is included in Appendix B. As specified in the CMAA (CH2M 2011), the monitoring of sediment and porewater will occur at 5-year intervals commencing with approval of this plan.

Analytical results from the sediment and porewater will be assessed to confirm reducing conditions beneath the Patapsco River. Maps detailing the sampling locations and graphs of the data compared to previous information will be used to evaluate any potential changes in the sediment and/or the porewater geochemical conditions. Areas where reducing conditions may not be occurring will be highlighted and discussed regarding implications to the selected containment remedy for the DMT site. The discussion will include potential reasons that the geochemistry has changed.

6. Reporting

Reports of the monitoring events will be completed at the conclusion of each sampling event after receipt of validated analytical results. These reports will summarize the data in tables and figures, and will provide a detailed evaluation of the data as described in Section 5.3. Proposed well repair, well redevelopment, well replacement, adjustments to the well network, and actions considered based on the data evaluation will be documented in the semiannual report.

7. References

Bennett, R.R., and R.R. Meyer. 1952. "Geology and Groundwater Resources of the Baltimore Area." Maryland Department of Natural Resources, Maryland Geological Survey. Bulletin #4.

CH2M. 2007. Quality Assurance Program Plan, Dundalk Marine Terminal, Baltimore, Maryland.

CH2M. 2009a. Interim Groundwater Sampling Plan, Dundalk Marine Terminal, Baltimore, Maryland. April 20.

CH2M. 2009b. Chromium Transport Study Report, Dundalk Marine Terminal, Baltimore, Maryland.

CH2M. 2009c. Human Health Risk Assessment, Dundalk Marine Terminal, Baltimore, Maryland.

CH2M. 2011. Corrective Measures Alternatives Analysis, Dundalk Marine Terminal, Baltimore, Maryland. January 14.

CH2M and ENVIRON. 2009. Sediment and Surface Water Study Report, Dundalk Marine Terminal, Baltimore, Maryland.

Chapelle, Francis C. 1985. "Hydrogeology, Digital Solute-Transport Simulation, and Geochemistry of the Lower Cretaceous Aquifer System near Baltimore, Maryland." *Maryland Geological Survey Report of Investigations 43*.

Hansen, H.J. 1968. "Geophysical Log Cross-Sections Network of the Cretaceous Sediments of Southern Maryland." *Maryland Geological Survey Report of Investigations No. 7.*

Tables

Table 1. Chromium Transport Study, Tidal Study Results

Shallow Wells			Up	per Sand Wells	3		M-Series Wells D-Series		D-Series Wells		
Well ID	Tidal Efficiency (%)	Lag Time (min)	Well ID	Tidal Efficiency (%)	Lag Time (min)	Well ID	Tidal Efficiency (%)	Lag Time (min)	Well ID	Tidal Efficiency (%)	Lag Time (min)
DMT-1S		•	DMT-49US	. 0	0	DMT-1M	62.7	45	DMT-81D	13.2	116
DMT-3S		44	DMT-50US ¹	10.6	-128	DMT-2M	66	139	DMT-82D	4.8	240
DMT-12S		1.4	DMT-50US ¹	9.2	-87	EA-10M	58.1	91	DMT-83D	52.4	9
DMT-13S		- 4	DMT-51US	0	0	DMT-34M	63.4	28			
DMT-14S	4	0	DMT-52US	0	0	DMT-35M	0	0			
DMT-15S		1.5	DMT-54US	0	0	DMT-36M	29.5	63			
DMT-16S			DMT-64US	45.2	63	DMT-37M	57.78	20			
DMT-17S		1.4	DMT-65US	8.1	52	DMT-38M	0	0			
DMT-18S		1.4	DMT-67US	6	240	DMT-60M	0	0			
EAC-3S		1.4	DMT-70US	39.8	4	DMT-77M	17.4	62			
EAC-4S		1.4	DMT-71US	14.6	88	DMT-78M	73.9	75			
EA-6S			DMT-72US	21.7	44	DMT-79M	1.6	240			
EA-7S			DMT-73US	13.9	43	DMT-80M	7.8	193			
EA-105	6.3	65	DMT-74US	5.5	201	EA-11M	68.28	37			
EA-115			DMT-75US	3.7	228	EA-13M	58.23	42			
EA-14S			TPZ-48	25.1	170	EA-14M	67.04	11			
EA-175		1.4	TPZ-49			EA-15M	62.94	43			
P-10		- (4)				EA-2M	3.13	194			
DMT-45S	58.4	43				EA-6M	30.85	104			
DMT-46S	18.1	66				EA-7M	39.8	93			
DMT-56S	15.8	89				EA-8M	6.67	201			
DMT-57S	56.1	48				EA-9M	44.74	55			
DMT-58S	69.6	37				EAC-1M	6.73	171			
DMT-2S						EAC-2M	8.15	149			
DMT-7S						EAC-3M	58.38	19			
DMT-8S		1.4									
DMT-9S											
DMT-10S											
DMT-20S											
DMT-25S											
P-3											
TPZ-B											
TPZ-24											
DMT-83S	23.3	62									

Note: Not all wells listed are still in use.

^{*}Not quantified due to lack of tidal response 1 = Two tidal studies were completed for well DMT-50US in November and December 2007.

Table 2. Sentinel Well Network/Well Gauging

Qty	Well Name	Screened Unit	Sampled (Y/N)	Location	Rationale
1	DMT-01S	Shallow Fill	Υ	Interior, Outside COPR	Assess potential lateral chromium migration
3	DMT-02S DMT-03S	Shallow Fill Shallow Fill	N N	Interior, Outside COPR Interior, Outside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
4	DMT-033	Shallow Fill	N	Interior, Outside COPR	Assess lateral groundwater movement
5	DMT-05S	Shallow Fill	N	Interior, Outside COPR	Assess lateral groundwater movement
<u>6</u> 7	DMT-06S DMT-07S	Shallow Fill Shallow Fill	N N	Interior, Inside COPR Interior, Inside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
8	DMT-08S	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
9	DMT-09S DMT-10S	Shallow Fill Shallow Fill	N N	Interior, Inside COPR Interior, Inside COPR	Assess lateral groundwater movement
11	DMT-103	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
12	DMT-12S	Shallow Fill	Y	Perimeter, South	Assess potential lateral chromium migration
13 14	DMT-13S DMT-14S	Shallow Fill Shallow Fill	N Y	Interior, Inside COPR Perimeter, South	Assess lateral groundwater movement Assess potential lateral chromium migration
15	DMT-15S	Shallow Fill	N	Perimeter, South	Assess lateral groundwater movement
16 17	DMT-17S DMT-18S	Shallow Fill	Y N	Perimeter, West	Assess potential lateral chromium migration Assess lateral groundwater movement
18	DMT-16S	Shallow Fill Shallow Fill	N N	Perimeter, West Interior, Outside COPR	Assess lateral groundwater movement
19	DMT-20S	Shallow Fill	N	Interior, Outside COPR	Assess lateral groundwater movement
20 21	DMT-22S DMT-26S	Shallow Fill Shallow Fill	N N	Interior, Inside COPR Interior, Outside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
22	DMT-27S	Shallow Fill	Y	Interior, Outside COPR	Assess potential lateral chromium migration
23 24	DMT-28S DMT-29S	Shallow Fill Shallow Fill	Y	Interior, Outside COPR Interior, Inside COPR	Assess potential lateral chromium migration Assess lateral groundwater movement
25	DMT-30S	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
26	DMT-31S	Shallow Fill	Y	Perimeter, South	Assess potential lateral chromium migration
27 28	DMT-32S DMT-33S	Shallow Fill Shallow Fill	Y N	Perimeter, South Interior, Inside COPR	Assess potential lateral chromium migration Assess lateral groundwater movement
29	DMT-39S	Shallow Fill	Υ	Perimeter, Outside COPR	Assess potential lateral chromium migration
30 31	DMT-40S DMT-41S	Shallow Fill Shallow Fill	N Y	Perimeter, West Interior, Outside COPR	Assess lateral groundwater movement Assess potential lateral chromium migration
32	DMT-41S DMT-42S	Shallow Fill	Y	Perimeter, West	Assess potential lateral chromium migration Assess potential lateral chromium migration
33	DMT-43S	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
34 35	DMT-47S DMT-56S	Shallow Fill Shallow Fill	Y	Upgradient, East 1501/1602 Below Liner	Assess background groundwater quality Assess potential vertical chromium migration
36	DMT-59S	Upper Saturated Zone	N	Upgradient, East	Assess vertical groundwater movement
37 38	DMT-61S DMT-62S	Shallow Fill Shallow Fill	N N	Upgradient, East Perimeter, Outside COPR	Assess upgradient groundwater movement Assess lateral groundwater movement
39	DMT-90S	Shallow Fill	Y	Perimeter, South	Assess lateral gloundwater movement Assess potential lateral chromium migration
40	DMT-91S	Shallow Fill	Y	Perimeter, South	Assess potential lateral chromium migration
41 42	DMT-92S DMT-93S	Shallow Fill Shallow Fill	Y	Interior, Outside COPR Interior, Inside COPR	Assess potential lateral chromium migration Assess lateral groundwater movement
43	DMT-94S	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
44 45	DMT-95S DMT-96S	Shallow Fill Shallow Fill	N N	Interior, Inside COPR Interior, Inside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
46	TPZ-27A	Shallow Fill	N	Upgradient, East	Assess lateral groundwater movement
47	TPZ-27B	Upper Saturated Zone	N	Upgradient, East	Assess vertical groundwater movement
48 49	TPZ-28 TPZ-29	Upper Saturated Zone Upper Saturated Zone	N N	Interior, East Interior, East	Assess vertical groundwater movement Assess vertical groundwater movement
50	TPZ-30A	Shallow Fill	N	Interior, East	Assess lateral groundwater movement
51 52	TPZ-30B TPZ-33	Upper Saturated Zone Shallow Fill	N N	Interior, East Interior, Outside COPR	Assess vertical groundwater movement Assess lateral groundwater movement
53	TPZ-35	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
54 55	TPZ-36 TPZ-37	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
56	TPZ-37	Shallow Fill Shallow Fill	N N	Interior, Inside COPR Upgradient, East	Assess lateral groundwater movement Assess upgradient groundwater movement
57	TPZ-58	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
58 59	TPZ-59 TPZ-61	Shallow Fill Shallow Fill	N N	Interior, Inside COPR Interior, Inside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
60	EA-02S	Shallow Fill	N	Interior, East	Assess lateral groundwater movement
61 62	EA-03S EA-06S	Shallow Fill Shallow Fill	N N	Interior, East Interior, Inside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
63	EA-07S	Shallow Fill	N	Inside 1501/1602 Cell	Assess lateral groundwater movement
64	EA-08S	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
65 66	EA-10S EA-11S	Shallow Fill Shallow Fill	Y	Interior, Outside COPR Perimeter, South	Assess lateral groundwater movement Assess potential lateral chromium migration
67	EA-12S	Shallow Fill	Υ	Interior, Outside COPR	Assess lateral groundwater movement
68 69	EA-15S EA-16S	Shallow Fill Shallow Fill	N N	Interior, Inside COPR Interior, Outside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
70	EA-17S	Shallow Fill	N	Perimeter, South	Assess lateral groundwater movement
71 72	EA-21S EAC-01S	Shallow Fill Shallow Fill	N Y	Interior, Outside COPR Upgradient, East	Assess lateral groundwater movement Assess background groundwater quality
73	EAC-01S EAC-02S	Shallow Fill	N N	Interior, Outside COPR	Assess background groundwater quality Assess lateral groundwater movement
74 75	EAC-03S	Shallow Fill	N N	Perimeter, South	Assess lateral groundwater movement
75 76	P-03 P-06	Shallow Fill Shallow Fill	N N	Interior, Inside COPR Interior, Inside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
77	P-07	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
78 79	P-09 P-10	Shallow Fill Shallow Fill	N N	Interior, Inside COPR Interior, Inside COPR	Assess lateral groundwater movement Assess lateral groundwater movement
80	P-11	Shallow Fill	N	Interior, Inside COPR	Assess lateral groundwater movement
81	S-1 S-2	Shallow Fill	Y	Perimeter, 1501/1602	Assess potential lateral chromium migration
82 83	S-2 S-3I	Shallow Fill Shallow Fill	Y	Perimeter, 1501/1602 Perimeter, 1501/1602	Assess potential lateral chromium migration Assess potential lateral chromium migration
84	S-3D	Shallow Fill	Υ	Perimeter, 1501/1602	Assess potential lateral chromium migration
85 86	S-4I S-4D	Shallow Fill Shallow Fill	Y	Perimeter, 1501/1602 Perimeter, 1501/1602	Assess potential lateral chromium migration Assess potential lateral chromium migration
87	S-5	Shallow Fill	Υ	Perimeter, 1501/1602	Assess potential lateral chromium migration Assess potential lateral chromium migration
88	S-6 S-7	Shallow Fill	Y	Perimeter, 1501/1602	Assess potential lateral chromium migration
89 90	DMT-49US	Shallow Fill Upper Sand	Y	Perimeter, 1501/1602 Interior, West	Assess potential lateral chromium migration Assess potential vertical chromium migration
91	DMT-50US	Upper Sand	Υ	Perimeter, West	Assess potential vertical and lateral chromium migration
92 93	DMT-51US DMT-52US	Upper Sand Upper Sand	Y	Interior, Inside COPR Interior, Inside COPR	Assess potential vertical chromium migration Assess potential vertical chromium migration
94	DMT-53US	Upper Sand	N	Interior, Inside COPR	Assess vertical and lateral groundwater movement
95	DMT-54US	Upper Sand	Υ	Upgradient, East	Assess background groundwater quality

Table 2. Sentinel Well Network/Well Gauging

Qty	Well Name	Screened Unit	Sampled (Y/N)	Location	Rationale
96	DMT-64US	Upper Sand	Y	Perimeter, West	Assess potential vertical and lateral chromium migration
97	DMT-65US	Upper Sand	Y	Perimeter, West	Assess potential vertical and lateral chromium migration
98	DMT-67US	Upper Sand	Y	Perimeter, South	Assess potential vertical and lateral chromium migration
99	DMT-70US	Upper Sand	Υ	Perimeter, South	Assess potential vertical and lateral chromium migration
100	DMT-71US	Upper Sand	N	Interior, Outside COPR	Assess vertical and lateral groundwater movement
101	DMT-72US	Upper Sand	Υ	Perimeter, South	Assess potential vertical and lateral chromium migration
102	DMT-73US	Upper Sand	Υ	Perimeter, South	Assess potential vertical and lateral chromium migration
103	DMT-74US	Upper Sand	N	Upgradient, East	Assess upgradient groundwater movement
104	DMT-75US	Upper Sand	N	Upgradient, East	Assess upgradient groundwater movement
105	DMT-01M	Medium-Depth	Υ	Interior, Inside COPR	Assess potential vertical chromium migration
106	DMT-02M	Medium-Depth	N	Interior, Outside COPR	Assess vertical and lateral groundwater movement
107	DMT-34M	Medium-Depth	Υ	Perimeter	Assess potential lateral and vertical chromium migration
108	DMT-35M	Medium-Depth	N	Interior, Inside COPR	Assess vertical and lateral groundwater movement
109	DMT-36M	Medium-Depth	Υ	Interior, Inside COPR	Assess potential vertical chromium migration
110	DMT-37M	Medium-Depth	Υ	Interior, Inside COPR	Assess potential vertical chromium migration
111	DMT-38M	Medium-Depth	N	Upgradient, East	Assess upgradient groundwater movement
112	DMT-60M	Medium-Depth	Υ	Upgradient, East	Assess background groundwater quality
113	DMT-77M	Medium-Depth	Υ	Perimeter, South	Assess potential lateral and vertical chromium migration
114	DMT-78M	Medium-Depth	N	Interior, Outside COPR	Assess vertical and lateral groundwater movement
115	DMT-79M	Medium-Depth	N	Upgradient, East	Assess upgradient groundwater movement
116	DMT-80M	Medium-Depth	N	Upgradient, East	Assess upgradient groundwater movement
117	EA-02M	Medium-Depth	N	Interior, Inside COPR	Assess vertical and lateral groundwater movement
118	EA-06M	Medium-Depth	Υ	Interior, Inside COPR	Assess potential vertical chromium migration
119	EA-07M	Medium-Depth	Υ	Perimeter, 1501/1602	Assess potential lateral and vertical chromium migration
120	EA-08M	Medium-Depth	N	Interior, Inside COPR	Assess vertical and lateral groundwater movement
121	EA-10M	Medium-Depth	N	Interior, Outside COPR	Assess vertical and lateral groundwater movement
122	EA-11M	Medium-Depth	Υ	Perimeter, West South	Assess potential lateral and vertical chromium migration
123	EA-13M	Medium-Depth	N	Interior, Inside COPR	Assess vertical and lateral groundwater movement
124	EA-14M	Medium-Depth	N	Perimeter, South	Assess vertical and lateral groundwater movement
125	EA-15M	Medium-Depth	N	Interior, Inside COPR	Assess vertical and lateral groundwater movement
126	EAC-02M	Medium-Depth	N	Interior, Outside COPR	Assess vertical and lateral groundwater movement
127	EAC-03M	Medium-Depth	N	Interior, Inside COPR	Assess vertical and lateral groundwater movement
128	DMT-81D	Deep Well	Υ	Perimeter, South	Assess potential lateral and vertical chromium migration
129	DMT-82D	Deep Well	Υ	Upgradient, East	Assess background groundwater quality
130	DMT-83D	Deep Well	Y	Perimeter, West	Assess potential lateral and vertical chromium migration

Table 3. Sentinel Well Network/Well Sampling

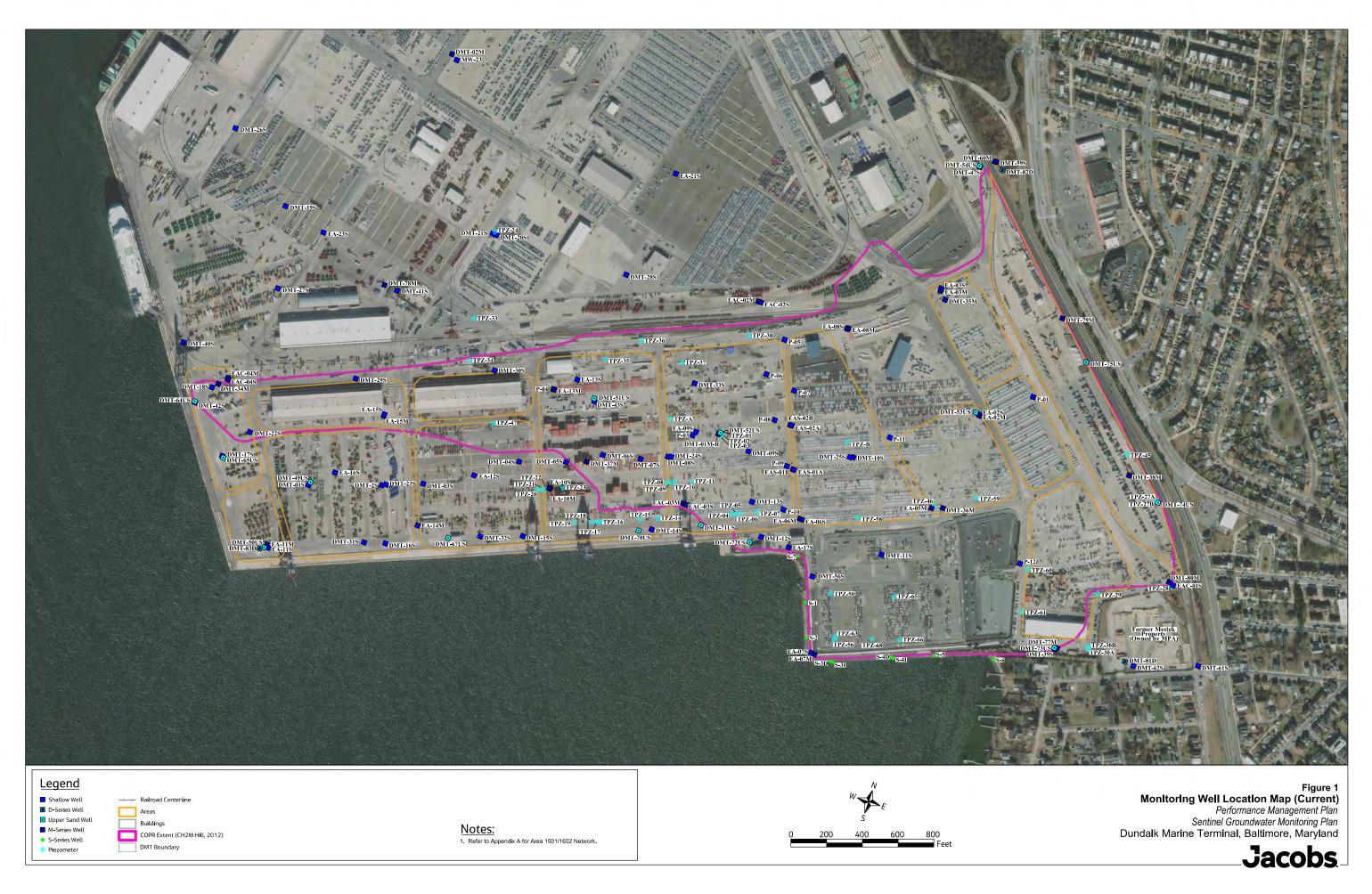
Qty	Well Name	Screened Unit	Sampled (Y/N)	Location
1	DMT-01S	Shallow Fill	Υ	Interior, Outside COPR
2	DMT-12S	Shallow Fill	Υ	Perimeter, South
3	DMT-14S	Shallow Fill	Υ	Perimeter, South
4	DMT-17S	Shallow Fill	Υ	Perimeter, West
5	DMT-27S	Shallow Fill	Υ	Interior, Outside COPR
6	DMT-28S	Shallow Fill	Υ	Interior, Outside COPR
7	DMT-31S	Shallow Fill	Υ	Perimeter, South
8	DMT-32S	Shallow Fill	Y	Perimeter, South
9	DMT-39S	Shallow Fill	Y	Perimeter, Outside COPR
10	DMT-41S	Shallow Fill	Y	Interior, Outside COPR
11	DMT-42S	Shallow Fill	Y	Perimeter, West
12	DMT-47S	Shallow Fill	Y	Upgradient, East
13	DMT-56S	Shallow Fill	Y	1501/1602 Below Liner
14	DMT-90S	Shallow Fill	Y	Perimeter, South
15	DMT-91S	Shallow Fill	Y	Perimeter, South
16	DMT-92S	Shallow Fill	Y	Interior, Outside COPR
17	EA-10S	Shallow Fill	Y	Interior, Outside COPR
18	EA-11S	Shallow Fill	Y	Perimeter, South
19	EA-118	Shallow Fill	Y	Interior, Outside COPR
20	EAC-01S	Shallow Fill	Y	Upgradient, East
21	S-1	Shallow Fill	Y	Perimeter, 1501/1602
22	S-2	Shallow Fill	Y	Perimeter, 1501/1602
23	S-3I	Shallow Fill	Y	Perimeter, 1501/1602
24	S-3D	Shallow Fill	Y	Perimeter, 1501/1602
25	S-4I	Shallow Fill	Y	Perimeter, 1501/1602
	S-4D	Shallow Fill	Y	
26 27	S-5	Shallow Fill	Y	Perimeter, 1501/1602
28	S-6	Shallow Fill	Y	Perimeter, 1501/1602 Perimeter, 1501/1602
29	S-7	Shallow Fill	Y	Perimeter, 1501/1602
30	DMT-49US	Upper Sand	Y	Interior, West
			Y	
31 32	DMT-50US DMT-51US	Upper Sand	Y	Perimeter, West
		Upper Sand	Y	Interior, Inside COPR
33	DMT-52US	Upper Sand	Y	Interior, Inside COPR
34	DMT-53US	Upper Sand	Y	Upgradient, East
35	DMT-64US	Upper Sand	Y	Perimeter, West
36	DMT-65US	Upper Sand	Y	Perimeter, West
37	DMT-67US	Upper Sand		Perimeter, South
38	DMT-70US	Upper Sand	Y	Perimeter, South
39	DMT-72US	Upper Sand	•	Perimeter, South
40	DMT-73US	Upper Sand	Y	Perimeter, South
41	DMT-01M	Medium-Depth	Y	Interior, Inside COPR
42	DMT-34M	Medium-Depth	Y	Perimeter
43	DMT-36M	Medium-Depth	Y	Interior, Inside COPR
44	DMT-37M	Medium-Depth	Y	Interior, Inside COPR
45	DMT-60M	Medium-Depth	Y	Upgradient, East
46	DMT-77M	Medium-Depth	Y	Perimeter, South
47	EA-06M	Medium-Depth	Y	Interior, Inside COPR
48	EA-07M	Medium-Depth	Y	Perimeter, 1501/1602
49	EA-11M	Medium-Depth	Y	Perimeter, West South
50	DMT-81D	Deep Well	Y	Perimeter, South
51	DMT-82D	Deep Well	Y	Upgradient, East
52	DMT-83D	Deep Well	Υ	Perimeter, West

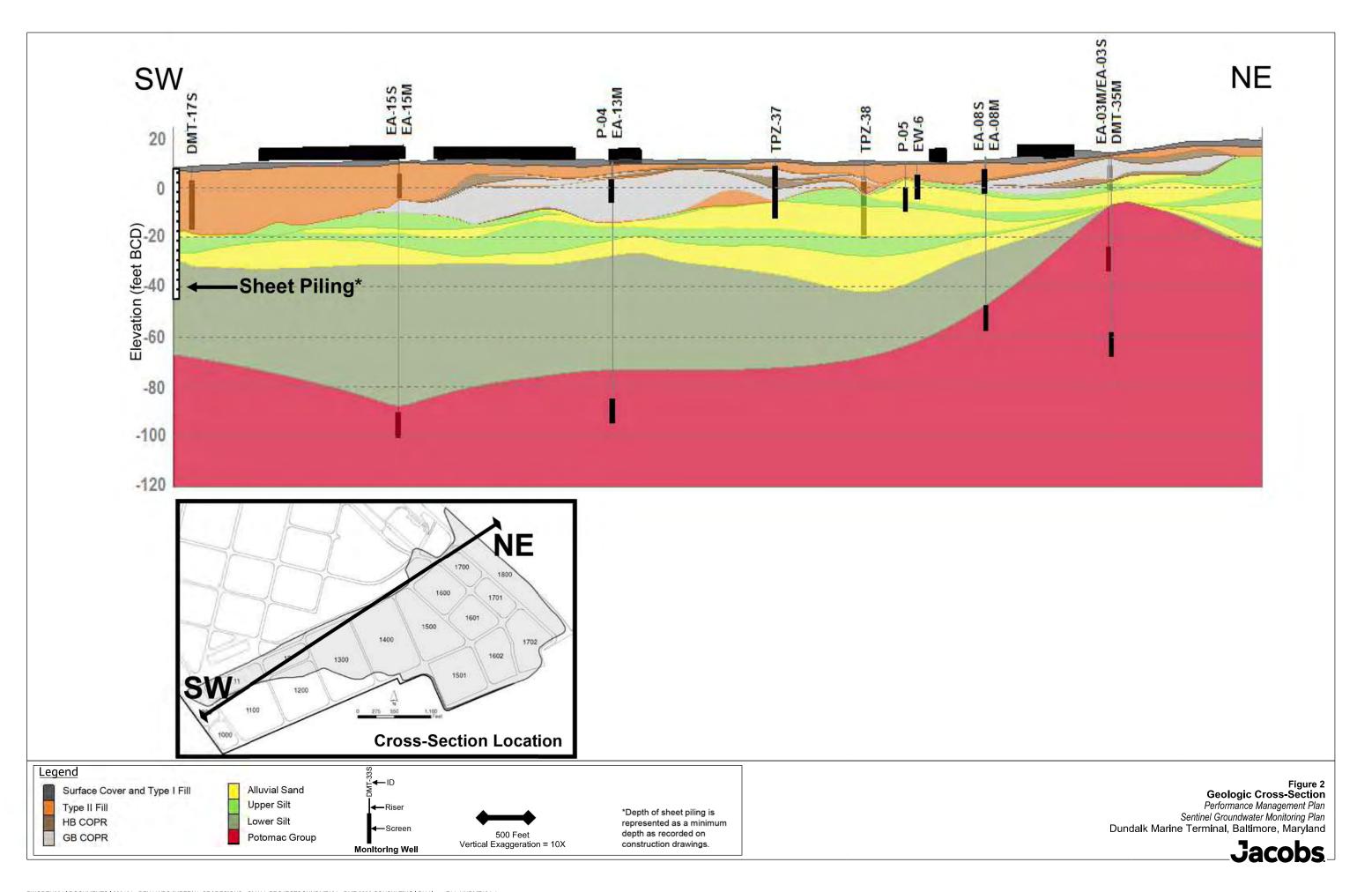
Rationale
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess background groundwater quality
Assess potential vertical chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess lateral groundwater movement
Assess potential lateral chromium migration
Assess lateral groundwater movement
Assess background groundwater quality
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential lateral chromium migration
Assess potential vertical chromium migration
Assess potential vertical and lateral chromium migration
Assess potential vertical chromium migration
Assess potential vertical chromium migration
Assess potential vertical chromium migration
Assess potential vertical and lateral chromium migration
Assess potential vertical and lateral chromium migration
Assess potential vertical and lateral chromium migration
Assess potential vertical and lateral chromium migration
Assess potential vertical and lateral chromium migration
Assess potential vertical and lateral chromium migration
Assess potential vertical chromium migration
Assess potential lateral and vertical chromium migration
Assess potential vertical chromium migration
Assess potential vertical chromium migration
Assess background groundwater quality
Assess potential lateral and vertical chromium migration
Assess potential vertical chromium migration
Assess potential lateral and vertical chromium migration
Assess potential lateral and vertical chromium migration
Assess potential lateral and vertical chromium migration
Assess background groundwater quality
Assess potential lateral and vertical chromium migration

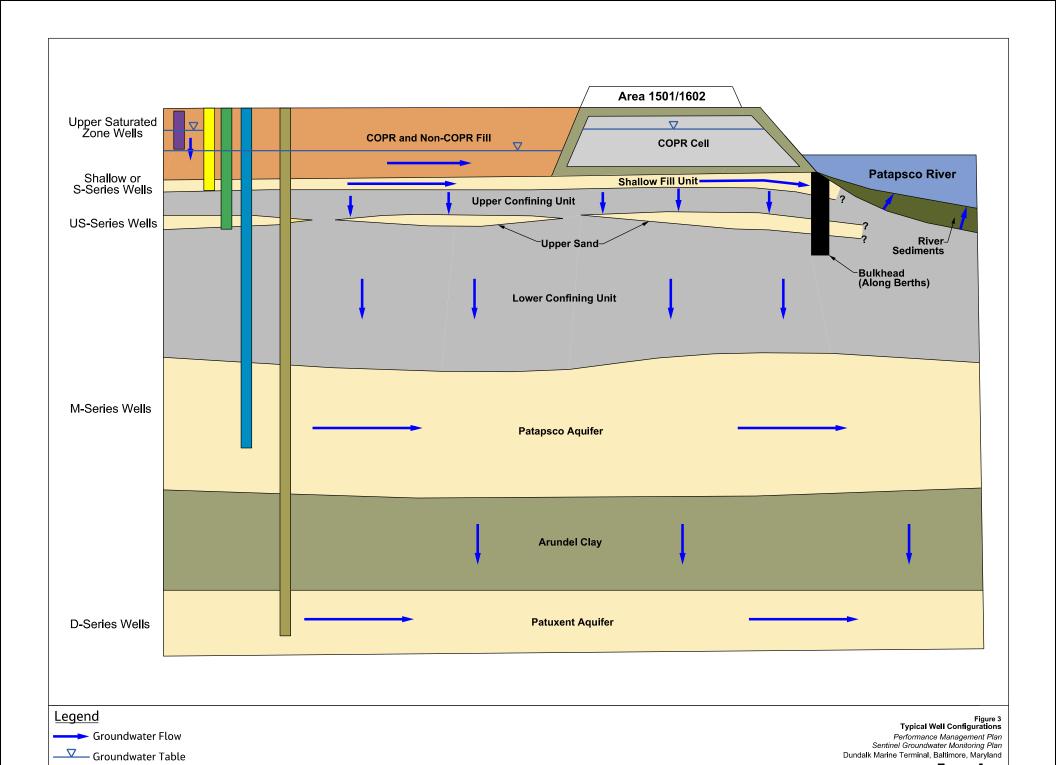
Table 4. Sentinel Well Network/Well Abandonment

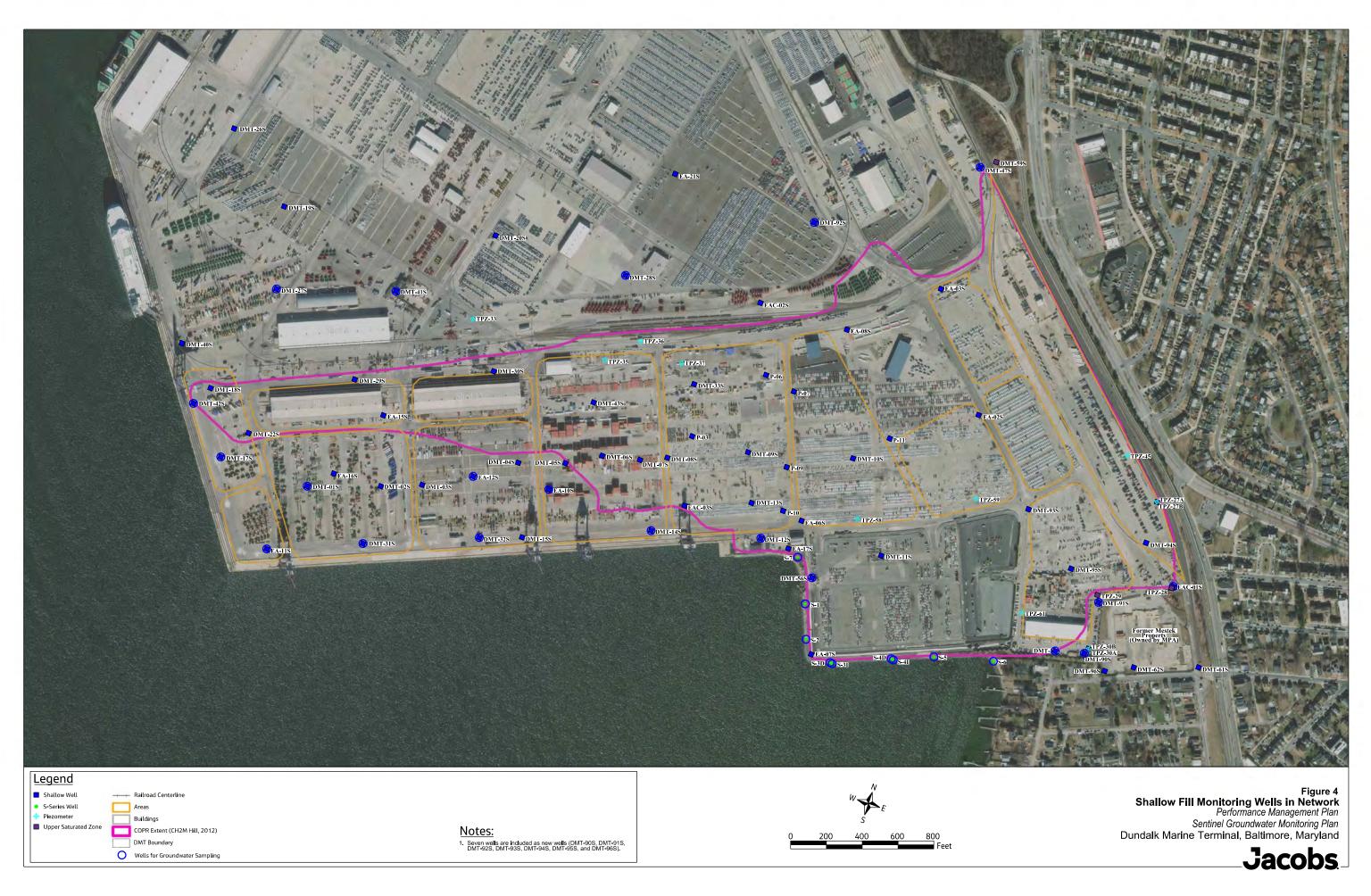
Qty	Well Name	Screened Unit	Location	Rationale
1	DMT-21S	Shallow Fill	Adjacent to DMT-20S	Duplicative
2	DMT-23S	Shallow Fill	Adjacent to DMT-2S	Duplicative
3	DMT-24S	Shallow Fill	Adjacent to DMT-8S	Duplicative
4	DMT-25S	Shallow Fill	Adjacent to DMT-10S	Duplicative
5	DMT-59S	Shallow Fill	Adjacent to DMT-47S	Duplicative
6	TPZ-A	Shallow Fill	Adjacent to EA-9S	Duplicative
7	TPZ-B	Shallow Fill	Between DMT-8S and EA-3S	Duplicative
8	TPZ-01	Shallow Fill	Near DMT-9S	Duplicative
9	TPZ-02	Shallow Fill	Near DMT-9S	Duplicative
10	TPZ-03	Shallow Fill	Near DMT-9S	Duplicative
11	TPZ-04	Shallow Fill	Near DMT-13S	Duplicative
12	TPZ-05	Shallow Fill	Near DMT-13S	Duplicative
13	TPZ-06	Shallow Fill	Near DMT-13S	Duplicative
14	TPZ-07	Shallow Fill	Near DMT-13S	Duplicative
15	TPZ-08	Shallow Fill	Between DMT-8S and EA-3S	Duplicative
16	TPZ-09	Shallow Fill	Between DMT-8S and EA-3S	Duplicative
17	TPZ-10	Shallow Fill	Between DMT-8S and EA-3S	Duplicative
18	TPZ-11	Shallow Fill	Between DMT-8S and EA-3S	Duplicative
19	TPZ-14	Shallow Fill	Near DMT-14S	Duplicative
20	TPZ-15	Shallow Fill	Near DMT-14S	Duplicative
21	TPZ-16	Shallow Fill	Between DMT-14S and DMT-15S	Duplicative
22	TPZ-17	Shallow Fill	Between DMT-14S and DMT-15S	Duplicative
23	TPZ-18	Shallow Fill	Between DMT-14S and DMT-15S	Duplicative
24	TPZ-19	Shallow Fill	Between DMT-14S and DMT-15S	Duplicative
25	TPZ-20	Shallow Fill	Adjacent to EA-10S	Duplicative
26	TPZ-21	Shallow Fill	Adjacent to EA-10S	Duplicative
27	TPZ-22	Shallow Fill	Adjacent to EA-10S	Duplicative
28	TPZ-23	Shallow Fill	Adjacent to EA-10S	Duplicative
29	TPZ-24	Shallow Fill	Adjacent to DMT-20S	Duplicative
30	TPZ-34	Shallow Fill	Adjacent to DMT-30S	Duplicative
31	TPZ-38	Shallow Fill	Between EAC-2S and P-06	Duplicative
32	TPZ-46	Shallow Fill	Near DMT-36M	Duplicative
33	TPZ-47	Shallow Fill	Between DMT-30S and DMT-4S	Duplicative
34	TPZ-60	Upper Saturated Zone	Near P-12	Duplicative with new wells
35	EA-09S	Shallow Fill	Near P-3	Interior, Inside COPR
36	EA-13S	Shallow Fill	Near DMT-43S	Duplicative
37	EA-23S	Shallow Fill	Near DMT-19S	Duplicative
38	EAS-01A	Shallow Fill	Near former EW	Used for Aquifer Test
39	EAS-01B	Shallow Fill	Near former EW	Used for Aquifer Test
40	EAS-02A	Shallow Fill	Near former EW	Used for Aquifer Test
41	EAS-02B	Shallow Fill	Near former EW	Used for Aquifer Test
42	EAC-04S	Shallow Fill	Adjacent to DMT-18S	Duplicative
43	MW-23	Shallow Fill	Far North Well	Distant Well
44	P-01	Shallow Fill	Near EA-2S	Duplicative
45	P-04	Shallow Fill	Near DMT-43S	Duplicative
46	P-05	Shallow Fill	Near P-6	Duplicative
47	P-08	Shallow Fill	Near P-7	Duplicative
48	P-12	Upper Saturated Zone		Duplicative with new wells
49	EA-03M	Medium-Depth	Interior, Inside COPR	Duplicative
50	EA-05M	Medium-Depth	Interior, Inside COPR	Duplicative
51	EAC-04M	Medium-Depth	Perimeter, West	Duplicative

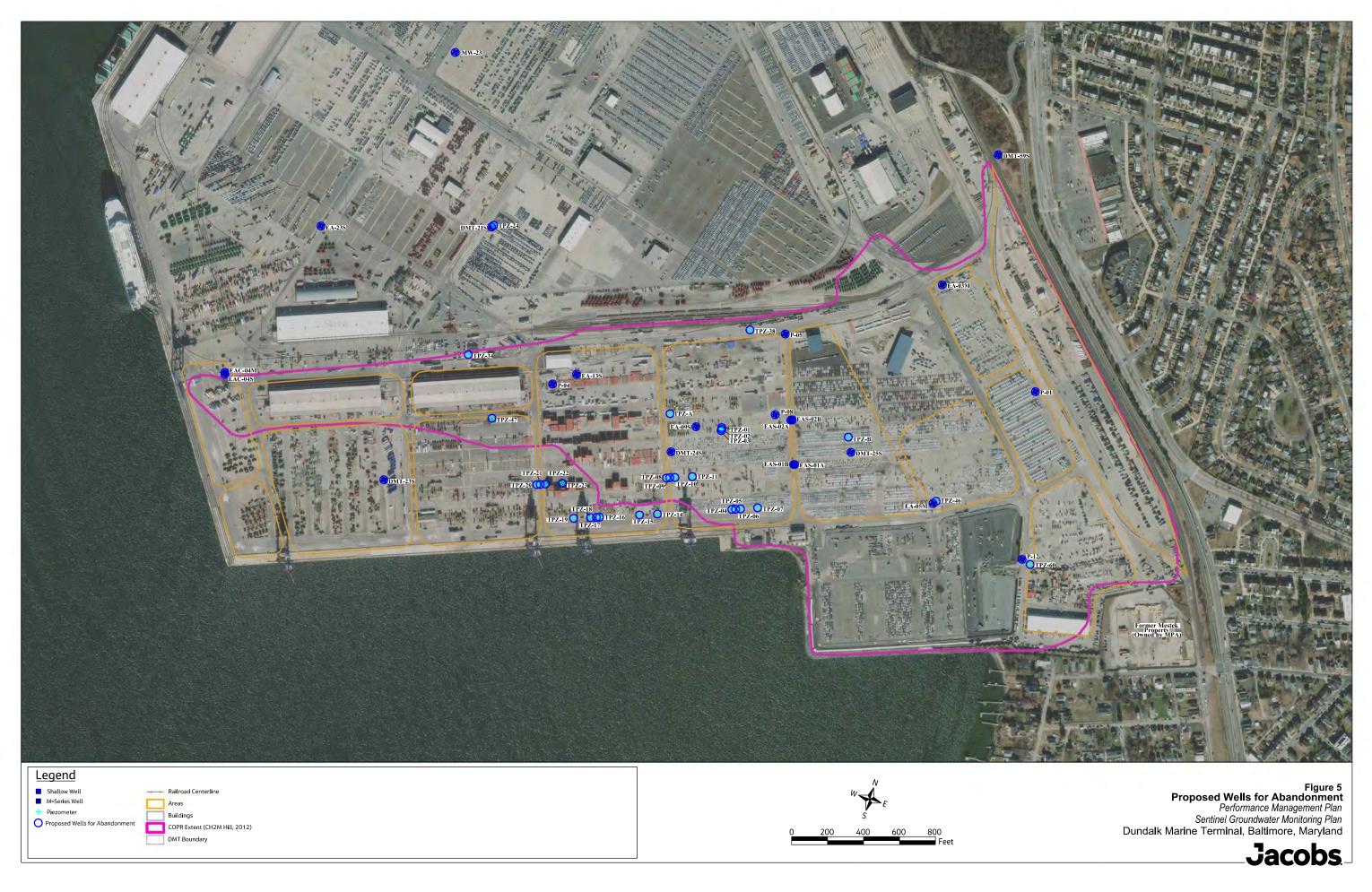
Figures

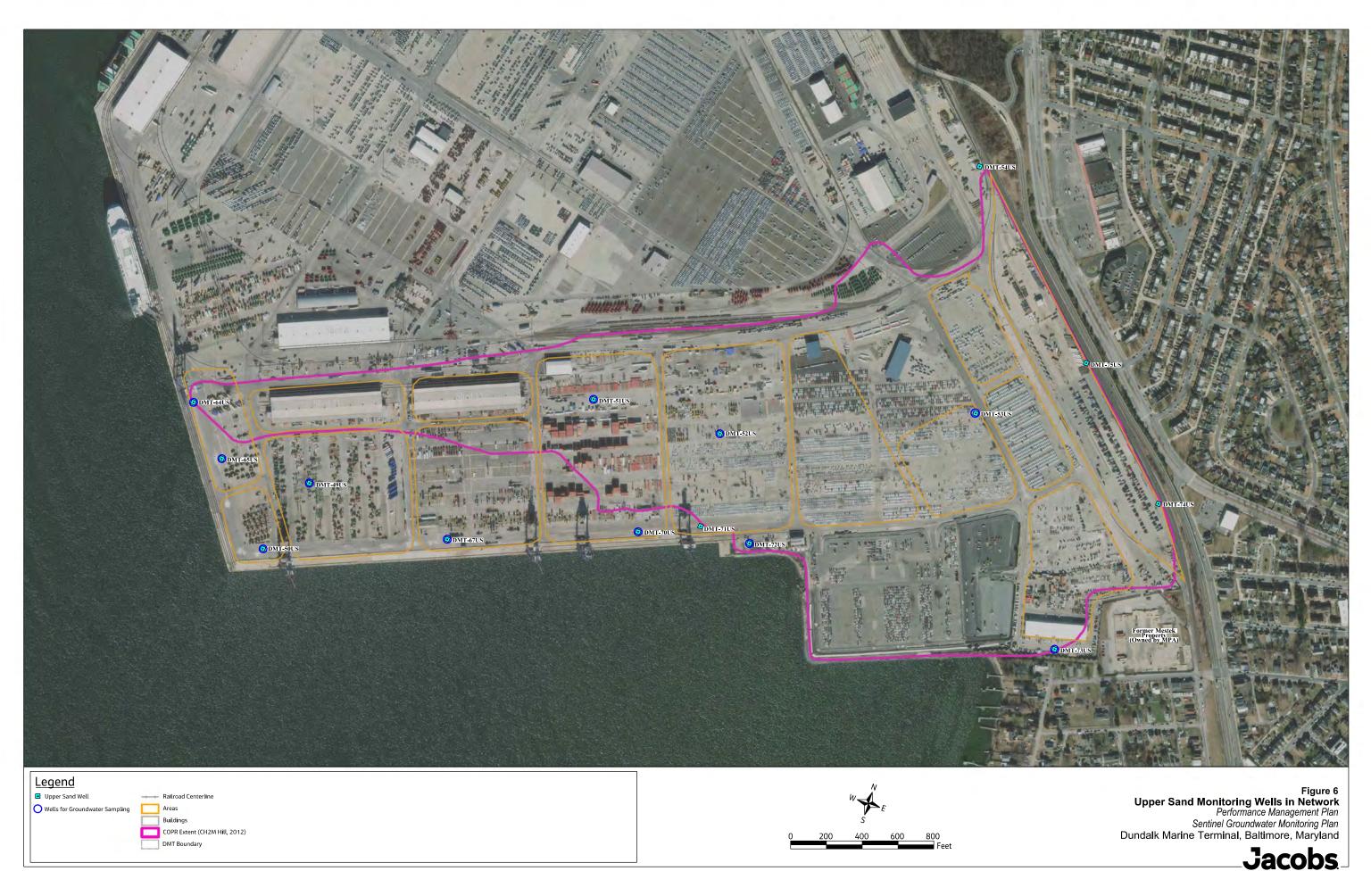


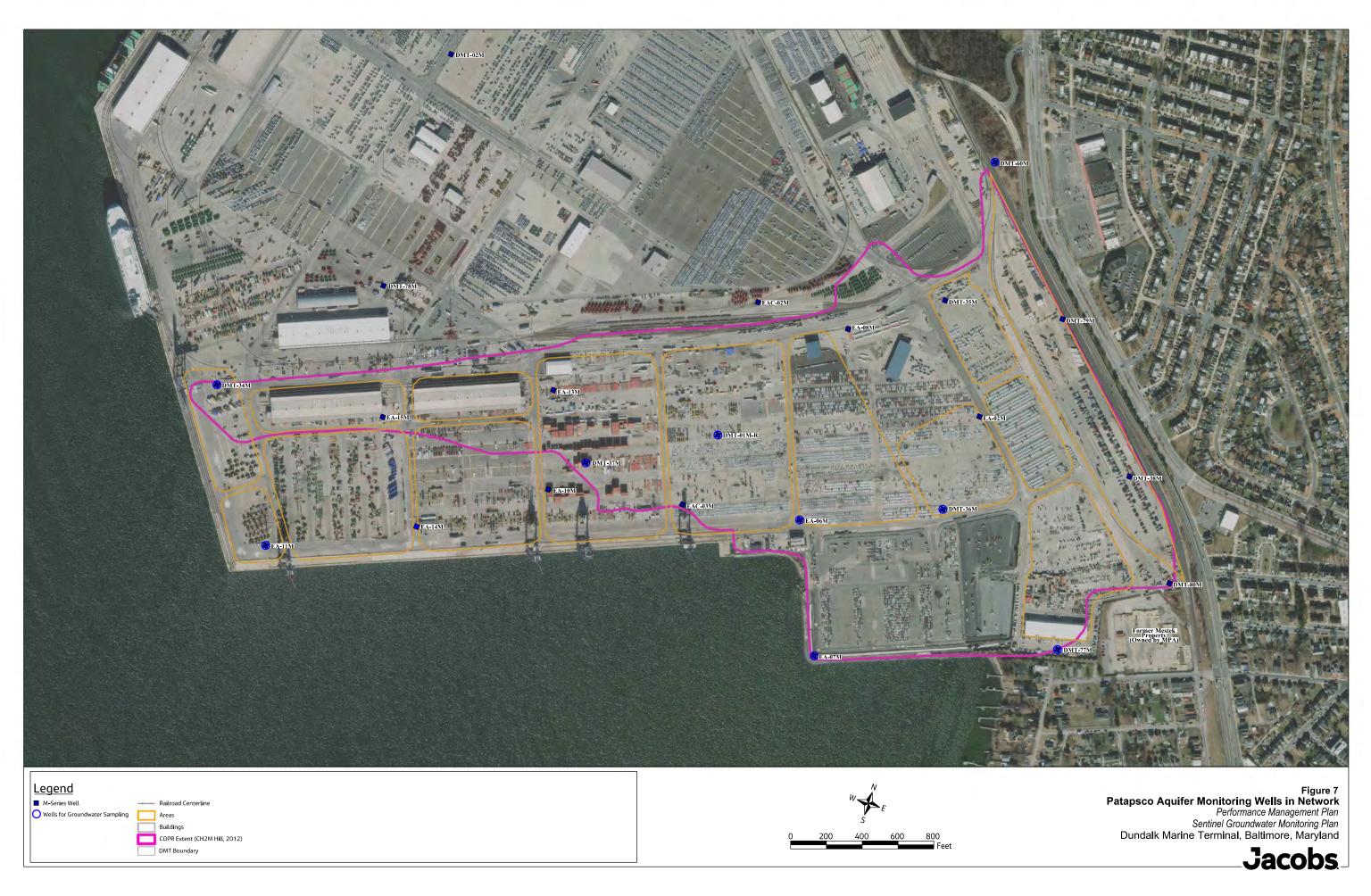


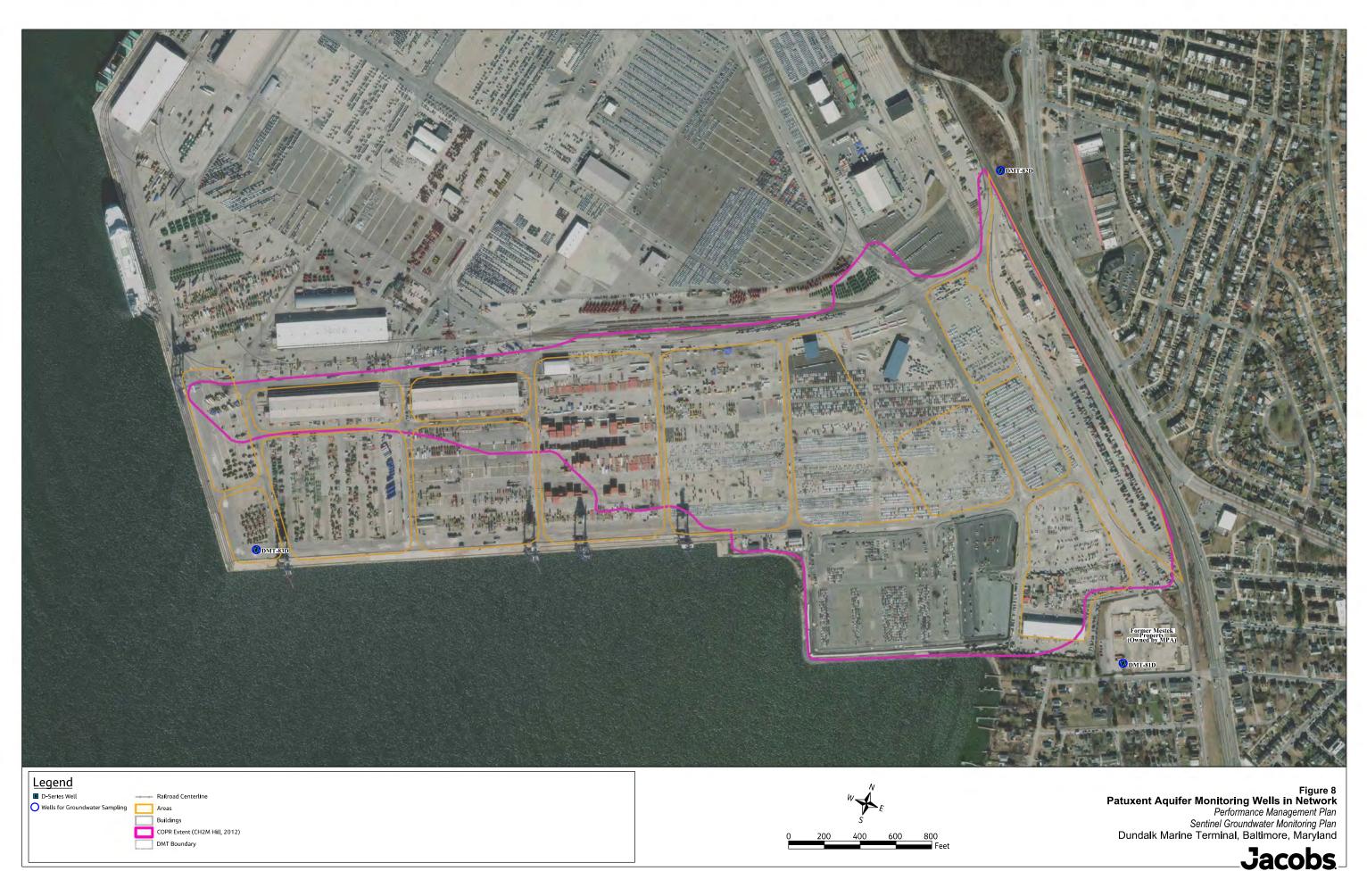


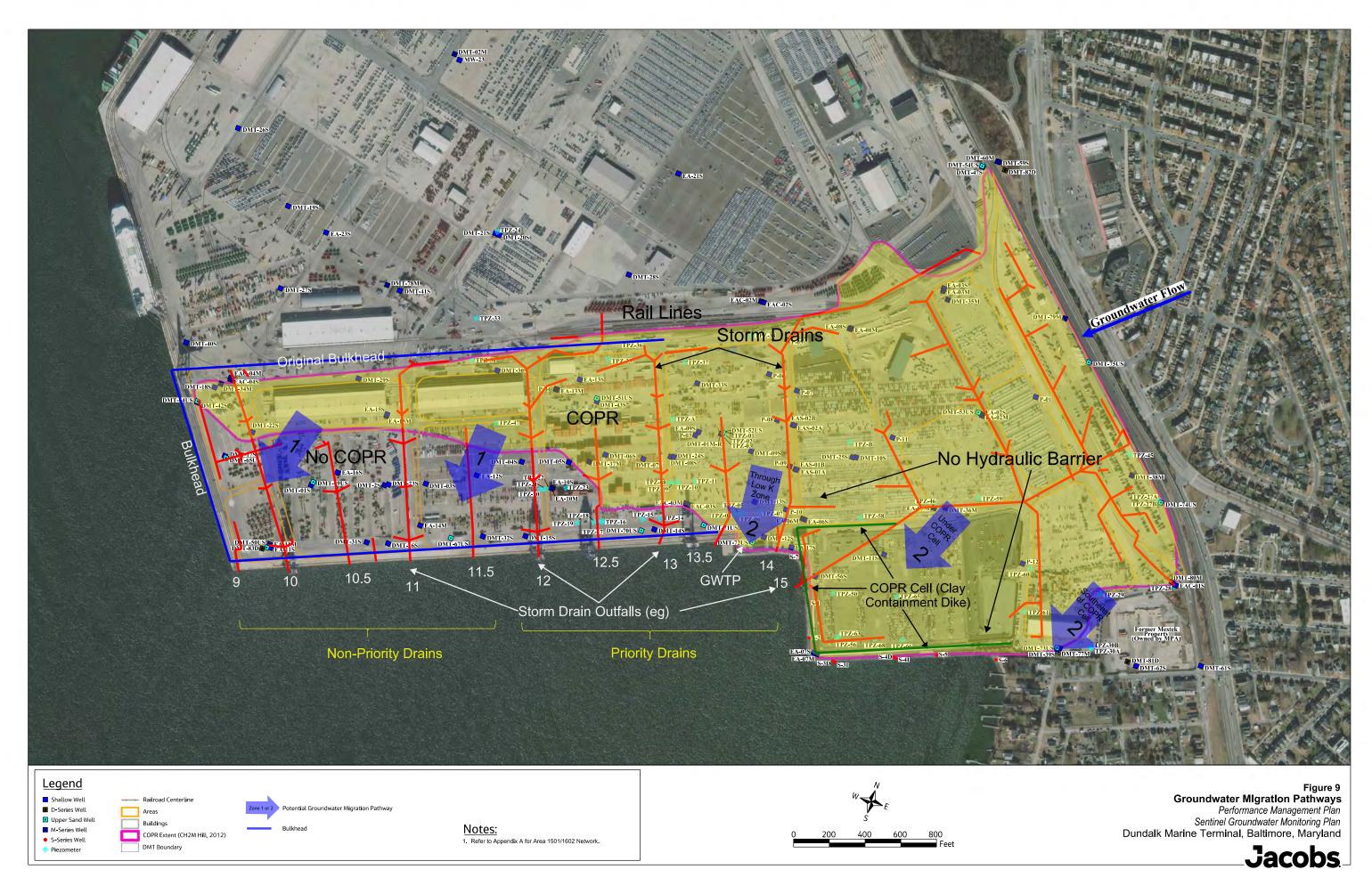












Appendix A Sentinel Well Location Plan for Area 1501/1602



HEALTH, SAFETY, ENVIRONMENTAL, PRODUCT STEWARDSHIP AND SUSTAINABILITY 115 Tabor Road, 4-D4

Morris Plains, New Jersey 07950 www.honeywell.com

March 5, 2021

Ms. Irena Rybak Administrator, Land Restoration Program Maryland Department of the Environment 1800 Washington Boulevard, Suite 645 Baltimore, MD 21230-1719

Re: Sentinel Well Monitoring Plan for Area 1501/1602, Dundalk Marine Terminal, Baltimore, Maryland

Dear Ms. Rybak:

Honeywell International Inc. (Honeywell) and the Maryland Port Administration (MPA) are submitting the enclosed Sentinel Well Monitoring Plan for Area 1501/1602 for the Dundalk Marine Terminal, in Baltimore, Maryland.

This report documents the approach for establishing the sentinel monitoring system, based on an evaluation of the existing network of monitoring wells located within Area 1501/1602.

If you have any questions or require additional information, please contact me at 973-216-7501.

Very truly yours,

Maria Kaouris

Remediation Manager

Enclosures

cc: Mr. Matthew Zimmerman /MDE

Maria Kanui

Mr. Bill Richardson/MPA Mr. Robert Munroe/MPA

Mr. Michael Daneker/Arnold & Porter

Sentinel Monitoring Plan Area 1501/1602

Dundalk Marine Terminal Baltimore, Maryland

Prepared for



115 Tabor Road Morris Plains, New Jersey 07950

Prepared by



Jacobs Engineering 2411 Dulles Corner Park Suite #500 Herndon, VA 20171

March 2021

Contents

1.	Introduction1.1Site Background1.21.22017 Supplemental Site Investigation2.21.32018 Predesign Investigation2.21.4Existing Monitoring Wells3.3	1 2 2
2.	Sentinel Monitoring Program	3 4
3.	Sentinel Program Summary	5
4.	References	5
Append	dixes	
A B	Soil Boring Logs for Wells To Be Abandoned Soil Boring Logs for Wells To Be Retained	
Tables		
1 2 3 4 5 6 7	List of Wells and Geotechnical Points Wells for Hydraulic Assessment Monitoring Perimeter Sentinel Wells Interior Monitoring Wells Vertical Sentinel Wells Movement Monitoring Wells Wells and Inclinometers for Abandonment	
Figures	S	
1 2 3 4 5 6 7 8 9 10 11 12 13	Site Map Typical Well Configurations Hydraulic Assessment Wells Sentinel Perimeter Wells Interior Observation Wells Vertical Sentinel Wells Cross Section Lines A-A', B-B', C-C', D-D', E-E', F-F', and G-G' Cross Section A-A' Cross Section B-B' Cross Section C-C' Cross Section D-D' Cross Sections E-E', F-F', and G-G' Inclinometer and Shape Accel Array Wells Inclinometer and Monitoring Wells for Abandonment	

1. Introduction

Honeywell International Inc. and the Maryland Port Administration are submitting this Sentinel Monitoring Plan for Area 1501/1602 at the Dundalk Marine Terminal (DMT), located in Baltimore, Maryland. The sentinel monitoring program is a requirement of the Corrective Measures Alternatives Analysis report (CH2M, 2011), which was approved by the Maryland Department of Environment (MDE) on July 30, 2012, as part of the requirements of the Consent Decree dated April 5, 2006.

Under Alternative 3, Enhanced Isolation and Containment, the selected remedial alternative, the chromium ore processing residue (COPR) containment system at DMT is to be maintained and the surface cover improved over the long term while contaminated groundwater is prevented from infiltrating into the priority storm drains within the COPR fill area. This alternative also requires the preparation of a Program Management Plan (PMP) at the completion of remedy implementation to provide a comprehensive framework for maintaining remedy performance and remedy protection in the future. The PMP is an observational-based process that represents a formalized and comprehensive approach for identifying, evaluating, and implementing contingent corrective measures. The PMP is intended to link monitoring and maintenance programs so that if a trigger is activated, an evaluation of additional corrective measures or enhancement of existing measures is initiated and implemented.

One of the elements of the PMP is the establishment of a sentinel groundwater monitoring network inland of the shoreline perimeter to measure groundwater flow and quality at the property boundary. The approach for establishing the sentinel monitoring system, based on an evaluation of the existing network of monitoring wells installed in Area 1501/1602, is described herein. Additionally, inclinometers and SAAs are in-place for monitoring COPR horizontal and vertical movement. These structures were also evaluated to ensure that adequate movement monitoring data will be collected for ongoing surface cover evaluation and maintenance efforts. The draft Area 1501 1602 Sentinel Monitoring Plan was submitted to MDE on September 11, 2020. This plan incorporates MDE review comments.

Only the sentinel and movement monitoring network for Area 1501/1602 is presented herein prior to implementing the containment remedy for Area 1501/1602, including the installation of the Service Road Strain Relief Trench (SRT) and the placement of an enhanced containment cap over the 21-acre Site, which will be performed during 2021 and 2022. The sentinel well network for the remainder of the COPR fill area inland from Area 1501/1602 will be developed once the containment cap has been installed.

1.1 Site Background

Area 1501/1602 encompasses approximately 21 acres in the southeast corner of DMT. The western and southern boundaries of Area 1501/1602 abut the Patapsco River. Much of this area was reclaimed from the Patapsco River channel in two phases of containment cell construction, for which COPR was used as fill, which occurred from about 1974 through 1982. Select borrow material consisting of a mixture of sand, silt, and clay was placed over the native silt and sand at the river's edge to an elevation of about 2 feet above the mean low-water mark and then was capped with a 1- to 2-foot layer of impervious borrow to create a clay liner. Above the clay liner, a clay containment dike was placed and graded to a 2:1 slope at the shoreline edges of Area 1501/1602, and COPR material was placed within the dike limits. Following placement in the containment cell, the COPR material was covered with a 1-foot-thick impervious layer (clay cap).

Containment measures were included in the design and construction of the cell to mitigate stormwater infiltration and to contain the COPR. An asphalt cap was placed over the clay liner and cap referenced above to minimize stormwater intrusion into the cells, thereby reducing the potential for transport of dissolved COPR constituents. A third, final phase of construction occurred across Area 1501/1602 in 1999–2002. During this final phase, the grade of Area 1501/1602 was raised by 7 feet through the placement of surcharge material intended to suppress COPR heave that had previously rendered the area unusable for port cargo storage operations. The surcharge fill was then covered with a 1-foot-thick layer of graded aggregate and 3 inches of surface course asphalt.

1



Along the sloped edge of the western and southern shorelines, gabion baskets filled with coarse stone were placed. More recently, additional riprap and jersey barriers were placed along portions of both the western and southern shorelines to reduce the potential for shoreline erosion.

SRTs were installed as two separate locations within Area 1501/1602 to address COPR lateral movement. One SRT was placed adjacent to the 15th Street drain located at the northern portion of Area 1501/1602. This SRT was installed in two sections: the first 200 feet in 2002, and the second, 380-foot section, in 2011. The second SRT was placed near the sound wall, which is approximately 80 feet in length, was installed in 2008 at southeast portion of Area 1501/1602. Additionally, a third SRT will be installed along the service road on the western and southern sides Area 1501/1602 as part of the capping of Area 1501/1602.

1.2 2017 Supplemental Site Investigation

A Supplemental Site Investigation (SSI) was performed in 2017 (CH2M, 2017) as a focused subsurface investigation of Area 1501/1602. This investigation supplemented previous investigations conducted at the Site—including the COPR Investigation, conducted between October 2006 and December 2008 (CH2M, 2009), and the *Heave Investigation and Minimization Study* (CH2M and Geosyntec, 2009), as updated through surface and subsurface displacement data from MRCE (2013–2020)—and is intended to characterize the subsurface conditions of this former COPR fill area.

Specifically, the purpose of this additional phase of investigation was to further evaluate the hydrogeology and geotechnical characteristics of the COPR cell by obtaining stratigraphic, hydrogeologic, and lateral subsurface and surface movement data to supplement the prior investigations conducted at the Site and to expand the heave monitoring and survey network. The findings from the SSI and data obtained from the installed instrumentation were used to update the site conceptual model (CSM) and further assess the effectiveness of the isolation and containment of the COPR fill. Additionally, data from the SSI were used to determine the need for any additional investigations for maintenance of the containment dike, cover system, stormwater management features, or other components of the Area 1501/1602 COPR containment cell, as further discussed in Section 1.3.

Field activities included drilling 101 soil borings and installing a network of subsurface instrumentation including vibrating wire piezometers, thermistors, Shape Accel Arrays (SAAs), and inclinometers in some of the borings. No borings drilled within the COPR cell penetrated the underlying basal clay liner. Initial findings of the investigation indicated the need for continued collection and analysis of data from the instruments installed in the SSI over a period of one or more years to enable further conclusions to be drawn regarding the COPR containment cell properties and behaviors. The results were also used to develop a CSM, which is a PMP post-closure care requirement.

1.3 2018 Predesign Investigation

In 2018, a geotechnical and groundwater Predesign Investigation was performed within Area 1501/1602 which included an uplands geotechnical investigation to evaluate the existing conditions and stability of the containment cell and a groundwater investigation along both the western and southern shorelines, which abut the Patapsco River, to evaluate the containment effectiveness of the existing clay containment dike. The *Predesign Geotechnical and Groundwater Investigation Work Plan* (Jacobs, 2018) was submitted to the MDE in June 2018.

The groundwater investigation consisted of installing well points at seven locations along the Area 1501/1602 western and southern shorelines to collect groundwater samples to assess whether the existing perimeter dike is providing adequate groundwater containment. The well points were installed outboard of the COPR containment cell to an elevation that is below the clay liner within the COPR cell. The seven well point locations (S-1 to S-7) along the shoreline were selected based on accessibility and coverage of the shoreline (Figure 1). The geotechnical investigation consisted of installing shallow and deep borings in the upland portions of Area 1501/1602, to evaluate the stability of the Area 1501/1602 containment cell in support of the development of a final containment remedy (i.e., enhanced containment cap). The locations of the geotechnical borings (SL-1 to SL-10) are illustrated in Figure 1.



The installation and sampling of well points along the southern and western shorelines was successful in evaluating the effectiveness of the existing containment dike. Groundwater elevation differences between the well points along the shoreline and the wells within the containment cell confirm the competency of the clay liner and clay dike in restricting groundwater flow beyond the limits of the containment cell. The wells installed along the western and southern shoreline will be used as part of the sentinel groundwater monitoring program.

The geotechnical investigation evaluated existing stability conditions for the containment cell and the stability of the remedy condition involving the addition of 2 feet of fill. Global stability analyses determined that placement of an addition of 2 feet of fill over the existing "surcharge" area of 1501/1602 will not adversely affect global stability. Additionally, the installation of a shoreline SRT along the existing service road will not reduce shoreline stability, provided the SRT alignment is sufficiently set back from the shoreline slope at a minimum of 14 feet inland from the crest of the shoreline slope.

1.4 Existing Monitoring Wells

As described on the previous sections, numerous investigations have been performed within the Area 1501/1602 boundaries since 1986, resulting in the existing 123 subsurface monitoring installations, including 105 groundwater monitoring wells and 18 geotechnical inclinometers, as illustrated on Figure 1. This network of existing wells and inclinometers were evaluated for inclusion in the sentinel monitoring program.

The monitoring wells were completed with either a vibrating wire piezometer grouted into the well bore or a section of well screen and casing. Typical configurations of the wells are shown on an idealized block diagram in Figure 2, which shows the vertical positioning of the wells. As such, there are wells that monitor the groundwater elevation above the COPR cell clay cap, within the COPR cell, and below the clay liner. The lateral locations of the wells extend from the center of the COPR cell to locations adjacent to the edges of the cell boundary, and beyond the cell boundary toward the Patapsco River. A table presenting the construction details for all the existing wells is provided as Table 1.

2. Sentinel Monitoring Program

An effective sentinel monitoring program would include wells above the clay cap, wells within the COPR cell, and below the clay liner to monitor groundwater levels, and inclinometers and SAAs to monitor subsurface ground movement. As such the following components of a monitoring program:

- Hydraulic assessment monitoring
- Sentinel groundwater monitoring
- COPR Movement monitoring

Existing wells and geotechnical locations were evaluated to determine which locations are suitable for continued use as part of a sentinel monitoring program. The wells and geotechnical locations deemed unsuitable or duplicative for monitoring were identified for abandonment. Table 1 provides a summary of the wells, the vertical location where the well monitors (i.e., above the clay cap, within the COPR cell, and below the clay liner), and a proposed use for the well, including abandonment.

As noted in Table 1, several wells provide data for multiple intervals and may also have a SAA installed. When considering wells to include, preference was given to these wells as they would provide additional data at one location. No new wells were considered necessary, as the existing wells at the Site are more than adequate to develop an effective monitoring program.

2.1 Hydraulic Assessment Monitoring

Hydraulic assessment monitoring focuses on monitoring water levels above the clay cap and in wells located adjacent to the 15th Street drain. A total of 21 monitoring wells were selected for the hydraulic



assessment, as detailed in Table 2. Five wells that are either screened above the clay cap or screened below the clay cap adjacent to the 15th Street drain were identified as duplicative or unnecessary for the monitoring network. These wells will be abandoned prior to installation of the Service Road SRT and enhanced containment cap over the next 2 years.

The water levels screened above the clay cap will evaluate the potential for surface water infiltration through the enhanced containment cap to be installed across Area 1501/1602 during 2021 and 2022. Water levels in wells adjacent to the 15th Street drain will be used to assess the potential presence of water within the COPR cell that could be from water exfiltration from the 15th Street storm drain.

Thirteen wells that monitor water levels above the clay cap were selected for hydraulic assessment monitoring (Table 2), as illustrated on Figure 3. These wells are located preferentially near the southwestern portion of the Site where the clay cap naturally slopes toward the shoreline.

Eight wells screened within the COPR cell and located adjacent to the 15th Street drain were selected as sentinel wells (Table 2) and shown on Figure 3. These wells are approximately equidistant along either side of the 15th Street storm drain. Groundwater elevation data from these wells would be used to prepare groundwater contour maps and periodic time-series graphs to assess the potential for surface water infiltration and to evaluate the integrity of the COPR cell adjacent to the 15th Street drain.

2.2 Sentinel Groundwater Monitoring

The sentinel groundwater monitoring network include wells screened within the COPR cell, along the perimeter of the cell, interior of the cell, and wells below the clay liner. Twenty-eight wells were selected (Table 3) along the perimeter of the COPR cell, as illustrated on Figure 4. These well locations represent an approximate equal spatial distribution throughout the COPR cell.

For monitoring the interior of the COPR cell, 10 wells were selected based on an even distribution of wells across the area (Table 4) as shown on Figure 5. Groundwater elevation data from these observation wells will be used in conjunction with the perimeter monitoring wells to generate a groundwater flow maps to confirm the hydraulic containment of the COPR cell and to assess the potential for groundwater flow within the cell.

Eighteen wells screened below the clay liner that monitor groundwater were selected to provide verification of vertical hydraulic containment of the COPR cell (Table 5). The location of these wells is shown on Figure 6. No existing wells that monitor water level below the clay liner were considered duplicative, and as such none are targeted for abandonment.

A total of 56 monitoring wells were selected to represent an effective sentinel groundwater monitoring network to assess the lateral and vertical hydraulic containment of the COPR cell. A total of 32 wells screened within the COPR cell were identified as duplicative or unnecessary for the monitoring network and these wells are targeted for abandonment.

Geologic cross-sections were constructed to illustrate the vertical positioning of the monitored zones for selected wells. The six cross-section locations are shown on Figure 7 and the sections A-A', B-B', C-C', D-D', E-E', F-F', and G-G' are included as Figures 8 through 12. Cross-section A-A' shows the details for the screened interval relative to the subsurface geologic materials. From this figure, the position of wells that monitor water levels above the clay cap, within the COPR cell, and below the clay liner are illustrated. Cross-section B-B' was included to show the position of the deepest well (EA-7M) relative to most of the wells. On this cross-section, there is a shallow sand unit that is monitored by the S-series wells (represented by S-2), and a thick sequence of low-permeability clayey silt separates this shallow sand from the deeper Patapsco sand that is monitored by the EA-7M well. The distribution of geologic materials indicates that the shallower sand would be the primary flow path to respond were the COPR cell integrity compromised. The remaining cross-sections illustrate the geologic conditions across the 1501/1602 Area in various orientations.



2.3 COPR Movement Monitoring

Monitoring of the surface and subsurface lateral displacement of COPR within the 148-acre fill area is performed under the 2006 DMT Consent Order. The movement monitoring results were reported to MDE in the *Heave Investigation and Minimization Study* (HIMS) (CH2M and Geosyntec, 2009), which has been updated since 2013 in a series of supplemental data reports. The 2009 HIMS report provided a detailed explanation of COPR mineralogy, chemical expansion mechanisms, physical manifestations of subsurface COPR expansion, and heave mitigation measures. The first update to this report was issued in 2013 as *Heave Investigation and Minimization Study Supplemental Data Report #1* (MRCE, 2013), and was followed by updates in 2014, 2015, 2016, 2017, 2018, and 2020 (MRCE, 2014–2018, 2020). This movement monitoring and reporting will continue, as an element of the PMP, after the completion of the containment remedy.

Lateral subsurface movement within and below the COPR cell are monitored using inclinometers and SAAs (Table 6). Thirteen existing inclinometers have reached the end of their usefulness based on deformation and will be abandoned. However, the remaining five inclinometers and 22 SAA locations are situated at locations shown on Figure 13 and depths suitable for continued subsurface monitoring. It is noted that five locations (DL-1-SAA, BRP-6, BRP-12, GP-7A, and GP-7C) are used solely for their SAA information. In addition, the existing Sound Wall and 15th Street Drain SRTs have numerous ports to assess the potential lateral COPR expansion and will remain in service.

3. Sentinel Program Summary

The evaluation of the network of existing groundwater wells has identified 68 well locations and five inclinometers are suitable to establish an effective sentinel monitoring program that would include hydraulic assessment and perimeter monitoring to confirm the lateral and vertical hydraulic containment of the COPR cell and to monitor lateral movement. Additionally, 37 well locations and 13 inclinometers are either duplicative or have reached the end of their usefulness and are selected for abandonment (Table 7). These locations are illustrated on Figure 14. The soil boring logs for wells that are planned for abandonment are presented in Appendix A, and those for wells to be retained are in Appendix B.

The abandonment of wells will be coordinated with the planned installation of the Service Road SRT during 2021 and construction of the enhanced containment cap in a phased approach during 2021 and 2022. There are 15 well locations that were closed in November 2020 along the western and southern service road in preparation of installing the SRT in 2021.

4. References

CH2M. 2009. COPR Investigation Report, Dundalk Marine Terminal, Baltimore, Maryland.

CH2M. 2011. Corrective Measures Alternatives Analysis, Dundalk Marine Terminal, Baltimore, Maryland. January.

CH2M. 2017. Area 1501/1602 Supplemental Site Investigation Report, Dundalk Marine Terminal, Baltimore, Maryland.

CH2M and Geosyntec. 2009. Heave Investigation and Minimization Study Report, Dundalk Marine Terminal, Baltimore, Maryland. May 29.

Jacobs. 2018. Predesign Geotechnical and Groundwater Investigation Work Plan. June.

MRCE (Mueser Rutledge Consulting Engineers). 2013. *Heave Investigation and Minimization Study Supplemental Data Report #1*. June.

MRCE (Mueser Rutledge Consulting Engineers). 2014. *Heave Investigation and Minimization Study Supplemental Data Report* #2. April.



MRCE (Mueser Rutledge Consulting Engineers). 2015. *Heave Investigation and Minimization Study Supplemental Data Report* #3. June.

MRCE (Mueser Rutledge Consulting Engineers). 2016. *Heave Investigation and Minimization Study Supplemental Data Report #4.* November.

MRCE (Mueser Rutledge Consulting Engineers). 2017. *Heave Investigation and Minimization Study Supplemental Data Report #5.* July.

MRCE (Mueser Rutledge Consulting Engineers). 2018. *Heave Investigation and Minimization Study Supplemental Data Report #6.* December.

MRCE (Mueser Rutledge Consulting Engineers). 2020. *Heave Investigation and Minimization Study Supplemental Data Report #7.* February.

Tables

Table 1. List of Wells and Geotechnical Points

No. Proceedings Process Proc		, , , , , , , , , , , , , , , , , , ,				VWP EI	evation	Well Screen	Elevation	Single or					
The color										•					
The Content The Content										•	. ,	` '			· · · · · · · · · · · · · · · · · · ·
Proc. Proc			_							0					
The color The				1						•	'		' °	Och Edge	Certainer
Fig. 200					_					•	<u> </u>			15th St Drain, SAA	Performance, Movement
March 1975 1976				1							<u> </u>		' °		
Proc. 20 Cont.				1							<u>'</u>		' °		
EPACH 1995 1996			_		_								· ·	,	· '
Proc. 20														Total of Brain, Grav	r chombance, weverhent
Proceedings	BRP-11	2015	Grouted	574434.0191	1448095.932	1.263	NA	NA	NA	Multiple	Below Cap	SAA	Plan to keep within monitoring well network to evaluate water level within cell. Also SAA		
Proc. Cont				1									, ,	SAA	Movement
Property 1975 Control Victorial				1						·	'			15th Ct Duain	Dowformones
				1							<u> </u>		1		
Proceedings					_					•				,	· · · · · · · · · · · · · · · · · · ·
Prop 1910 Company 1910 Company 1910 1			Grouted			2.534								0 1	<u> </u>
Page 1					_										<u> </u>
1997 27 2015 2016				1							- ' '		·	<u> </u>	
				1							<u> </u>			•	
				1							17	None		, , , , , , , , , , , , , , , , , , ,	
BPP-52 7010 Concust 17-231-251 48091 17 48051 18091				1						•	<u> </u>				
PRP-14 7016 Chande February Change C				1						•	•		' °	•	
PRP-52 7016 Crimate FEAST 157 Left 146 177.0 MA NA NA NA NA NA NA NA				1						•	<u> </u>			· · · · · · · · · · · · · · · · · · ·	
PRP-53 7216 October				1						•	· · · · · · · · · · · · · · · · · · ·			, , , , , , , , , , , , , , , , , , ,	
Sept. 2015 Oraced 76/17/56 145095.77 1.1 NA NA NA Sigle Seav. Org. Nove Plant to detection Section				1							<u> </u>		, v		
BPP-50 2015 Counted CF47934 146601.04 4.05 NA				1		5.888			NA	Single	<u> </u>		Plan to abandon		
BPS-32 2015 Ground 57,548.60 14,489.00 24,430 N. N. N. N. N. N. N. Single Select Cop Note Plan to absorded											•				
EMP-39 2016 Oracle Security Securi				1						•	<u> </u>			Cell Interior	Sentinel
EBP-40 2015 Granted 574109 72 44600569 4.005 NA NA NA NA NA NA Striple Debtor Cap Nome Part to base path memoratoring well review store under some store and the store of the standard of the store of the standard of the s				1						·	•				
PRP-41 2015 Granted 70/3809/27 1448095/32 A322 NA NA NA Na Single Rebox Cap None Part to leasy white monitoring well network to evaluate water level within cell Call Interior Smitted Call Interior Smitted Call Interior Smitted Call Interior Smitted Call Interior Smitted Call Interior Smitted Call Interior Smitted Call Interior Smitted Call Interior Smitted Call Interior Call Interior Smitted Call Interior Smi				1						•				Cell Interior	Sentinel
SPR-12 2016 Grounder SPR-968-971 1449-185-214 1.30			_		_	2.828				Multiple					
1879-14 2015 Grounded 57/0205-59 144898-357 3.471 NA NA NA Stright Storov Cap None Plat to began pullim monitoring well relevont to evaluate water level within cell. Cell interor Sentinel Cell i				1						·	· · · · · · · · · · · · · · · · · · ·				
EMP-44 2015 Grounded 574231.5284 1448710.2386 2.388 N.N. N.N. N.N. Strige Below Cap None Plan to keep within monthing well retwork to evaluate water frew within cell. Cell Edge Sentinel				1						•	·		' °	Cell Interior	Sentinel
SRP-45 2015 Grounder 57426-42 1448971 8 3.84 NA NA NA NA NA Single According to water feel only to evaluate water feel adone one! Cell Edge Sentine!				1						·	•			Cell Interior	Sentinel
BRP-447 2015 Grouted S7994-89 4146914-88 1,46 NA NA NA Single Below Cip None Plan to batamom September S7940-10 September September S7940-10 September September S7940-10 September				1						•	<u> </u>				
Section Company Comp	BRP-46	2015	Grouted		1448080.89	13.426	NA	NA	NA	Single	Above Cap	None	Plan to keep within monitoring well network to evaluate water level above cell.	Above Cap	Performance
DL-15AA 2018 Grouped 573803,8051 1448011.05 NA NA NA NA NA NA NA N						1				•	· · · · · · · · · · · · · · · · · · ·				
Discrimination Disc				1							<u> </u>			844	Movement
DMT-115 2006 Open 5744/30-8821 1448/175-396 NA NA -2.9 -2.29 Single Below Liner None Plan to keep to monitor deep Below Liner Sentinel					_					0					
DMT-TW2			_		_					_					
DMT-TW2 2011 Open 5734014.6 1448454.992 NA NA 6.5 1.5 Single Below Cap None Plan to beand on Plan to keep to monitor deep Below Liner Sentinel					_			-6.9					<u> </u>	Below Liner	Sentinel
EA-7M 1987 Open 673744.5189 1448006.41 NA NA 68.37 97.37 Single Below Liner None Plan to keep to monitor deep Below Liner Sentinel								<u>7</u>							
EA/TS 1987 Open 573747.4446 1447366.576 NA NA 6.54 2.54 Single Below Cap None Plan to keep			_		_									Below Liner	Sentinel
EW-2					_							None		Cell Edge	Sentinel
GP-10A 2015 Grouted 574153.65 144893.12 2.766 NA NA NA Single Below Cap None Plan to keep within monitoring well network to evaluate water level within cell. Inside SRT. Cell Edge Sentinel GP-10C 2015 Grouted 574071.4694 1447875.695 0.327 NA NA NA Single Below Cap None Plan to keep within monitoring well network to evaluate water level within cell. Also SAA. Cell Edge Sentinel GP-10C 2015 Grouted 574071.4694 1447875.695 0.327 NA NA NA Single Below Cap None Plan to keep within monitoring well network to evaluate water level within cell. Also SAA. Cell Edge, SAA Sentinel, Movement GP-2C 2015 Grouted 574006.5727 1447896.498 1.472 10.472 NA NA Multiple Below Cap Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel Cell Edge Sentinel Cell					_					•				Cell Interior	Sentinel
GP-10C 2015 Grouted 574144.81 144894.11 1.										•				Cell Edge	Sentinel
GP-1A 2015 Grouted 574071.4694 144787.6955 0.327 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, SAA Sentinel, Movement GP-2A 2015 Grouted 574006.5727 1447898.498 1.472 10.472 10.472 NA NA Multiple Below Cap Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement GP-3A 2015 Grouted 574001.5837 1447898.6472 1.541 10.341 NA NA Multiple Below Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement Multiple GP-3A 2015 Grouted 573817.0101 144795.8517 4.525 9.025 NA NA Multiple Below Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement Multiple GP-4A 2015 Grouted 573817.0101 144795.8517 4.525 9.025 NA NA Multiple Below Cap, Above Cap None Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel Movement CP-4A 2015 Grouted 573817.891 4448116.456 0.955 11.955 NA NA Multiple Below Cap, Above Cap None Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel None Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel None Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel None Plan to keep within monitoring well network to															
GP-2A 2015 Grouted 5740016.5727 1447898.498 1.472 10.472 NA NA Multiple Below Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement GP-3A 2015 Grouted 573824.7818 1447975.8556 2.538 11.0341 NA NA Multiple Below Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement Multiple Below Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement Multiple Below Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement Multiple Below Cap, Above Cap None Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel, Movement San Saa, Cell Edge, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel, Movement San Saa, Cell Edge, Above Cap Performance, Sentinel, Movement San Saa, Cell Edge, Above Cap Performance, Sentinel, Movement San Saa, Sa										•		SAA		Ŭ	Sentinel, Movement
GP-2C 2015 Grouted 574001.5837 1447886.472 1.541 10.341 NA NA Multiple Below Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement Advanced by Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement Advanced by Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap, SAA Performance, Sentinel, Movement Advanced by Cap, Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Advanced by Cap, Above Cap Performance, Sentinel, Movement SAA															
GP-3A 2015 Grouted 573824.7818 1447975.856 2.538 11.038 NA NA Multiple Below Cap. Above Cap SAA Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap Performance, Sentinel, Movement Cell Edge, Above Cap Performance, Sentinel Cell Edge, Above Cap					_					_				- 57 - 17	
GP-3C 2015 Grouted 573817.0101 1447958.517 4.525 9.025 NA NA Multiple Below Cap, Above Cap None Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel GP-4C 2015 Grouted 57385.8188 1448114.622 2.305 11.605 NA NA Multiple Below Cap, Above Cap None Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel GP-5C 2015 Grouted 573872.9513 1448269.223 3.517 NA NA NA Single Below Cap, Above Cap None Plan to keep within monitoring well network to evaluate water level within cell and above cap. Cell Edge, Above Cap Performance, Sentinel GP-5C 2015 Grouted 573872.9513 1448269.223 3.517 NA NA NA Single Below Cap None Plan to abandon P										_					
GP-4C 2015 Grouted 573785.8188 1448114.622 2.305 11.605 NA NA Multiple Below Cap, Above Cap None Plan to keep within monitoring well network to evaluate water level within cell and above cap. Also SAA. Cell Edge, Above Cap Performance, Sentinel GP-5A 2015 Grouted 573872.9513 1448269.223 3.517 NA NA NA NA Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to abandon Single Below Cap None Plan to keep within monitoring well network to evaluate water level within cell. Cell Edge Sentinel Single Sing										_					· · · · · · · · · · · · · · · · · · ·
GP-5A 2015 Grouted 573872.9513 1448269.223 3.517 NA NA NA None Plan to abandon GP-5C 2015 Grouted 573856.9 1448275.27 3.011 NA NA NA Ningle Below Cap None Plan to keep within monitoring well network to evaluate water level within cell. Cell Edge Sentinel GP-6C 2015 Grouted 573911.6541 1448406.701 3.251 NA NA NA Nine Plan to keep within monitoring well network to evaluate water level within cell. Cell Edge Sentinel GP-7A 2015 Grouted 573910.6541 1448406.701 3.251 NA NA NA NA Nine Plan to keep within monitoring well network to evaluate water level within cell. Cell Edge Sentinel GP-7A 2015 Grouted 573970.2839 144852.684 4.39 NA NA NA Multiple Below Cap SAA Plan to keep for movement monitoring SAA Movement GP-8A 2015 Grouted					_									0 ,	,
GP-5C 2015 Grouted 573856.9 1448275.27 3.011 NA NA NA Single Below Cap None Plan to abandon GP-6A 2015 Grouted 573930 1448399.49 2.817 NA NA NA Single Below Cap None Plan to keep within monitoring well network to evaluate water level within cell. GP-6C 2015 Grouted 573911.6541 1448406.701 3.251 NA NA NA Single Below Cap None Plan to keep within monitoring well network to evaluate water level within cell. GP-7A 2015 Grouted 573987.823 1448535.718 3.234 NA NA NA NA Multiple Below Cap SAA Plan to keep for movement monitoring GP-7C 2015 Grouted 573970.2839 1448542.649 4.39 NA NA NA NA Multiple Below Cap SAA Plan to keep for movement monitoring GP-8A 2015 Grouted 574040.24 1448652.08 3.019 NA NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA Cell Edge, SAA Sentinel, Movement GP-8C 2015 Grouted 574022.8362 1448659.885 2.197 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA. Cell Edge, SAA Sentinel, Movement					_									Cell Edge, Above Cap	Performance, Sentinel
GP-6A 2015 Grouted 573930 1448399.49 2.817 NA NA NA Single Below Cap None Plan to keep within monitoring well network to evaluate water level within cell. GP-6C 2015 Grouted 573911.6541 1448406.701 3.251 NA NA NA Single Below Cap None Plan to keep within monitoring well network to evaluate water level within cell. GP-7A 2015 Grouted 573987.823 1448535.718 3.234 NA NA NA NA Multiple Below Cap SAA Plan to keep for movement monitoring GP-7C 2015 Grouted 573970.2839 1448542.649 4.39 NA NA NA NA Multiple Below Cap SAA Plan to keep for movement monitoring GP-8A 2015 Grouted 574040.24 1448652.08 3.019 NA NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA Cell Edge, SAA Sentinel, Movement GP-8C 2015 Grouted 574022.8362 1448659.885 2.197 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA. Cell Edge, SAA Sentinel, Movement					_										
GP-7A 2015 Grouted 573987.823 1448535.718 3.234 NA NA Multiple Below Cap SAA Plan to keep for movement monitoring SAA Movement GP-7C 2015 Grouted 573970.2839 1448542.649 4.39 NA NA NA Multiple Below Cap SAA Plan to keep for movement monitoring SAA Movement GP-8A 2015 Grouted 574040.24 1448652.08 3.019 NA NA NA Multiple Below Cap SAA Plan to keep for movement monitoring SAA Cell Edge, SAA Sentinel, Movement GP-8A 2015 Grouted 574040.24 1448652.08 3.019 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA Cell Edge, SAA Sentinel, Movement GP-8C 2015 Grouted 574022.8362 1448659.885 2.197 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring										0				Cell Edge	Sentinel
GP-7C 2015 Grouted 573970.2839 1448542.649 4.39 NA NA NA Multiple Below Cap SAA Plan to keep for movement monitoring Plan to keep for movement monitoring well network to evaluate water level within cell. Also SAA Cell Edge, SAA Movement GP-8C 2015 Grouted 574040.24 1448652.08 3.019 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA Cell Edge, SAA Sentinel, Movement GP-8C 2015 Grouted 574022.8362 1448659.885 2.197 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA. Cell Edge, SAA Sentinel, Movement				1							<u> </u>		i ü	- 3	
GP-8A 2015 Grouted 574040.24 1448652.08 3.019 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA Cell Edge, SAA Sentinel, Movement GP-8C 2015 Grouted 574022.8362 1448659.885 2.197 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA. Cell Edge, SAA Sentinel, Movement SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA. Cell Edge, SAA Sentinel, Movement SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA.											<u> </u>		, ,		
GP-8C 2015 Grouted 574022.8362 1448659.885 2.197 NA NA NA Multiple Below Cap SAA Plan to keep within monitoring well network to evaluate water level within cell. Also SAA. Cell Edge, SAA Sentinel, Movement				1							'		, ,		
					_									J ,	,

Table 1. List of Wells and Geotechnical Points

Sentinel Monitoring Plan, Area 1501/1602, Dundalk Marine Terminal

	Year				VWP EI	evation	Well Screen	Elevation	Single or Multiple					
Well ID	Installed	Well Type	Northing	Easting	VWP1	VWP2	Тор	Bottom	Purpose Well	Monitoring Interval(s)	Other Use(s)	Proposed Status	Well Location	Program
GP-9C	2015	Grouted	574100.48	1448841.45	3.062	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
INC-1501-C	2007	Open	574233.5827	1448047.946	NA	NA	NA	NA	Single	NA		Plan to abandon		
INC-1501-D	2007		574248.5841	1448084.991	NA	NA	NA	NA	Single	NA	Inclinometer	Plan to abandon		
INC-1501-I	2007		574029.5994	1447950.663	NA	NA	NA	NA	Single	NA	Inclinometer	Plan to abandon		
INC-1501-J	2007	Open	573973.2842	1448062.592	NA	NA	NA	NA	Single	NA	Inclinometer	Plan to keep	INC	Movement
INC-1501-K	2007		573867.9787	1448175.161	NA	NA	NA	NA	Single	NA	Inclinometer	Plan to abandon		
INC-1501-L	2007		574006.084	1448106.892	NA	NA	NA	NA	Single	NA	Inclinometer	Plan to keep	INC	Movement
INC-1501-M	2007	Open	573912.8722	1448225.496	NA	NA	NA	NA	Single	NA	Inclinometer	Plan to abandon		
INC-19 INC-27VW	2010 2010		574783.8839 574191.7453	1448806.892 1447929.722	NA NA	NA NA	NA NA	NA NA	Single	NA NA	Inclinometer	Abandoned due to damage Plan to abandon		
INC-27VW	2010		574481.9366	1447929.722	NA NA	NA NA	NA NA	NA NA	Single	NA NA	Inclinometer			
INC-44	2010	Open	573761.0283	1447983.085	NA NA	NA NA	NA NA	NA NA	Single Single	NA	Inclinometer	Plan to abandon		
INC-44	2011		573748.1489	1448018.268	NA NA	NA NA	NA NA	NA NA	Single	NA	Inclinometer	Plan to abandon Plan to abandon	+	
INC-45			574063.4867	1448749.781	NA NA	NA NA	NA NA	NA NA	Single	NA NA	Inclinometer	Plan to abandon		
INC-46	2011	Open	574139.2603	1448946.768	NA NA	NA NA	NA NA	NA NA	Single	NA NA	Inclinometer	Plan to abandon		
INC-47			574242.6299	1447890.886	NA NA	NA NA	NA NA	NA NA	Single	NA	Inclinometer	Plan to keep	INC	Movement
INC-49	2012	Open	574190.4459	1447941.951	NA NA	NA	NA NA	NA NA	Single	NA	Inclinometer	Plan to abandon	1140	IVIOVEITIENT
INC-50	2012		574219.0965	1449049.583	NA NA	NA	NA NA	NA NA	Single	NA		Plan to keep if debris is succesfully removed	INC	Movement
P-12	2001	Open	574658.8505	1448959.292	NA	NA	10.15	14.15	Single	Outside Cell	None	Plan to keep	Cell Edge	Sentinel
S-1	2018	Open	574023.0589	1447845.934	NA	NA	-9.3	-19.3	Single	Below Liner	None	Plan to keep	Below Liner	Sentinel
S-2			573826.9567	1447926.658	NA NA	NA	-15.9	-16.9	Single	Below Liner		Plan to keep	Below Liner	Sentinel
S-3D	2018	Open	573751.8893	1448119.703	NA NA	NA	-16.3	-17.3	Single	Below Liner	None	Plan to keep	Below Liner	Sentinel
S-3I		_	573752.2308	1448118.595	NA	NA	-11.5	-12.5	Single	Below Liner		Plan to keep	Below Liner	Sentinel
S-4D	2018	Open	573890.5621	1448451.287	NA	NA	-10.4	-15.4	Single	Below Liner	None	Plan to keep	Below Liner	Sentinel
S-4I	2018	Open	573890.9754	1448449.512	NA	NA	-8.5	-9.5	Single	Below Liner	None	Plan to keep	Below Liner	Sentinel
S-5	2018	Open	573986.0278	1448681.98	NA	NA	-8.9	-18.9	Single	Below Liner	None	Plan to keep	Below Liner	Sentinel
S-6	2019	Open	574116.0768	1449016.034	NA	NA	-16.5	-18.5	Single	Below Liner	None	Plan to keep	Below Liner	Sentinel
S-7	2018	Open	574264.9151	1447701.367	NA	NA	-0.2	-5.2	Single	Below Liner	None	Plan to keep	Below Liner	Sentinel
SL-1	2018	Grouted	574503.15	1448435.69	6.5	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
SL-10	2018	Grouted	574504.95	1448732.86	3.6	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
SL-2	2018	Grouted	574462.45	1448580.66	6.5	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
SL-4	2018	Grouted	574353.04	1448868.59	4	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
SL-5	2018	Grouted	574187.58	1448898.99	3.3	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
SL-6	2018	Grouted	574067.05	1448755.23	4.3	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
SL-7	2018	Grouted	573941.7478	1448467.447	3.4	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
SL-8	2018	Grouted	573766.505	1448037.06	1	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
SL-9	2018		573935.6584	1447923.214	1.4	NA	NA	NA	Single	Below Cap	None	Plan to abandon		
TPZ-50	2010	Open	574113.0214	1447972.64	NA	NA	4.4	-0.6	Single	Below Cap	None	Plan to keep within monitoring well network to evaluate water level within cell.	Cell Interior	Sentinel
TPZ-51	2010	Open	574326.4581	1448066.911	NA	NA	6.5	1.5	Single	Below Cap	None	Plan to abandon		
TPZ-52	2010		574556.13	1448335.451	NA	NA	7.7	1.7	Single	Below Cap	None	Plan to abandon		
TPZ-53			574491.2841	1448832.092	NA	NA	10	5	Single	Below Cap		Plan to abandon		
TPZ-54	2010		574195.684	1448910.892	NA	NA	8.2	2.2	Single	Below Cap	None	Plan to abandon		
TPZ-55	2010	Open	573992.3871	1448462.813	NA	NA	7.4	2.4	Single	Below Cap	None	Plan to abandon		
TPZ-56	2010	Open	573867.9842	1448087.692	NA	NA	4	-1	Single	Below Cap	None	Plan to keep within monitoring well network to evaluate water level within cell.	Cell Interior	Sentinel
TPZ-57	2010	Open	574235.9923	1448335.979	NA	NA	8.1	3.1	Single	Below Cap	None	Plan to abandon		
TPZ-61	2010	Open	574396.3552	1449066.676	NA	NA	2.6	-7.4	Single	Below Cap	None	Plan to keep within monitoring well network to evaluate water level within cell.	Cell Edge	Sentinel
TPZ-63	2010	Open	573885.0931	1448089.063	NA	NA	13.1	8.1	Single	Above Cap	None	Plan to keep within monitoring well network to evaluate water level above cell.	Above Cap	Performance
TPZ-64	2011	Open	574028.7279	1448100.301	NA	NA	15.7	10.7	Single	Above Cap	None	Plan to abandon		
TPZ-65	2011	Open	574223.8211	1448330.143	NA	NA	18.7	13.7	Single	Above Cap	None	Plan to keep within monitoring well network to evaluate water level above cell.	Above Cap	Performance
TPZ-66	2011	Open	573995.5765	1448450.064	NA	NA	16.6	11.6	Single	Above Cap	None	Plan to keep within monitoring well network to evaluate water level above cell.	Above Cap	Performance
TPZ-67	2011	Open	573888.6052	1447944.483	NA	NA	15	10	Single	Above Cap	None	Plan to abandon		
TPZ-68	2011	Open	573943.3741	1448293.719	NA	NA	15.9	10.9	Single	Above Cap	None	Plan to keep within monitoring well network to evaluate water level above cell.	Above Cap	Performance

Notes: VWP = Vibrating Wire Piezometer Elevations in ft (NAVD88)

Table 2. Wells for Hydraulic Assessment Monitoring

				VWP EI	levation	Well Scree	n Elevation	Single or Multiple	
Well ID	Well Type	Northing	Easting	VWP1	VWP2	Тор	Bottom	Purpose Well	Monitoring Interval(s)
Wells Adja	cent to 15th St	reet Drain				-			
BRP-06A	Grouted	574485.18	1448090.04	4	NA	NA	NA	Single	Below Cap
BRP-07	Grouted	574325.94	1447953.66	4.189	NA	NA	NA	Multiple	Below Cap
BRP-08	Grouted	574229.17	1447873.89	1.399	NA	NA	NA	Multiple	Below Cap
BRP-09	Grouted	574507.62	1448174.09	2.582	NA	NA	NA	Multiple	Below Cap
BRP-11	Grouted	574434.02	1448095.93	1.263	NA	NA	NA	Multiple	Below Cap
BRP-12A	Grouted	574337.83	1448098.93	3.16	NA	NA	NA	Single	Below Cap
BRP-13	Grouted	574254.06	1447980.31	2.225	NA	NA	NA	Single	Below Cap
BRP-14	Grouted	574157.6	1447904.39	0.743	NA	NA	NA	Multiple	Below Cap
Wells Abo	ve Clay Cap								
BRP-16	Grouted	573742.78	1448000.8	0.962	9.962	NA	NA	Multiple	Below Cap, Above Cap
BRP-17	Grouted	573877.42	1448309.17	2.534	11.534	NA	NA	Multiple	Below Cap, Above Cap
BRP-46	Grouted	573963.68	1448080.89	13.426	NA	NA	NA	Single	Above Cap
GP-2A	Grouted	574006.57	1447898.5	1.472	10.472	NA	NA	Multiple	Below Cap, Above Cap
GP-2C	Grouted	574001.58	1447886.47	1.541	10.341	NA	NA	Multiple	Below Cap, Above Cap
GP-3A	Grouted	573824.78	1447975.86	2.538	11.038	NA	NA	Multiple	Below Cap, Above Cap
GP-3C	Grouted	573817.01	1447958.52	4.525	9.025	NA	NA	Multiple	Below Cap, Above Cap
GP-4A	Grouted	573801.77	1448106.46	0.965	11.965	NA	NA	Multiple	Below Cap, Above Cap
GP-4C	Grouted	573785.82	1448114.62	2.305	11.605	NA	NA	Multiple	Below Cap, Above Cap
TPZ-63	Open	573885.09	1448089.06	NA	NA	13.1	8.1	Single	Above Cap
TPZ-65	Open	574223.82	1448330.14	NA	NA	18.7	13.7	Single	Above Cap
TPZ-66	Open	573995.58	1448450.06	NA	NA	16.6	11.6	Single	Above Cap
TPZ-68	Open	573943.37	1448293.72	NA	NA	15.9	10.9	Single	Above Cap
Wells for A	bandonment								
BRP-10	Grouted	574422.26	1448106.42	1.499	NA	NA	NA	Multiple	Below Cap
BRP-38	Grouted	574020.12	1447997.62	13.385	NA	NA	NA	Single	Above Cap
BRP-48	Grouted	573922.49	1448024.63	13.087	NA	NA	NA	Single	Above Cap
TPZ-64	Open	574028.73	1448100.3	NA	NA	15.7	10.7	Single	Above Cap
TPZ-67	Open	573888.61	1447944.48	NA	NA	15	10	Single	Above Cap

Table 3. Perimeter Sentinel WellsSentinel Monitoring Plan, Area 1501/1602, Dundalk Marine Terminal

				VWP E	levation	Well	Screen	Single or Multiple	
Well ID	Well Type	Northing	Easting	VWP1	VWP2	Тор	Bottom	Purpose Well	Monitoring Interval(s)
Perimeter	Wells for Se	ntinel Moni	toring						
BRP-01	Grouted	574544.6	1448037.46	2.768	NA	NA	NA	Single	Below Cap
BRP-02	Grouted	574435.23	1447902.46	2.018	NA	NA	NA	Single	Below Cap
BRP-16	Grouted	573742.78	1448000.8	0.962	9.962	NA	NA	Multiple	Below Cap, Above Cap
BRP-17	Grouted	573877.42	1448309.17	2.534	11.534	NA	NA	Multiple	Below Cap, Above Cap
BRP-18	Grouted	574199.82	1449073.59	-15.207	3.793	NA	NA	Multiple	Below Cap, Below Liner
BRP-20	Grouted	574313.38	1449004.61	4.43	-17.57	NA	NA	Multiple	Below Cap, Below Liner
BRP-21	Grouted	574420.78	1448804.7	10.585	NA	NA	NA	Multiple	Below Cap
BRP-23	Grouted	574439.55	1448860.2	-17.308	1.692	NA	NA	Multiple	Below Cap, Below Liner
BRP-25	Grouted	574589	1448832.17	-16.964	6.036	NA	NA	Multiple	Below Cap, Below Liner
BRP-28	Grouted	574611.92	1448617.02	-16.397	5.603	NA	NA	Multiple	Below Cap, Below Liner
BRP-31	Grouted	574621.01	1448314.67	-25.715	-0.715	NA	NA	Multiple	Below Cap, Below Liner
BRP-45	Grouted	574254.42	1448871.8	3.64	NA	NA	NA	Single	Below Cap
EA-7S	Open	573747.44	1447986.58	NA	NA	6.54	2.54	Single	Below Cap
GP-10A	Grouted	574163.65	1448933.12	2.766	NA	NA	NA	Single	Below Cap
GP-10C	Grouted	574144.61	1448941.16	4.281	NA	NA	NA	Single	Below Cap
GP-1A	Grouted	574071.47	1447875.7	0.327	NA	NA	NA	Multiple	Below Cap
GP-2A	Grouted	574006.57	1447898.5	1.472	10.472	NA	NA	Multiple	Below Cap, Above Cap
GP-2C	Grouted	574001.58	1447886.47	1.541	10.341	NA	NA	Multiple	Below Cap, Above Cap
GP-3A	Grouted	573824.78	1447975.86	2.538	11.038	NA	NA	Multiple	Below Cap, Above Cap
GP-3C	Grouted	573817.01	1447958.52	4.525	9.025	NA	NA	Multiple	Below Cap, Above Cap
GP-4A	Grouted	573801.77	1448106.46	0.965	11.965	NA	NA	Multiple	Below Cap, Above Cap
GP-4C	Grouted	573785.82	1448114.62	2.305	11.605	NA	NA	Multiple	Below Cap, Above Cap
GP-6A	Grouted	573930	1448399.49	2.817	NA	NA	NA	Single	Below Cap
GP-6C	Grouted	573911.65	1448406.7	3.251	NA	NA	NA	Single	Below Cap
GP-8A	Grouted	574040.24	1448652.08	3.019	NA	NA	NA	Multiple	Below Cap
GP-8C	Grouted	574022.84	1448659.89	2.197	NA	NA	NA	Multiple	Below Cap
P-12	Open	574658.85	1448959.29	NA	NA	10.15	14.15	Single	Outside Cell
TPZ-61	Open	574396.36	1449066.68	NA	NA	2.6	-7.4	Single	Below Cap
	Wells for Ab	andonment							
BRP-03	Grouted	574340.5	1447832.37	4.713	NA	NA	NA	Single	Below Cap
BRP-26	Grouted	574550.98	1448634.28	0.268	NA	NA	NA	Single	Below Cap
BRP-34	Grouted	574420.58	1448671.6	5.888	NA	NA	NA	Single	Below Cap
GP-1C	Grouted	574066.17	1447863.4	2.375	NA	NA	NA	Single	Below Cap
GP-5A	Grouted	573872.95	1448269.22	3.517	NA	NA	NA	Single	Below Cap
GP-5C	Grouted	573856.9	1448275.27	3.011	NA	NA	NA	Single	Below Cap
GP-9A	Grouted	574119.82	1448833.55	3.64	NA	NA	NA	Single	Below Cap
GP-9C	Grouted	574100.48	1448841.45	3.062	NA	NA	NA	Single	Below Cap

Table 3. Perimeter Sentinel WellsSentinel Monitoring Plan, Area 1501/1602, Dundalk Marine Terminal

				VWP E	evation	Well Screen		Single or Multiple	
Well ID	Well Type	Northing	Easting	VWP1	VWP2	Тор	Bottom	Purpose Well	Monitoring Interval(s)
SL-1	Grouted	574503.15	1448435.69	6.5		NA	NA	Single	Below Cap
SL-10	Grouted	574504.95	1448732.86	3.6		NA	NA	Single	Below Cap
SL-2	Grouted	574462.45	1448580.66	6.5		NA	NA	Single	Below Cap
SL-4	Grouted	574353.04	1448868.59	4		NA	NA	Single	Below Cap
SL-5	Grouted	574187.58	1448898.99	3.3		NA	NA	Single	Below Cap
SL-6	Grouted	574067.05	1448755.23	4.3		NA	NA	Single	Below Cap
SL-7	Grouted	573941.75	1448467.45	3.4		NA	NA	Single	Below Cap
SL-8	Grouted	573766.51	1448037.06	1		NA	NA	Single	Below Cap
SL-9	Grouted	573935.66	1447923.21	1.4		NA	NA	Single	Below Cap
TPZ-52	Open	574556.13	1448335.45	NA	NA	7.7	1.7	Single	Below Cap
TPZ-53	Open	574491.28	1448832.09	NA	NA	10	5	Single	Below Cap
TPZ-54	Open	574195.68	1448910.89	NA	NA	8.2	2.2	Single	Below Cap

Table 4. Interior Monitoring WellsSentinel Monitoring Plan, Area 1501/1602, Dundalk Marine Terminal

				Elev	ation	Well Scree	n Elevation	Single or Multiple	
Well ID	Well Type	Northing	Easting	VWP1	VWP2	Тор	Bottom	Purpose Well	Monitoring Interval(s)
Interior We	ells for Moni	itoring					•	•	
BRP-29	Grouted	574533.55	1448298.73	6.667	NA	NA	NA	Multiple	Below Cap
BRP-32	Grouted	574343.15	1448154.15	3.726	NA	NA	NA	Single	Below Cap
BRP-33	Grouted	574408.3	1448471.91	3.375	NA	NA	NA	Multiple	Below Cap
BRP-36	Grouted	574279.04	1448338.51	5.415	NA	NA	NA	Single	Below Cap
BRP-39	Grouted	574108.72	1448209.89	4.306	NA	NA	NA	Single	Below Cap
BRP-42	Grouted	574046.77	1448408.71	4.204	NA	NA	NA	Single	Below Cap
BRP-44	Grouted	574231.53	1448710.33	3.388	NA	NA	NA	Single	Below Cap
EW-1	Open	573909.3	1448080.59	NA	NA	8	1	Single	Below Cap
TPZ-50	Open	574113.02	1447972.64	NA	NA	4.4	-0.6	Single	Below Cap
TPZ-56	Open	573867.98	1448087.69	NA	NA	4	-1	Single	Below Cap
Interior We	ells for Abar	donment							-
BRP-35	Grouted	574175.44	1448085.77	3.1	NA	NA	NA	Single	Below Cap
BRP-37	Grouted	574249.01	1448530.46	4.425	NA	NA	NA	Single	Below Cap
BRP-40	Grouted	573875.29	1448048.3	2.828	NA	NA	NA	Multiple	Below Cap
BRP-41	Grouted	573936.27	1448265.23	4.023	NA	NA	NA	Single	Below Cap
BRP-43	Grouted	574095.59	1448636.37	3.474	NA	NA	NA	Single	Below Cap
BRP-47	Grouted	573954.58	1448046.8	1.46	NA	NA	NA	Single	Below Cap
DMT-TW1	Open	573898.95	1448111.77	NA	NA	7	2	Single	Below Cap
DMT-TW2	Open	574014.6	1448454.99	NA	NA	6.5	1.5	Single	Below Cap
EW-2	Open	574278.05	1448275.92	NA	NA	10.47	0.47	Single	Below Cap
TPZ-51	Open	574326.46	1448066.91	NA	NA	6.5	1.5	Single	Below Cap
TPZ-55	Open	573992.39	1448462.81	NA	NA	7.4	2.4	Single	Below Cap
TPZ-57	Open	574235.99	1448335.98	NA	NA	8.1	3.1	Single	Below Cap

Table 5. Vertical Sentinel WellsSentinel Monitoring Plan, Area 1501/1602, Dundalk Marine Terminal

				Elevation		Well Scree	en Elevation	Single or Multiple	
Well ID	Well Type	Northing	Easting	VWP1	VWP2	Тор	Bottom	Purpose Well	Monitoring Interval(s)
BRP-18	Grouted	574199.82	1449073.6	-15.207	3.793	NA	NA	Multiple	Below Cap, Below Liner
BRP-20	Grouted	574313.38	1449004.6	4.43	-17.57	NA	NA	Multiple	Below Cap, Below Liner
BRP-23	Grouted	574439.55	1448860.2	-17.308	1.692	NA	NA	Multiple	Below Cap, Below Liner
BRP-25	Grouted	574589	1448832.2	-16.964	6.036	NA	NA	Multiple	Below Cap, Below Liner
BRP-28	Grouted	574611.92	1448617	-16.397	5.603	NA	NA	Multiple	Below Cap, Below Liner
BRP-31	Grouted	574621.01	1448314.7	-25.715	-0.715	NA	NA	Multiple	Below Cap, Below Liner
DMT-11S	Open	574430.88	1448175.9	NA	NA	-2.9	-22.9	Single	Below Liner
DMT-56S	Open	574170.12	1447839.5	NA	NA	-6.9	-16.9	Single	Below Liner
EA-7M	Open	573744.52	1448006.4	NA	NA	-86.37	-97.37	Single	Below Liner
S-1	Open	574023.06	1447845.9	NA	NA	-9.3	-19.3	Single	Below Liner
S-2	Open	573826.96	1447926.7	NA	NA	-15.9	-16.9	Single	Below Liner
S-3D	Open	573751.89	1448119.7	NA	NA	-16.3	-17.3	Single	Below Liner
S-3I	Open	573752.23	1448118.6	NA	NA	-11.5	-12.5	Single	Below Liner
S-4D	Open	573890.56	1448451.3	NA	NA	-10.4	-15.4	Single	Below Liner
S-4I	Open	573890.98	1448449.5	NA	NA	-8.5	-9.5	Single	Below Liner
S-5	Open	573986.03	1448682	NA	NA	-8.9	-18.9	Single	Below Liner
S-6	Open	574116.08	1449016	NA	NA	-16.5	-18.5	Single	Below Liner
S-7	Open	574264.92	1447701.4	NA	NA	-0.2	-5.2	Single	Below Liner

Table 6. Movement Monitoring Wells

		1	1	Movement	
Well ID	Well Type	Northing	Easting	Monitoring Type	
Wells for Movement					
DL-1-SAA	Grouted	573853.6651	1448061.169	Shape Accel Array	
DL-2-I	Open	574621.31	1448479.55	Inclinometer	
INC-1501-J	Open	573973.2842	1448062.592	Inclinometer	
INC-1501-L	Open	574006.084	1448106.892	Inclinometer	
INC-48	Open	574242.6299	1447890.886	Inclinometer	
INC-50	Open	574219.0965	1449049.583	Inclinometer	
BRP-06	Grouted	574481.6574	1448092.779	Shape Accel Array	
BRP-07	Grouted	574325.94	1447953.66	Shape Accel Array	
BRP-08	Grouted	574229.1737	1447873.886	Shape Accel Array	
BRP-09	Grouted	574507.62	1448174.09	Shape Accel Array	
BRP-11	Grouted	574434.0191	1448095.932	Shape Accel Array	
BRP-12	Grouted	574325.2623	1448040.438	Shape Accel Array	
BRP-14	Grouted	574157.6013	1447904.385	Shape Accel Array	
BRP-16	Grouted	573742.781	1448000.799	Shape Accel Array	
BRP-17	Grouted	573877.4188	1448309.165	Shape Accel Array	
BRP-18	Grouted	574199.8173	1449073.588	Shape Accel Array	
BRP-21	Grouted	574420.78	1448804.7	Shape Accel Array	
BRP-29	Grouted	574533.55	1448298.73	Shape Accel Array	
BRP-33	Grouted	574408.3	1448471.91	Shape Accel Array	
GP-1A	Grouted	574071.4694	1447875.695	Shape Accel Array	
GP-2A	Grouted	574006.5727	1447898.498	Shape Accel Array	
GP-2C	Grouted	574001.5837	1447886.472	Shape Accel Array	
GP-3A	Grouted	573824.7818	1447975.856	Shape Accel Array	
GP-7A	Grouted	573987.823	1448535.718	Shape Accel Array	
GP-7C	Grouted	573970.2839	1448542.649	Shape Accel Array	
GP-8A	Grouted	574040.24	1448652.08	Shape Accel Array	
GP-8C	Grouted	574022.8362	1448659.885	Shape Accel Array	
Movement Wells for	Abandonme	nt			
INC-1501-C	Open	574233.5827	1448047.946	Inclinometer	
INC-1501-D	Open	574248.5841	1448084.991	Inclinometer	
INC-1501-I	Open	574029.5994	1447950.663	Inclinometer	
INC-1501-K	Open	573867.9787	1448175.161	Inclinometer	
INC-1501-M	Open	573912.8722	1448225.496	Inclinometer	
INC-19	Open	574783.8839	1448806.892	Inclinometer	
INC-27VW	Open	574191.7453	1447929.722	Inclinometer	
INC-30VW	Open	574481.9366	1448147.388	Inclinometer	
INC-44	Open	573761.0283	1447983.085	Inclinometer	
INC-45	Open	573748.1489	1448018.268	Inclinometer	
INC-46	Open	574063.4867	1448749.781	Inclinometer	
INC-47	Open	574139.2603	1448946.768	Inclinometer	
INC-49	Open	574190.4459	1447941.951	Inclinometer	

Table 7. Wells and Inclinometers for Abandonment

1	Date	F	ate Well Type VWP Elevation Well Screen Eleva		. =14'	1			T				
Well ID	Installed	Date	weii iype	N a mtla im m	Faatina					Single or Multiple			Dancet Description of Latest Information
Well ID		Abandoned	One stand	Northing	Easting	VWP1 4.713	VWP2	Top	Bottom	Purpose Well	Interval(s)	Rationale	Report Document and Latest Information
BRP-03	2015	2020	Grouted	574340.5	1447832.4	1.499	NA	NA NA	NA NA	Single	Below Cap	Duplicative. Using BRP-02, BRP-07, and BRP-08	SSI (Jan 2018). Last groundwater elevation measured Dec 2019 at 7.88 ft
BRP-10 BRP-26 *	2015 2015		Grouted		1448106.4 1448634.3	0.268	NA NA	NA NA	NA NA	Multiple	Below Cap	Duplicative. Using BRP-09, BRP-11, and BRP12A Duplicative. Using BRP-28	SSI (Jan 2018). Last groundwater elevation measured May 2019 at 7.67 ft
			Grouted	+		5.888	NA	NA NA	NA NA	Single	Below Cap		SSI (Jan 2018). Mechanical failure with no data collected.
BRP-34 *	2015		Grouted		1448671.6	3.1	NA NA	NA NA	NA NA	Single	Below Cap	Duplicative Using BRP-21	SSI (Jan 2018). Last groundwater elevation measured May 2019 at 9.23 ft
BRP-35 *	2015		Grouted		1448085.8	4.425	NA	NA NA	NA NA	Single	Below Cap	Duplicative Using BRP-32, BRP-39, and TPZ-50	SSI (Jan 2018). Last groundwater elevation measured Jan 2018 at 6.59 ft
BRP-37	2015		Grouted		1448530.5	13.385	NA	NA NA	NA NA	Single	Below Cap	Duplicative. Using BRP-33, BRP-36, BRP-42, and BRP-44	SSI (Jan 2018). Last groundwater elevation measured Jan 2019 at 9.04 ft
BRP-38 BRP-40	2015 2015		Grouted	+	1447997.6 1448048.3	2.828	NA NA	NA NA	NA NA	Single	Above Cap	Duplicative. Using GP-2A and BRP-46 Duplicative. Using TPZ-56 and TPZ-63	SSI (Jan 2018). Last groundwater elevation measured Jan 2019 at 12.86 ft SSI (Jan 2018). Last groundwater elevation measured Apr 2019 at 8.37 ft
BRP-41	2015		Grouted		1448265.2	4.023	NA NA			Multiple	Below Cap	Duplicative. Using BRP-17 and BRP-39	
BRP-43	2015		Grouted Grouted		1448636.4	3.474	NA NA	NA NA	NA NA	Single	Below Cap Below Cap	Duplicative. Using andGP-8C and BRP-44	SSI (Jan 2018). Last groundwater elevation measured Jan 2018 at 8.88 ft SSI (Jan 2018). Last groundwater elevation measured May 2019 at 9.24 ft
BRP-47	2015	<u> </u>	Grouted	+	1448046.8	1.46	NA NA	NA NA	NA NA	Single Single	Below Cap	Duplicative. Using TPZ-56 and TPZ-63	SSI (Jan 2018). Last groundwater elevation measured May 2019 at 9.24 ft
BRP-48	2015				1448040.6	13.087	NA NA	NA NA	NA NA	•	+		, , ,
DMT-TW1	2015	<u> </u>	Grouted		1448111.8	NA	NA NA	NA 7	2	Single Single	Above Cap Below Cap	Duplicative. Using TPZ-56 and TPZ-63 Duplicative. Using TPZ-56 and TPZ-63	SSI (Jan 2018). Last groundwater elevation measured Dec 2020 at 12.89 ft Internal use only. Last groundwater elevation measured Jun 2020 at 10.21 ft
DMT-TW1	2011		Open	574014.6	1448455	NA NA	NA NA	6.5	1.5				. 0
EW-2	2011	<u> </u>	Open		1448275.9	NA NA	NA NA	10.47	0.47	Single	Below Cap Below Cap	Duplicative. Using BRP-42, GP-6C and GP-8C Duplicative. Using BRP-36	Internal use only. Last groundwater elevation measured Jan 2018 at 8.92 ft GGIR (Aug 2019). Last groundwater elevation measured Jun 2020 at 8.75 ft
GP-1C	2015	2020	Open		1446275.9	2.375	NA NA		NA	Single	Below Cap	·	SSI (Jan 2018). Last groundwater elevation measured Mar 2019 at 10.07 ft
GP-1C GP-5A *	2015	2020	Grouted		1447863.4	3.517	NA NA	NA NA	NA NA	Single	Below Cap	Duplicative Using GP-2C	SSI (Jan 2018). Last groundwater elevation measured Mar 2019 at 10.07 ft
GP-5A GP-5C	2015	2020	Grouted	573856.9	1448275.3	3.011	NA NA	NA NA	NA NA	Single	 	Duplicative Using BRP-17, GP-4A, and GP-6A	, ,
GP-9A *	2015	2020	Grouted		1448275.3	3.64				Single	Below Cap	Duplicative Using BRP-17, GP-4C, and GP-6C	SSI (Jan 2018). Last groundwater elevation measured Mar 2019 at 8.75 ft
GP-9A GP-9C *	2015	2020	Grouted		1448841.5	3.062	NA NA	NA NA	NA NA	Single	Below Cap Below Cap	Duplicative. Using GP-8A and GP-10A Duplicative. Using GP-8C and GP-10C	SSI (Jan 2018). Last groundwater elevation measured Mar 2019 at 10.65 ft SSI (Jan 2018). Last groundwater elevation measured Mar 2019 at 7.77 ft
-	2018	2020	Grouted			6.5	NA NA	NA NA	NA NA	Single	<u> </u>		, , ,
SL-1			Grouted		1448435.7					Single	Below Cap	Duplicative Using BRP-28, BRP-31, and BRP-33	GGIR (Aug 2019). Last groundwater elevation measured Mar 2019 at 10.17 ft
SL-10	2018 2018		Grouted		1448732.9 1448580.7	3.6 6.5	NA NA	NA NA	NA NA	Single	Below Cap	Duplicative Using BRP-21, BRP-25, and BRP-28	GGIR (Aug 2019). Last groundwater elevation measured Apr 2019 at 8.13 ft
SL-2	2018		Grouted	574353.04	1448868.6	4	NA NA	NA NA		Single	Below Cap	Duplicative Using BRP-21 and BRP-28	GGIR (Aug 2019). Last groundwater elevation measured Mar 2019 at 10.13 ft
SL-4 SL-5	2018	<u> </u>	Grouted		1448899	3.3	NA NA	NA NA	NA NA	Single	Below Cap Below Cap	Duplicative. Using BRP-20, BRP-23, and BRP-45 Duplicative. Using GP-10C	GGIR (Aug 2019). Last groundwater elevation measured Jan 2018 at 4.36 ft GGIR (Aug 2019). Last groundwater elevation measured Mar 2019 at 10.50 ft
SL-5 SL-6	2018	2020	Grouted		1448755.2	4.3	NA NA	NA NA		Single	Below Cap	·	, , ,
SL-0 SL-7	2018	2020	Grouted		1448467.4	3.4	NA NA	NA NA	NA NA	Single	Below Cap	Duplicative Using GP-8C and GP-10C	GGIR (Aug 2019). Last groundwater elevation measured Mar 2019 at 12.40 ft
SL-7 SL-8	2018	2020	Grouted Grouted		1448037.1	3.4	NA NA	NA NA	NA NA	Single Single	Below Cap	Duplicative. Using GP-6C Duplicative. Using BRP-16 and GP-4C	GGIR (Aug 2019). Last groundwater elevation measured Mar 2019 at 12.18 ft GGIR (Aug 2019). Last groundwater elevation measured Mar 2019 at 10.25 ft
SL-0 SL-9	2018	2020		+	1447923.2	1 1					- '		, , , ,
	2010	2020	Grouted			1.4 NA	NA NA	NA 6.5	NA 1.5	Single	Below Cap	Duplicative. Using GP-2C and GP-3C Duplicative. Using BRP-12A	GGIR (Aug 2019). Last groundwater elevation measured Mar 2019 at 11.59 ft
TPZ-51 TPZ-52	2010		Open		1448066.9	NA NA	NA NA	6.5 7.7	1.5 1.7	Single	Below Cap		GWMR (Oct 2020). Last groundwater elevation measured June 2020 at 13.12 ft
TPZ-52	2010	<u> </u>	Open		1448335.5 1448832.1	NA NA	NA NA	10	5	Single	Below Cap	Duplicative Using BRP-29 and BRP-31	GWMR (Oct 2020). Last groundwater elevation measured June 2020 at 15.44 ft
TPZ-53	2010	<u> </u>	Open	574195.68		NA NA	NA NA	8.2	2.2	Single Single	Below Cap	Duplicative. Using BRP-21, BRP-23, and BRP-25 Duplicative. Using GP-10C and BRP-45	GWMR (Oct 2020). Last groundwater elevation measured June 2020 at 18.67 ft GWMR (Oct 2020). Last groundwater elevation measured June 2020 at 16.00 ft
TPZ-55	2010		Open Open	573992.39		NA NA	NA NA	7.4	2.4	Single	Below Cap	Duplicative. Using GP-6C and BRP-42	GWMR (Oct 2020). Last groundwater elevation measured June 2020 at 10.00 ft
TPZ-57	2010		Open	574235.99		NA NA	NA NA	8.1	3.1	Single	Below Cap	Duplicative. Using BRP-36	GWMR (Oct 2020). Last groundwater elevation measured June 2020 at 11.34 ft
TPZ-64	2010		Open	574028.73		NA	NA	15.7	10.7	Single	Above Cap	Duplicative. Using BRP-46	Internal use only. Last groundwater elevation measured Dec 2019 at 13.48 ft
TPZ-67	2011	2020	Open	+	1447944.5	NA	NA	15.7	10.7	Single	Above Cap	Duplicative. Using GP-2A and GP-3A	Internal use only. Last groundwater elevation measured Aug 2018 at 5.03 ft
INC-1501-C	2007	2020			1448047.9	NA	NA	NA	NA	Single	Below Cap	No longer usable. Using BRP-12	HIMS. Last used Mar 2011
INC-1501-D	2007		Inclinometer		1448085	NA	NA	NA NA	NA NA	Single	Below Cap	No longer usable. Using BRP-12	HIMS. Last used Sep 2012
INC-1501-D	2007		Inclinometer		1447950.7	NA NA	NA NA	NA NA	NA NA	Single	Below Cap	No longer usable. Using GP-2A and GP-2C	HIMS. Last used Jun 2013
INC-1501-K	2007				1448175.2	NA NA	NA NA	NA NA	NA NA	Single	Below Cap	No longer usable. Using BRP-17	HIMS. Last used Mar 2011
INC-1501-N	2007			573912.87		NA	NA NA	NA NA	NA NA	Single	Below Cap	No longer usable. Using BRP-17	HIMS. Last used Jun 2013
INC-19	2007	2020	Inclinometer		1448806.9	NA NA	NA NA	NA NA	NA NA	Single	Below Cap	Abandoned due to damage	HIMS. Last used Nov 2014
INC-27VW	2010	2020		574191.75		NA	NA NA	NA NA	NA NA	Single	Below Cap	No longer usable. Using BRP-08, BRP-14, and INC-48	HIMS. Last used Mar 2012
INC-30VW	2010			574481.94		NA	NA NA	NA NA	NA NA	Single	Below Cap	No longer usable. Using BRP-06 and BRP-09	HIMS. Last used Jun 2013
INC-44	2010	2020	Inclinometer			NA NA	NA	NA NA	NA NA	Single	Below Cap	Abandoned due to construction. Using BRP-16	HIMS. Last used Dec 2015
10	2011	2020	oioinictei	37 37 31.03	. ++7 000.1	14/7	14/7	14/7	14/5	I - II I I I I I I I I I I I I I I I I	Dolow Oap	p bandshod add to conditional. Coming Dist - 10	1 mmo. Edd: 4004 500 2010

Table 7. Wells and Inclinometers for Abandonment

Sentinel Monitoring Plan, Area 1501/1602, Dundalk Marine Terminal

	Date	Date	Well Type			VWP EI	evation	Well Screen	Elevation	Single or Multiple	Monitoring		
Well ID	Installed	Abandoned		Northing	Easting	VWP1	VWP2	Тор	Bottom	Purpose Well	Interval(s)		Report Document and Latest Information
INC-45	2011	2020	Inclinometer	573748.15	1448018.3	NA	NA	NA	NA	Single	Below Cap	Abandoned due to construction. Using BRP-16	HIMS. Last used Jun 2013
INC-46	2011	2020	Inclinometer	574063.49	1448749.8	NA	NA	NA	NA	Single	Below Cap	Abandoned due to construction. Using GP-8A and GP-8C	HIMS. Last used Jun 2013
INC-47	2011	2020	Inclinometer	574139.26	1448946.8	NA	NA	NA	NA	Single	Below Cap	Abandoned due to construction. Using INC-50	HIMS. Last used Jun 2013
INC-49	2012		Inclinometer	574190.45	1447942	NA	NA	NA	NA	Single	Below Cap	No longer usable. Using BRP-08, BRP-14, and INC-48	HIMS. Last used Aug 2014

Notes: Below cap is in COPR.

* - Non-functional VWP

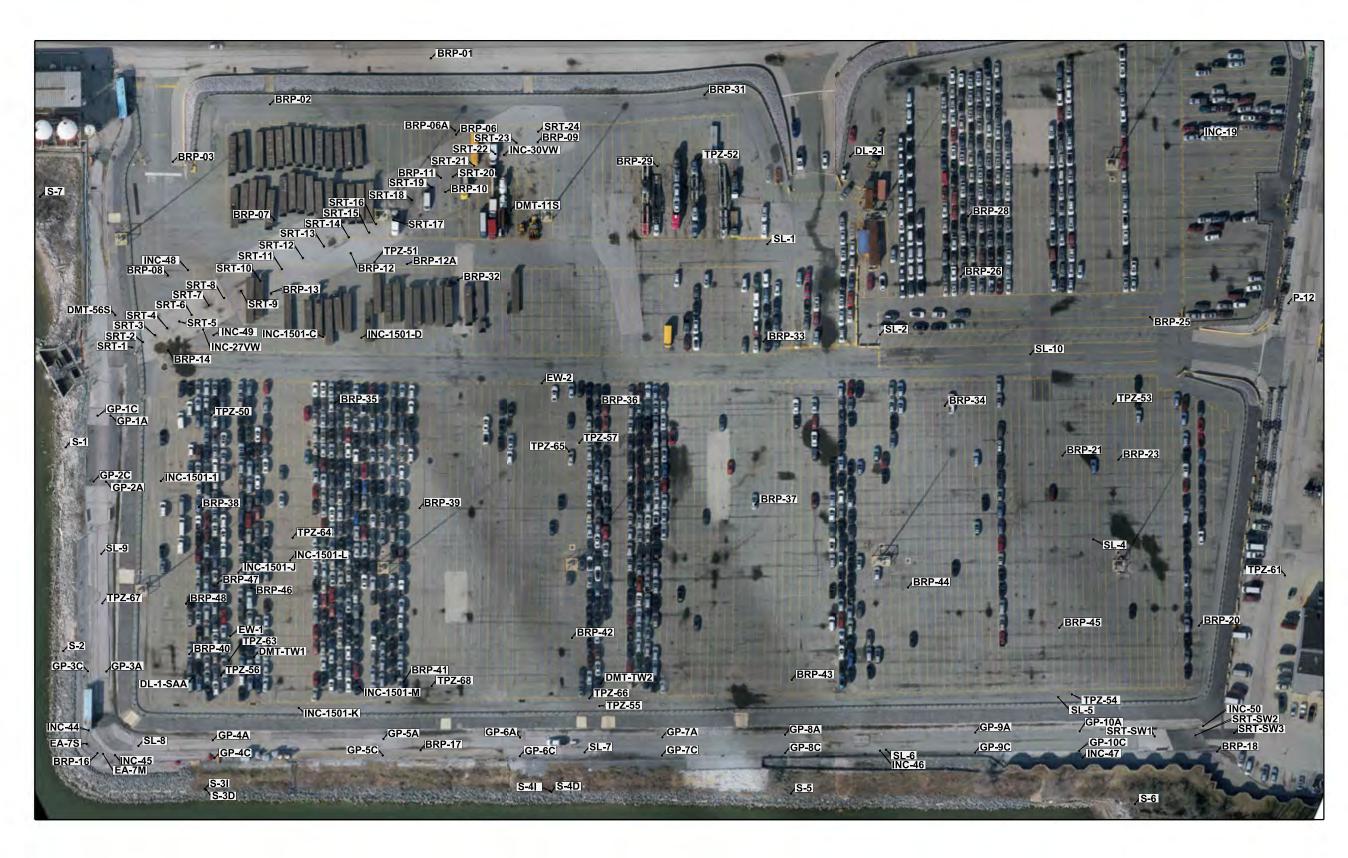
GGIR - Geotechnical and Groundwater Investigation Report (August 2019)

GWMR - Semiannual Groundwater Monitoring Report (October 2020)

HIMS - Heavy Investigation and Minimization Study, Supplemental Data Report #7 (February 2020)

SSI - Area 1501/1602 Baseline Supplemental Site Investigation Report (January 2018)

Figures



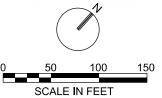


Figure 1 Site Map Sentinel Monitoring Plan, Area 1501/1602 Dundalk Marine Terminal Baltimore, Maryland

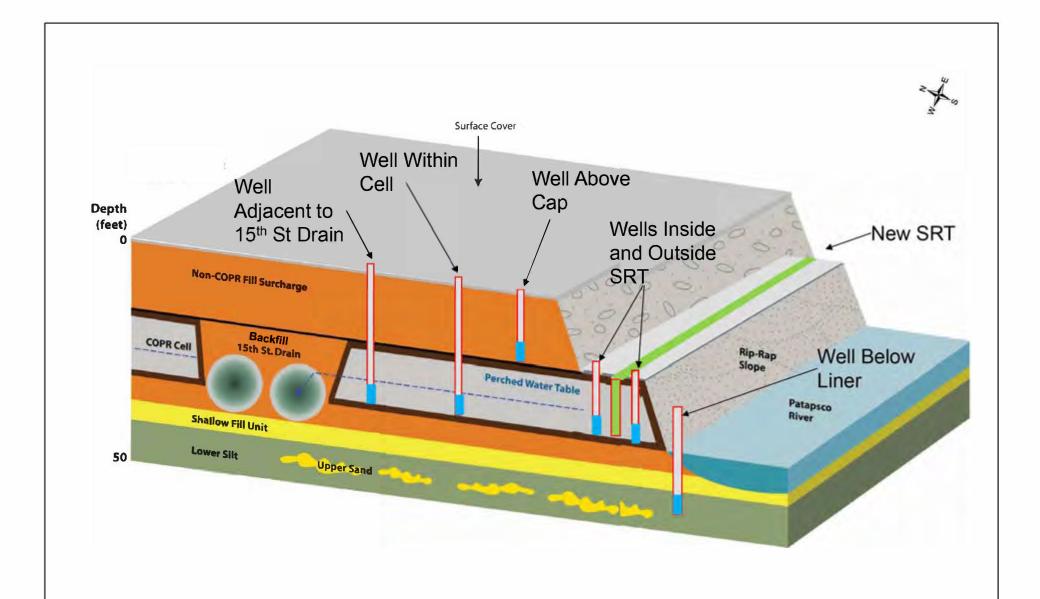
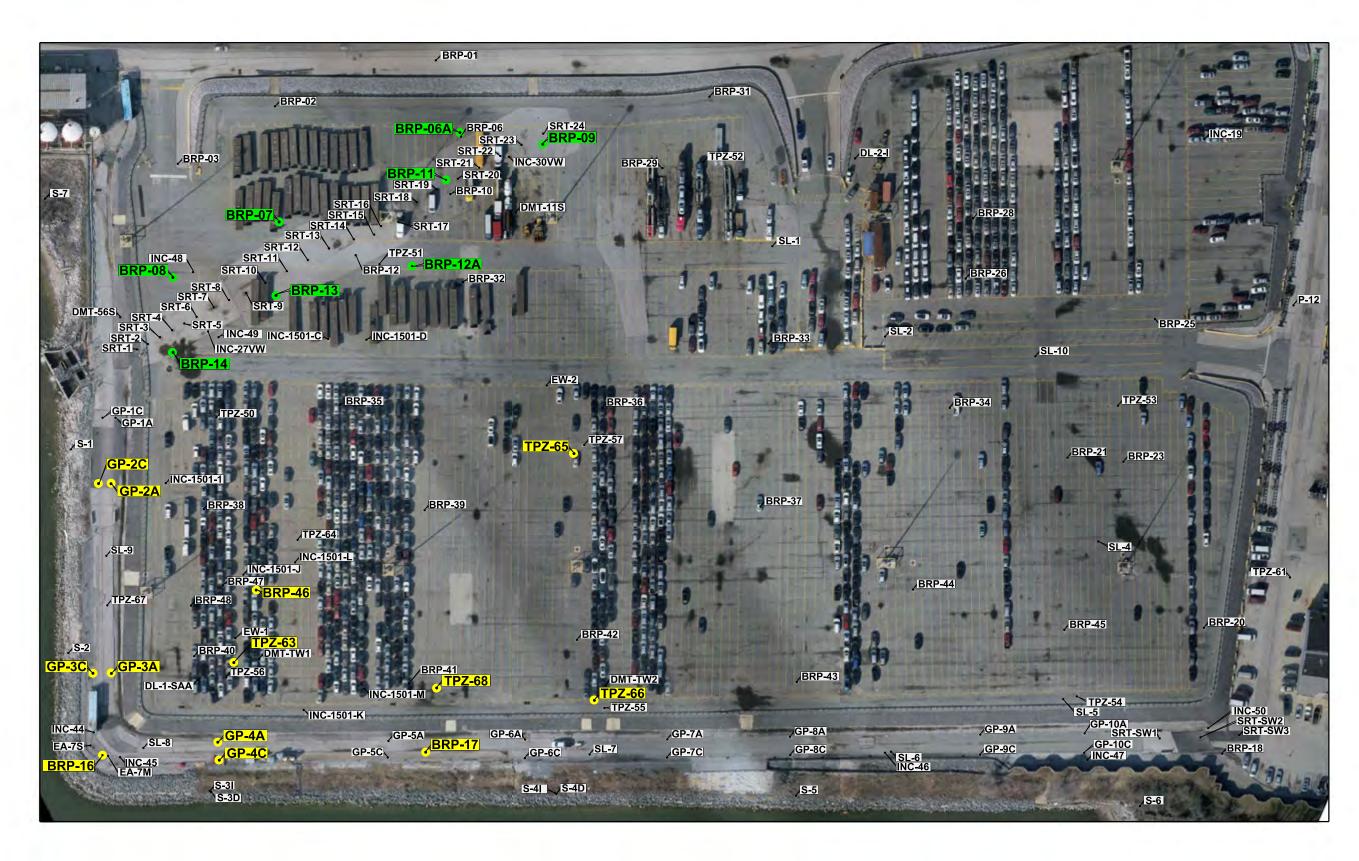


Figure 2

Typical Well Configurations
Sentinel Monitoring Plan -Area 1501/1602
Dundalk Marine Terminal Baltimore, Maryland

Jacobs





Wells Above Clay Cap

Wells Adjacent to 15th St. Drain

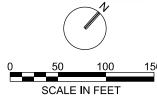


Figure 3
Hydraulic Assessment Wells
Sentinel Monitoring Plan, Area 1501/1602
Dundalk Marine Terminal
Baltimore, Maryland







Sentinel Perimeter Wells

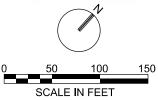


Figure 4
Sentinel Perimeter Wells
Sentinel Monitoring Plan, Area 1501/1602
Dundalk Marine Terminal
Baltimore, Maryland







Interior Observation Wells

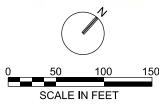


Figure 5 Interior Observation Wells Sentinel Monitoring Plan, Area 1501/1602 Dundalk Marine Terminal Baltimore, Maryland







Vertical Sentinel Wells

▲ (~ -25 Ft. Elevation)

△ (~ -90 Ft. Elevation)

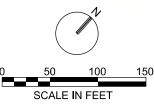
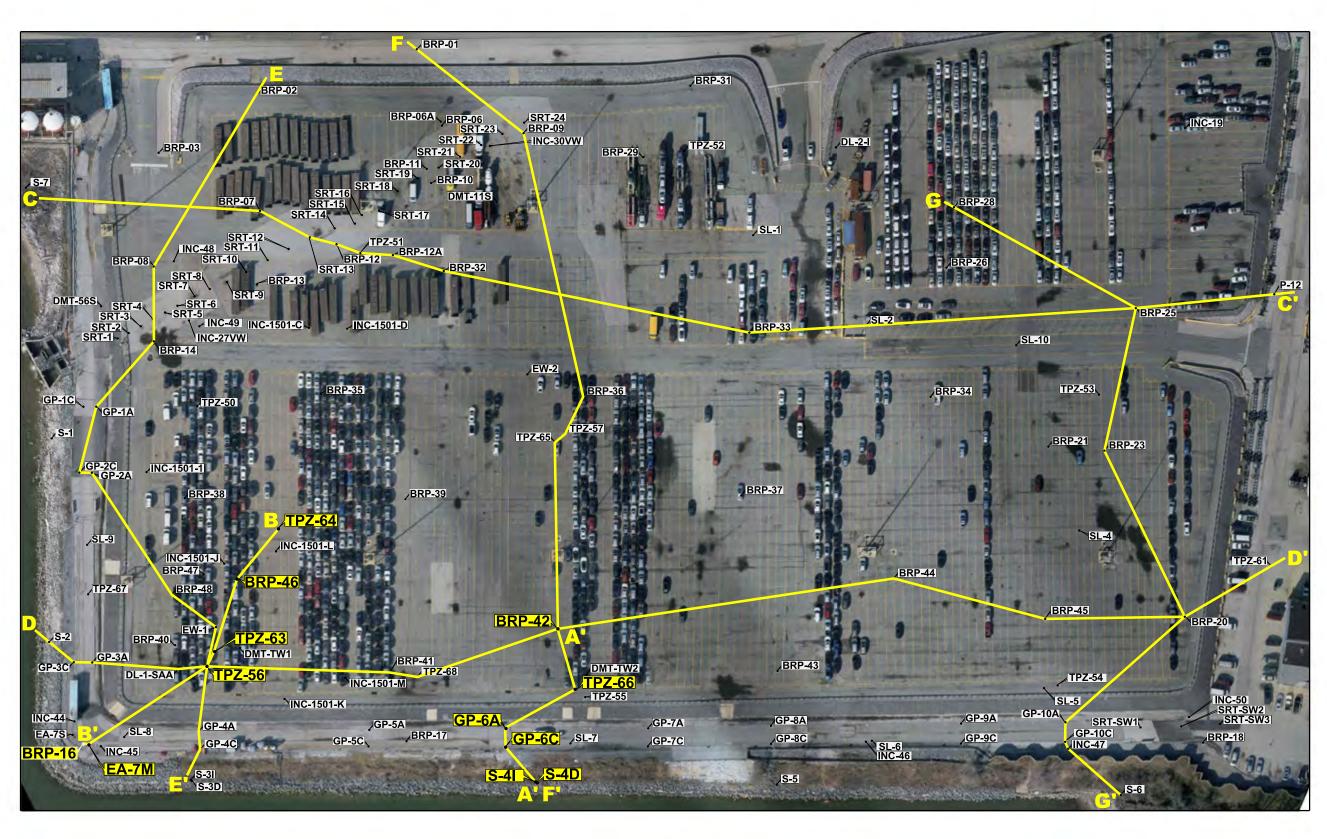


Figure 6
Vertical Sentinel Wells
Sentinel Monitoring Plan, Area 1501/1602
Dundalk Marine Terminal
Baltimore, Maryland





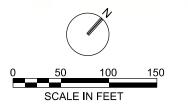
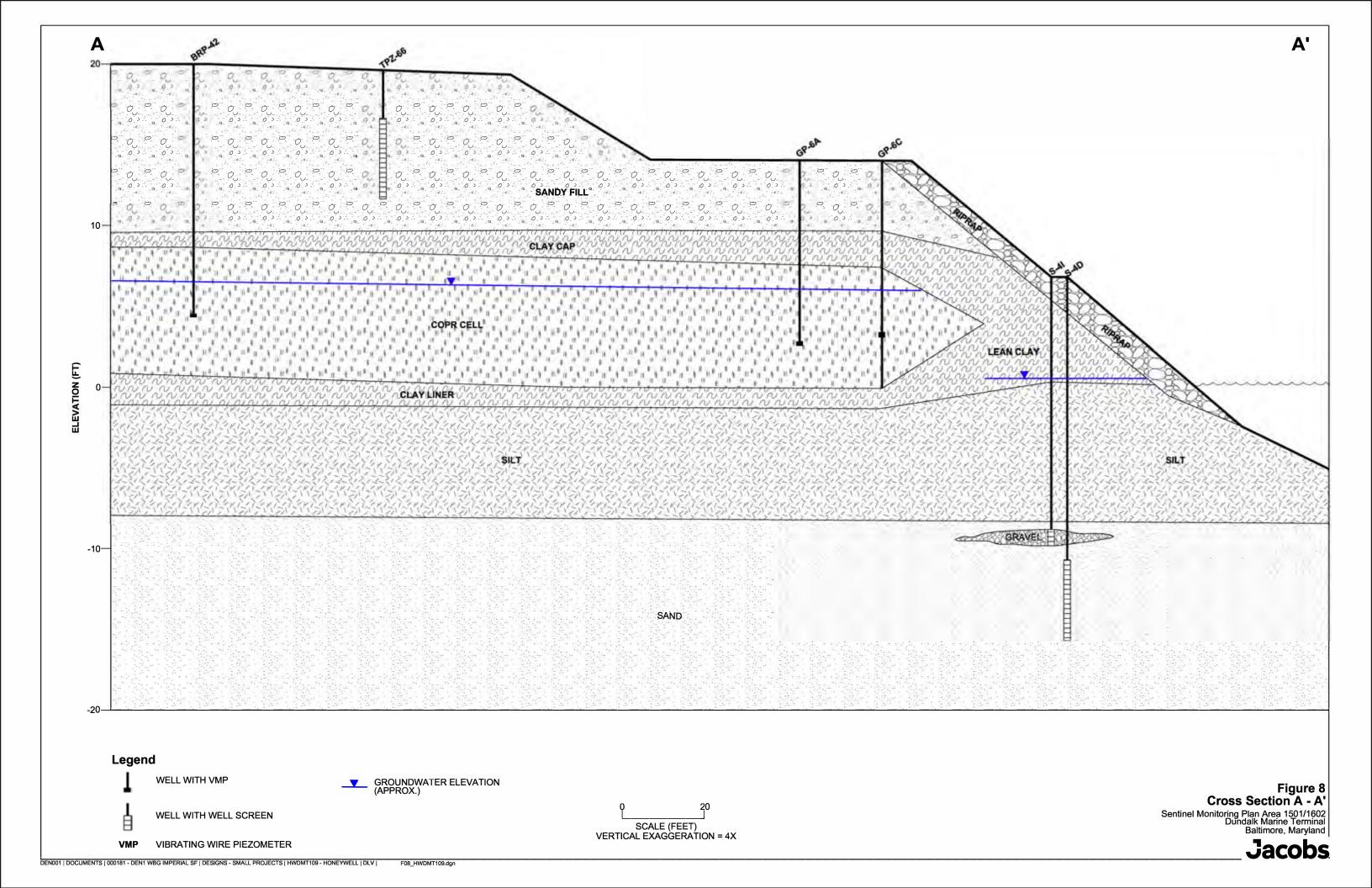
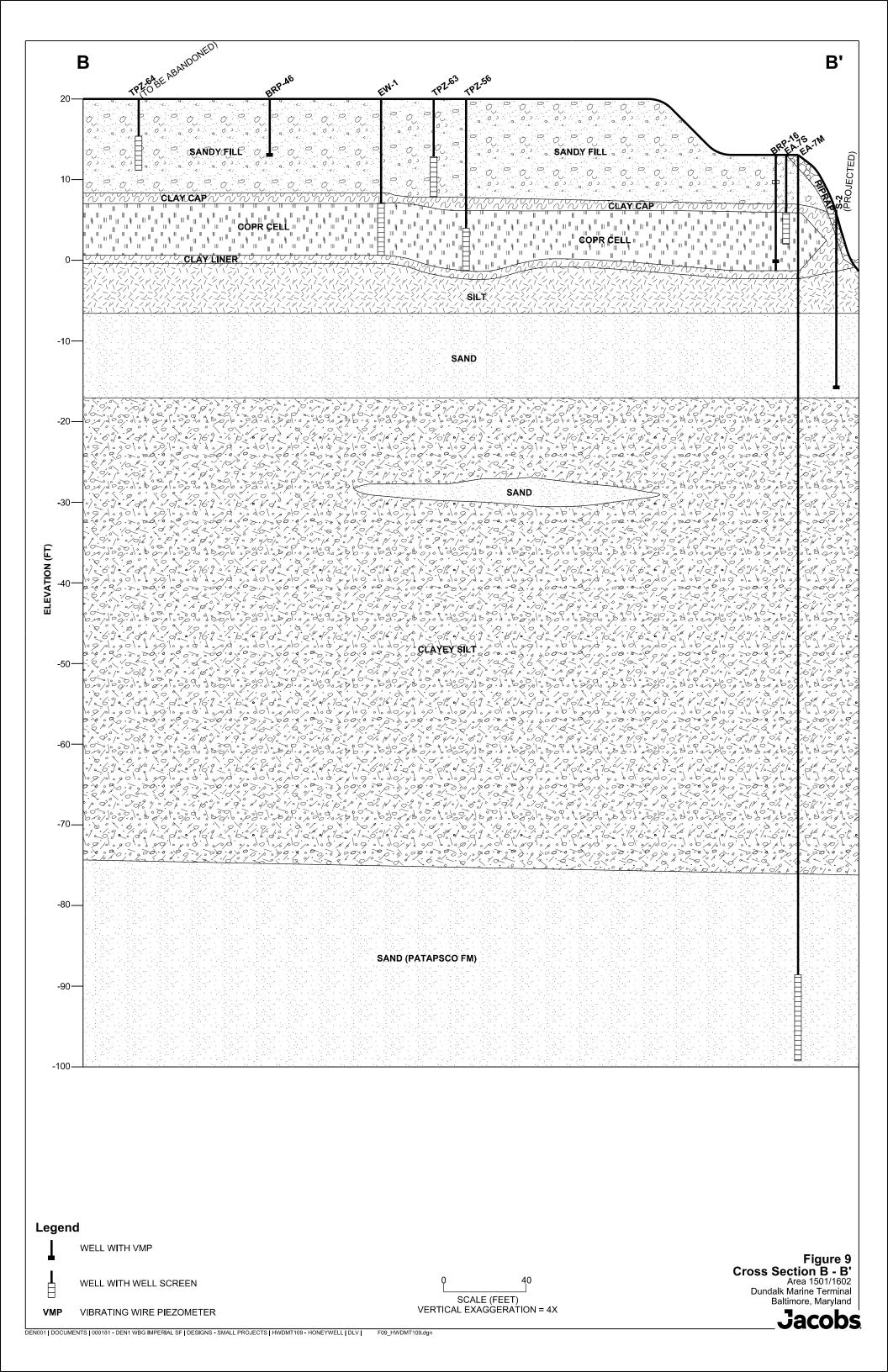
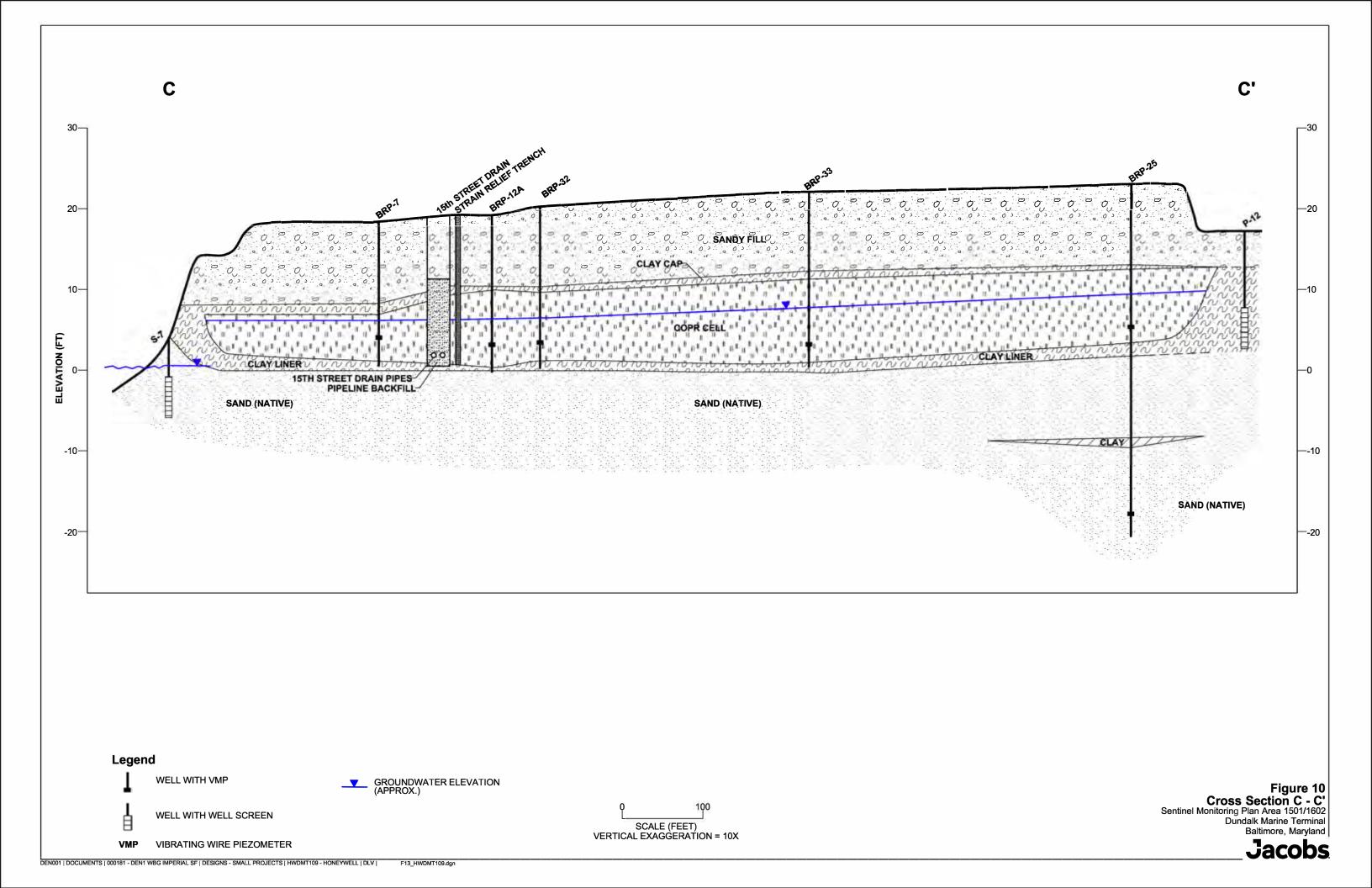
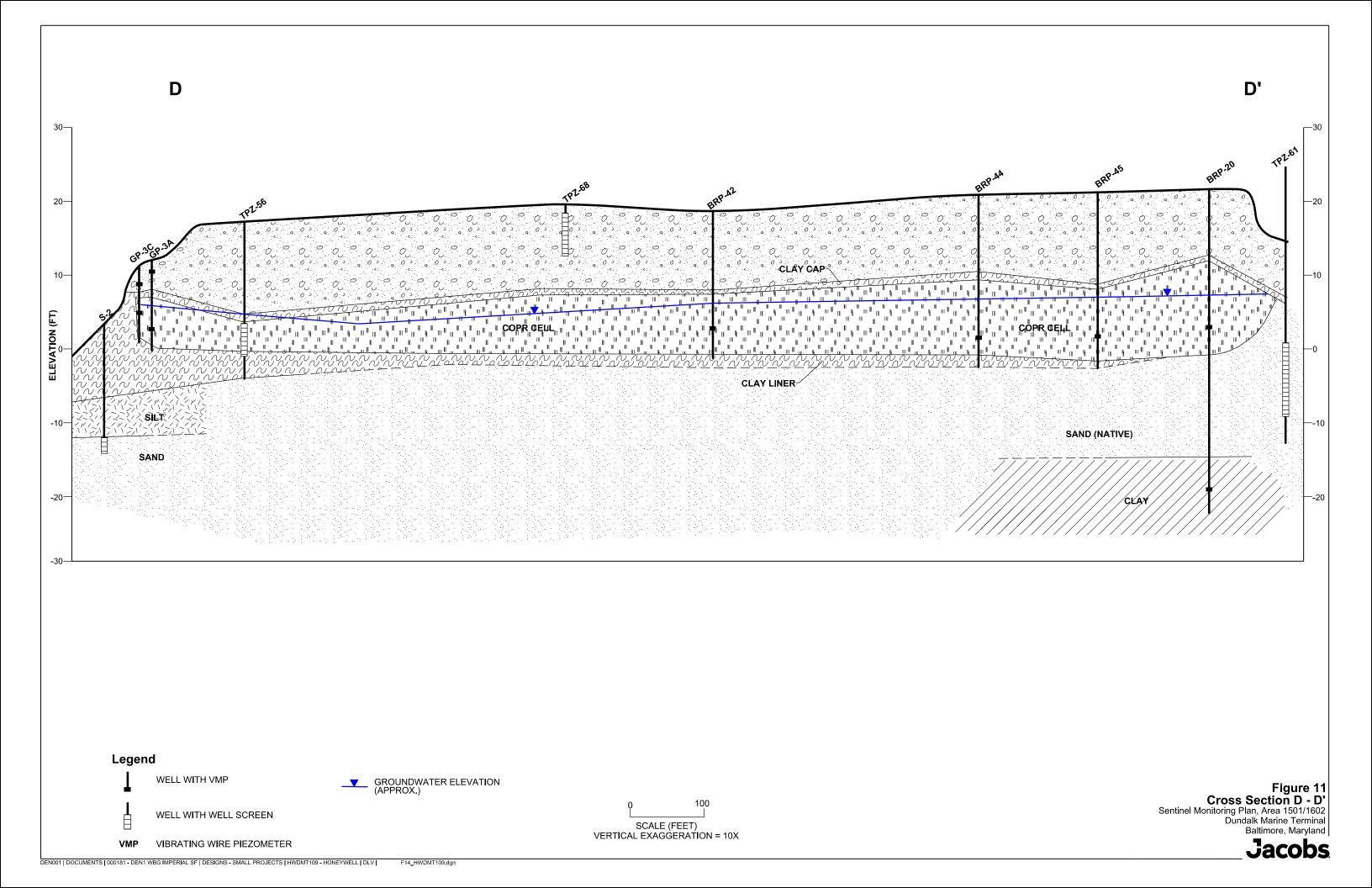


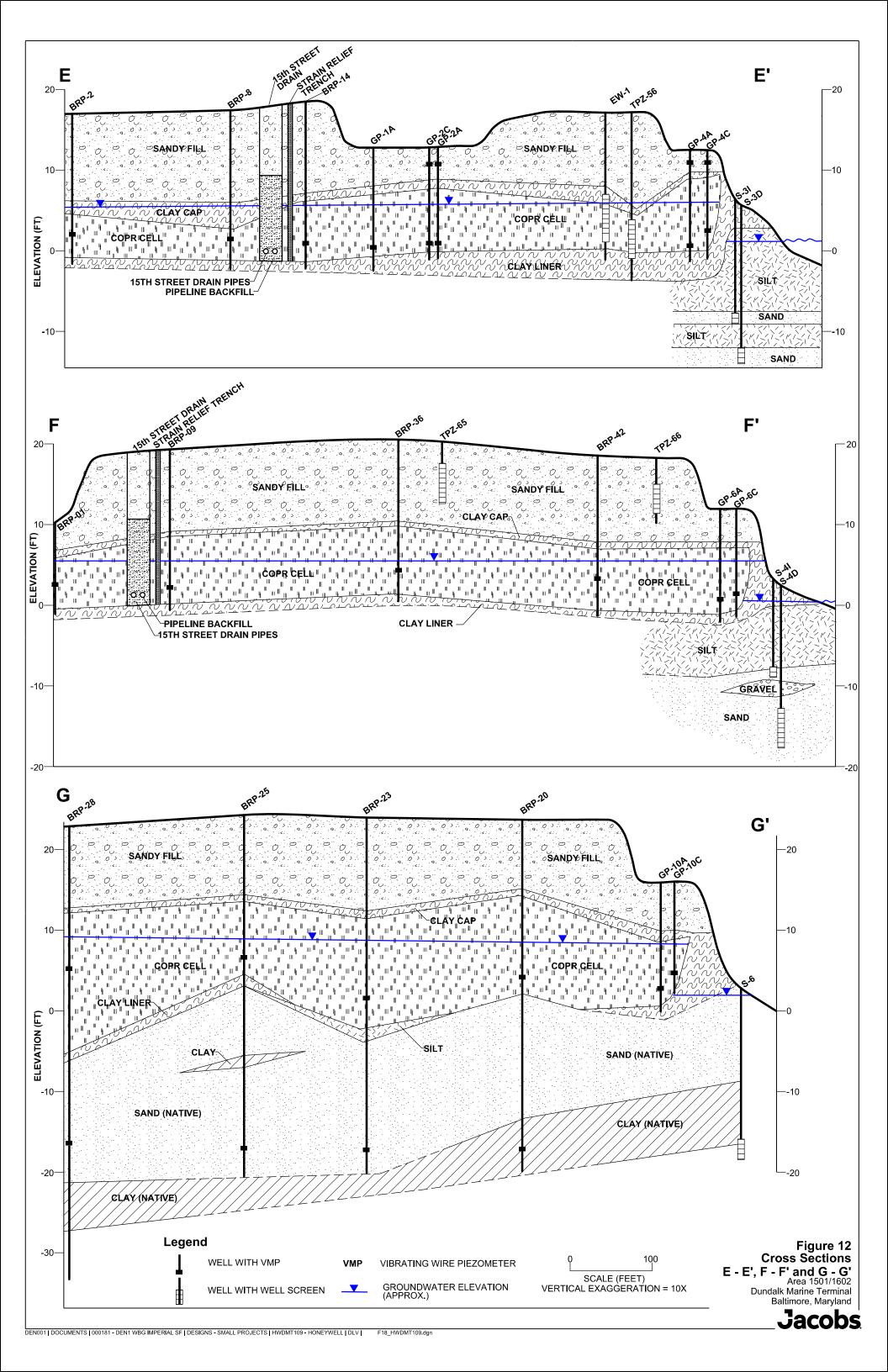
Figure 7
Cross Section Line
A - A', B - B', C - C', D - D',
E - E', F - F' and G - G'
Sentinel Monitoring Plan, Area 1501/1602
Dundalk Marine Terminal
Baltimore, Maryland













Legend

Inclinometer

Shape Accell Array Wells

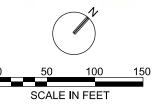


Figure 13 Inclinometer and Shape Accel Array Wells Sentinel Monitoring Plan, Area 1501/1602 Dundalk Marine Terminal Baltimore, Maryland







Inclinometers and Monitoring Wells to be Abandoned

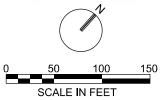


Figure 14 Inclinometers and Monitoring Wells for Abandonment Sentinel Monitoring Plan, Area 1501/1602 Dundalk Marine Terminal Baltimore, Maryland



Appendix A Soil Boring Logs for Wells To Be Abandoned



BORING NUMBER: BRF 3

Sheet: 1

SOIL BORING LOG

PROJECT: Honeywell DMT 35 I

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Aquifer Drilling and turing, ine

DRILLING METHOD AND EQUIPMENT: SONIC STOP DELL MINE 170; SPT Sampling

WATER LEVELS:

START: 12 | 7 | 15

FINISH: 12/7/15

LOGGER: Killetin veolula

(ft)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	rype	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0-	N.	Saute breeze corts	24		3" Asphalt 0.25'- well graded gravel with sound and silt CGW-GH, derk brown (2.54R 4/2), dry 1 Coarse grained Sand.	0'-3'-Advanced the sonic Macrocot, 3"-ID -Harddrilling conditions.
	28,	çS	124	8	3-11- Stly SATTO Witingraved (SM), brown (75 TR 472), dry, fine to medium grained soul 3.5'- 0.25" thick rubber membrane 3.5'- Some our above, clay seams	38-Switched to Split spoon Sampling: 140-16 hammer: 3º dia Sampler. DPC-
4-	577	ç	n	19	4.1'- Polyethylene membrane Same as alsove	DPC-
5-	3 th	çs	*GP	5	Same as alone	DPC-



BORING NUMBER: BRP-3

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(t)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
	55	ÇŞ	90	100/34	6.0' - Same as about, traceclary 6.5' - 3' thick Asphalt	3******
7-	5-6	SONIC HACEO	15	-	6-75'-2" thick Asphalt 2-0'- Poorly graded gravel with sand (Gill, group (254R 711), dry coarse graindsand -, Road based aggregate	Advanced sonic mains cone to 8' togé - DPC-
8 -	\$s-7	SS	12"	16	7-91-Silly SMVD (SN), Red. Brown (7.542) dry I fine to medium grained send 80' - 700rly graded SAND with gravel (SP), brown - gray (7.54R 4/2), moist medium to coase grained sand	
	5-8	3,5	127	12	9.01 - Same as above 9.51 - Silty Samo (SM), Red-Brown (2.54R 5/8), moist, fine to medium grained Soud, trace gravel and wooder preces:	DPC-
0-	15-9	55	121	13	lood- same as above 10:3'-sandy lean CLMY (CL), tan (GLD): dry, Low plasticity, stiff, trace grand	s Hsay) ppc-
11-	-55	93	12	13	11.05- Lean CLAY (CU), Red (1044/4), dry, Low plackicity, very stiff, 11.75' - Silty strue (SM), Brown-greens gray (7.54R 4/3), dry, particulate	V/(-



BORING NUMBER: 889-3

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

2 S	AMPLE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
LOW	(H) YS	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
DEPTH BELOW (ft)	TYPE RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
12-	55 12	us 53	12.01_ come as above 12.81. Itan contact, Red (1074/4), Amil Lowplasticity,	12.5'- 6" op outer caring installed to 13' bgg
13			Bottom of boring @ 13:01 bgs.	

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122

PROJECT LOCATION BORING LO	B	INDALK	MARINI LE, MARY		INAL	SHEET	BRI-3 4 OF	4
	CTION EQU		SONEC SH	AP BRILL M	2 1707 KA	SPT Samplin	9	
ORING EC	UIPMENT		F STABIUZING BOI	REHOLE				
NOT OF OOR		TYPE OF FE	3 4 4 4	******		<u> </u>		
YPE OF BOR	ING RIG	DURING CO		CASING USE	64	YES		101
RUCK		MECHANIC		DIA., IN.	- 6	DEPTH, FT. FROM		
BARGE		OTHER		DIA., IN.		DEPTH, FT. FROM		
	TRACK	OTHER		DIA., IN.	-	DEPTH, FT. FROM	-	
TYPE AND	SIZE OF:			DRILLING M	UD USED	YES	NO NO	,
-SAMPLER	_				F ROTARY BIT, IN	_		
J-SAMPLER		46		TYPE OF DRI	LUNG MUD	_		
SAMPLER ORE BARRE	85.50	mpling - 3"	10 sample				No.	
ORE BIT	_	-	_	AUGER USE	IAMETER, IN.	YES	NO	1
RILL RODS	_			TIPE AND D	MMEICH, IN.	_		
VATER LEV	/EL OBSERV	ATIONS IN BORE		DEPTH TO WATER		CONDITIONS	S OF OBSERVATION	
	-	Tel mer mese	our mor ordano	The state of the s		CONDITIONS	or observation	
PIEZOMETO	ER INSTALLI		ves	NO SKET	CH SHOWN ON	See	piezomeki	log
TANOPIPE:		TYPE	Handpipe	ID, IN.	2" LEN	IGTH, FT.	3 TOP ELEV	
NTAKE ELEN	MENT:	TYPE		00, IN.	LEN	IGTH, FT.	TIP ELEV.	
ILTER:		MATERIAL		00, IN.	LEN	IGTH, FT.	BOT, ELEV	٧.
AY QUAN	TITIES Y SAMPLE BO	DING	III FF		NO 0525 617	W 71100 011101		
WIN. UN			UN. FT.			Y TUBE SAMPLES	-	
		110	UN. FT.		OTHER:	STURBED SAMPLES	-	
.5" DIA. U-S	NG IN ROCK							
.5" DIA. U-S	NG IN ROCK	, Ac	wifer Drill	ina and s	eship, Inc	~		
5" DIA. U-S ORE DRILLII ORING CO BRILLER	NTRACTOR	an Karshi	CK	U	HELPERS		elen	
ORE DRILLI	NTRACTOR	an Karshi	CK	nth 5 feet	HELPERS	Chris Th	elen IE 121:	Tie.

BORING NO. BRP-3



BORING NUMBER: BRP-10

Sheet: 1

SOIL BORING LOG

PROJECT: Honeywell DMT SST

ELEVATION:

LOCATION:

DRILLING CONTRACTOR: Aquifer Dosling and testing, line. DRILLING METHOD AND EQUIPMENT: Sonic samp Drival May 170; SPT sampling

				START: 12) was FINISH: 12/10/15	LOGGER
S	AMP	T	STANDARD PENETRATION		LOGGER: K. chaturedula
NUMBER	TYPE	RECOVERY (RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
دع	o cupic		-	1.0'- Sily sparo (IM) with gravel, grave	Advanced the 3°ED Sonice macro core to 10'bgs.
	- 1	36 ^v		-01 - woven geotenking	brc.
					Pre-
			- Lu.		bpc -
force Martin Conta		Ŋ	5.6	man of Asphair	Dpc _==
	NUMBER CARE	CONTRE MACAN CURE TYPE	Service Macho congression of the service of the ser	NUMBER CONT. MACAO CORE TEST RESULTS 8"-6"-6" (I)	SAMPLE STANDARD PENETRATION TEST RESULTS SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY Sample black (50), dry, coase growing Sample black (50), dry, coase growing Sample black (50), dry, coase growing Sample black (50), dry, coase growing Sample black (50), moist, gravel, gray brown (3.54R 41), moist, fincto making growed Sand 10-Sily short (11) with gravel, gray brown (3.54R 41), moist, fincto making growed Sand 10-Sily short (11) with gravel, gray brown (3.54R 41), moist, fincto 10-Sily short (11) with gravel, gray brown (3.54R 41), moist, fincto 10-Sily short (11) 20-cone as above 10-Sily short (11) 20-cone as above



BORING NUMBER: BRP-10

Sheet: 2

SOIL BORING LOG

PROJECT:

ELEVATION:

LOCATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

***	IEKL	EVELS	>:		START:	FINISH:	LOGGER:
OW (ft)		SAMPI	LE £	STANDARD PENETRATION TEST	SOIL D	DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (# - 6 9 1.9		DENSITY OR	SCS GROUP SYMBOL, RE CONTENT, RELATIVE CONSISTENCY, SOIL RE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 7 8 9	5-7	3	36		to-ts-same a. 7-5'- Clayer SAN Noist, time to me 3:0'-8-5'- Pat Cl woist, kashff, h	ed GRANTELL with same by, coarse granned same s above or (SC), Red (1014)41, divergrained same by (CU), Red (1044)44 ighly plastic (SM). Red-Brown 1/4), dry, fine ficulate to highly	DPC-
10-	, s, u	5	2ª	18 3h	10 o'- same a o slightly Indu		DPC+ 10.01 - 6" 00 outer casing finetalled to 100' bgs. 10.5'- switched to ss sampling; 140-16 hammer; 30-in. drop; 3" to sampler



BORING NUMBER: BRP-10

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

æ	T	SAMF	LE	STANDARD		LOGGER,
) ×	_	T		PENETRATION TEST	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12-				39	12.0'-13.5'_same as above.	
13-	35.75	55	181,	30 _		
14				IS	13:2-13:6'-Brick and pieces of glass 13:6'-Poorly graded sand with silt (SP-5M), gray-black (NZ), moist, coance grained sand, particulate (GB COPR)	DPC +
15-	چې ل ه	Ŷ	184	16 -	Same as above, wet, bright green particles	14.0'. groundwaker encountered during distling
- -	~	55	(1)	15	16.0'- Same as above, wet,	DPC+
13-	3-7			50/4"	16-21-Silfy SMND (SM), Red-Brown- (7-5 YR 4131, Moist, fine to medium grained sand (HB copp) Same as above, green-julion particles	SS refusal encountered; Advanced the hole to 19.0' bgs, using 3' ED sonic macrocole
(8-	s.S.	2000	125	-	pai ries	DPC+

ch2m:

PROJECT NUMBER: 10587

BORING NUMBER: BRP-10

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH: 4.1 LOGGER:

			·····	r	7.15	
N (ft)	s	AMPL		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
-8/	ss ⁻⁹	25	12"	20 - 27 -	18-0'- Poorly graded sand with silt (SP-SM), gray-black (N2), Let, medium to coakegrained sand, Particulate. (GB copp), green and gray seams.	DPC+
19 -	८५१०	ςs	<i>ς</i> ''	12 -	19:0'- sity sand (sH), Red-Brown (107416), cref, fine to medium grained Land, particulate (coppand non-copp)	DPC+1-
-	55-11 [.]	SS	6"	17	sane as above	DOC +/_
20-		31:	4"	7	20.0-20 5 losse gray black to brown M-c sund GB copp	20.01-6"000 uter casing installed to 20.01 bgs
	55-13	311	b"	4	20.5-21 loose wet Red nottled with white medium placeterly Fat day (EH)	DPC-nane
21-					1" Puc installed to 21 depth	Bose have from thestel at 21
-				-	1 Vwp installed at 20	Vwp Aw Toll. P. 9734
-					1518424	Dgr. Tr. 17 1. 9500 Slowy T220,21,28432
				- -	1 thermister installed at 16, interface of HB & GB COPR	1 22-122
-				-	Thurmonto Air temp= 16.2	
-				-	thermister Air Oppin Gury 17,9	

Mueser Rutledge Consulting Engineers 14 Penn Plaza • 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 www.mrce.com BORING NO. SHEET **PROJECT** MARINE TERMINAL FILE NO. LOCATION SURFACE ELEV. BALTIMORE, MARYLAND **BORING LOCATION** See location Plan DATUM Souic Samp Prill Max 170; SPT Sampling **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING CASING USED** TRUCK MECHANICAL DIA., IN. DEPTH, FT. FROM SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM то BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER TUCK TYPE AND SIZE OF: DRILLING MUD USED YES **D-SAMPLER** DIAMETER OF ROTARY BIT, IN. **U-SAMPLER** TYPE OF DRILLING MUD S-SAMPLER CORE BARREL AUGER USED YES **CORE BIT** TYPE AND DIAMETER, IN. **DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL IN. SAMPLER HAMMER, LBS. 40 AVERAGE FALL, IN. 30 TYPE OF HAMMER ntomatic WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER **CONDITIONS OF OBSERVATION** PIEZOMETER INSTALLED **SKETCH SHOWN ON** STANOPIPE. TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER MATERIAL OD, IN. LENGTH, FT. BOT. ELEV. **PAY QUANTITIES** 3.5" DIA, DRY SAMPLE BORING LIN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK UN. FT. OTHER:

HELPERS

BORING NO.

Joseph Arrayo

BRP-10

DRILLER

REMARKS

BORING CONTRACTOR

RESIDENT ENGINEER



BORING NUMBER: BRF- 16

Sheet:

SOIL BORING LOG

PROJECT: Hindusell DIHT SEE

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Agus Fer dilling and festing Tex

DRILLING METHOD AND EQUIPMENT: Some samp DAII mas 170; SPI Sampling

WATER LEVELS:

START: Wills FINISH: 11 15 15

LOGGER: K.chadupvedula

£	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (T)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
)	54	6			3-4" Asphalt Poorly graded samp withground (GP) gray (GLEY 1 4/104) dry, press of Asphalt. Road based aggregate	Advanced the hole from 0'-10' using 2'20 sorre macro core
111111		SONIC MARCH WAS	36"			DRZ-
111111					30 -400 - sily spus with gravel (SM), dark brown-grows (2545/4), mostly fire to medium grained Sand	tipe -
Title Line	52	SONY C MATRE COLE	36"		40-60 - Same as above	
-					60 - Poly ethiclane lines	



BORING NUMBER: BRP-26

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

SAI	MPLE	STANDARD	SOIL DESCRIPTION	COMMENTS
NUMBER	rype RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND
NUN	TYPE	6" - 6" - 6" (N)	STRUCTURE, MINERALOGY	INTRUMENTATION
Sv			6.0'- Poorly graded SAND (SP) mixed with crushed asphalt, done gray-black (ND) alry	DPC-
			701. 9.751- some as above.	prc-
Source MACKED CORE	36		9.0' - decrease in Asphali content 9.0' - Fat CLAP (CH), Red Ciny Why),	DPC-
		9	moret. highly plash r. verystift 10.0' - Poorly graded samo with grave! (SP) dark gray black (W7), dry, Fred based aggregate	10.01. 6' OD Outer casing installed to a depth of 10 base 10.51. Below 10! Switched to 55 sampling; 100-16 hammes 50-tm. drop " 3" dia sample. DPC -
5574 5	5 211	14	11:01 - clayey SATUD (SC), Red- white (1044/10), moist, fine to medium growned Sand	DPC-



BORING NUMBER: BRP - 24

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

1	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	W.		RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,	
DEPTH	NUMBER	TYPE	RECOVI	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION	
1-	cs-5	5.5	911	50/3"	Sity savo (SM), greenish-brown (754 413) dry. particulate (COPP)	DPC +	
-				13		12-75'- spoon houncing Advanced to 14-0' bas with	
-					selb some as about, Reddich-Ringe	3, 19 savis moun cou	
3-1-1	5.6	Korio Soul	,15"	-	to muderately indurated, laminated		
1-	F.22	son's	NR/	20/1,	Same as whome	Advanced to 15 of logs with 3" 10 some mother case.	
-		notro	12"				
-	55-8	SS/	1 6	50 31	15.0'-16.5'- same at atove	150 - spoon bouncing Advanced to 120 bgs with some marro core	
1 1 1		ioul	749				
					LLS'- Poorty graded SAND with Site (SR-SM), dark gray-black (N2),	pre 4	
-					most, medium grained Eard particul (GE COPE)	ule	
1	55-9	2.2	NR	12	No Recovery		
	5 10	SS	y"	36	green CASTR 412), dry, particulate	0F/+	



BORING NUMBER: BRP- 26

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(u)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
8 -	55-11	çs	10	21 .	Same ou abone		
	55-12	15	C'n	19	Poorly graded SAND With silt (SP-SM), dorn gray-Steen (2:57413), Wet, particulate (GB cope)	DPC +	
9-	cs43	53	6"	13	same as above , wet		
	55-14	57	WI	11	some as above, wet	2010' - water excountered	
0-	55-15	22	1/11	9 .	Same as above	during drilling.	
	gg-16	¢\$	6,,	9	Same as above	-	
1-	55-17	2.2	611	10	Same as above	DPC-F	
	55-18	55	6"	25	some as above	DPC+	
-	51-10		(a.1)	9	some as above	polyarced to zon bas	
	55-20	Ç.	611	()	same as above	DPC +	
-	55-21		Gi1	15	Same as above, with grown	DPC+	
1	55-D	SS	6	33	same as above, contains	brc +	



BORING NUMBER: BRP-26

Sheet: 5

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
4-	51-13	55	Ç u	27	Some as above,	DPC +
	SP.VV	¢\$	60	33	Poorly graded sord with grave! (GP), brown-gray (SYR 4/2), wet, medium to coarse growed sord,	Returned drilling on 11 forth
5-	32.25	22	6"	- 14	same as above	255' Advanced the 6" op
5-				-	Bottom of borning and 255 bgs	25' bas .
		-,				
-						
4						
1						
1.1.1.						



BORING NUMBER: BRP 34

Sheet: 1/7

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: ~ +25

DRILLING METHOD AND EQUIPMENT: Froste Moth Drill Max 170; Some Sampling WATER LEVELS: START: 0745 FINISH: LOGGER: Mark Chancy

					0 145	
£)	S	AMPL		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0-	5-1	311 Soni U		NA -	Road base J Asphalt	15 mins stand by time Needed to fist I vwp OPC - near
1			2.5'	- - -		
3 -				- - -	2.5'-40' Dry Redbrown f-C silly sand trace gravel (SM)	DPC-none
4 -	5-2	311 Sonit	4'	-	4-60 Dry Red brown with orange Particles f-M silty sand truce gravel, trace clay (GM-CL)	DPC-none
5-					1 (711-22)	
4-	4			-		



BORING NUMBER: BRP-34

SOIL BORING LOG

PROJECT: Dundal & Marine Terminol

DRILLING CONTRACTOR: ADT

ELEVATION: ~+25

DRILLING METHOD AND EQUIPMENT: Fruste Muth Drill Max 170; Sonic Sampling WATER LEVELS: START: FINISH: LOGGER: MANY Chancy

£	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -	5-2	311 SonrC	4'	NA -	60-70 Roud base/Asphelt	geomembrane & 6.0 DPC-none
7 -	5-3				7.0-8.0 Dry brown f. m silty sand trace gravel (SM)	DPC-none
8 -	-		•	 - -	8.0-8.5 Rood basel asphalt	DPC-none
9 -			:	- -	Brown f-(sand trace gravel, sit (SM)	DPC-none
16.	_			<u>.</u>	9.5-10 Dry and mottled with white Kateley (CH)	DPC-none PP=2.75
	- 44-1	2" 00 544047	13"	5-12 100/4	16-11' Dry mottled with white Fat Clay (CH) (Hard)	PPC-none PP=3.75
12	-54	3 ²¹ SoniL	2'	NA .	11-11'4" Dry very dense red brown highly indicated from sitty sand, true grave 11'4"-12' Dry very dense red Grown highly indivated f. M sitty sand trace grave (GM)(+B)	DPC+ strong refusel DILY advanced with south to 12' DPC+ Strong



BORING NUMBER: BRP-34

Sheet: 3/7

SOIL BORING LOG

PROJECT: Dundal K Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: ~ +25

DRILLING METHOD AND EQUIPMENT: Frost Moth Drill Max 170; Sonic Sampling WATER LEVELS: START: FINISH: LOGGER: Mark Chancy

£	S	AMPLI	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12 -	55-2	54mbr	0	100/5 _	T_{2}^{2}	No sample in spoon
-	5-5	311 Sonic	11	NA	12'5"-13' Same 95 S-4 (HB)	DPC++ very strong
13-	45-3 5-6	Sonil	8"	100/3" NA	13'-13'3" Day Nery dense red f. msilly sand (SM)(HB) 13'3"-14 Same as S-5 (HB)	OPC++ vay strong (etus=1 a+ 13 3 advised w some +>14 OPC+strong
14-	554 5-7	311 Sounic	8".	100/4"	14'-14'4" Same as 55-3 (HB) 14'4'-15' Dry red to gray; sh black f-m silty sand trace gravel [HB) and (GB) intermingled	DPC+ Strong (CDS) all 4" and vanied to 15 willowic DPC++
15-	55-5	Sampur		(00 ₁₁ , -	15.6'- Silty SAND (SM), Red-Brown (7-54R4/3), dry, slightly to moderate indurated, (HBCOPR)	le DPC+ (SS refusal: Advanced the hole to 16' bgs USing 5" DD
16 -	S-8 S5-h	2"07,2 Samp	15,	NA -	Same as above,	16:0'- Ss refusal; Advanced the hole to 17/1598 using 2"10
	S-9	3" sonic	2"	NA	Same as above	Sonic macro cove
17 -	55-7	2" o Der Sampler	54	50/5".	17-0'- Poorly graded sours with silt (SP-CM), dark-gray-black (NZ), wet, Particulate, coarse gradined sand, (GBC OPE)	14-0'-ss refusal; advanced the hole to 18 bgs using 3" ID sonic macro core
18 -	5-10	30 mic	g"	NA .	Same as anove	17.0-groundwater encountered during dritting DPC+

Ch2m:

PROJECT NUMBER: 10587

BORING NUMBER: BP-34

Sheet: 417

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: \sim †25

DRILLING METHOD AND EQUIPMENT: Frost Moth Drill Max 170; Sonic Sompling WATER LEVELS:

START: FINISH: 12:30 LOGGER: Mark Chancy

€	S	AMPLI		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	туре	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18 -	ss-8	Soul,	6"	47 -	18.0'- Sily SAMD (SM), Red-Brown - gray (7:54R 4/3), moist, fine to mentium grained sand, particular (COPR)	DPC 1
į.	559	Semble 5,00	6"	24	Same as above	DPC+
19 -	55-10	200D Sample	- 3"	50/3" -	Same avahove	Prol -Ss refusal: Advanced 3's D Sonic mocrocon to 2001 bgs.
	SYI	31. 52×16	21	NA .	19.5-20 Red brown f. m silty sand, moderately indurated (HB) (5M)	DPC+Strong
20-	35-11	30mbr. 31.00	311	50/3"	20'-20'3" Wet very dense red brown moderately industri f. M sitty sound (SM) (HB)	
	5-12	31' Soul	1	NA -	20'3"-21 Dry led brown moderately into a test f-M 51/ty sixely trace gravel (SM) (HB)	advanced supports to 21
21 -	59-17	2:1 Sampler	10"	22 -	21-21'6" Motst very stiff Red in offled with a words	DPC 6"00 mery to 21' PP=4.5 tof
22-	1		:			2" stradpipe installed Two thermisturs installed
. 30	-			T ₂ .	Thermaneter oir temp 13.5° Thermister Air temp 14.6° 22.7 Thermister 14.62) 19.1	T2 D14 & through COPR T, D17 interface of GB&HB
				T	hermister 14,62) 19.1	
24	1			-		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET Dindulk Marine Terminal FILE NO. **PROJECT** Baltimore Mary land SURFACE ELEV. LOCATION DATUM **BORING LOCATION** Sonic Somp Drill Max 170; SPT Sompling **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED CASING USED TYPE OF BORING RIG **DURING CORING** TO DEPTH, FT. FROM TRUCK **MECHANICAL** DIA., IN. HYDRAULIC DIA., IN. DEPTH, FT. FROM SKID DEPTH, FT. FROM OTHER DIA., IN. BARGE OTHER TYPE AND SIZE OF: YES DRILLING MUD USED **D-SAMPLER** DIAMETER OF ROTARY BIT, IN. TYPE OF DRILLING MUD U-SAMPLER 55-50mpler: 2" Oily Sampl S-SAMPLER YES AUGER USED **CORE BARREL** CORE BIT TYPE AND DIAMETER, IN. **DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. 140 30 AVERAGE FALL, IN. SAMPLER HAMMER, LBS. Automatic TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE CONDITIONS OF OBSERVATION DEPTH OF CASING | DEPTH TO WATER TIME DEPTH OF HOLE DATE PIEZOMETER INSTALLED YES SKETCH SHOWN ON เพก STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. TYPE LENGTH, FT. TIP ELEV. INTAKE ELEMENT: OD, IN. FILTER: MATERIAL OD, IN. LENGTH, FT. BOT. ELEV. **PAY QUANTITIES** NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. DRY SAMPLE BORING LIN. FT. 3.5" DIA. U-SAMPLE BORING LIN. FT. NO. OF 3" UNDISTURBED SAMPLES OTHER: CORE DRILLING IN ROCK LIN. FT. BORING CONTRACTOR Joseph Arrayo DRILLER REMARKS DATE RESIDENT ENGINEER 12-9-15

BORING NO.

BRP-34



BORING NUMBER: BEF -35

Sheet: |

SOIL BORING LOG

PROJECT: Honeywell DAT SSI

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Aquater Drilling and feeting the

DRILLING METHOD AND EQUIPMENT: SONIC SMIF DELL MAR 170; SPT Sampling

WATER LEVELS:

START: 11315 FINISH: 11315 LOGGER: K.Choturyedula

(£		SAMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 -		MANARO CARE			3" Asphall 125'-12'- Poorty groated sand (SB). light groy (CLEY) 71104), day, afternating layers of Asphalt and Sand	Advanced from 01-10' with 3' It some mero con
2-	5-1	SOLVIE AN	re.		1-c'- Lea's sitty carno with gravel (SM), gray-Racum (2:54 574), wast, fire to modium grained sand	pre-
3 -						DPC - 201-ground 2 cales observed on the bodside of the all matricine
4		O CORE			60'-50'- Same as above	DPC -
5	SiV	Sorue Mreke	36	=	5.6'- Polyethaline layer chrounts-of 5.0'-6.0'- Poolly Fraded sand with gravel (SP), Red-white (2.5'12 6/6), moist, medium grained Sand	DPC-



BORING NUMBER: BRP-35

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£		SAMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	02		ERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
DEPTH	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -	5-76	10			6.617.01- Alterrating layon of Asphalt and Pourly graded sand with grant tsp. dray (GLEY) 7/1047, dry	bpc-
9-	S				70'-g.6'- INJ SAMD with gravel (SN), gray-brown (2.54 slu), wet, time to medium gravinal soul	bpc-
1 1 1 1 1 1	5-3	AMERICA CONTO	36"			
1 1 1 1		201405			95'-10.0' tean CLAY (ch), Red (107 4/10), moist, medium plasticity	DPC-
0-1-1-1	55.74	55	***	5 - 15 -	10-0'- Silty SAND (SN), Red-Brown (7578 516), dry, slightly indurated, (HB COPE)	los - Switched to SS sompling below 10'; 140:16 hammed; 30-Indrop; 3"dia, Sampler. Installed the 6" 00 outer (asing to a depth of 10' bgs breat
1-1-1				50 6 -		- haveneed to hale to 13 bgs with 3" IP Source macro core



BORING NUMBER: BRP-35

Sheet: 3

SOIL BORING LOG

PROJECT: LOCATION: ELEVATION: DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH:

(£)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
7-	5-5	Share MACKE	5n ₀		some as above, dry, particulate	Drc+
15	- 55° b	90	25	25 74 30 78	14.5 - 14.5 - same as above 14.5 - 155 - Poorly graded Enno with Sit (58-SM), dark gray black (N2), wet, particulate, bright green particles (GB copp) - 155'- STHy Shrum (SH), Red-Brown greenish (2.57R 5/6), moist, particulate - to slightly inducated, laminated	
16	-55°	ss	2	50/24	Same of show, wet	outer paring to 15' bgs - Advanced the hole to 19' bgt with 3" 10 sons
13	755	12 8	bi	35	same as about , wet	
10	85	55	2	12	some no above, wet	/



BORING NUMBER: BEP 35

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH: LOGGER:

(ft)	S	AMP	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	ER		RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING
DEPTH	NUMBER	TYPE	RECOV	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18 -	Stolo	53	Š ^{tl}	71 -	some as above, greens bartiles	DPC 4-7
9 -	55-11	22	6"	42	18.5'- Poorly graded soud with silt (SPSM) darkgroup-black (NZ), wet, forticulate (AB COPE)	Drc ++
	55-12	53	3"	62/311	Same as allower	DPC +
	5.13	more rock	6"	- 1	1991 - Tal CLAY (CH), Red (DY 4/4)	Movement that I's to coming make there to soid bys .
0 -					Free Luidard black	5" so outs raving
-					Bottom of boring @ 200' bgs.	advanced to so o' Egs
21-				_		-
1						
-				0.0		
ı -				-		
-						
3-				-		
-						*
-						
Na				-		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 www.mrce.com BORING NO. SHEET PROJECT DUNDALK MARINE TERMINAL FILE NO. LOCATION BALTIMORE, MARYLAND SURFACE ELEV. **BORING LOCATION** DATUM Sonic Samp Drill Man 170; SPT Sampling TEST/INSPECTION EQUIPMENT REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING** CASING USED TRUCK MECHANICAL DIA., IN. 00 DEPTH, FT. FROM SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER rapla TYPE AND SIZE OF: DRILLING MUD USED YES D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER CORE BARREL AUGER USED NO YES CORE BIT TYPE AND DIAMETER, IN. DRILL RODS CASING HAMMER, LBS. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. IUD AVERAGE FALL IN. 30 TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION PIEZOMETER INSTALLED YES See Stardpipe NO SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL OD, IN. LENGTH, FT. BOT, ELEV. PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING UN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK UN. FT. OTHER: BORING CONTRACTOR Drilling and Fishin Brian DRILLER hice HELPERS REMARKS RESIDENT ENGINEER K. Charinedula DATE 13

CH	2	RA	HI	11.
US. CO.		DE 1007 HE		

PROJECT NUMBER 10587 BORING NUMBER 37 SHEET | OF 7

SOIL MAME, USES GRIOUP SYMBOL GOLOR, MOISTURE CONTENT, RELATIVE DENSITY OF CONSIST PRIORY, SOIL STRUCTURE, MINERALOGY Asphit Load > CSC H H H H H H H H H H H H	ELS.		EQUIP	,	start 220 Finish soil description	COMMENTS.
Asphatt Road Sesc 4-4'8" Red Grown Siltys and, trace grave fely		/				The state of the s
NA 4-4'8" f.m. trace gravely chy	-1	30			Asphalt/Road besc	Sample under pressure
4-4'8" from siltys and, trace gravel, clay			NA			
4-4'8" f.m. Prace gravel, clay		M		NA		
Red brown Siltys and, trale gravely clay						
-52 311 31 NA 4'8"-53611 Black					4-4'8" f.m. Red brown silty sand, trace gravely clay	
y aver as prairy low saise	12	Sonic	31	NA	4'8" 5'3' Black asphalf Kondbase_	- Geo Membrane



PROJECT NUMBER: 10587 BORING NUMBER: BRP37 Sheet: 2/7

SOIL BORING LOG

5	SAMPL	E	STANDARD	SOIL DESCRIPTION	LÖGGER: Mark Chancy
NUMBER	md Xsi	RECOVERY (fi)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRIBLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
5-2	311 Soni'L	31	-	Asphalt road base.	5-2 photo not taken
5-3	311 Sonic		-	7-7'8" light brown f M sitty send, trace gravel	PPC-
The second secon				78'-8'4" Black walgraded gravel f-1 sand appealt 84"-9'6" Red brown silty fe (sand with green & orange seems some gravel	53 junto met men
	-			96"-10' Brown f- in sund trace	Geosynthe fil Membrane 210
55-	31° 00 55	20'	12-15 -	Mod-highly inclurated grayish white fine to medium sand, som gravel Bott 12 day led Clay mottled with green orange particles Liner	DPC-



BORING NUMBER: BRP37 Sheet: 3/7

SOIL BORING LOG

PROJECT: Pundalk Marine Terminal

LOCATION: Baltimore, MO

ELEVATION: wt 23

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Sonic Samp Drill max 170; SPT sampling

WATER LEVELS:

START: FINISH: LOGGER: Mark Changy

£	SAMPLE			STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF:CASING, DRILLING RATE, DRÍLLING FLUID LOSS TESTS AND INTRUMENTATION
-	SY	311 5121L	21	NA =	Red brown f-M sand with gravel (HB)	DPC+
- 13 - -	35-2	311	N.6 (CCo	4"/50	Nophoto	refosal advanced with some Con
-	5-5	311 SoniL	1	NA :	13-14 Red f-M sand, trule gravel (HB)	13-14 DK++
	为子	211	21	9-24 43-13	Mod to highly industed f-Msilty sandtrake gravel (HB) bill how will the sile gravel (HB) epitts for an interpretation	Top 10" DPC++ Bot+14" DPC+
15 -					Red brown mod indurated with green to orange particles, trace grovel HB	
61.1.1		311		14/11/110	Highly industed grestack with 16-16'10"	0P/ + :
	55-4	OD	香	45 4/100 -	trace grave (GB)	split good from 17-18'
η -	545	311	8"	67 4/08 -	Highly 17-178" Sports orange particles HB M-(Sand trase grave (HB)	ofc+
8-	5-6	311 Sinil	17"	13	178"-18" AN Down C.C.	son' (fom 118"-18"

BORING NUMBER: BEP 37

SOIL BORING LOG

PROJECT: Oundally Marine Terminal

LOCATION: Baltimore, MD

PROJECT: Donda IR Mar INC. 1 DRILLING CONTRACTOR: ADT

ELEVATION: ~ 123

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Son/C Samp Orill Max 170; SPT Sampling

WATER LEVELS:

START: FINISH: Q LOGGER: Mark Chancy O

(Ft)	S	AWPL	£	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW	NUMBER,	TYPE	RECOVERY (#)	TEST RESULTS	SOE NAME, USCS GROUP SYNBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	DEPTH OF CASING, DRILLING HATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18_	5 4	311	, 1	9 /	Dark Brann same as above	pref
(-	5-7	So hil	1	NA	Red from C-M sand with green particles sinc gravel (HB)	OPCT
19 -				-		00.1019414
	46	311	NO TLC	4"/50	640)	advanced with some to 20
-	1		1.51	NA	Bott 1 Red brown claver sand war yours	DPC+
20-	55-7	300	4"	4/100	Highly Inducated Red brownf-C sand somegrand HB	ppct
	5-9	311 Soul	12"	NA	Red brown f. M sand some gravel	advance with source to 21 DPC+ Installed 12-3-15
<i>u</i> -	55-8	3059	3"	7	21-21'3' Red Fat Clay Liner	Thermometer 7.9° Thermister T'017'84° Air
-1						. 19.4 Slu
22 -				et-mail		thermister 12014572 A.
-						19.1 Slu
23-				_		
LY -				Prof.		

New York, NY 10122

Mueser Rutledge Consulting Engineers

14 Penn Plaza - 225 West 34th Street

DEPTH OF HOLE

T: 917 339-9300 F: 917 339-9400

www.mrce.com

BORING NO. SHEET FILE NO. **SURFACE ELEV.**

andalk Marine Terminal **PROJECT** Bultimore, Mary land Sec location Plan **LOCATION BORING LOCATION DATUM** Soule Samp Drill Max 170; SPT sampling **TEST/INSPECTION EQUIPMENT REFERENCE CODES/STANDARDS** BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING CASING USED** NO **TRUCK** MECHANICAL DIA., IN. DEPTH, FT. FROM TO SKID **HYDRAULIC** DIA., IN. DEPTH, FT. FROM то **BARGE OTHER** DIA., IN. DEPTH, FT. FROM TO rack **OTHER** TYPE AND SIZE OF: DRILLING MUD USED YES **D-SAMPLER** DIAMETER OF ROTARY BIT, IN. TYPE OF DRILLING MUD **U-SAMPLER** 55-Sampler; 3" Diam Sampler S-SAMPLER **CORE BARREL AUGER USED** YES TYPE AND DIAMETER, IN. **CORE BIT**

DRILL RODS 3"ID Sonic Macrolore CASING HAMMER, LBS. AVERAGE FALL, IN. 30 SAMPLER HAMMER, LBS. AVERAGE FALL, IN. TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE

DEPTH OF CASING DEPTH TO WATER

	_				
PIEZOMETER INSTAL	IED	YES	NO SKI	ETCH SHOWN ON	
T TEEDINE TER TROTAL	LLD	123			
STANDPIPE:	ТҮРЕ		ID, IN.	LENGTH, FT.	TOP ELEV.
INTAKE ELEMENT:	TYPE		OD, IN.	LENGTH, FT.	TIP ELEV.
FILTER:	MATERIAL		OD, IN.	LENGTH, FT.	BOT. ELEV.
PAY QUANTITIES					
3.5" DIA. DRY SAMPLE B	ORING	LIN. FT.	-	NO. OF 3" SHELBY TUBE SAMPLES	<u> </u>
3.5" DIA. U-SAMPLE BO	RING	LIN. FT.		NO. OF 3" UNDISTURBED SAMPLES	
CORE DRILLING IN ROCK		LIN. FT.		OTHER:	<u> </u>
	_	Agrifer Shick	0.10. 17	.1.	
BORING CONTRACTO	OR	Hyviter	Drilling and Tr	sting	
DRILLER	Brian Karg	Shick		HELPERS Chris Phek	2 n
REMARKS	2 thermist	ors installe			
RESIDENT ENGINEER	Ma	irk Chancy		DATE	12-2-15

CONDITIONS OF OBSERVATION

DATE

TIME



BORING NUMBER: BRP-38

Sheet:

SOIL BORING LOG

PROJECT: HON EYWELL DAT IST

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: AGUIFER DRILLING AND TESTING, ENC

DRILLING METHOD AND EQUIPMENT: SONICSAMP DRILL MAY 170; SPT SAMPLING

WATER LEVELS:

START: 11/19/15 FINISH: 11/19/15 LOGGER: K. Chatur vedula

Œ	S	AMPL	.E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft) NUMBER	~		ERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
	NUMBER	TYPE	RECOVERY	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0-	5-1	SONIE BENEY MACRO COPLE	364		3' Asphalt 0.25'-1.0'- Poorly graded GRANEL with sand (GP), dry, gray GLEYT S/104) ody, mired with crushed Pieces of Asphalt 1.0'- Silty strust with gravel (SM), Red-Brown (104R 416), moist, three to medium grained Sand 2.0'- Geofer ble	Advanced the hole from 0'-81 using 3" ID sonic macro core DPC -
,					3.0'-5.0' - Same as above	DFC-
	gV	Sonte Marko Carce	36		5.6'- clayey sand with gravel (SC), Brown-gray (104124/14), moist, fine to medium grained Savel	DP1-



BORING NUMBER: BRP-38

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

€ _	SAMP		E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
		MALKO CORE			6.01. 7-51 - same at ahow.	DPC-
1-1S	- 1	SONIC MACK	24		25'- Asphalt, multiple layers 2" thick, crushed per pieces.	70'-6" OD outer casing installed to adepth of
					Bottom of Boring Q 8.0' bgs	21 kg-

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BRP- 32 www.mrce.com BORING NO. SHEET PROJECT DUNDALK MARINE TERMINAL FILE NO. LOCATION BALTIMORE, MARYLAND SURFACE ELEV. BORING LOCATION DATUM TEST/INSPECTION EQUIPMENT SONIC CAMP DRILL MAX 170 REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG DURING CORING CASING USED YES TRUCK MECHANICAL DIA., IN. 0.D DEPTH, FT. FROM SXID. HYDRAUUC DIA., IN. DEPTH, FT. FROM BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER TRACK TYPE AND SIZE OF: DRILLING MUD USED YES D-SAMPLER DIAMETER OF ROTARY BIT. IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER DNIC MACED CORP CORE BARREL AUGER USED NO YES CORE BIT TYPE AND DIAMETER, IN. DRILL RODS CASING HAMMER, LBS. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. 140 30 AVERAGE FALL, IN. TYPE OF HAMMER Automati WATER LEVEL OBSERVATIONS IN BOREHOLE DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF DESERVATION 0 8-0 NE CLOUDY DRIZZLE PIEZOMETER INSTALLED YES NO SKETCH SHOWN ON PIEZONETER RELOPD PVC STANDPIPE: ID, IN. 2 LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL 00, IN. LENGTH, FT. BOT. ELEV. PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING UN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK UN. FT. OTHER: **BORING CONTRACTOR** AQUIFER DRILLING AND TESTING, PAVE

HELPERS

CHRIS

11/19/115

PHELEN

DATE

DRILLER

REMARKS

RESIDENT ENGINEER

BRIAN

KARSHICLE

Chaturredula



BORING NUMBER: BRP-46

Sheet: 1/5

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINIAL

LOCATION: BALTIMORE

ELEVATION: ~ +19

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MOUTIDRILL XL MAX

V (ft)		SAMPL	T	STANDARD PENETRATION	N/80/15 SOIL DESCRIPTION	LOGGER: M. CHARLES L-LINCOS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
			S. College	A I I I I I I I I I I I I I I I I I I I	BRN. C.M. SAND, SM GVL, TR. CLAY	DPC-
	2		3'	11/A	4-018 CARK ERN OF TERMO, SM SVL, CLAY	
					5.5 - U DKBRM CLAN	GEOMENIBRANE @



BORING NUMBER: BRP 40

Sheet: 2/5

SOIL BORING LOG

PROJECT: DMT

WATER LEVELS:

LOCATION: BALTIMORE, MD

ELEVATION: ~ + 19

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT:

START: 1400 FINISH: 1600 LOGGER: LUNCOLNI!

					11/30	PA, CHA	
€		AMP	7	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USC\$ GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
ι ·	- 6-2 cons		3.	N/A	G. RED. FIRM M.C SAND,		
า -	-	3" 50110		,	7-8 HON DK ERN CLAY		
3 -	5-3		2'4"	N/A -	8. GRY, GRY. GREEN C. M SANID, SMGVL (ROAD BASE	WOVEN FABRIC	
٦ -				- - -	9.5-10 RED CLY, SMGN		
D -	S<-1	W.		٤ 1	CLAYEY C. M. SAND, SM GW. BOT 7" RED CLAY MOTT.	DPC-	
- - 12-	5-5			50/5"	F. M. SAND (HB) RED BRN HIGHLY INDURANT F. M. SAND (HB) RETO BRN SILTY F. M.	ED DPC7	



BORING NUMBER: BRP.40

Sheet: 3/5

SOIL BORING LOG

PROJECT: DMT

LOCATION: BALTIMORE MD

ELEVATION: ~+19

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAY 170

WATER LEVELS:

FINISH: 1400 LOGGER: M. CHAMON START: 1400 11/30/15

₽	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
ELOW (RY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12 -	55.10	3" 5.5"	U*	99 _	12-12-5 HIGHLY INDURATED RED-BRN, BRN, GRY SILTY M-F SAND,	DPC+7
- 13 -		302		-	TR GUL, GREEN PARTICI 12.5:13.2 DUSSILG (HB)	EJ
-	5,7		. X	N/A	13.2-13.7 BLK M. F SAND, TR. GREEN PARTICLES (GB) 13.7-14 DO SS-16 (HB)	
14 -	55-8	305.5.	3"	50	141-14.5 DK GPY M-FSAND, SM GREEN PARTICLES (GB)	WATER IN DPC+
15 -	51	8001K	∖ ∂"	H/A	14.5.16 MOD-HIGHLY INDURATED RED.BRN C.F SAND, SM GVL, TR. GREEN PARTICLES (HB)	14-14,5 DPC+
- 16 -		3"		-	16.16.75 BRN, RED BRN	DPC+
1	55- 16	0.D. 55.	12"	3 -	GULY C. M. SAND	
[] -				-	THE WHITE	EOB @ 17.0'
- - -				_		

14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET Dundalk Marine Terminal **PROJECT** Baltimore, Maryland See boring location plan LOCATION **SURFACE ELEV. BORING LOCATION** DATUM Sonic samp Orill max 170; SPT sampling **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING** CASING USED **TRUCK** MECHANICAL DIA., IN. DEPTH, FT, FROM SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM **BARGE** OTHER DIA., IN. DEPTH, FT. FROM OTHER Track TYPE AND SIZE OF: DRILLING MUD USED YES **D-SAMPLER** DIAMETER OF ROTARY BIT, IN. **U-SAMPLER** TYPE OF DRILLING MUD 35-Sampler: 3" dia sampler S-SAMPLER **CORE BARREL** AUGER USED **CORE BIT** TYPE AND DIAMETER, IN. **DRILL RODS** 3"ID Sonic Macrocore AVERAGE FALL, IN. CASING HAMMER, LBS. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. 30 TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE | DEPTH OF CASING | DEPTH TO WATER CONDITIONS OF OBSERVATION VW Piezo meter Installation Record pg 4/5 PIEZOMETER INSTALLED SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL OD, IN. LENGTH, FT. BOT. ELEV. **PAY QUANTITIES** 3.5" DIA. DRY SAMPLE BORING I N FT NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING LIN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK LIN. FT. TOR Acroifer Prilling and testing, Inc Brian Karshick HELPERS 1 vw piezometer and 2 thermistors installed R M. Chancell Lincoln **BORING CONTRACTOR DRILLER** Chris Phelan REMARKS RESIDENT ENGINEER

Mueser Rutledge Consulting Engineers

BOR-4_JAN2013

BORING NO.

BRP-40



BORING NUMBER: BRR41

Sheet: 1/7

SOIL BORING LOG

PROJECT: Dundalk Marine Terminal

LOCATION: Baltimore, Maryland

ELEVATION: ~)니

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Sonic Samp Drill Max 170; SPT Sampling
WATER LEVELS:
START: 09: 45 FINISH: LOGGER: M. Chancy

	T .				12-4-15	LOGOLIN. MI. Chancy
2 (3)		SAMPI	1	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0-	5-1	311 Sonic		NA -	Roud base asphalt, well graded	DC-none
-			1.51	-	gravel	6"00 casing to 5'
-				-		
1 -						
				- -		
2 -				-		
				-		
				-		
3 -				- - 	·	
				-	35-40 0. Pala (14. CM c. d	
				-	3.5-4.0 Bry Rad brown Silty f-M sand tracegraves (SM)	DPC- none
4 -	5-2	311 Souic		NA	4.0-6.6'	
		50000	3	///	Dry Red brown with orange particles Silty f-M sand trace gravel	DPC- none
				4	(SM)	
5-				7		
		-		1		
6-		l				Geomembrane @ 600



BORING NUMBER: BRP-41

SOIL BORING LOG

PROJECT: Dundalk Marine Terminal

LOCATION: Baltimore, Maryland

ELEVATION: ~ + 14 DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Sonic Samp Drill Max; SPT samples

WATER LEVELS:

START:

FINISH:

LOGGER: Mark Change

	_			071110100		The Control of the Co
√ (ft)	<u>s</u>	AMPL	Ι _	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -	5-2	311 Soull	31	NA -	60-7.0 Dry red brown with orange Particles Silty f-M sand, trace gravel (SM)	DPK- none L" OD casing to 10'
7 -	5-3	311 SoniC	2.5	NA :	7-9.0 Damp Dark brown with orange Particles f-M Silty sand (SM)	DPC - none
8 9	·			- - -		
				_	9.0-9.5 Dry whitish concrete with gravel, f-M sand (S) 9.5-100 Dry Red Fat Clay (CH) (top Clay liner)	DPC-none
-	55-1 54	20050 300 50 00 50 00 50 00 00 00 00 00 00 00 0	2.5'	45-100/2" _ N4 -	10-10.5 Dry Rod brown Highly Indirated Medium to Course Sand, trace gravel (6) HB 10.5-12 Dry Red brown M-C sand with	Refuse 10 10.5' Advance wissonic to 12' DPL+ strong DPC+ strong
12-	-	Gara l		- -	gravel (5) (HB)	J



BORING NUMBER: BRP41

Sheet: 3/7

SOIL BORING LOG

PROJECT: Dundalk Marche Terminal
ELEVATION: + 14 DRILLI

LOCATION: Baltimore, Maryland

DRILLING CONTRACTOR: ADT DRILLING METHOD AND EQUIPMENT: Sonic Samp Drill Map 170; SPT sampler

WATER LEVELS:

START:

FINISH: 01-15 LOGGER: Mark Ch

				•	, , , ,	51AR1. FINISH: 01:15	LOGGER: Mark Chancy
	V (ft)		SMIP	T	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
0	DEPTH BELOW	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	. SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
î	12 -	55-2	200 55	1.5	41-22 -	12:0-14 Dry and brown much rately indirected f-C sand, trace gravel (5) (HB)	Mrc 1 show
		à			7		to 151
***************************************	, ,				3-11 -	13.0-14.0 Dry red brown with green particles f-M Sand (5) (GB)	OPC+ strong
	14 -	95-3	2"00 65	5"	´]	14-14'5" Pense mol'st Brown for sand, true silt (SM) (IGB)	(Asul 2 14'5" DPC + Strong
	15		3" Sonic	1.5	, -	145-15.0 MOIST Red Brown with orange Silly f-M sand trace gravely trace clay (SM-CL)	DPC+ Strong
		554 5-6	211 60 311 Somil	3"	NA -	5-15'3" wet verydense brown Silty f-m sand (SM) (GB) 6'3"-160 Maist Red brown Sitty Sand with Gravel (SM) (HB)	DPC tstrong Souric Sample saturated DPC tstrong
	16-15	5-7	dio 3" Sort	114"	$\Lambda \Lambda \Lambda = 17$	(HB)	OPC+ strong (etusal 162' rod just bancing advance with sonic to 17' Sonic Sample saturated
	7 - 5	_0	3"	None	17	7-175 moist red brown f-c silly sand trace fravel (SM) (GB)	Fod buners 100/0" pact advanced with somic to 18'
	18 –		SoniC		NA -	5" 12"	OPC+ Strong



PROJECT NUMBER: 1658 7

BORING NUMBER: BRP4

SOIL BORING LOG

PROJECT: Rudall Marine Termine?

ELEVATION: ~+ 14

LOCATION: Baltomore, Maryland

DRILLING CONTRACTOR: AOT

DRILLING METHOD AND EQUIPMENT: SONIC Samp On'll May 170; SPT Sample.

WATER I EVELS:

START: FINISH: LOGGER: Mark Change

	S	AMPL	F	STANDARD		
<u>₹</u>			·	PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТУРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18'-	557	2" GD	3"	8/6" -	18'-18'3" Dry loose Red mottled with white streaks (CH)	OPC+
-	~			-	streaks (CH)	Borny terminated a 186"
-				-		During terminated w 186
-				-		
-				_		Thermisture installed
			7	-		10 13'
-				_		w 7 5
-				-		ا سر سرا
				-		15.5
				•		
-				_		
-				-		
-				-		
				1		
				_		
-				_		
-				4		
				1		
]		
				4		
				4		
-				4		
				7		
]]		
				_		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET Dundalk Marine Terminal FILE NO. PROJECT Baltimore, Maryland See livetion plan SURFACE ELEV. LOCATION DATUM **BORING LOCATION** Sonic Samp Dill Max 170; SPT sampler **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED CASING USED **DURING CORING** TYPE OF BORING RIG MECHANICAL DIA., IN. DEPTH, FT. FROM **TRUCK** HYDRAULIC DIA., IN. DEPTH, FT. FROM SKID DEPTH, FT. FROM **OTHER** DIA., IN. **BARGE** OTHER YES TYPE AND SIZE OF: DRILLING MUD USED DIAMETER OF ROTARY BIT, IN. **D-SAMPLER** U-SAMPLER TYPE OF DRILLING MUD 55-Sampler; 2" diam sampler S-SAMPLER AUGER USED **CORE BARREL** TYPE AND DIAMETER, IN. **CORE BIT** DRILL RODS CASING HAMMER, LBS. AVERAGE FALL, IN. 140 30 SAMPLER HAMMER, LBS. AVERAGE FALL, IN. Automotic TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE CONDITIONS OF OBSERVATION DEPTH OF HOLE DEPTH OF CASING | DEPTH TO WATER DATE TIME SKETCH SHOWN ON PIEZOMETER INSTALLED LENGTH, FT. TOP ELEV. STANDPIPE: TYPE ID, IN. TIP ELEV. LENGTH, FT. INTAKE ELEMENT: **TYPE** OD, IN. OD, IN. LENGTH, FT. BOT. ELEV. FILTER: MATERIAL

BORING NO.

NO. OF 3" SHELBY TUBE SAMPLES

NO. OF 3" UNDISTURBED SAMPLES

Tough

DATE

BRP-41

DRILLER REMARKS

PAY QUANTITIES

3.5" DIA. DRY SAMPLE BORING

3.5" DIA. U-SAMPLE BORING CORE DRILLING IN ROCK

BORING CONTRACTOR

RESIDENT ENGINEER

LIN. FT.

LIN. FT.

LIN. FT.

TOR Aquifer Drilling and Testing
TUNY HELPERS

12 thermistors installed

ER Mark Chancy



BORING NUMBER: BREVE

Sheet: 1

SOIL BORING LOG

PROJECT: Horey all DMT SET

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Aquifer drilling and teching Inc.

DRILLING METHOD AND EQUIPMENT: SOME SHAN DRILL NAM 170; SPT. Sampling

WATER LEVELS:

START: 11 1115 FINISH:

ISH: LOGGER: Chattavedula

(ft)	3	SAMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER		RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,	
DEP	NUM	TYPE	REC	6" - 6" - 6" (N)	STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION	
	51	Spaint Mitchia copts	12"		Poorly Product Sent with Silt and gravel 151-500, Bray, brown Gilly my day, fine to midium grained sand	Advance from al 10' with some manneau 3" 20 DPC -	
4	c.v	SOUT MRCEL CORE	361		4.75' gentratile fabric encountered 4.75' - gentratile fabric encountered 4.75' - 6.5' - 5.14's sound (SM), with trace growel, dark brown group (BAR) moist, fine growned sound 6.5' rose tolgethylese three encountered 6.5' - 2066-2" Asphalt 6.6' - 5.14y sond (SM) with good Fid-Brown (25.18.316), dry office grained Sound	DP1 = 3/2)	



BORING NUMBER: BRP-43

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	5	AMPL	LE	STANDARD	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
	5.3	SOME MARROCORE	7,6		4.01-8.51- Poorly graded GRMEL Light Sand (Sp), gray-shi te (GR) 1 dry, Road based aggregate. 4/109) 8.51-991- Poorly graded SMO (SP), light Red-white (STRE) yry, medium to coarse grained sand 1.9-51-51-ty EAND (SM), Red-white (STRE) dry, fire grained sand	DPC =	
	55° ³	5%	74"	19	Sound and sold (Grant, dark gray, &	10.0 - Bolow to switched to SS sampling, 140-16 hom 30 in drop : 3 dies Sampler sur) SYR 5/6)	



BORING NUMBER: BRY-43

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£	S	AMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
13-	82-5	100	NR	100 20	Silty Shoro (SM), Red-Brown (754 4/6) dry, thre to medium grained sand, Particulate to single by Indusored (Mr. core)	- Advanced the some mocrecort to 14 bgs - No Recovery in split	
4	Strife	55	121	8 6 14 22	Theof - Poorly graded sone with SH (SRSM), dorn gray-black (NE) Moish. Particulate, bright Jam particles (SR core)	- encountered groundward at 151 bgs during drilling	
6-1-1-1	55-7	55	c,"	100/4"	Redistrict day faithful are break- stigle indicated incided (HE CHE)	- havanced the 31 to sonit make the to	
7-	55 B	ŠŠ	10 to 10	10	No receiving		
7	6-22	33	5"	100 24	175'- Pourly emoled simils with six (s dorse good-house (ne) weet, Partico bright street partition (are carr)	este	



BORING NUMBER: BRALLS

Sheet: 14

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	TEST RESULTS SOIL NAME, USCS GI COLOR, MOISTURE CO		SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
2 -	55-7	55	NE	50/0" -	Same as above, dry , particulate	Advanced the some mount
414	واءي	دن	6"	56	same as above, particulate to slightly indurated, laminated	
9-	2541	52	3"	100)3"	same as above, particulate, wet, bright green and tellows Particles	Advanced the somer many cort to so bgs
-	5-12	S	6"		come as about, parkentake to moderately Indurated, laminated	20-d-Installed to 6"01
20-	51-11	2.2	616	11	Same as about, particulate to	Dollage
-	(I-14	23	6"	8	dry the ground sand	414) bpc-
-					Bottom of honing @ 21-0' bgs	
1 1				-		
1						
-				-		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 www.mrce.com BORING NO. SHEET PROJECT DUNDALK MARINE TERMINAL FILE NO. LOCATION BALTIMORE, MARYLAND SURFACE ELEV. **BORING LOCATION** DATUM TEST/INSPECTION EQUIPMENT Sovic Samp Dail Map 170; SPT-Sampling REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG DURING CORING CASING USED TRUCK MECHANICAL DIA., IN. OD DEPTH, FT. FROM TO SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM TO BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER Track TYPE AND SIZE OF: DRILLING MUD USED YES D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER CORE BARREL AUGER USED YES V NO CORE BIT TYPE AND DIAMETER, IN. DRILL RODS 2 100 Sovie Hack Core CASING HAMMER, LBS. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. AVERAGE FALL, IN. TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION 11/11/15 10 15 Duning prilling Sce Standplpe Installation le q PIEZOMETER INSTALLED YES NO SKETCH SHOWN ON STANOPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE CO, IN LENGTH, FT. TIP ELEV. FILTER: MATERIAL 00, IN. LENGTH, FT. BOT. ELEV PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING UN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK UN. FT. OTHER: **BORING CONTRACTOR**

BORING NO. BRP-43

Phelen

DATE

DRILLER

REMARKS

RESIDENT ENGINEER

Balan

Chaturvedula



BORING NUMBER: BRP-47

Sheet:

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILLIXL MAX 170

WATER LEVELS:

START: 0950 FINISH: 1300 LOGGER: L LINCOLN

- 11	1	Į١	ĺ	ı	5

3	SAMPLE			STANDARD	11/11/15	
S		PENETRATION		PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (#)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
2 -	S´	CORE	24°	~ / A	4" ASPHALT BRN, DK GRY SILTY F.C EAND, SM GW (FILL) GRY FC SAND, SM GW, TR. SILT (RDAD BASE AGGREGAT	DPC-
3 u 5 v	5-2	3" O.D. SONIC		H/A	4-7.0 DK BRN, RED, DK GRY F.C SAND, SM GVL, CLAY (FILL)	DPC-



BORING NUMBER: BRP-47

Sheet: 2

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION:

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS:

START: 0950 FINISH: 1300 LOGGER: LLINICOLM

£	SAMPLE		STANDARD	SOU DECCEPTION			
3			€	PENETRATION TEST	SOIL DESCRIPTION	COMMENTS	
DEPTH BEI OW (#)	NOW BER	TYPE	RECOVERY (#	RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
4	5.7	x ·	3	- N/A -	BRN, RED, DEGRY FIC SAND SM GVL, CLAY (FILL)	DPC-	
7		CORCE			7-8 DO S-Z		
8	- S-3	3" SOVIC	3	71/14	8.8.5 ASPHALT 8.5.9 GRY C.F. SAND, SM. GVL (ROADBASE) 9-10 DK GRY, GRAN-ELVE F.C. SAND, SM. SILT, GVL (ROAD BASE)	DPC- DROVE DPC- CASING TO 10'	
10		2" SPOON		8 -	SAND, TR. GVL	DPC-	
12-	- 55-W				11.2. 11.5 DK BRM, RED SILTY F.M. SAND, SM. GW., WUSD FRAG. 11.5-12 GRY, GREEN, BRN OF SAND, SM. GW., TR SILT	DPC- FABRIC DEOTEXTILE OU.8'	



BORING NUMBER: BRP-47

Sheet: 3

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION:

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS: \

START: 0950

FINISH: 1300 LOGGER: L. LINCOLN

Γ	£	SAMPLE		STANDARD		1	
	W (ft)		11411	1	PENETRATION	SOIL DESCRIPTION	COMMENTS
	рертн весом	NUMBER	ТҮРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
	2 -		S. D. S. S. S. S. S. S. S. S. S. S. S. S. S.	197	39 -	12-12.5 BRN, DK GRY C-F SAND, SM BM (FILL) 12.5-12.8 RED CLAY MOTT, WHITE	DPC. PP=1.6
\\\\ \frac{1}{3}	3 -	ss:5		\^(- - -	12.8-14 GREEN, BRN, GRY F.M. SAND, TR. SILT, WEAKLY INIDURATED (COPE)	DPC+
	. 1						
1/4	- 4	55.U		1"	50/1"	14-14.6 BLK, GREEN M-F SAND, TR. YELLOW PARTICLES (GB)	DPC+ BROKEN WINDE
15	+	5,7	SOLTIC SOLTIC	17"	NIA	H.U-15.5 BRN F-C SAND, TR GVL, SILT, MOD INIDURAD (HB COPR)	FRAGMENTS @14.L'
14	+		5.00 5.00		- 30 -	SILTY F. M. SAND, SM. GRY SILTY F. M. SAND, SM. GM. TR. YELLOW PARTICLES, MOD. INDURATED (HB COPP.) IU-1U-4 DK BRN SILTY M-FSAND (HB COPP.)	DPC1
17	, _	درجی			(el) - - Bo -	IN.Y. IV. 7 BLK, GREEN M.F GAND TR. YELLOW PARTICLES (GB) IV.7. 17 RED. BRN, DK GRY SILTY F.M. SAND, MOD LITH. (HB) TIO- 17.3 RED. BRN CLAYEY F.M. EAND, TR. GV	DPC [†]
149	- - - -				18 -	17.3.18 GRY, GREEN, RED. BRN SILTY F.M SAND, TR. GM, YELLOW PARTICLES, MOD. INDURATED (HB)	DPC+



BORING NUMBER: BRP.47

Sheet: 식

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +20

DRILLING CONTRACTOR: ADJ

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: 0950 FINISH: 1300 LOGGER: L. LINICOLN

	-	A BADI		1	L. LINCOCK	
\ (£)	- 3	AMPL	1	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
195 - -	55:9	2000 J	;· Le"	24 - -	18-18.5 DK BRN M-F SAND, TR SUT (HB) MOIST	DPC+
n -	- 55-W		じ	32 -	185-19 RED BRN, BRN, DKARY SILTY IN-F SAND, SM GNL, TR RED CLAY (HB) MOIST, WORKLY INDURATED	DPC1
-	555 II	555 N		17	DK BRN, RED BRN C.F. SAND, SM SILT, TR GIV., RED CLAY (HB) MIST MAKLY (NDVRATED	DPCT
1	_{SS} -12		3"	50/3" -	BRN SANDY GUL, TR SILT	DPC+
20 -	5-13	3"SONIC	(ę u	N/A	RED. BRN, BRN, DK GRY SILTY F.M. SAMD, TR GN, MOD. INDURATED	WOOD FIBERS (?)
21 -	55-1 ^{LA}	85021/ 0.D.		7 -	RED CLAY MOTT, YELLOW, WHITE TR. FIM BAND SEAMS	EUB @ 21.0'
_				- - -		
22 -				-		
-				- - -		
3-				= = = = = = = = = = = = = = = = = = = =		
21-				-		

ŧ	New Tork, INT TO	122
J	T: 917 339-9300	F: 917 339-9400
	www.mrce.com	

** wi	w.mrce.c	om	333-5400				BORING NO		3RP-4-	
PROJECT	T~~(INITIAL	K MAR	INE T	FOMIL	.i Δ1	FILE NO.	1059		<u></u>
LOCATION		ALTIM		RYLAN		471-	SURFACE E		~ + 20	^
BORING LO			E PLAN		9.3	<u> </u>	DATUM			
				<u> </u>				***************************************	*******	
TEST/INSPE	CTION EQ	UIPMENT								
REFERENCE	CODES/ST	TANDARDS	The second section of the second section of the second section of the second section s		ngapa-pagapangapangapan akhishisha asika	ومرور والمراجع والم والمراجع والمراجع والمراجع والمراجع والمراجع والمراجع والمراجع و	And the second stranger of the second strangers	in designation des	- i deje si sa riska minima kilomining di	
BORING EO	UIPMENT		DS OF STABILIZIN	<u>G BOREHOLE</u>	•					
TV0F 0F 000	INIC DIC		OF FEED		sano neen			1		
TYPE OF BOR TRUCK	ING KIG		G CORING		ASING USED	/ L VI	ــــــــــــــــــــــــــــــــــــــ	YES	NO	n 1/=
SKID	. /	HYDRA	ANICAL		IA., IN.	4"	DEPTH, FT. F		<u> </u>	
BARGE		OTHER	***************************************		IA., IN. IA., IN.		DEPTH, FT. F		T(
OTHER		OTHER		U			DEPTH, FT. F	NUIVI		
OTTIER.		 								
TYPE AND S	SIZE OF:			Di	RILLING MUD	USED		YES	No	
D-SAMPLER		0.D S	PLIT SPOON			ROTARY BIT, IN.	L	,	1	
U-SAMPLER					PE OF DRILLI	NG MUD		·	·····	
S-SAMPLER	**********	***************************************							***************************************	
CORE BARRE	L 3"	SONIC		AU	UGER USED			YES	NO	
CORE BIT				TY	PE AND DIAN	AETER, IN.				
DRILL RODS		····								
				C	ASING HAMM	ER, LBS.	-	AVERAGE F	ALL, IN.	Salaharan Berkel Wittermann
				54	AMPLER HAM	MER, LBS.	140	AVERAGE F	ALL, IN.	30
				TY.	PE OF HAMM	1ER				
MATERIES	ADCCO	/ATIONIC 181 0	00511015							
WATER LEV	EL OBSEK	VATIONS IN B	UKEHULE							 -
DATE	TIME	DEPTH OF H	OLE DEPTH OF C	ASING DEPTH T	O WATER	TER CONDITIONS OF OBSERVATION				
						***************************************	4.A. 4.Q			
		-								

				l						
				 1						
PIEZOMETE	RINSTALL	ED (YES	NO	SKETCH	SHOWN ON	E	<u>'. (, </u>		
			7.10					- 0		
STANDPIPE:	·FaiT	TYPE	PVC), IN.		IGTH, FT.	20	TOP ELEV.	~+19.0
INTAKE ELEM	IENI;	TYPE	PVC	*	D, IN.		IGTH, FT.	5	TIP ELEV.	~ 0.0
FILTER:		MATERIAL		O	D, IN	LEN	IGTH, FT.	an little for some or find the species of white the service of	BOT. ELEV.	open suppress, ordered) of tradented Material
DAY CHIAND	FITIEC									
PAY QUANT 3.5" DIA. DRY		NOING.	LIN. FT.		etr	n nr 35 curi n	V THIRE CANADIC	_		
3.5" DIA. U-S			LIN, FT.		-		Y TUBE SAMPLE TURBED SAMPL			
CORE DRILLIN		1110	UN. FT.	4 -1	-		I UNDED SAMPL	E3	***************************************	
TOUC DUILD	TO ST DUCK		uiv. rl.		0	THER:				
BORING CO	NTRACTO	R	AQUIFE	e too.	111512	2 ANID	TEST	114		
DRILLER		,	LOMEQUE			ELPERS	DYLA		EWELL	and or a company of a special Kalling
REMARKS	_101	~7_IB	CUMERUCE				DICA	<u> </u>	<u> </u>	and the state of t
RESIDENT E	NGINEER	1540	ANDRA L	INICOL-	N	· · · · · · · · · · · · · · · · · · ·		DATE	11/11/15	
			A 71 1	114000	<u> </u>	······································		, 	-141112	



BORING NUMBER: BRP-48

Sheet:

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +20

DRILLING CONTRACTOR: ADT

WATER LEVELS:

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIPRILL XL MAX 170

START: 0850 FINISH: 0905 LOGGER: L. LINCOLN

₽	⊋ SAMPLE		F	STANDARD	4/4/15	
€ ≥		- 11111	r	PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
2 - 3	<i>S</i> ¹		3	N/A	BRN SILTY FM SAND, SM	DPC-
	S-Z		3'	N/A -	GVL	DPC-



BORING NUMBER: BRP-48

Sheet: 2

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH: 0905

		AMBI		STANDARD	0(03	T		
-€-	S	AMPL		STANDARD PENETRATION	SOIL-DESCRIPTION	COMMENTS		
DEPTH BELOW (ft)	NUMBER	RECOVERY (#) STATE (N) .999 (#)		TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION		
				-	6-7.5 BRN C F SAND, SM GVL, SILT (SURCHARGE FILL)	DPC.		
17-	5-3		24"	N/A -				
8				_	7.5.8.0 BRN CLAYEY F. M SAND, SM GVL (FILL)	DPC-		
				_		EDB @ 8.0'		
				-				
				- -				
-				<u>-</u>				
				- - -				
				-	·			
				-				

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. BRP- 48 www.mrce.com SHEET DUNDALK MARINE TERMINAL FILE NO. **PROJECT** SURFACE ELEV. ~ +20 LOCATION BALTIMORE DATUM **BORING LOCATION TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED CASING USED **DURING CORING** TYPE OF BORING RIG DEPTH, FT. FROM TO MECHANICAL DIA., IN. TRUCK TO DEPTH, FT. FROM DIA., IN. SKID HYDRAULIC DEPTH, FT. FROM TO DIA., IN. BARGE OTHER OTHER √ NO YES TYPE AND SIZE OF: DRILLING MUD USED DIAMETER OF ROTARY BIT, IN. D-SAMPLER TYPE OF DRILLING MUD U-SAMPLER S-SAMPLER ✓ NO YES AUGER USED **CORE BARREL** 3" SONIC TYPE AND DIAMETER, IN. **CORE BIT** DRILL RODS AVERAGE FALL, IN. CASING HAMMER, LBS. HO AVERAGE FALL, IN. SAMPLER HAMMER, LBS. 30 TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION DEPTH OF HOLE DATE TIME 6.7' 0 1/11/15 1300

PIEZOMETER INSTALL	<u>ED</u>	VES [NO ŞKE	TCH SHOWN ON	P. 4	
TANDPIPE:	TYPE	2	ID, IN.	LENGTH, FT.		TOP ELEV.
NTAKE ELEMENT:	TYPE		OD, IN	LENGTH, FT.		TIP ELEV.
HLTER	MATERIAL		OĐ, IN.	LENGTH, FT.	Substantistic and interesting	BOT, ELEV.
PAY QUANTITIES 3.5" DIA, DRY SAMPLE B		LIN. FT. LIN. FT.	A Property	NO. OF 3" SHELBY TUBE SAM		
3.5" DIA. U-SAMPLE BOR CORE DRILLING IN ROCK	.5" DIA. U-SAMPLE BORING CORE DRILLING IN ROCK			NO. OF 3" UNDISTURBED SA OTHER:	WALE?	
BORING CONTRACTO	R	AQUIFER	DRIWING	AND TESTIN	16	
DRILLER	MAN	PALOMEG	100	HELPERS DY	LLON	JEWELL
RESIDENT ENGINEER	KL	SANDRA	LINCOLM		DATE	4/4/15
						1 1



BORING NUMBER: 97-10

Sheet:

SOIL BORING LOG

PROJECT: Toneywell DMT 15

LOCATION:

DEPTH BELOW (ft)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6"-6"-6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
1	Ç-l	SONIE MACIZO.	34		ti" Asphalt 0.3- Crushed Cortrele, Churks 2-5" dia 13 concrete:	Advanced the borchele from of 21 using 30 to sonic mains done. -Hard drilling: Contrete pounder thying from the macro core as the Sample is collected DPC-	
3 -					3-0' - silty sand (cm), dans brown (2-51R 3/3), moist, fire to medium sand	Dpc-	
5-	Ç-).	SIC MATER	118.		hol-same as above, with graves		
		20802			(9 SYR 4/6), with grave	DPC-	



BORING NUMBER: GP. 16

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6"-6"-6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
	5-7	If MMTRocang	rr."		Gra- some as about with	DPC=
-		Soult			7.8. clayer sprop (se). Ped-Yellows (107714), most, the terredium Sand, trace gravel	
	523	55	20"	28	8.0'-8.2'. Fot CLANCON, Red-White day, very shift 8.2'. Silly shrows entingravellish. Gray brown-real, moist, time to indian grained sand greenish particles cropp and non-copy	DDC +1=
0-	çe ^{yl} l	55	6"	2,5 41 8	To of - Poorly graded sand with clay and gravel (sp.sc), Red- Brown - group, wet, medium to (oan's grained sand (cope and hon-lopp)	100- G'op outer casing Installed to 10 bgs. 10 o'- groundwater encounter during drilling Drett-
2-				4		



BORING NUMBER: GILLO

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

DEPTH BELOW (ft)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS	
	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6"-6"-6"(N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
12-	35	CT.	8"	10	12-0'- Poorly graded sand (Sp. daric gray (alexi alian), well medium graded sand	DPC-	
3-	ssolo	95.	121	21	13:01-Clayer sand with growel (sc), Red-Brown (7:54R416), wet, fine to medium grained sand	DPE -	
IM-	53-7	22	U ^q	7	the's same as above.	DPC-	
- 2					Bottom of boring P130 bgs		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET DUNDALK MARINE TERMINAL FILE NO. 10587 PROJECT SURFACE ELEV. LOCATION BALTIMORE, MARYLAND DATUM BORING LOCATION TEST/INSPECTION EQUIPMENT REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED YES NO DUR:NG CORING CASING USED TYPE OF BORING RIG DIA, IN DEPTH, FT. FROM MECHANICAL TRUCK TO DEPTH, FT. FROM HYDRAULIC DIA., IN. SKID DEPTH, FT. FROM TO DIA., IN OTHER BARGE OTHER NO YES DRILLING MUD USED TYPE AND SIZE OF: DIAMETER OF ROTARY BIT, IN. D-SAMPLER TYPE OF DRILLING MUD U-SAMPLER S-SAMPLER NO YES AUGER USED CORE BARREL TYPE AND DIAMETER, IN. CORE BIT DRILL RODS AVERAGE FALL, IN. CASING HAMMER, LBS. Sonir marro Lore AVERAGE FALL, IN. SAMPLER HAMMER, LBS. 4-0 TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE CONDITIONS OF OBSERVATION DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER TIME DATE 12.0 00 during diffling 1218/15 11.23 10.0 VWP Tratallation NO SKETCH SHOWN ON PIEZOMETER INSTALLED ID, IN LENGTH, FT. TOP ELEV. TYPE STANDPIPE: TIP ELEV. OD, IN LENGTH, FT. INTAKE ELEMENT: TYPE BOT. ELEV LENGTH, FT. 00, IN. FILTER MATERIAL PAY QUANTITIES NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. DRY SAMPLE BORING LIN FT. NO. OF 3" UNDISTURBED SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. CORE DRILLING IN ROCK UN. FT OTHER: and secting tre-**BORING CONTRACTOR** HELPERS DRILLER 3 notalled REMARKS 12/18/18 DATE RESIDENT ENGINEER 9P-10 BORING NO.



BORING NUMBER: GP. SA

Sheet:

SOIL BORING LOG

PROJECT: Honeywell DM7 SST

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Aquifer drilling and testing Inc.

DRILLING METHOD AND EQUIPMENT: SUNIC MAR SMALP DELLE MAR THE; SIM Sampling

WATER LEVELS:

START: 11/2/15

FINISH: 11/2/15 LOGGER: K. Chaturredula

DEPTH BELOW (ft)	5	AMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
	NUMBER	ш	RECOVERY (#)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND	
DEP	NON	TYPE	REC	6" - 6" - 6" (N)	STRUCTURE, MINERALOGY	INTRUMENTATION	
) - 1 - 1	SH	Deb MACRO CORE	26"		2" Asphalt 0.21-1.21 - Poorly graded gravel with SAND (ap), gray-black/104R4 dry, road based aggregate 1.21-1.51 - Fat CLAY (CH).	Advanced to 3'bgs with 3'ID some macro core	
-		ige.		-	Red (104 0-14), moist, highly plash		
11111		Source			1.5'-3:6'- Silly SAND (SM), Brown Reddish-JHERN (10 YK 4)3), dry, fine to medium grained sand, particulate to slightly indusated (COPF)	cample tested with diphenyl Carbands for the presence of COPIE	
-						+ 2	
111					Same as above, higher fines context	Advanced the 2026 Some mores core toom	
-		CORE		-			
-	52	ACES CO	24	7			
-		SOM 21 MOZ					
-							
-				1			
-				-			



BORING NUMBER: GP-5A

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

3	S	AMPL	E	STANDARD SO PENETRATION SO	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	R		RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEFTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,	
DEPTH	NUMBER	TYPE	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION		
6-		3765			same as above swet	ground water observed at 6-01 tigs during drilling. Advented the 3" cools main som 6-91	
8	5	SONIE MALER	32"		gas!-8'- silty struc (sn), Clark gray and black (N2), wet, medium grained sand perticulate (GB copp) 8:0'-9:0'- sity struc (sn), Red-Brown and green (10489/3), most particulate to slightly inducated. (HB copp)		
۹ -	gg.iq	5.5	5"	21	Black and Red - Brown (1048 4/3), Wet, infranciscod layers of GE and HIS COPPE, while charelessed part	to a depth of 16' bys.	
10-	55-5	\$5	15,1	15 48	to 0'-10.75'- Sily SAND (SM), dark gray and (10') R 3/2) And bright green (S) 6/6), but, medium grained sound, Particulate to stightly indurated (GB COPR) 10.75' - Silty SAND (SM), Red-Brown		
11.	55-1	55	12"	67	(1078 5/6) Wedmedium grained tand 110'-1125' - Sami as above 1125'-12-0' - Silty Shun (Shi) dark gra and bright grun (546/6); but , medium Stand Slightly indusated (GE (OFE))	g Coresin	



BORING NUMBER: 47-5A

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT: WATER LEVELS:

START:

FINISH:

DEPTH BELOW (ft)	S	AMPL	E	STANDARD PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL DESCRIPTION	COMMENTS
	NUMBER	TYPE	RECOVERY (ft)		SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
7 -	55-74	22	6"	81	Silty SAND (SM), Briwn-gray and greenish (2575)4) I wet, fine to medium grained sand, slightly indurated (HB COPR)	and a cooling to 13:0' bg i
	8-22	ŠŠ.	411	25	Book not clay CCHI, Red (1049)4" West highly plastic	
3-					Bottom at the boring at 12 feet bys	



BORING NUMBER: GP-50

Sheet: |

SOIL BORING LOG

PROJECT: Foreywell DMT SSI

LOCATION:

DRILLING CONTRACTOR: Aquifer drilling and desting The DRILLING METHOD AND EQUIPMENT: SONIC SMAP DELL HAR THE STY Sampling WATER LEVELS:

DRILLING CONTRACTOR: Aquifer drilling and desting The DRILLING METHOD AND EQUIPMENT: SONIC SMAP DELL HAR THE STY Sampling and desting The DRILLING CONTRACTOR: Aquifer drilling and desting The DRILLING METHOD AND EQUIPMENT: SONIC SMAP DELL HAR THE STY SAMPLE S

£ .	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
2	SONIE MATTE CATE	5-1	14"	6" - 6" - 6" (N)	2" Asphalt Poorly graded GRAVEL LILSONA (GP) gray and black (104R 4/1), dry Alkinating layer of Asphalt and gravel to the spoor	Advanced the sonic matro core 2"10 to 31 bgs	
3	52	SONIC MACKE COPE &	1511		30-3.25 fat CLAY (CH), Red (104 ALA), moist, highly plastic. 3.25 f. Silty sand with gravel (SM), Reddish - Brown and green (1048-414) dry, fine to medium grained sand (COPR)	3- Advanced the some macro core 32 p from 3-6. sample facted with diphenyl carbatide to datermine the presence of coppe	



BORING NUMBER: 91-50

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

€	S	AMPL	E.	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (#)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
1 - 1 - 1		9302			6.0'-6.2' - Same as above 6.2'-10' - Silty SANO (SM), darkgray (10 YR 4/2), Wet, Medium grained sand (GB COPR) Particulate 7.0' - Silty SAND WHI gravel (SM)	6.0%. Advanced 3° ED sonic mach core from 6'-3' water encountered od approximately 52' bgs during disting	
111111	\$-3	SONIE MACED CO	24"		Reddish - Brown and greenish (104R b) u dry, fine to medium grained sand, 1 (4B COPR)		
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	5-14	SS	12"	20	gol- Poorly graded some with silt (dork gray and black (N2) streetium grained sand, particulate, (GISCOPR)	spise),	
1 1 1 1 1	SIT	55	124	28 83	same as above, slightly to moderately indurated	installed to adepth of	
	55.10	çs	51	100 5"	same as above	Advanced to 12' using 3" Its sonic marrocore	



BORING NUMBER: GP-50

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

DEPTH BELOW (ft)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
2 -	55-7	22	G"	30	gray Sand clay.		
20	65-8	55	6.11	30	same as above, moderately to highly indurated 130'- same ad above		
1	SSA	SS	Øa	13	134- Fat CLAY (CII), Red (1044)4) highly plastic		
7					Bottom of boring at 125 bgs		



BORING NUMBER: GIA

Sheet:

SOIL BORING LOG

PROJECT: Harrowell DMT SSI

LOCATION:

ELEVATION: DRILLING CONTRACTOR: Manufer of the grand tenting The DRILLING METHOD AND EQUIPMENT: SOME SAMP DELL MEN 170; SET SAMPLES

WATER LEVELS:

START: 11315

FINISH: 11315 17:40

LOGGER: K. Chadra Vestula

(#)	SAMPLE			STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS		
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION		
1. 1. 1. 1. 1.		CORE			3" Asphalt 0.25+2-01. Poorly graded GRAVEL with sand and sill LGP-GM, recently gray (104R 6/2), moist - Road based aggregate	Advanced the hole Using 3'tD Sonic macrocore		
1 1 1	5-1	IC MACES	321			2 of Polyethylene membro encuntered, in the spoon		
1 1 1 1		SOUR			2:01- Poorly graded sants with siltard gravet (so sm), gray-brok (10 mg 4) 3), moist, medium to loanse grained sand	ovs.		
-					3:5'-40'- same as above.			
1 1 1 1	5.74	MACRE CARE	79"	-	trol-4.51. Clayer samo (se), Ped-white (1044) 47, dry, time to medium granded sand.			
1 1 1		SOMIC		-	4.5'-55'- Poorly graded GRAVEL wi Sand (GP), Black (ND), dry, Road based aggregate	45.		
1 1 1 1					5.5'-6:0' - same as above, tan-white Cloyne 8/21, most, loanse grained sand			



BORING NUMBER: G.P-9A

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

LOGGER:

V (ft)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	rype	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
30	NN S	Some MACRO CORE	36,	5 - 5 (N)	Silty Sand (SM), dark gray and brown Llove 4/4), day offer to medium grained sand, particulate (HB copp) 70'- Same as above, Red-Brown and greenish. (104x4/6)	Sample tested with diphenyl carbabide for the presence of LOPR
) -	54	SONJENARER	12"		"some ac above slightly to moderately inducated,	installed the 6000 outer casing down to let by:
0 -	3	53	10"	12 23	same as above	sampling, 160-16 hamrer
1-	55	Ç,	5"	50/54	110'- Poorly graded smus with Silt (SP-SIN, gray-black Indiand greenist, bet, Particulate (CB COPR)	



BORING NUMBER: 9 P. 9A

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

LOGGER:

(H)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS		
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION		
12-	55-7	5.5	6"	18	Silty SAND (SN), Brown-gray and gluenish (10 YR 4/3), dry, Particulate to slightly indurated (HB COPE)			
3	528	22	NR	2.8	No Perovery	13.5'- Ground wealth		
	5.9	ss.	6,1	15	Proving gradeal SMAD WITH SITE (SP-SH), dark gray and black (AB), bet, medium gravind stand, Particula	anisan Kered pleasing obilly		
-	55/13	55	NR	50/14	No Recovery	Advanced to 145 bgs Using 3"ID south marrorage		
	g-11	55	6"	22	same arohove	-		
5-	52-12-	SS	611	9\$	moist, fire ground soud			
		-1			Bottom of boring @ 15-5' bgs.			



BORING NUMBER: GP-9C

Sheet: 1

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +\~

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAY 170

WATER LEVELS: **

START: 0830

FINISH: 1130 LOGGER: L. LIN COLN

į		SAMPL	.E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
ANO INC.	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
1 2 3	52		\$7°	_	4" ASPHALT 6" TANI C-M SAND, TR GM, SILT (FILL) 2" ASPHALT BEN C-F SAND, SM GM, TR SILT (FILL) 3-3.2 BRN C-F SAND, SM GM, TR SILT (FILL) 3.2-3.4 BLK ASPHALT 3.4-3.8 WHITE C-M SAND, SM GYL, 3.8-4.2 BRN, BLK C-F SAND, GM 4.2-4.7 RED CLAY MOTT WHITE, YOLOW, SM C-F SAND, GM 4.7-6 MOD. LITH BRN, GRY, YELLOW SILTY F-C SAND, SM GYL (4B)	M NO DPC
u						



BORING NUMBER: GP-9C

Sheet: 2

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION: ~ + 17

DRILLING CONTRACTOR: SDS

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: ~ + 6

START: 0830 FINISH: 1130 LOGGER: L LINCOLN

<u> </u>	5	SAMP	l F	STANDARD	T	LINCOLN
≥	 `		Т	PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (#)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
\(\begin{align*}	5.3		27'	H/A	6-9. GRY, YELLOW, MOD. HIGHLY LITHIFIED SILTY F.C SAND, SM GVL (HB)	
8 -		SONIC		-		
9 -	· 54	bori	8"	N/A	9-10 RED. BRN, GRY, YELLOW CLAYEY F. C SAIND, TR. GVL (HB)	DPC+
10	5.5	3" 0.D. 55	12"	20 -	10-11 RED BRM, GRY, BLK, YELLOW MOD HIGHLY LITTH! SILTY F-M SAND, TR. GYL (HB)	
n -	35-10		12"	42 -	11-12 DO SS-5 (HB)	



BORING NUMBER: GP-9C

Sheet: 3

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~+17

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAY 170

DRILLING CONTRACTOR: STOS

WATER LEVELS:

START: 0830 FINISH:

LOGGER: L. LINCOLN

€.	s	SAMPLE		STANDARD	SOIL DESCRIPTION	COMMENTS
¥ (₹			(ft)	PENETRATION TEST	SOIL DESCRIPTION	COMMENTS
DEРТН ВЕLOW (ft)			RY (I	RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
H B	BER		OVE		COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL	RATE, DRILLING FLUID LOSS, TESTS AND
EPI	NUMBER	TYPE	RECOVERY	6" - 6" - 6" (N)	STRUCTURE, MINERALOGY	INTRUMENTATION
-		ļ	Œ			
12 -	1	3"		9	12-13' NO RECOVERY	
-		ο.Ω· 55	0"	_		
-	55:7	20		-		
]		·	15 -		
13 -						
-				21 _	13-13.7 DL GRY, GREEN	13'- WATER IN SPOON
-	1-8		12"	_	F.M SAND, TR. SILT (GB)	
-	55.8		10	· -		
-				53 -	137-14 BRM, GRY, YELLOW	
14 -				· -	SAND TR. GUL (HB)	
-	100		Z"	9 -	14.14.4 BRM, GRY, GVLY SAND,	DPC+
-	50			-	SM SILT GRED BRN, GRY SANDY CLAY	
-	55,0		6"	7 -	14.5-15,0 RED CLAY MINT	
-	55			_	WHITE, LELLOW SIM TAN MIF	EOB @ 15.0°
15 -						108 6 15.0
_						
_						
-				_		
16 -		:		· —		
-				<u>-</u>	•	
-				***		
-	}			-		
-						
17 -						
-				-		
-						
-				-	·	
.o				-		
18 -		L				

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122

BOR-4_JAN2013

T: 917	nrce.com					i gran artika	6P-9C	
DJECT	· · · ·		MARRINI	e mooss	16.1 A1	SHEET	OF OF	
CATION				E TERM	INAL-	_ FILE NO	587	
RING LOCAT		SEE	E, MARY	LAND		SURFACE ELEV.	<u>~+17</u>	
T/INSPECTIO								
ERENCE CO	DES/STAND	ARDS		Market and the second s			The state of the s	
RING EQUIP	MENT AND	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	<u>F STABIUZING BO</u>	REHOLE				
E OF BORING I	DIC	TYPE OF FEE	from a first of	CASING USE			NO	
ick	MG .	MECHANICA		DIA., IN.		VES PROM		.
	 	HYDRAULIC	<u> </u>			DEPTH, FT. FROM		ro
GE		OTHER	-	DIA., IN.		DEPTH, FT. FROM		ro
IER		OTHER	-	DIA., IN.	<u> </u>	DEPTH, FT. FROM	. 	ro
		-						
E AND SIZE	OE•			D011111C 14		- Line	[]	
	Ur,			DRILLING M	5 T T T T T L L	YES	No	
AMPLER			 		OF ROTARY BIT, IN.			
AMPLER AMPLER				TYPE OF DRI	MUD			
IMPLER IE BARREL	24.6	- 116		A1100A1100				
IE BAKKEL IE BIT	<u>3" Sc</u>	M(C		AUGER USER	IAMETER, IN.		NO	
L RODS	-			I TPE AND D	IAMEIER, IN.	gradient gewant		1
T KODS	-			CACINIC IIIA	****			
				CASING HAN	NIMER, LBS.	AVERA	GE FALL, IN.	
			A CONTRACTOR OF THE CONTRACTOR	= 44 45 45 45				
ITER LEVEL O	DBSERVATIO	INS IN BORE	HOLE	SAMPLER HA	AMMER, LBS. MMER		GE FALL, IN.	30
		INS IN BORE		化二氯甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基甲基	MMER	140 AVERA		30
				TYPE OF HAP	MMER			30
				TYPE OF HAP	MMER			30
				TYPE OF HAP	MMER			30
				TYPE OF HAP	MMER			30
				TYPE OF HAP	MMER			30
				TYPE OF HAP	MMER			30
				TYPE OF HAP	MMER			30
	TIME DE		DEPTH OF CASING	TYPE OF HAI	MMER			30
DATE T	ISTALLED	PTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER NO SKET	MMER	CONDITIONS OF	OBSERVATION	30
COMETER IN	ISTALLED	PTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER NO SKET	CH SHOWN ON	CONDITIONS OF	OBSERVATION TOP ELEV.	30
COMETER IN NOPIPE:	ISTALLED TYPE TYPE	PTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER DEPTH TO WATER NO SKETT ID, IN. OO, IN.	CH SHOWN ON LENC	CONDITIONS OF	TOP ELEV.	30
COMETER IN NOPIPE:	ISTALLED TYPE TYPE	PTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER NO SKET	CH SHOWN ON LENC	CONDITIONS OF	OBSERVATION TOP ELEV.	38
COMETER IN NOPIPE: IKE ELEMENT: ER;	ISTALLED TYPE MAT	PTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER DEPTH TO WATER NO SKETT ID, IN. OO, IN.	CH SHOWN ON LENC	CONDITIONS OF	TOP ELEV.	30
OMETER IN OMETER IN OMETER IN EXERCISE SERVICE OFFICE SERVICE OFFI	ISTALLED TYPE MAT	PTH OF HOLE	VES	DEPTH TO WATER DEPTH TO WATER NO SKETT ID, IN. OO, IN.	CH SHOWN ON LENG LENG LENG	CONDITIONS OF	TOP ELEV.	30
COMETER IN NOPIPE: KE ELEMENT: ER: OUANTITIE: DIA. DRY SAM	ISTALLED TYPE TYPE MAT	PTH OF HOLE	VES UN. FT.	DEPTH TO WATER DEPTH TO WATER NO SKETT ID, IN. OO, IN.	CH SHOWN ON LENC LENC LENC NO, OF 3" SHELBY	CONDITIONS OF STH, FT. STH, FT. STH, FT.	TOP ELEV.	30
COMETER IN NOPIPE: KE ELEMENT: ER; QUANTITIE: DIA. DRY SAMPI	ISTALLED TYPE TYPE MAT S APLE BORING LE BORING	PTH OF HOLE	VES VWP	DEPTH TO WATER DEPTH TO WATER NO SKETT ID, IN. OO, IN.	CH SHOWN ON LENC LENC LENC NO. OF 3* SHELBY NO. OF 3" UNDIST	CONDITIONS OF STH, FT. STH, FT. STH, FT.	TOP ELEV.	30
COMETER IN NOPIPE: NKE ELEMENT: ER: QUANTITIE: DIA. DRY SAMPI	ISTALLED TYPE TYPE MAT S APLE BORING LE BORING	PTH OF HOLE	VES UN. FT.	DEPTH TO WATER DEPTH TO WATER NO SKETT ID, IN. OO, IN.	CH SHOWN ON LENC LENC LENC NO, OF 3" SHELBY	CONDITIONS OF STH, FT. STH, FT. STH, FT.	TOP ELEV.	30
COMETER IN NOPIPE: AKE ELEMENT: ER: QUANTITIE: DIA. DRY SAMPI E DRILLING IN	ISTALLED TYPE TYPE MAT S APLE BORING LE BORING I ROCK	PTH OF HOLE	VES VIN P LIN. FT. LIN. FT. LIN. FT.	NO SKET	CH SHOWN ON LENC LENC LENC NO. OF 3* SHELBY NO. OF 3* UNDIST OTHER:	CONDITIONS OF STH, FT. STH, FT. STH, FT. TUBE SAMPLES URBED SAMPLES	TOP ELEV.	30
COMETER IN NOPIPE: AKE ELEMENT: ER; DIA. DRY SAMPI DIA. U-SAMPI E DRILLING IN	STALLED TYPE MAT S APLE BORING I ROCK	ERIAL SC	VES VIN FT. UN. FT. UN. FT.	DEPTH TO WATER DEPTH TO WATER NO SKETT ID, IN. OO, IN.	CH SHOWN ON LENC LENC NO. OF 3" SHELBY NO. OF 3" UNDIST OTHER:	CONDITIONS OF STH, FT. STH, FT. TUBE SAMPLES URBED SAMPLES	TOP ELEV. TIP ELEV. BOT. ELEV.	30
OMETER IN OMETER	ISTALLED TYPE TYPE MAT S APLE BORING LE BORING I ROCK	ERIAL SC	VES VIN P LIN. FT. LIN. FT. LIN. FT.	NO SKET	CH SHOWN ON LENC LENC LENC NO. OF 3* SHELBY NO. OF 3* UNDIST OTHER:	CONDITIONS OF STH, FT. STH, FT. STH, FT. TUBE SAMPLES URBED SAMPLES	TOP ELEV. TIP ELEV. BOT. ELEV.	30
COMETER IN NOPIPE: IKE ELEMENT: ER: QUANTITIE: DIA, DRY SAMPI DIA, U-SAMPI E DRILLING IN	STALLED TYPE TYPE MAT S APLE BORING LE BORING I ROCK LACTOR	ERIAL SC	VES VWP UN. FT. UN. FT. UN. FT. UN. FT. PHIC DE	NO SKET	CH SHOWN ON LENC LENC NO. OF 3" SHELBY NO. OF 3" UNDIST OTHER:	CONDITIONS OF STH, FT. STH, FT. TUBE SAMPLES URBED SAMPLES	TOP ELEV. TIP ELEV. BOT. ELEV.	5

Elevation: 17.673

Page 1 of 2

Site: Dundalk Marine Terminal

Boring No: DMT-TW1



Northing: 573899.856 Driller: Parratt Wolff, Inc.
Easting: 1448112.085 Method: Hollow Stem Auger

Consultant: CH2M Hill

Geologist: C. Reed

Total Depth: 18.0 Ft

Start Date: 07/21/2011

					Geolog	gist: C. Reed	
Depth Ft	Rec	DPC	PID	USCS	Soil Code	Stratum Description	
0		-				Asphal/bubbase. POORLY GRADED SAND with CLAY and GRAVEL (SP-SC), dark yellow brown (10YR 3/4 and with Ittle angular gravel, (firm).	i), dry to molef, fine to coarse
-				SP-SC			
5 —		-		SP-SC		PODRLY GRADED SAND with CLAY and GRAVEL (SP-SC), dark yellow brown (10YR 3/4 and with little angular gravel and black geotextile fabric at 0.5' of this interval, (firm).	is), dry to molet, fine to coerse
-				ML		SILT with SAND (ML), dark gray (10YR 4/1), dry, with fine sand, (loose). PODRLY GRADED SAND with CLAY and GRAVEL (SP-SC), dark yellow brown (10YR 3/	D day to moist fine to coerse
		-		SP-SC		sand with little angular gravel, (firm).	
		+		мн		QIRAVELLY ELASTIC SILT with SAND (MH), very dark gray (10YR 3/1), dry, with fine to m and little fine to medium sand, (loose).	odum subangular gravel
10 —	Ч						

Page 2 of 2

Site: Dundalk Marine Terminal

Boring No: DMT-TW1



Northing: 573899.856 Easting: 1448112.085 Driller: Parratt Wolff, Inc. Method: Hollow Stem Auger

Consultant: CH2M Hill

Total Depth: 18.0 Ft

Start Date: 07/21/2011

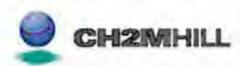
Elevation: 17.673 Geologist: C. Reed

	_			-			
Depth Ft	Rec	DPC	PID	USCS	Soil Code	Stratum Description	
10	ü					LEAN CLAY (CL), red (2.5YR 4/8), dry to moiet, with trace fine sand, (etiff).	
1				CL			
t		++		SP		POORLY GRADED SAND (SP), strong brown (7.5YR 4/6), wat, fine to medium send with (medium dense).	trace all, strongly indurated,
						POORLY GRADED SAND (SP), strong brown (7.5YR 4/6), wot, fine to medium sand with (medium dense).	trace silt, weakly indurated,
ł						(meteorin opinio).	
Ţ		++					
				SP	нв		
15 +							
1							
		**					
		**					
Ţ						LEAN CLAY with SAND (CL), red (2.5YR 4/5), moist, with fine send, (s0ff).	
		-		CL			
18.0							

Page 1 of 2

Site: Dundalk Marine Terminal

Boring No: DMT-TW2



Northing: 574014.616 Easting: 1448455.513

Elevation: 19.776

Driller: Parratt Wolff, Inc. Method: Hollow Stem Auger

Consultant: CH2M Hill

Total Depth: 19.0 Ft

Start Date: 07/22/2011

Geologist: C. Reed

			Geolo	gist. O. Need	
Depth DPC	PID	USCS	Soil Code		
0 -			Code	Asphalt/subbase. POCRLY GRADED SAND with CLAY and GRAVEL (SP-SC), dark brown (10YR 3/3), dry swith little fine to medium subangular gravel, (firm).	io moist, fine to medium sand
5 —		SP-SC			
		MH		GRAVELLY ELASTIC SILT with SAND (MH), dark gray (10YR 4/1), dry, fine to medium sa angular gravel. (loces). LEAN CLAY (CL), red (2.5YR 4/8), dry to most, with trace fine sand and fibrous geotandie	

Page 2 of 2

Site: Dundalk Marine Terminal

Boring No: DMT-TW2



Northing: 574014.616 Driller: Parratt Wolff, Inc.
Easting: 1448455.513 Method: Hollow Stem Auger

Elevation: 19.776 Consultant: CH2M Hill

Geologist: C. Reed

Total Depth: 19.0 Ft

Start Date: 07/22/2011

					000.0	giot. O. Nebu	
Depth Ft	Rec	DPC	PID	USCS	Soil Code	Stratum Description	
10				CL		LEAN CLAY (CL), red (2.5YR 4JS), dry to moiet, with trace fine sand and fibrous geotextile	febric, (eliff).
		++		SP	GB	POORLY GRADED SAND (SP), efrong brown (7.5YR 4/5), moiet to dry, fine to medium se indurated, (medium dense).	nd with trace allt, moderately
15		**		SP	НВ	POORLY GRADED SAND (SP), elrong brown (7.5YR 4/5), www, fine to medium aand with (medium dense).	
15		+		SP	НВ	POORLY GRADED SAND (SP), strong brown (7.5YR 4/6), moist to dry, fine to medium se indurated, (medium dense).	nd with trace sit, weakly
1		+		SP-SM	GB	POORLY GRADED SAND with SILT (SP-SM), very dark gray (10YR 3/1), wet, fine to med gravel, particulate, (toose).	ium sand with trace subangular
		+		SP	POORLY GRADED SAND (SP), very dark brown (10YR 2/2), wel, fine to medium send with little fine angular gravel, weekly indurated, (dense).		
1		+		SP-SM	GB	POORLY GRADED SAND with SILT (SP-SM), very dark gray brown (10YR 3/2), wot, fine trace angular gravel, particulate, (medium dense).	to medium grained sand with
		+		CL		LEAN CLAY with SAND (CL), red (2.5YR 4/8), moist, with fine eend, (atiff).	

JA	CO	B						Page 1 of 2	
ana (1141)			3		SOIL BOR	RING	LOG		
PROJECT: H ELEVATION: DRILLING MET ATD WATER LI	HOD AND	5	ENT USE	DRILLING CONTRACTOI Parratt Wolff ED: Charles 1400 START 1410 7/24/2 END: 110/5 7/21/8 LOGGER: 121/24/24 Marc SOIL DESCRIPTION USCS COUNTRIS					
DEPTH BELOW SU				SOL	ESCRIPTION	USCS		COMMENTS	
INTERVAL			COPR	SOIL NAME, USCS GROU	P SYMBOL COLOR		BLOW	DEPTH OF CASING, DRILLING	
RE	COVERY (IN		Strata	MOISTURE CONTENT, RE	LATIVE DENSITY.	1	CODIVIS	RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATION	
	E/TYP	_		OR CONSISTENCY, SOIL MINERALOGY	STRUCTURE,	1		DRILLING ACTIONS/DRILLER	
	1.00	(COPR)	4-3	MINERALOGY				COMMENTS	
- 0				ישוצרי ויחים	huse			South His red	
-1 4	1 531			3.14 , 2.5 732	5 Soul & Coperation there	24.			
- 4		3		hour, niame, h	er word		1)	Thesh thereses	
- z z	0 225	-		28-321-500		SA	1	0,3	
-	1		- 1	32-48" - CLN15	y F. Sand Some	1	3		
- 9				Gowl, 5 Yr 3	/2 (DAME MENGIN BON-1)		,	1	
-		-	- 1	Jucist, Mestun			6		
2		- 1	- 1	Transfer Transfer	1 1000 00000		1.5-		
-		-	- 1			1 1	11		
-						-		Pal Sona alte aprenia	
	9 6-1			48-59"- No Hacon	2007	4.7	5		
4 13	223	-	- 1	54 - 72" - 500		T	-		
- 425	1	-	- 1			- "	5		
- /			- 1				6		
- 6		-	- 1				0		
3 1	1 1		- 1				10	-	
-		-					70		
- 6 13	_			72-78"-No Action	2		9	-	
- 6 15	13/	- 1	- 1	78-85 "- son	-	SP-In	21	15	
- L.		-	- 1	35. 14" - winte Ca	eg F	-12	31		
- 1-3	1 1		- 1	56-96 - Lamer			24	- 2	
	1 1	-	- 1	SC- 10 - toluice	ic. Op main		6		
- 5	1 1	- 1	- 1				A-S	7 -	
-	1 1	-	- 1			- 1	25		
-								loved the from 618, -	
- 5 13	222	_		96-101" - NO MONON	se)		29	THE MILL THAN ONLY	
- 0 /	-3	7		101-102"- WHITE U	say c. com			1 19	
- 10		-	4	soli (Grussi) Day, ma	on They GARDEL, GEBY)		28	1	
- 10				11-112"- F. hm. Son	1. Incost, 5414/6		15		
		-	1	POLIDERIN RELL MAN H	MONTH, NOUNCY Croses		12	1	
-			- 1 1	10 -17 w' - The C.Su.	terratilit savel		8	-	
-		-	_ []	7.5 70 7/3 (Dane A	Mann) Day Lovey Well		0		

PROJECT NUMBER

BORING NUMBER

Page Z of Z

SOIL BORING LOG

_	VATER					D: CME \$5, HSA START: 1410 7/16/15/END: 1645 7/3		LOGGE	R: h Mr. H Truck
PTH	BELOW:	AL (FT)	E (FT) ERY (IN) SAMPLE #/TYPE	DPC	COPR Strata	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	USCS	BLOW COUNTS	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS
111111111	10 1 7 15	14"	5.56	(COPR)	118	120-124"- 100 MELOLOGY 124-125"- SDD 125-134"- (CAY, 2.5 TH 5/6 (MED) F. SENDY (Ley, MILLEY, TYLEY, FORMY LANDON, MANTICE METH USING 134-135"- 1. to M. S. and with 51H 54A 4/6 (MELONIA ARO) Day bear Device, PORMET GROUPED, LERKEY MONTED	CL.	5 10 75/3	POEXET TOUS >4-66 Descrit & HAT Land & late
11111111	12.C.1	11,	557	**	HZ	144 - 162 " - SDD	5m	21 17 100/6	
11-11-11-11	14 4 15	24"	278	+4	45 C3	168-185"- SAD 185-182"- F. John Silly Sonditude Gerry, 5444 (DANK GRAY), MOIST, WENY DEMOND, PEDAM GRADED CAR NUMBER	24	68 18 64	ADVINCES HOW FROM 12 to 16
	10 4 0%		229	++ ++ +4.	HB HB	192-207 "- F. to M. Sont. Sign 4/6 (YOLLOWSH MODDAY, MODES, FORMAL) GLESOD, WOSHELY HALMATO SUST-210"- F. to A. SILLY SILL thus GROWNER, 54 T/1 (DONIE GRUY) DAY, MODELLY, FORMALY CROSS	Sh	30 34 13	
	176	5	5310	++	GB	210°-211": No recovery 211"-216": SAA (207"-210") WET	sM	20	Bach P. Stude
NOTE OF	184 8	6"	5511	11	HB	5+ 4/. (Deall cont), moin, stiff,	Sm CL	9	PP=2.5+\$
	1873	1	2215	-		F.S. 1. SY 1/1 (DANK GIVY) WOT, POFT		3	PF: 0.75 4sf

3.				PROJECT NUMBER	BORING NUMBER			Page of 3
JA	CC	B	S		SOIL BOI	RING	LOG	
ROJECT: H	lonevwell	DMT	_			N : Baltim	ore, MD	
LEVATION: RILLING MET	+24.	5	ENT USE	DRILLING CONTRACTO	1	1		E. Chuckey South
TD WATER L	EVEL:			START: P(Y) 7/2X/	END: 100 7 28	USCS	LOGGE	COMMENTS
INTERVAL RI	(FT) COVERY (II		COPR Streta	SOIL NAME, USCS GROUP MOISTURE CONTENT, RE	SYMBOL, COLOR, LATIVE DENSITY,		BLOW	DEPTH OF CASING, DRILLIN RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATIO
	SAMP #/TYF			OR CONSISTENCY, SOIL S MINERALOGY.	STRUCTURE,			DRILLING ACTIONS/DRILLER COMMENTS
-01-	11	1	1	Ashir				31/4" ID HSA Sample WLAWR
-0.5	2 55/		-	10-12": No rec	nou		19	
- +	7-31	-		10-12 Caravelly	F-C SAITE SI CE-SW	Sp- SM	34	1
- 2		_	1	54 5/2 /	OLIVE GRAY)			16
-				Dry, De	nse, well Graded		22	
- 2 1	2 SS2			24"-36": No 180	overy		2	
- to /	3 552	_		30-40" : SILTY F	SAND (SM)	SM	6	
A		~		104R 41	3 (Brown)		6	
-		-		40-48" = SAND 1048-4, Moist, S	l. Darse, Roorly Grad y CLAY (200) (SC) (3 CBROUN)	SC	7	Advance HSA
4 20	553			48"-52" : No re	roven		4	
4 20	ار ا	-		52-67" : SAA G		56		PP>46
6		-		07"-72" : weaths	Base	20	4	
		_		er Bla	CK F-C SA, SIUSIU		19	
			- 1	Day 3	(SN) leng Dense	_	20	
					-	SM		
6 20	554			12"-76": No rec			16	
40		-		16"96" SAA (ed Asphalt		29	
8'		-		tav-li	ed Asphalt, ce consistence	SM		-
		_	- 1			1 1	32	Alaman artan
							54	Advance HSA :
81 00	355		a	6-100" No rec	nery		21	0
10 20		-	k	DIOG F-CSA,	SM GV TRSI (SPS 5/1 (Blush Gry)	4)		
10				Dm N	all lovaried my D.	95W	100	7
		-	13	9-115": F.MS	A. TREVI, SILSP SA		4	PP=2.56
				7.5 46	and frances			Atvance HSA
		-		Dud' be	orly braded,	ch		4.8.

FIED COIL OF ACCIDIOATION

": SUTY CLNY, TRESPICE)
2.5 YR 416 (RED)
POOTLY GRADIN, MOISE

1/		:O	P		PROJECT NUMBER BORING NUMBER 10(0/01/2 SL-2			Page 2 of 3
	10	.U	B	3	SOIL BOR	NG	LOG	8
DJECT	Hon	eywell D	MT		LOCATION :	Baltimo	ore, MD	
VATIO	N: 4 2	4.5		ENT USE	DRILLING CONTRACTOF Parratt Wolff			* ***** 711. (*****
WATE	R LEVI	EL:	-QOII M	1	ED: CME SS HSA START: @00 1/28/6 END: 1/20 1/2/	BIIB	LOGGE	I.ZWMDZIN (JAKOB
TH BELOV	W SURFA			COPR	SOIL DESCRIPTION	USCS		COMMENTS
The state of		VERY (IN)		Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR,		COUNTS	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
		SAMPLE			MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,			TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER
		WTYPE	(COPR)		MINERALOGY.			COMMENTS
- 10	10	SSIO	_		120"-178" : SAA (115"-170") (CL)	1	0	pp=8.5 tsf
- 10	10	350	1		126"-130" : F-M STLTY SAND (SM)	CL	3	
- 101	1		100	VVa	iour sile (Vermanish Brn)		100/4"	pall ahead to 12'.
10.	1		++	HB.	DAN PANIM Granded Vidense			
-					Dry Party Graded voense moderately indurated	SM		
7					100000000000000000000000000000000000000			
-								
1.2	1	eco		un	144"-148": SAA (128"-150")		100/4"	100
- 16	14	557	++	4B	7.5 YR 416 (Strong Brn)		100/4	Vi.
-123								
-						SM		Almen in a strat-
						1		Physical HEA 8212.
-								Drill ahead to 14'.
								1017.
i.i.	-	110	-	НВ	168"-182": F SILTY SAND, TR GULGAD		71	
H	23	558	++	TIU	7.5 YE 4/6 (Strong DM)			
- 10			. 4		ory, very Dense	50	32	
15.9			44	GB	182-188" : F-M SILTY SAND (SM)	SM	32 25 10/5"	
			44	09	2,5 y 2,5/1 (BLACK).		100/5"	
			alex.		Moist, Poorly Granded			and the same of th
-			74	HB	186-191" : SAA (166"- 182")			Advance HSA
				THE .		-		12-110
110	110	559	++		192-199: No recovery		20	
10	ilo	-	41.5	HB	147"-204": SAA (168"-182")	N.	10	
10			41			511	24	
			4.0	Ca	208"-244"; SITA (182'-188")		Sec.	
-			++	CB	200 -201 13HH (102)		70 3"	
			++					
-				-	OU " DIG" CAN SUITU SONIO (SAN	-		
18	2	55	++	613	216"-218" FM SILTY SAND (SM)		50/2"	1
10	Cal	10	++		tellow cope natures smrty inturated	SM	12	
18.5	912				try is dense, promised			
-	-				222" - 225": FM SILTY SAND (SM)			
185		55	#	GB		51	40	
to	0	11				Div		
VA.	*		++	ua	meist, pearly amplat			
111			- 1	HB	nearly indurated	-	-	_

225"-228": SAA (1108"-182") Wearly swillvaled

•	, ,	•	-	B	0	51-2			Page 3 of 3				
			.U	В.	9	SOIL BORING LOG							
JE	CT:	Hone	ywell D	т		LOCATION: Baltimore, MD							
ш	NG MI	ETHO	AND E	QUIPME	ENT USE	DRILLING CONTRACTOF Parratt Wolff ED: START:0000 128 K END: 11000 712	zelik	LOGGE	I ZMUDZIN CJACO				
HB	ELOW	SURFAC	E (FT)			START: 000 128 K END: 110/ 7/2	USCS	3.0.5	COMMENTS				
ľ	NTERV		ERY (IN)		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY,		COUNTS	DEPTH OF CASING, DRILLIN RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATIO				
١			SAMPLE #/TYPE	DPC		OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.			DRILLING ACTIONS/DRILLER COMMENTS				
+	19		27	(COPR)		228"-230": SMA (225"-228")	M						
-	15	6	55	++		230" - 234": F-M SANDY CLAY, 2.5YR 4/8 (CES) MOIST, FOREY GRADE	sc	15					
-	11.5	_	_			MOIST, FEETING GRADED							
						and of being at 195'.							
			J - 8				П						
-													
1							П						
				- 1			1 1						
				- 1	- 1		П						
					- 1		Ш		4				
-		- 1		- 1			ш						
			- 1	- 1			Ш						
				- 1	- 1		Ш		-				
				- 1	- 1		Ш						
			- 1	- 1	- 1		ш						
							ш						
				- 1									
				- 1									
1													
									-				

J	A	C	0	R	8	706612	BORING NUMBER		U	Page of 3			
					_	SOIL BORING LOG							
ELEVA DRILL ATD V	TION ING MI	+2 THOD	well DI	MT QUIPMI	ENT USE	DRILLING CONTRACTO	LOCATION :	Baltimor	e, MD				
DEPTH	BELOW	SURFAC	E (FT)			START : 1605 TEXT	PEND: USY 7/16		LOGGER	H-M/MISOMMET.			
	HAIENV	ML (FT)	ERY (IN)		COPR Strata	SOIL NAME, USCS GROV	ID COMPANY	USCS	BLOW	COMMENTS DEPTH OF CASING, DRILLIN			
			SAMPLE B/TYPE	OPC (COPR)		MOISTURE CONTENT, R OR CONSISTENCY, SOIL MINERALOGY.			COUNTS	RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATIO DRILLING ACTIONS/DRILLER			
	30	010	1	4/4	4/4	DSPANET Rang	2. (2.1)			COMMENTS			
1-	0.54	107			-	12-24* - 5 1	Sp 19 (0.6.)	1	1				
1111	2	12	221	1 1		Maria de A	E. Sand, trace silt, al, 1070 3/4 (Ance and) And, measure,	27.4					
-			_			STATE OTANO	og' werh at Sirth		16				
1-	2	18	552	1		24-30"-NOV	iscory		3				
1	4			=		30-31 - 219 2	mb, Nyr 3/2 (very and	V 2"	7				
1	,			-		37-48"- SANDY	CLAN . 2. 84x 4/6 (KB)		7				
1-1				-		Most, Ne	DIVIN, MONEY GALDED	CL	11	Lorked bon: 3-242t			
-	4	20	223	_		48-52"-NE 1	ldwynn	-		opported has by or			
-	to	-	333			52 - 72" - 640	1-74143 131	W	3				
1	6			-		free gravel,	10 Yn 3/2 (VERY DOLK	20	7				
-	0			-		LOOVED COM	Deta Well Pense		2				
-				-					4				
-	6	24	224	-		72 - 75 "- 56A 79 - 88 " - 1991	MIT (DAKE)		65				
3	8			-		88-96" - F.L	oC. Sind, trocors: IL, GLET 2 4/1 (DONN		33				
-				-		MUSH CAN)	Dry Moone 11.00	2.4	29				
-		M		,		CAND.	27		13	LIDVALION NOW From y L			
-	8	53	222	7		57-101" - FI	mond	7		ADVA (05 PM			
-	40			-			h tough SILTY JOHA, 18 7/2 (PINK) MUST,	24					
-	10					101-110"- 5107	John Some Grance, Chil	5m	13				
-				-		110-170" - CLAYER	E COUNTY LOUD LOUGH GAS	DVA	10				
3				-		POINTY COM	The second second		12				

PROJECT NUMBER BORING NUMBER

Page Z of 3

SOIL BORING LOG

			eywell D			LOCATION :	Baltimor	e, MD	
RILL TD V	ING M	ETHO	DAND	EQUIPM	ENT USE	DRILLING CONTRACTOI Parratt Wolff D: CMT SS HSA			77 1 k 1
ЕРТН	BELOW	SURFA	CE (FT)	_	1	START : 1/25 1/25 1/25 1/26/18		LOGGER	I. Zmila Brine
	INTERV	AL (FT)			COPR	SOIL DESCRIPTION	USCS	LOGGE	COMMENTS
		RECO	VERY (IN)		Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR,		BLOW	DEPTH OF CASING, DRILLING
			SAMPLE M/TYPE	_		OR CONSISTENCY SOIL STRILL THE		COUNTS	RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION
-				(COPR)		MINERALOGY.	1	-	DRILLING ACTIONS/DRILLER
-	10	12	225	-		170-101			COMMENTS
1	10		-			132-136" - No staturary		6	
	12			-	. 0	136-143, 20 10		100	Total Lord
-	16					25 m de (nen), Chy, Twee good.	CL	10	Toxum ov 1.75 th
-						(en too	95	11	
1				_		192 4000 4 11 4 1 5	S 1	15.0	3
_				4		Stat, 59 of (construe) Dry, Long	Spire	-1	
-			Cr.	-		Marc Carres?	, n	1	Man P Calin
-	12	9	22.3	-		INCI-154" - NO AFINELY			DANNES #30 100 8415
_	1.					189 -169 - CLOY 7 5 Varely Can	CL	20	
-	1					MICHET, VENT JATE, POUNCY GODED	· CC		Carrelin - de la Carrella
-	14			+		169-116" 5 1 5 5		70%	forem est 1.51sf
-						169-168"- F. L.C. Smild Good, 10xx s/6	- '	6	
-			h .			(Yournes Brond mess, Louis,	Popular		
_						Princes Grano	5.00		
-	-		-						
-	14	9	228	+	15.5	168-177" -F. to M. S.H, Soni,		3.	
3	1	1			HB	- 11 7/6 (YOU THE WAS .) CARE	10 m	21	
-	~,0			7		Prous, triebure, proper Green,		25	
-	15			~		LEGALI PROMISE			
-									H-
		1117							
-		122							
-	15	,	0.	7.	100	10 . 7			
	3	6	223	++	His	180-181" - 500	5m	15	
-					(415	181-186"- F. torn, Shal, Son, sur (103)	211	0.0	15
-	126				100	and the factor of all went the way			
	5%	1	00	++	lis	HOM GRUDIA , COSTI NODALS	-	-	
		6	2200	41	1115	184-192" - Ttom. S.Hy S.J. SYRU/6	SP.	85	
-	4-					(Youcours Brun) com , most,	200	80	
-	16'5"		-	-	_	meoning Poently Grades, weekly subvites			Assessment
	161			4.1	120		120		Advancautua From 12 h
	le t	6	22"	++	113	194-200"- SOA	50-	66	Sources was 3" spoons
-	-2	.,,					Sm	136	2 3140
-	4.8"								
	1.18			_					
-/	- 2	6	3/2	++	-	201-102-300	Sh	7-	Senpres mine 3" specis
	10	100	212	7.4		202-207"- F. to M. Soul, Some Sill		37	3.446.
	173					(GIS) ST W/2 (OLIVE GRIND) MOIST, DONNE		1	
_	77.7					Pouncy Grades, CEPIL MEDILES	1		

Page

3 or 3

JACOBS

PROJECT NUMBER BORING NUMBER
706612 5L-4

SOIL BORING LOG

FVA	TION	Hone	ywell D	MT		LOCATION	Baltime	ore, MD	
RILLI	NG ME	THO	AND E	QUIPMI	ENT USE	DEILLING CONTRACTOI Parratt Wolff D: Chie SS 1/5%			I. Znalin poss
IDN	MIEK	LEVE	L:		8	START: 1605 7/25/18 END: 1075 7/26/18	_	LOGGE	R: M. HANNETON MRCE
	BELOW		E (FT)			SOIL DESCRIPTION	USCS		
	NTERV		ERY (IN)		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR.		COUNTS	DEPTH OF CASING, DRILL RATE, DRILLING FLUID LOS
- 1		17.7	SAMPLE			MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,			TESTS, AND STRUMENTAT DRILLING ACTIONS/DRILL
			#/TYPE	DPC (COPR)		MINERALOGY.			COMMENTS
1.	17'3"	G	503	++	HB	208-211 - FEM. SILLS SYN YG	2-2	35	Somewhat 3" spour
	10	0	110			(Yours Brow) Corn, most, best,		1	
3	17/0			77	G-B	TOUNG (W) MOIST, DON'S (DON'S) CHOOK (CHOOK) CAS	SY-	es	
-	1773		Sig	++	GC/	70 -121 - F toth Silly Sand, Sta 4/6	-	22	Somples was 3" 4000
		6	214		1HB	CHENTON CHENT INTERMINED WITH SYY/Z	20.24		
-	4-0			13-	1113	(or we (mus) must, very bourd, toonly			
1.0	13'5"			7.7		CHAMPI, CO'N HEREET IN GB			
	15'5'	1	25.52	4.1	GB	222-228"- F. to M. Silly South, 54 4/2	58-3n	95	Sported Long 3" space
-	+	6	40	100	0.9	Z 5 712 5/8 (red) ment feet bout forms		100	
-	19"					Grades, Care Mould, mederately I MONESTED			
-			-	-	-	729-235"- F. H. M. SILTI SOND, SYR W/6	20.0		6
	15'	C	5316	17	123	Cyntimina Bath I heist, Dans , rocky	76-7K	30	Senter him 3" spans
-	10		200			CHOUSE, L'KERLY MONTER, 25 71 4/8(ETW)			
-	157	1				Cany or bottom or succ			
-	107	_		-	-	+35-234 "G" Z56-270"- F. H.M. Shay			,
-	19'3	4	53/7	1-1-	74.0	Shab , 7. I Mi She (STANGER BROWN) MEIST.	Se-sn	Jour	Serentioning 34 Spoon
-	+	1				V. Dense, rocker Cheston, moderately		100/4	2 31001
-	202	1				4 - 272 - 075	_		
		_		1/1-	PB.	ZY 3-246"-500	50.52	7./	Sentero mora 3" stoom
-	20'2'	3	2217	1.1-	1-17	246 - 249"-110		70/3	7 200
-	1=	1							SPOON BONNEINE
_	20'9"								
_	20'9	-	-	++	AB	250 -256" - SOO, COM HODRES PRESENT	Sern	20	Sorriestina 3" space
-	1.	6	5/5		HB	Era Era July College Control	1.54.5	38	3 36000
-	+0								
_	214							_	
-	214	-	250	++	HB	257 - 260" - Spa	¥-5m	19	Sonfeed Fire 30 Your
	200	6	So	1.0	1412	260-263"-1 to M. S./h Jand, 544/2		11	3,000
-	211	1 10		44	GB	(orine cut) Dut, we pink theury arepes,			
-	511		=	000	- 20	MERKEY INDAMELED	/		
	2111	6	2521	++	HB.	264-270"- 1.+M. Silh Sand, 7.5415/6	Sem	3	Samples word 3" Jepon
-	to	6				(STAPH BROWN) WET, VERY LOOSE	.,	2	2 21000
-	226	1				Tooky Greden			
_		-	~	+	110	270 - 273"- 5AB	Fig.	16	14.10
`-	226	6	235	1	HB	273-276"- M. LC.S. L, 541 2.5/1	200	17	MY INTERIOR LA
-	to					(BLACK) WIST, LOCATO, FOOLLY GRASOS			S1, pa
	23			-		(general transfer of the said			
- 1									

			_	-		706612	SL-5			Page 1 of 3			
J/	/C			35	>	SOIL BORING LOG							
EVATION	T: Hor ON: GMETH	DD A	4.	1	NT USE	LOCATION: Baltimore, MD DRILLING CONTRACTOI Parratt Wolff ED: Cかき Ss ット・スラールSp START: では3つ コフェットをND: 13-00 フノェットを LOGGER コファルト・ファル・ハル・ルール							
	RVAL (FT	SAM	(IN) MPLE YPE	DPC COPR)	COME	SOIL NAME, USCS GREMOISTURE CONTENT, OR CONSISTENCY, SO MINERALOGY.	RELATIVE DENSITY.	UNCS	BLOW	DEPTH OF CASING, DPILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS			
- 65	0			COPRI		Asphalt 1800	d Bace			AW rods used to sample			
-0:+2	0		551	1 1 1 1		loyr Br Dng, P	eny to someworth some (sp-sm) 414 (aung yellow) own) avrly Gooded encountered@22		10 22 6				
214	1 10	+ 5	552	1 (1 1)		24"-34": SA 34"-66": SA 36"-48" 2.	y, sm si, sm f-c sa, travilla syr 4/4 (Brown)	d spsn	-	Advance auger 0-4' bgs.			
4-0	jo 2'		ss3	1 1 1 1	y .	48-51": NOOR	ecovery * (C14)(257R4)	(4)	2004	pp = 2.25 tsf			

JAC	OB:	S	706612	TL.L			Page Z of 3			
			SOIL BORING LOG							
ROJECT: Honeyw LEVATION: 1 ZA RILLING METHOD A TO WATER LEVEL:	NO EQUIPMI	ENT US	START: 0930 1/1/	4.25" lica	leylir		RIZMAL JACOBS, MIS			
INTERVAL (FT) RECOVERY SAI		COPR Strata	SOIL NAME, USCS GROI MOISTURE CONTENT, R OR CONSISTENCY, SOIL MINERALOGY.	UP SYMBOL, COLOR, ELATIVE DENSITY.	USCS	BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER			
- 8' - 12	SY -		72-80"-5A 80-87"- F. Somo Con (on (clay Burn Soul of Silt, 2.57 & 4/4 XBurn) Olun, fosnet Gorovi	CC SPC SM	1 2 6 13	COMMENTS			
- 8 21 S	22 .		96-108- 1 108-117" - N GRENDI, the GRENDIDAY,	Spaner of C. Soul, Some of 16 (2.575/1 Louse, Well Grando	54x 5x	24	Advance auger -			
16 17 SS +- 12	56 -		Day SETT, PO 126 - 132"- Sons birale	THE 4/1) (Brown) OCRLY CABOUD M. L. C. S. J. HET MOST	cl Sm	18 13 15 18	Advance auger- 8-12'			
12 17 SS	1	нβ	YELLEWITH RELE POERLY CARDI 154-154"- COPP	LITY CLOT (SYNS)(S) MODSF, MEDIUM, ED L (Hang BROOF) V.F. 4 ILT, SYNS/4 (MODDU	CL SP-SA A Bro	3 8 37	op = 3.5 tsf -			
17 3 5	8 +	10	SAA		SM	100				

	JACOBS					PROJECT NUMBER	BORING NUMBER 5'(J-5		Page 3 of 3			
J	14	C	U.	E	5	SOIL BORING LOG							
RILLIA TO W	TION : NG ME ATER	THO LEVE	eywell 44. I D AND		MENT US	DRILLING CONTRACTOR ED: Chie 55 4 4 START: 6730 7/21/18	125" 11-10 1/2-1	lis		ER To Junio Trais			
	TERVA	L (FT)	SAMPLE	E		SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		USCS	BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS. TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS			
+	8		229	+ + + + + + + + + + + + + + + + + + + +	HB GB	192-202-566 202-212- COI 50HD (574), MEDIUM, MODIUM COPR MODIUM	M (GB) V.F. L.F. WHATE CHOY) MUST, CHOSTOLICE CO.	D.A	21 14 19 15	DSILVEY advancement will HSA beginning - at approx-14 bgs			
- 4	.0	2	22	++++	Ga	711 - 235 1		5.54	8 14 12 39				
20.		0	22	4	G-B	240-2 19 240-1 18 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	(544/1-Dene Gary) , medium, famely	50.5%	23	Begin 6" Split Spen =			
20 10 21	56		12		GB	200		D'SA	17				
21 40.			13	++	Gg HB	2 xx1) (14x 2/	CB) L(HB) V.F. L.M. H-MODDING BEN) POONLY (MODE)	8:55	43				
ZI.	3	3 2	14		棉	268-272-50 258-261-NO	(112) CO 12 - 12 CO 17 C	cı	7	PP>3.5 tsf			
						MUIST, FILM, POO	VA CHARD						

B.O. B. B ZZ A bgs.

	_		-	A	PROJECT NUMBER BORING NUMBER			Page 1 of 3
JA	C	U	B	5	SOIL BOR	ING	LOG	
ROJECT:	341	-1.2		CNTHE	DRILLING CONTRACTOF Parratt Wolff	Baltim	ore, MD	
D WATER	LEVE	L:	EGUIPM	ENTUS	START: 0145 1 18 18 END: 1020 11818	Luscs	LOGGE	R:F. COSIAS/M. HAMILY
PTH BELOW INTERV	AL (FT)	SAMPLI	E	COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,		BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER
- 01 - tv	1	#/TYPE	(COPR)		Asphalt wad base			onlied of 3/4"ID - fuger (1"OD) (Howar STEN)
-06 -06 to	13	85.1	1 1		0-11": No recovery 11"-24": FM sould, trace fine gravel, th sill (sp-sm) (fill) 10 YR 4/4 (corre YellowishBrn) Swaangular gravel, lowghorson Wedi vim Dense Moist		4 7 5	Sample: W/AW rods
2' 10' 4'	11 5	5-2	1 1 1		24"-37": No recovery 37"-39": Claujey silt Lune) 2.5 YR 418 (RED) AND M SA WI SOME CLAY 10 YR 714 LVERY Falle Brown) Moist to wet Well Graded 39"-48": F-M SAND, Some F-M Gravel. tr silt (SP-SM)(Find) 10 YR 9.5/2 (Palle brange Yellow Subangular gravel, Med. Spherisity Well Craded		5M5-6	Advance auger
41 2	4 3	5-3	-	4	18"-56": clayey "sand strace fine gravel (sc) 104R 413 (Brown) wet, well Graded Medium Pense	SC	28167	

Wet, yellow cope crystalline

modules well Grades

Mulio auger

6-101.

	JACOBS				PROJECT NUMBER S(Page 3 of 3		
<i>J</i>	40	U.	D.	3	SOIL BOR	ING	LOG			
EVATIO	Hone ON: 1 METHO	1-7 2	-	ENT USE	DRILLING CONTRACTOI Parratt Wolff ED: (NUL SS + 1SH) START: 0.45 IKIK END: 1030 IKIK END: 1030 LOGGER: F. (OSTAS/M-HAIU					
HEEL	ELOW SURFACE (FT) NTERVAL (FT) RECOVERY (IN)			COPR Strata	COPR SOIL NAME USCS CROUD SYMBOL COLOR		BLOW COUNTS	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS.		
		SAMPLE M/TYPE	DPC (COPR)		OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.			TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS		
-10' - to - 12	. 1	SS6	++	HB GB	120"-121": No v eravery 122"-133"" SATA (101"-114") (SM) 133"-138": SATA (114"-120") Melst (SM)		35 39 33 96			
			4+	₩B	138"-144": SM (78"-87") [ML) Pry	SM	10			
			++			ML				
[2 - 40 - 14		557	++	НВ	144"-157": No recovery 157"-165": F-M SAND, SOME F-M GVL trad 7.5 VR 5/4 (BROWN) SM) Mod. indurated Pry Dense	5M	30 29 17 14			
			++	6B	165-168: F-M SHND, SOME SILT, tr gravel (SM) GLEY 131N (Greenish Gray) Moist to wet		,	Advance auger 10'-14'.		
- 14'b	9	358	4+		168"-114": Silty Sand grading to Sittyciay (SM/ML-CL) 10R 5/6 (RED)	SMI ML	19	Pig chattration - auctor aluncarons at 11. Hule taped at 14.5 ft offerss-8		
- 141 - to	6 6	559	١		174"-181: Silty clay (ML)	ML	12	PP=05tsf		

End of boring at 15' bgs.

RUJECT NUMBER 706612

SL-

Page

RO.	JECT:	Hor	neywell	DMT	_	SOIL BO		1000	9		
LEV	ATION	4 - 0	15	2	MENT INC	DRILLING CONTRACTOF Parrait Wolff	N : Balt	imore, MD			
TD	VATE	R LEV	EL:	EQUIPN	IENT US	DRILLING CONTRACTOF Parratt Wolff ED: CME 55 MUD POTTAR START: 0820 1/13/1/8 END: 0900 1/17/1/8		1			
rin	INTER	VAL (FT)	ACE (FT)		COPR	SOIL DESCRIPTION	SOIL DESCRIPTION USCS LOGGER: I				
			SAMPL #/TYPE	E	Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		COUNT			
1111	0 40					Asphalt road base			7" of ID Auger used to open hole		
11111	060	14	227	-		0-10": No recovery 10-21: F-M SAND, some clay (Sw: 2:54 4/3 COLINE BROWN) MOIST, LOOSE, BONY GRADE	1	5 18 17			
111111				-+		21"-24": F-C sand and gravel, some GLEY15/11 (greenish gray) Dry, Loose, well evaded COPE impacted Fill					
-10	1	12	<i>S</i> S2	_		24'-36": No recovery	BW-	7			
	4					54 SII (GRAY) (GUGM) bry, Loose, well Graded	GIU	933			
				1		47"-48": F-c sand, some gv1, trace 51+ 51+	GW-				
				-		Dry, wose, well Gradel					
				~					1		
4	7	18	223	-		48"-54": No recovery 54"-63": WAY (W)		MH 8	set 4" casing to 3 ft bgs. Introduces		
K	6,			-		Z.5 VR 5/6 (RED) Moist, high plasticity	CL	-37 49	Mud at 4ft (using 37/8" Pox but) PP = 0.25-1.0 tsf		

PROJECT NUMBER 706612 BORING NUMBER

Page 2 of 3

SOIL BORING LOG

PROJECT: Honeywell DMT LOCATION: Baltimore, MD

ELEVATION: +15.9 DRILLING CONTRACTOF Parratt Wolff DRILLING METHOD AND EQUIPMENT USED: CME 55, MUD POTARL ATD WATER LEVEL: START: 0830 1/13/18 END: 0900 1/17/18 LOGGER: I. 7 LULDE IN / M. YAMILTON DEPTH BELOW SURFACE (FT) INTERVAL (FT) COPR BLOW DEPTH OF CASING, DRILLING SOIL NAME, USCS GROUP SYMBOL, COLOR, RECOVERY (IN) COUNTS RATE, DRILLING FLUID LOSS. MOISTURE CONTENT, RELATIVE DENSITY. SAMPLE OR CONSISTENCY, SOIL STRUCTURE. TESTS, AND STRUMENTATION. WITYPE DPC DRILLING ACTIONS/DRILLER MINERALOGY. (COPR) COMMENTS 63"-72": F-M sand with silt (sm) HB ++ 10 YR 4/6 ware yellowish Brown) Very dense, well Graded SM ++ 12 554 72"-84" : Slough ++ 24 84"-95" : F-IN SANDW SILT (SM) 38 to 100/5" 2.5 YR 4/8 (RED) ++ very dense, well graded HB SM ++ Drilled ahead from 7'5" to8' 95"-96" : Gray cobble ++ BL 20 96"-100": No recovery 555 ++ 68 160" - band F-M sand w/ selt 58 to 119": 101 22 ++ 10 YR 4/6 (DOK 33 HB Yellowish Brown) SM very dense, Poorly Gradd 49"-120" ++ 1000000000 FILLSAND, WITHSILT 54 25 Y/1 (BLACK)(SM) weakly indurated, ++ Poorly Graded

PROJECT NUMBER 706612

BORING NUMBER

Page 3 of 3

SOIL BORING LOG

PROJECT: Honeywell DMT
ELEVATION: + (5 , 9 (BX)) LOCATION: Baltimore, MD

DRILLING CONTRACTOF Parratt Wolff

EPTH BELOV	V SURFA	CE (FT)			ED: CME 55 MMD POTARY START: 0820 7 1311 END: 0900 717118	Uscs	LOGGE	R:I.ZULDZIN/M.H
INTER	VAL (FT)	VERY (IN)		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY,	0303	BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS
		#/TYPE	DPC (COPR)		OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.			TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS
- 10	18	SSG	++		120-126: No recovery		12	
- 10				GB			22	
- 12			++		126-129" = SANOW TRACESUT	SP-SM	15	
-			1000		1000 GLEY 1 10/3/1 (very	-	12	
			+4		Moist (sp-sm), Dange			
-			17	6B	129-132" F SAND, SOME SICT, TRAUS	- 11		
-					129-132": FSAND, SOME SICT, TRACE F GRAVEL (SIX)	SM		
-				63	132"-136" SAA 126"-129"			
-			++	HB	1310"- 126" : F-W SAND SMEUL (SIN)			-
-				- 0	Brown Mod Indurated			
-				GB	139"-144": SAA (126"-129")			
-12'	q	557	14		144: 186 : Slough		3	_
- 10	1	-	44		3		3	switched to 3"
- 3							0	and drove
-112			4.1					from 12-13.5'
-								to obtain better visual classification
-								Blaucaints 10-27-25
- 13		558	++		ISG-102": F SAND AND SILT (SM)		25	WOULS VO SUNT
136	8	7	++	GB	GLEY 1 1043/1 (Vely Dark Greenish Gray) trace white	SM	-	with 2" split speen.
-100	9	-		GB	Oreenish Gray) trace white most pooch cond and make to	V. Dein	.	-
-186"	1.	559	-		162"-163": CLAY (CL)		6	PP=0.5tf -
	(in she)				MIXED WIGH COPP	CL	0	1
14	5111E)	-	-	-	2.5 YR 4/6 (RED) MIXED WI GB COPP CLAY (CL) 2.5 YR 4/6 (RED) SHIFT TO V. SHIFT MAIST			
7 1					End of being at 140' bgs.			pp=pocket =
-								- paremonent
								_
-								1
-								-
-1								_

PROJECT NUMBER 706612

BORING NUMBER

Page

n 3

set 4" casing to

3ft bas

Introduce mud

at 4 A bgs.

24

34

SOIL BORING LOG

PROJECT: Honeywell DMT ELEVATION: +17 LOCATION: Baltimore, MD DRILLING CONTRACTOF Parratt Wolff DRILLING METHOD AND EQUIPMENT USED: CIME 55 MUD ROTARY ATD WATER LEVEL : END: DEPTH BELOW SURFACE (FT) 2/12/18 LOGGER: 7 NUDZIN/M. HAMILTONS SOIL DESCRIPTION USCS INTERVAL (FT) COMMENTS COPR SOIL NAME, USCS GROUP SYMBOL, COLOR, BLOW RECOVERY (IN) Strata DEPTH OF CASING, DRILLING COUNTS MOISTURE CONTENT, RELATIVE DENSITY, RATE, DRILLING FLUID LOSS, SAMPLE OR CONSISTENCY, SOIL STRUCTURE, TESTS, AND STRUMENTATION. #/TYPE DPC MINERALOGY. DRILLING ACTIONS/DRILLER (COPR COMMENTS 0 SHEWE SHE Asphalt road base T" Ø ID AVERR used to open hole 0-6 carg 6" to 24": clayey sand 0-6" 551 SC + Moist, Loose, Poorly Graded, Slight Plasticity W. 21 COPE-Impacted FILL 24"-38" : Slough 10 552 11 + to. 6 38"-48": F.M silty sand (SM) 3M 9/10 + 7.54R S/6 (Strong Biown) 4 100/4

Dny, Very dense, Peorly Graded

48"-12": SAA (SM)

copp-impacted Fill

COPR-Impacied Fill

+

+

++

4

10

10

2211

553

PROJECT NUMBER

706612

BORING NUMBER

Page 2 of }

SOIL BORING LOG

		SURFAC	CE (FT)			START: 1605 THIR END: 1150 7/12	USCS	LOGGE	R: T. ZMUDZINIMI
	INTER	RECOVERY (IN)		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY,		BLOW COUNTS	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,	
			#/TYPE			OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.			TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS
11111				+					
1111			1	++					
1111111111	6 to 8'	15.	554	++		72"-81": slough BI"-94": SMA (SM) 7.5 YR 5/6 (Strong Brown) COPR-IMPACTED FILL Q4"96": Clay (L) 2.5 YR 5/6 (RED)	CL	39 10 20 15	pp=packet penetrometer
						Mottled with white clay wet, soft, high plasticity Poorly graded			PP=0.25tsf -
	8' to	4 (in shal	355	1 1		96"-105": Slough 105":104": SAA (CL) 2.5 YR 5/6 (RED)	CL	WH WH	pp=1.5-1.75tsf
	a' to	9	356			109"-123": Slough 123"-132": SAA (CL) 2.5 YR 5/6(RED)	a	WH WH 2	PP=0:25 tsf

PROJECT NUMBER 706612

ORING NUMB

Page 3 of 2

LIN

SOIL BORING LOG

PROJECT: Honeywell DMT

ELEVATION: +13

DRILLING CONTRACTOF Parratt Wolff

ATD WATER LEVEL:

START

PROJECT: Honeywell DMT

LOCATION: Baltimore, MD

DRILLING METHOD AND EQUIPMENT USED: CMF 55 MUD ROTARY

DEPTH	BELO	W SURF	ACE (FT)			START: 100 7/11/18 END: 1150 7/12/18	_	LOGGE	ER: I. ZHUIDZIN/M. HAM
		VAL (FT			COPR	No. of the control of	USCS		COMMENTS
		RECO	SAMPL		Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY,		COUNTS	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
			#/TYPE	DPC (COPR)		OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		_	TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS
-				-	17	see page 2			SOMMENTS
11111				_			CL		
11111	11' to	6	557	ーサナ	G8	182 - 136": SAIA (CL) 2.5 YR 5/6 (RED) 136"-137": F-M SAND MIHSILT (SM) 2.5 Y 4/1 (DARK GRAY) 137"-138": F-C SAND, DARK GRAY) SY 4/2 (SUIVE GRAY) MOTST, LOOSE	CL SM	29	Well Graded
	10" to 12"	6	\$32	++		138"- 144": SAA (SM) 54 4/2 COLIVE GRAY) COPR IMPACTED FILL		22	* cope impabled
-	12'	4 con shor)	SS9	++	GB	144"- 150": F-M SAND W SILT (SM) 2.5 Y 4/1 (DARKERAY) WEAKLY INDURATED	SM	9	
-	26°	4	5510	++	68	150"-152": SLOVELY 152"-155": F-MSHND WITH SILT (SM) 2:5 Y 4/1 (DARK GRAN) DANSE, WELLY INDURAND 155"-156": F-MSAND W SILT, SOME		14	215 YR 5/6 -
-	to 36"	4	SS11	۲		156"-158": NO RECOVERY 158"-162": SANDY CLAY (SC) MOD PLASTICITY 2.5 YR 516 (RED)	se	9	pp=1.15 tsf -
	3'6" to 14'	6	S512	+	- 1	162"-167": SAA (SC) SOFT, 2.5 YR 516 (RED) 167"-168": F-M SAND WISHT (SM) SY 4/2 (OLIVEGRAY)	sc	9	pp=0.5 tsf -
11111						B.O.B @ 144 bgs			PP= pocket - Penetrometer -
-		· i							=

PROJECT NUMBER 706612 BORING NUMBER

SL-9

Page /

013

SOIL BORING LOG

ATD V	ING M	LEVE	AND E	QUIPMI	ENT USE	D: CINE 35 - MYD ROTERY START: 1355 7/10/18/END: 1100 7/11/18		LOGGE	8: - 7. 1. Jay																	
_	BELOW			-	1	SOL DESCRIPTION	USCS	LOGGER: I. 7 mulein AK																		
	INTERVAL (FT) RECOVERY (IN) SAMPLE			INTERVAL (FT) RECOVERY (IN)		INTERVAL (FT) RECOVERY (IN)					COVERY (IN)		RECOVERY (IN)		COVERY (IN)		VERY (IN)						SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,		BLOW COUNTS	DEPTH OF CASING, DRILLI RATE, DRILLING FLUID LOS TESTS, AND STRUMENTATION
			MYPE	(COPR)		MINERALOGY.			DRILLING ACTIONS/DRILLE COMMENTS																	
-				L,		0-6"- NO NOCONON (DSWING BOSE)																				
1.1				_		6-11"- NO RECEIVERY 11-24" - GRAVELY SHID (SH)		6																		
1.1	6	134	SSA			10 YR 5/3 (BRUNN) MOUST, MEDIUM,		5																		
1	to Z'			-		M. L.C. GALVED, F. L.C. SOLD, SOME M. L.C. GALVED, BAGGLED & SHORPYLLOW,																				
Ξ	6"					trace silt, Cabille at 2 fe biss		10																		
1				-				24																		
4								7.7																		
1.1.1.																										
	2'	21"	S\$- 2	+		30-33"-NO RECOVERY 33-43"-SA (SW)		22																		
1.00	16 4			+		112 41 F E L N C 1(co)	se	27																		
1 1 1 1	6"			+		46-56" - FILL MOTEUDE (FILL) GLEY Z 5/1 (GREENISH GHEY), DAY, MEDIUM DEHSE, WELL GRODED, CRYSTOLIZON		14																		
1.1.1						SO-52" - F. to M SALA (SP) SYR 6/6 (REDDISH YELLOW) MOISE,	FILL	20																		
1				++		52-54" - Grower (GW) 54 6/2 (PALS	SP																			
-						GRADED MEDIUM DENS, NELL GRADED SLOVEH	GM.																			
-	4'	Z4"	55-3	1		(REDDISH YOULD) F. L. C. SILT, Sond		3																		
-	6			+		GRODED MOIST, LOSE, HELL		8																		
				11		60-62" - CAYSTOLIZED COM (5 42 4/2)		8																		
-				++		(PALE OLIVE) HOND, LITHIFIED, STRONGLY INDVINITION GZ-68"- F. tom. SIGT, SAND, AND GRANGE,		7																		
-				4.	AB.	(FILL) MOIST, LOOSS, WELL GREDED STRY	(PRIN	H 780																		

THENDERD, V.F. & F. SAND HITT STUT,
TRY, MEDIUM DENDRY, POOLLY GRADED
70-72" - SAD (FILL)

PROJECT NUMBER 706612 BORING NUMBER

Page Z of 3

SOIL BORING LOG

DRIL ATD	ATION LING M WATER	ETHO	AND E		ENT USE	DRILLING CONTRACTOF Parratt Wolff ED: Cm2 SS mus (withing START: 1335 Tier LEO TIII/IS LOGGER: 27 mb; OM				
DEPTH	INTERV	AL (FT)	ERY (IN) SAMPLE		COPR Strata	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	USCS	BLOW COUNTS	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATIO DRILLING ACTIONS/DRILLER COMMENTS	
13	6-9	ib	554	++ ++ ++	НВ	12-76" clay (cl) SYR 4/6 (Vellowish Red) Moist, soft, Mod. Plastic, Some U-c gravel 76"-88" F to M sand with silt (SW-SM) Moist, De Nery derke, Well graded, Weakly Induvated 10 YR 4/10 (Darkyellowish Brown)	SW-SM	14 45 100/4"	B" CAKING O STI	
9 =	8 to 10	14	555	+- ++ ++	HB GB	46'-106": Slough 106"-128": SAA (SW-SM) 118"-120" F to M sand with SH+ (SP-SM) Moist, 2.5Y 4/1 (Dark Gray) Dense, weakly Indurated	Me-ave	67 19 44 38		
0 1 1 1 1 1	10		SSG	++ ++	GB	120-132" SAA (SP-SM) 2.5 Y 4/1 (DAVE GRAY)	SP-3M	12 42		

PROJECT NUMBER 706612 BORING NUMBER

Page 3 of 3

SOIL BORING LOG

PROJECT : Honeywell DMT LOCATION : Baltimore, MD

ELEVATION: + 14.4 DRILLING CONTRACTOF Parratt Wolff
DRILLING METHOD AND EQUIPMENT USED: CME. 55. MMD POTARY

	WATER H BELOW			_	1	START: 335 THOUR END: 100 11118	Linco	LOGGE	R:I, ZMUDZIW/M. HAMI	
-		AL (FT)			COPR		USCS	BLOW	DEPTH OF CASING, DRILLING	
	1	RECO	SAMPLE		Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY,		COUNTS	RATE, DRILLING FLUID LOSS,	
			#/TYPE	DPC (COPR)		OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.			TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS	
,	11'6"	(ina (ina	557	++	68	132"-138": SAA (Sp. SM) 2.5 y 4/1 (Dank Gray)	SP- SM	21	start 6" sampling out 11 ft lags.	
	116"	(187 (187 (24)	558	++	6B	138"-144": SAY (SP-SM) 25 Y A/1 (Darkeray)	54- 5M	45		
1	12 to 126	(IN SHIE)	SSA	++	GB	144"-150"; SAA (SP-SM) ZISY 4/1 (DUTE GRAY) LEMSE OF SYR 4/6 (Vellow Rel) (SP-SM)	SP-	50#·		
1 1 1 1	12'6"	(in)	55/0	++	GB	150"-156": SAA (SP-SM) 2.5 Y 4/1 (Dark Gray) (nystalized copp Nature)	sp-sM	25		
11111	13/6"	(in shock	SSIL	++	68	156°-172": SAA (SP-SM) 2.5 Y 4/1 (FOUR Bray) Intermitted with 5 YR 4/6 (Vellow Red.)	SP- SM	35		
The state of the state of	136	(m)	SS 2	+		2.5 YR 4/6(RED). 1171 termixed with GB Moist, are medium. Poorly graded, high Plasticity Find of boning at 14 ft.	CL	7	PP21.5 tsf	
Transfer at Least	0									

JACOBS'

PROJECT NUMBER

SU-LO

Page 1 of 3

SOIL BORING LOG

BILLIN	IG M	+ 2.	ywell D		ENT USE	DRILLING CONTRACTOF Parratt Wolff		-	C-11/2 C
- ***	J. FL	LEVE		QUIPME	ENT USE	ED: CMB 55 135A START: 1415 7/28/18 END: 1715 7/28/15	2	LOGGE	S. Zonden Breaks
TH BE	TERV	SURFAC	E (FT)		1	SOIL DESCRIPTION	USCS		COMMENTS
			SAMPLE #/TYPE	DPC	COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER
-				(COPR)	-				COMMENTS
-					L.	Asphalt road base			sampled what nots
-	25	12	SSI	1 1		12"-12": No recovery 12"-24": Gravelly F.C. sand, travesilt 1.5 yr 4/2 (Brown) (SP-SM) Dry, Well Graded, V. Dense	SP- SM	30 31 5	
Τ,	104	19	SS2	1 1		24"-30": No recovery 30'-33": SAA (12"-24") Black mesh encountered at 33". (uven) 33"-43": Sitty clay, trf sa (cc)	SP- SM	5 7 18 17	
-				-		542 413 (Riddish Brown) Moist, Poorly Graded 43"-48": weathered asphalt Blacksitty F-C SA (SM)	sM		Advance HSA
-	4 40 6	13	SS3	1 1 1		48"-59": No recovery 59"-70": F SANDY SILT (ML) 7.5 YR 3/2-(Dark Brown) Moist, About Granded Medium 70"-72": Weathered asphalt Black in word liner. encountered at 70".	ML	4 8 3 14	
- 4	b 3	įb	S54	1 1 1		12-80 ": No rorovery 80-90": weathurd asphalt silty 1-m so and gvl Black and gray well graded, bry very dense	5W	15 16 23 19	Advance HSA
	0,30	16	SSS	, , ,		96"-102": No recovery 102"-108": SAA (80"-96") 108"-12": F-CSAND, TESIU (SP.SM) 1.5 YR 713 (FINE) DEY, PROLY GRADED 112"-113": CLAY (CL) MONT, PROPLY GRADED, SHA	SM SY-SM CL	2900	112": pp = 0.25 - 15f - 15": pp = 2.25/5-

113"-170": F SANDY CLAY (CL)
2.5 YR 416 (RED)
POOLLY Graded, Moist

PROJECT NUMBER BORING NUMBER SL-10 706612 **JACOBS** Page 2 of 2 SOIL BORING LOG PROJECT: Honeywell DMT ELEVATION: +25. O DRILLING CONTRACTOR

DRILLING METHOD AND EQUIPMENT USED: CMC SS H SP

ATD WATER LEVEL: START: 1415 112818 END: LOCATION: Baltimore, MD DRILLING CONTRACTOF Parratt Wolff LOGGER: M. HANDUNCINCOS 7/28/18 COMMENTS INTERVAL (FT) BLOW DEPTH OF CASING, DRILLING RECOVERY (IN) SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION. OR CONSISTENCY, SOIL STRUCTURE. M/TYPE DPC MINERALOGY. DRILLING ACTIONS/DRILLER (COPR) COMMENTS 120"-121": NO recovery ++ 556 B 10 10 121"-12": +SATA (113"-120") 8 122"-124": weathered asphalt to 52 124"-127" : F-C SAISM SILT (SM) SM HB 12 ++ 59 127"-137": SAA (113"-120") CL Advance HSA 153' - 14": SAA (124"-127") HB ++ SM 8-12'. 557 144-150": No recovery 12 18 27 ++ 190"-168": F-C SA, SM GUL, SE (SM) to HB 21 10 YR SIG (yellowish Bm) 14 +4 SM Dry Dense ++ Well Graded 15 168-173": No recover 558 14 19 173"-1-7" : F.W. SAND, TE SILT (SESM) 5 Sp. SM ++ HB 40 7. SYR SIG (Strong Bm) Dry, Peorly Graded, wose 6 16 7 SM + 177"- 183" : F. SAND, SOME SILT (SM) 7 183"- 192": F-M SILTY SAND (SM)

SY 2.5/1 (BLK) WICOPE NODLES 14 ++ GB SM Advalle HSA 12'-16. 192"- 194" : No recovery 2Z SS9 16 44 GB 6 194"-216" : F. M SILTY SAMD (SM) 10 18 8 54 2.5/1 (BLACK) SM 44 11 MOIST, POORLY GRADED Medium Dense 10 4+ YELLOW COPR natules 216"-220": No recovery SSIU 18 20 220'- 240" : SAA (194"-216") to 20 16 SM 15 15 Advance HSA 110-20.

PROJECT NUMBER **BORING NUMBER** 70101012 of 3 51-10 3 Page **JACOBS** SOIL BORING LOG PROJECT: Honeywell DMT LOCATION: Baltimore, MD ELEVATION: +25.6 DF DRILLING METHOD AND EQUIPMENT USED: DRILLING CONTRACTOF Parratt Wolff J.ZMUDZIN (JAKO 35) M. HAMUTAN (WEGT) COMMENTS START : 1415 1/28 18 HSA END: 1715 ATD WATER LEVEL : 7/28/18 | LOGGER: DEPTH BELOW SURFACE (FT) SOIL DESCRIPTION USCS INTERVAL (FT) DEPTH OF CASING, DRILLING COPR BLOW SOIL NAME, USCS GROUP SYMBOL, COLOR, RATE, DRILLING FLUID LOSS, RECOVERY (IN) MOISTURE CONTENT, RELATIVE DENSITY, TESTS, AND STRUMENTATION. SAMPLE OR CONSISTENCY, SOIL STRUCTURE. DRILLING ACTIONS/DRILLER M/TYPE DPC MINERALOGY COMMENTS (COPR 20 \$ 240-246: SAA (194-216") ++ +0 8 SM 11 GB 0 Yellow COPR nature 20.5 wet 246"-252" : SNA (194"-216") 20.5 55 GB 4+ intermixed with SY 64 14 to SM 12 0 (Pale olive) 21 wet 252":258": SAA (194"-216") 21 SS GB ++ to 6 Yellow natule 13 SM 21.5 Wet, Poorly Graded 258"-264": SAR (194"-216") 21.5 55 63 ++ +0 Wet, Parry Graded 10 6 SM 14 wasty Indurated 22 Advance HSA 264"-276": SNA (194"-216") SS15 ++ wet, Poorly Greded weakly Induvated GB -10 0 SIM 6 22.5 270"-276": SAA (194"-216" 22.5 SS GB wenkly Indurated SM 6 ++ to Wet, Poorly Grade 16 23 279 276" - 200" : SAA (194"-216") GB ++ 23 SS SM 40 8 279"-282" - Brown Wood 17 + 23.5 6 282"- 288": SILTY F-MSA, TR 235 SS loye 4/4 (DK Yell Bin) wet, poorly evaded SM 6 18 40 (1) 24' 15 End of banny at 24.0!

JACOBS

PROJECT NUMBER 706612

BORING NUMBER

EW-2 Page 1 of 3

SOIL BORING LOG

PROJECT : Honeywell DMT LOCATION: Baltimore, MD ELEVATION:

ELEVATION: DRILLING CONTRACTOF Parratt Wolff
DRILLING METHOD AND EQUIPMENT USED: CONE 55 HSIA
ATD WATER LEVEL: START: 95/18 09/5 END: 4/5/18
DEPTH BELOW SURFACE (FT) SON DESCRIPTION LOGGER: L. Paterink USCS

-		SURFAL	C1F11	_		SOIL DESCRIPTION	USCS		COMMENTS
	INTERV	RECOV	ERY (IN)		COPR Streta	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY.		BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION
			MTYPE	DPC (COPR)		OR CONSISTENCY, SOIL STRUCTURE. MINERALOGY.			DRILLING ACTIONS/DRILLER COMMENTS
1.1.1.1	0-					no sample			
Tritte Line	1-3	12	551	4.00.00		10-1.8 no recovery 1.6. 2.0 silt, dry, loose who sand, dark gray 20-23 grave, 1005c, gray 23-30 sand, dry, medium 1042311		1 4 4 7	
11111111111	3-5	, 4	55-2	1.1.4.		3.0-3 c no recovery 3.6-4.5 fine to med sand, some silt, dry 104R211 45-50 same as 2.0-2.3		8 6	
	5 7	10	55	$A=A\cdot A$		50-54 no recovery 54-59 sand, little gravec 1005e, dry 59-6-3 sity sand, dred, bry, little clary 63-6-6 sitty clary, red, dry 66-3-0-6 SA 59-6-3		6 12 36 50/4	
1 1 1 1 1 1 1 1	7 9	15	55	1. 3. 4		70.7.5 no recovery 15.7.7 SAA 77-82 Sand, little grame 100Se dry,104R313 8290 clayey Sitt, Med-tim dry		38 24 13	
1111	97	1.0	50 5	1		9.0.94 no recovery 9.4-9.8 silty sand, Med, dry		4	

EN-2 Page 2 of 3

SOIL BORING LOG

PROJECT: Honeywell DMT ELEVATION

JACOBS

LOCATION Baltimore, MD

TH BELO	W SURFAC	E/FT\			TART 9(5/19 OYIS END 9/5/19		1. Li Raknik
	RVAL (FT)	ERY (IN)		COPR Strate	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY	BLOW COUNTS	DEPTH OF CASING, DRILLIN RATE, DRILLING FLUID LOSS
1		WTYPE	DPC (COPR)		OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY		TESTS, AND STRUMENTATIO DRILLING ACTIONS/DRILLE COMMENTS
1111		55 le			18-10.4 clay, red 2.54R416 dry firm	50/4	
- 1		55	++	НВ	11.11.2 no recovery 11.2-11.7 Silt, dry, firm moderately indurated	22	
	13/10	ľ	++	GB	11.7:12.2 clayer sitt, day	50/2	
-	3 0.4	55	++	нв	13.0-13 4 silt, dry, littlesand weaking indurated 25 yr 416	50/4	
-	5 04	55	++	нв	15.0-SAA except moist	17	
-11	5 8	54	++	4B HB	16.0-16.2 clayer silt, moist 2574413 latine brown), 16.2-16.5 silty - lettle sand, moist 2578416, warry inducted	32	
- 1	x 0.3	4	++	48	14.5-14.7 54 14.0-162	5072	
-13	\$ 05	55	++	GB HB	122-12.5 Silt, moist, 201846 weeky-med indurated	.79	
-13	5 0	55	H	GB	125-18.0 SA 18.0-14.2	27	

BORING NUMBER

EW-2

Page 3 of 3

SOIL BORING LOG

PROJECT Honeywell DMT

JACOBS

LOCATION Ballimore, MD

		LEVE		_		START: 7/5/16 0515 END: 9/5/16	LOGGER	
	BELOW	_	E (FT)		CARLE	SOIL DESCRIPTION	JSCS.	COMMENTS
	INTERV		ERY (IN)		COFR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR: MOISTURE CONTENT, RELATIVE DENSITY,	COUNTS	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION
			MYPE	DPC (COPR)		OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	1	DRILLING ACTIONS/DRILLER COMMENTS
-	18		55	++	98	18.0-18.2 SAA		
5	18.5	0.5	14	++	нв	182-185 Silt, moist, Little sans	32	
1	195	٥٤	55	++	HB	18.5-19,0 SAA	7-7	
11111	19	35	55	++	НВ	190 192 SHA	27	
5	195	p	SS (7			no recovery		
-	20	oS	55	+		20.0-205 sand wigrams,	6	
5	205		55			20.5 - 21.0 SHM	4	
11111	21	65	55			210-21.5 SAA	10	
7	25		55 20			21.5-220 gravel, moist some sand		
9-111	22-	05	95 21			Some grave, moist	8	
1 1	25	05	55 22	T		125-23.0 SAM	6	

)n(ey		Page 1 of 4	Site: Dundalk Marine Terminal Boring No: INC-19 CPT No: CPT-148A	(5-100)	: 574783.9 1448806.9 n: 25.19	м	ethod:	Hollow	Stem	Auger	lff, Inc.) M Hill, Inc.)	GV	al Depth: 30. / Depth: 0.0 i	Ft
Depth Ft	Recov.	Sample	DPC	Soil Code	1	Soil Description	uscs	Comments	w	Gs	PL	LL	P200	qt tsf	fs o tsf	res ohm-m	u . ₁₀ ft ₁₀₀
5					gray (10 YR 4/1) n	D SAND with GRAVEL (SP) - nottled with black (10 YR 2.5/1), nd white (10 YR 8/1), dry to moist,	SP		12								
															3	Loc	us

Site: Dundalk Marine Terminal Boring No: INC-19

Boring No: INC-19 CPT No: CPT-148A Northing: 574783.9 Easting: 1448806.9

Elevation: 25.19

Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Consultant: J. Rodgers (CH2M Hill, Inc.)

Total Depth: 30.0 Ft GW Depth: 0.0 Ft

Date: 10/24/2006

Depth	Recov.	Sample	DPC	Soil	Soil Description	uscs	Comments	Tw	Gs	PL	LL	P200	qt	fs	res	u
Ft 8	ž	ID	DEC	Code	Soil Description	0303	Comments	+**	GS	PL		F200	tsf 500	tsf ,	ohm-m	.10 ft 10
			9		POORLY-GRADED SAND with GRAVEL (SP) – gray (10 YR 4/1) motified with black (10 YR 2.5/1), red (2.5 YR 4/8) and white (10 YR 8/1), dry to moist medium to coarse sand.	SP										
10					SILTY-CLAY (CL) -strong brown (2.5 YR 5/8), dry, hard, low plasticity.	ČL	Increased DPT hammer intensity. Driller states material is like concrete.	15						- - - - - - - - - -		
			1		No Recovery		Refusal at 11 feet. Used auger and rock core to open hole.									
			++	НВ	Minimal recovery. Recovered sample was coarse sand, strong brown (10 YR 4/8), dry.		Material extremely milicum to penetrate with DPT. Sampler bent when retrieved from hole. CPT refusal in 12.9 feet						·			
15													/	5	> Loc	

BORING LOG Page 3 of 4 Site: Dundalk Marine Terminal Northing: 574783.9 Driller: A. Chapel (Parratt Wolff, Inc.) Total Depth: 30.0 Ft Boring No: INC-19 Easting: 1448806.9 Method: Hollow Stem Auger GW Depth: 0.0 Ft Honeywell CPT No: CPT-148A Elevation: 25.19 Consultant: J. Rodgers (CH2M Hill, Inc.) Date: 10/24/2006 Depth 3 Sample Soil fs qt res DPC Soil Description USCS Comments W Gs PL LL P200 ID Code tsf tsf ohm-m, ft 15 /---J + + + WC-18-SQL4618 POORLY-GRADED SAND with GRAVEL (SP) HB -strong brown (7.5 YR 5/8), dry, medium to coarse SP CPT refusal at 18.9 feet. I I I I I1 1 1 1 I I I I I1 1 1 1 INC FRECUING! 3.20 20 1 + 1 + 1111 1111 POORLY-GRADED SAND (SP) -bluish black LILI (GLEY 2 2 5/1 108), becomes strong brown (7.5 YR SP 5/8) at 22-23 feet, moist, fine to medium sand 1 + 1 + 1111 27 3.01 1.1.1.1 2.92 50 MO-19-BOH422X 1111 1111 1111 1111 1111 Locus

Site: Dundalk Marine Terminal Boring No: INC-19

CPT No: CPT-148A

Northing: 574783.9

Easting: 1448806.9 Elevation: 25.19 Driller: A. Chapel (Parratt Wolff, Inc.) Method: Hollow Stem Auger

Consultant: J. Rodgers (CH2M Hill, Inc.)

Total Depth: 30.0 Ft GW Depth: 0.0 Ft

Date: 10/24/2006

			_													
Depth Ft	ecov.	Sample	DPC	Soil	Soil Description	uscs	Comments	w	Gs	PL	LL	P200	qt	fs	res	u
Ft	2	ID	DPC	Code	Soil Description	USUS	Comments	**	GS	PL	LL	P200	tsf	tsf	ohm-m	ft
24	I			GB	POORLY-GRADED SAND (SP) -bluish black (GLEY 2.2.5/1.108), becomes strong brown (7.5 YR 5/1 at 22-23 feet, moist, fine to medium sand.) 5P										
25							DPT push difficult and slow.									
			Ĩ		LEAN CLAY (CL) -gray (10 YR 5/1) and yellowish brown (10 YR 5/8), dry, hard, low plashcity, trace organics.	CL		75								
П	ı				SILTY SAND (SM) -moist, low plasticity, sand tens at 27.6-27.8 feet	SM										
					Collect shelby tube sample.											
30.0	_			Bore	hole completed as an inclinometer location.			-	_							



BORING LOG Page 1 of 4 Honeywell Site: Dundalk Marine Terminal

Boring No: INC-44

Northing: NA Easting: NA Elevation: NA Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 36.0 Ft GW Depth: 0.0 Ft Date: 12/13/2011

Soil Depth Blow S PID DPC USCS Stratum Description Sample Comments Ft Count Code Asphalt. 0 Subbase. WELL GRADED GRAVEL WITH SILT AND SAND (GW-GM), dark bro (7.5YR 3M), dry to moist, fine to medium subrounded gravel with some file to coarse sand and silt, (loose). GW-GM LEAN CLAY (CL), red (2.5YR 4/8), moist, with trace fine sand. CL No recovery within known COPR cell. Refusal/no return beyond 4" bgs, advance augurs to 14" and continue DPT. 5

Site: Dundalk Marine Terminal

Boring No: INC-44

Northing: NA Easting: NA Elevation: NA Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 36.0 Ft GW Depth: 0.0 Ft

Date: 12/13/2011

					Geologis	t. C. Re	ed / A. Louder	
Pepth Ft 10	Rec	PID	DPG	Blow Count	USCS	Soil Code	Stratum Description No rescovery within known COPR cell. Refusal/no return beyond 4' logs, adjustice augurs to 14' and continue DPT.	Sample Comments
15 —					CL		LEAN CLAY (CL), RED (2.5YR 4/8), dry to moist, with trace fine to coarse sand and fine subangular gravel, (stiff).	
20			-					

Site: Dundalk Marine Terminal

Boring No: INC-44

Northing: NA Easting: NA Driller: Parratt Wolff, Inc. Method: Direct Push Total Depth: 36.0 Ft GW Depth: 0.0 Ft

Elevation: NA Consultant: Date: 12/13/2011 Geologist: C. Reed / A. Louder Soil Depth Blow S PID DPC USCS Sample Comments Stratum Description Ft Count Code LEAN CLAY (CL), RED (2.5YR 4/8), dry to moist, with trace 20 fine to comise sand and fine subangular gravel, (stiff). CL GRAVELLY SILT WITH SAND (ML), brown (7.5YR 4/4), moist, with some line to coarse rounded gravel and little fine to coarse eand, (firm). ML ELASTIC SILT (MH), very dark gray (7.5YR 3/1), moist, with trace fine sand, (firm). MH WELL GRADED SAND WITH SILT AND GRAVEL (SW-SM), gray (7.5) R 5/4), well lime to coarse sand with little silt and trace fine in burgular gravel, (medium firm). SW-SM SANDY LEAN CLAY (CL), olive (5Y 5/3), moist to wet, with fine to coarse sand, (soft). 25 CL SANDY LEAN CLAY (CL), light gray (7.5YR 7/1), moist to wet, with fine to coarse sand. (soft). CL

Site: Dundalk Marine Terminal

Boring No: INC-44

Northing: NA Easting: NA Elevation: NA Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 36.0 Ft

GW Depth: 0.0 Ft Date: 12/13/2011

Depth Ft	GIA Rec	DPC	Blow Count	USCS	Soil Code	Stratum Description	Sample Comments
30				CL		SANDY LEAN CLAY (CL), light gray (7.5YR 7/1), moist to wet, with hine to coarse sand, (soft).	
				CL		SANDY LEAN CLAY (CL), brown (2.5YR 5/4), moist to wet, with fine to coarse sand, (soft).	
35 —						No recovery.	

Site: Dundalk Marine Terminal

Boring No: INC-45

Northing: NA Easting: NA

Elevation: NA

Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 36.0 Ft GW Depth: 0.0 Ft

Date: 12/12/2011 - 12/13/21

Depth		-		Blow	1	Soil		
Ft	Rec	PID	DPC	Count	USCS	Code	Stratum Description	Sample Comments
							Asphalt.	
		П						
							Subbase gravel.	
			40					
							WELL GRADED GRAVEL WITH SILT AND SAND (GW-GM), dark brown (7 5/FR 5/4), dry to moist, fine to medium subrounded gravel	
					GW-GM		with some time to coarse sand and silt, (loose).	
		+					SANDY LEAN CLAY (CL), yellow red (5YR 4/6), dry to moist, fine to coerse sand with little silt and subrounded gravel, (soft).	
+					CL			
							POORLY GRADED SAND (SP), brown (7.5YR 5/4), moist, fine to medium sand with trace sit, weakly indurated.	
					SP			
							SANDY LEAN CLAY (CL), red (2.5YR 5/8), wet, fine to coarse	
							sand with little subangular gravel and trace silt, (soft).	
İ					CL			
$^{\circ}$			++					
+					SP	НВ	POORLY GRADED SAND (SP), dark red brown (2.5YR 3/4), moist, fine to medium sand with trace slit, weakly indurated.	
					SP-SM	GB	POORLY GRADED SAND WITH SILT (SP-SM), brown (7.5YR 5/2), moist, with trace fine subrounded gravel, particulate.	

Site: Dundalk Marine Terminal

Boring No: INC-45

Northing: NA Easting: NA Elevation: NA Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 36.0 Ft GW Depth: 0.0 Ft

Date: 12/12/2011 - 12/13/21

				Geologia	0 110	od / A. Loudel	
Rec	PID	DPG	Blow Count	USCS	Soil Code	Stratum Description	Sample Comments
						No recovery	
						LEAN CLAY (CL), red (2 SYR 4/8), moist, with trace fine sand, (frm).	
		٠		CL			
	-					GRAVELLY SILT WITH SAND (ML), brown (7.5YR 4/4), moist, with some fine to coarse subrounded gravel and little fine to coarse sand, (firm).	
				ML			
				мн		ELASTIC SILT WITH SAND (MH), very dark gray (7.5YR 3/1), moist to wet, with little fine to medium sand, (soft).	
	Rec	Rec G	PID DPC	PID DPC Count	PID DPC Count USCS	PID DPC Count USCS Code CL ML	Blow Count USCS Soil Stratum Description No recovery CLAN CLAY (CL), red (2.5YR 4/8), moist, with trace fine sand. (f/mr). CIL GRAVELLY SILT WITH SAND (ML), brown (7.5YR 4/4), moist, with some fine to coarse subrounded gravel and little fine to coarse sand. (f/mr). ML ELASTIC SILT WITH SAND (MH), very dark gray (7.5YR 3/1), moist to wet, with little fine to medium sand, (scit).

Site: Dundalk Marine Terminal

Boring No: INC-45

Northing: NA Easting: NA Elevation: NA Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 36.0 Ft

GW Depth: 0.0 Ft

Date: 12/12/2011 - 12/13/21

			-	Occiogic		Su / A. Loudel	
Depth Ft	GIA Rec	DPC	Blow Count	USCS	Soil Code	Stratum Description	Sample Comments
24						ELASTIC SILT WITH SAND (MH), very dark gray (7.5YR 3/1), moist to well, with little fine to medium sand, (soft).	
25 -		2					
	20						
				МН			
1							
+						POCHLY GRADED SAND WITH SILT (SP-SM), very dark gray (7.5YR 3/1), wet, fine sand with some silt, (very soft).	-
				SP-SM			
30 +						SILTY SAND (SM), gray (7.5YR 5/1), wet, fine to medium sand, (very soft).	
+							
				SM			
+							
						No recovery. Too soft.	
35 +							
36.0							

BORING LOG

Page 1 of 3

Honeywell

Site: Dundalk Marine Terminal

Boring No: INC-46

Northing: NA Easting: NA

Elevation: NA

Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 38.0 Ft GW Depth: 0.0 Ft

Date: 12/14/2011 - 12/15/21

Depth Ft	Rec	PID	DPC	Blow	uscs	Soil Code	Stratum Description	Sample Comments
							HSA wrocut characterization to 12' due to known material. Begin DPT at 12' to define bottom clay depth.	
-								
			**		SP-SM	GB	POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM), dark gray (SY 4/1), moist to wet, fine to medium sand with some silt and fine to coarse subrounded gravel, yellow particulate present.	
			+		SP-SM	НВ	POORLY GRADED SAND WITH SILT (SP-SM), strong brown (7.5YR 4/6), moist, fine to medium sand with little subangular gravel, weakly indurated.	

BORING LOG

Page 2 of 3

Honeywell

Site: Dundalk Marine Terminal

Boring No: INC-46

Northing: NA Easting: NA

Elevation: NA

Driller: Parratt Wolff, Inc.

Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 38.0 Ft GW Depth: 0.0 Ft

Date: 12/14/2011 - 12/15/21

					Geologis	t: C. Rei	ed / A. Louder	
Depth Ft	Rec	PID	DPC	Blow Count	USCS	Soil Code	Stratum Description	Sample Comments
15 —					SP-SM	НВ	PODRLY GRADED SAND WITH SILT (SP-SM), strong brown (7.5YR 4/6), most, file to medium sand with little subangular gravel, wearby indurated. SANDY LEAN CLAY (CL), red (2.5YR 4/5), dry to moist, with some fire to medium sand, (very stiff).	
			1		CL			
20 —					ML		GRAVELLY SILT WITH SAND (ML), brown (7.5YR 4/2), moist to wet, with some fine to coarse sand and fine to medium subangular gravel (stiff).	
					мн		ELASTIC SILT (MH), dark gray (7.5YR 4/1), wet, with trace fine sand and petroleum odor, (very soft).	
					SM		SILTY SAND (SM), gray (7.5YR 6/1), wet, medium sand with some silt, (loose).	
25 —							No recovery.	
28				l				

Site: Dundalk Marine Terminal

Boring No: INC-46

Northing: NA Easting: NA Elevation: NA Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 38.0 Ft

GW Depth: 0.0 Ft

Date: 12/14/2011 - 12/15/21

Depth Ft	Rec	PID	DPG	Blow	USCS	Soil Code	Stratum Description	Sample Comments
28	Ī				SM		SILTY SAND (SM), reddish yellow (7.5YR 6/5), wet, medium sand with some sit, (loose).	
30 +					мн		ELASTIC SILT WITH SAND (MH), very dark gray (YR 3/1), wer, with some fine to medium sand, (very soft).	_
	1						No recovery	_
35 —								
			-		мн		ELASTIC SILT WITH SAND (MH), very dark gray (YR 3/1), wet, with some fine to medium sand, (very soft).	

BORING LOG Page

Page 1 of 3

Site: Dundalk Marine Terminal Boring No: INC-47

Honeywell

Northing: NA Easting: NA Elevation: NA Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 38.0 Ft GW Depth: 0.0 Ft

Date: 12/15/2011 - 12/16/21

Depth Ft	Rec	PID	DPC	Blow	uscs	Soil Code	Stratum Description	Sample Comments
						1.101	Not sampled. Drillers used HSA to 12' bgs -not sampled since interval material already know. DPT at 12'.	
-								
			++		SP-SM		POORLY GRADED SAND WITH SILT AND GRAVEL (SP-SM), dark gray (SYR 4/1), wet, fine to medium sand with some silt and fine to medium subangular gravel, particulate.	
5			-		CL		SANDY LEAN CLAY (CL), red (2.5YR 4/6), dry to moist, with some fine to medium sand and trace fine subangular gravel, (very stiff).	

BORING LOG

Page 2 of 3

Site: Dundalk Marine Terminal Boring No: INC-47

Honeywell

Northing: NA Driller: Parratt Wolff, Inc.
Easting: NA Method: Direct Push

Elevation: NA Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 38.0 Ft GW Depth: 0.0 Ft

Date: 12/15/2011 - 12/16/21

				Geologis	t: C. Rec	ed / A. Louder	
Depth Ft	OIP &	DPC	Blow Count	uscs	Soil Code	Stratum Description	Sample Comments
15		5-8		CL		SANDY LEAN CLAY (CL), red (2.5YR 4/6), dry to moist, with some hine to medium sand and trace fine subangular gravel, (very skiff)	
						SANDY LEAN CLAY (CL), red (2.5YR 4/6), dry to moist, with some him so medium sand and little fine to medium subround gravel, (stiff).	
		÷		CL			
1				ML	=1	GRAVELLY SILT WITH SAND (ML), brown (7.5YR 4/2), moist	
+ 05	7 1			ML		to wet, with some fine to coarse sand and fine to medium subangular power. (astr). No recovery (Poorly graded sand in top of macrocore sampler.	
						SILTY SAND (SM), pale olive (5Y 6/3), wet, fine to medium sand with some sit, (loose).	
25 +				SM			
				SM		SILTY SAND (SM), reddish yellow (7.5YR 6/5), wet, fine to medium sand with some silt, (loose).	
-						ELASTIC SILT (MH), very dark gray (7.5YR 4/1), wet, with	
+				мн		trace fine sand, (soft).	
₃₀ ∐				L		LL	

BORING LOG Page 3 of 3

Honeywell

Site: Dundalk Marine Terminal

Boring No: INC-47

Northing: NA Easting: NA Elevation: NA Driller: Parratt Wolff, Inc. Method: Direct Push

Consultant:

Geologist: C. Reed / A. Louder

Total Depth: 38.0 Ft

GW Depth: 0.0 Ft

Date: 12/15/2011 - 12/16/21

Depth Ft	Rec	PID	DPC	Blow Count	USCS	Soil Code	Stratum Description	Sample Comments
30							ELASTIC SILT (MH), very dark gray (7.5YR 4/1), wet, with trace line sand, (soft).	
		Ш	6					
					мн			
35 —	ı	Ш	1.00					
+								
			-					
38.0								

Site: Dundalk Marine Terminal

Boring No: INC-27VW

Northing: 574193.2 Easting: 1447924.8

Elevation: 20.4

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Mary Velasquez

Total Depth: 36.0 Ft

GW Depth: 12.0 Ft Date: 01/24/2010

epth c	Rec	DPC	Blow	uscs	Soil Code	Stratum Description	Sample Comments
						Asphalt	
+	Į	æ	5-13-9	sc		Clayey Sand and Gravel (SC) -large gravel fragmenta (fill)	geotextile fabric liner encountered
						Sity Sand (SM) -dark brown (10 YR 3/3), medium dense, dry, fine sand, trace subangular fine gravel, (fill material)	
		9	8-8-12-15	SM			
+			2-4-4-5	SM		Sity Sand (SM) dark brown (10 YR 3/3), loose few coarse angular gravel, dry	geotekile fatrić liner encountered
+				GM		Sity Gravel (GM) -dark grayish brown (10 YR 3/2), coarse gravel, stemp, medium asnae	
+			9-7-12-50/5	SM		Sity Sand (SM) -brown (10 YR 3/3), medium dense, few coarse angular gravel, dry	
+						Sity Sand (SM) -olive gray (5 Y 4/2), road base, concrete, moist, medium dense	refusal at 7 9" with spiril spoons, auger to 9.0"
		-	6-10-5-6	SM			



Site: Dundalk Marine Terminal

Boring No: INC-27VW

Northing: 574193.2

Easting: 1447924.8

Elevation: 20.4

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Mary Velasquez

Total Depth: 36.0 Ft

GW Depth: 12.0 Ft

Date: 01/24/2010

Depth Ft	Rec	DPC	Blow	uscs	Soil Code	Stratum Description	Sample Comments
9				SM		Sity Sand (SM) -olive grey (5 Y 4/2), road base, concrete, moist, medium dense	
		1.0	6-10-5-6	SM		Sity Sand (SM) -brown (7.5 YR 5/4), trace fine to coarse angular gravel	_
			1	SM		Sifty Sand (SM)-brown (10 YR 3/3); medium dense, lew coarse angular gravel, dry	
0		3	4-8-50/3			Fill malerial (soncrete, asphalt)	
	/	+	4-8-50/3	SM	НВ	Silly Sand (SM) idark yellowish brown (10 YH 3/5), most, dry in last 0.2', fine to medium sand, very fine angular gravel, very dense, strongly indurated	refusal at 11.2' with split spoons, auger to 12.0'
+		-		SM	GB	Sify Sand (SM) black (10 YR 2/1), trace yellow ricdules, wet, fine sand, particulate	
				SM	HB	Sity Sand (SM) -dark, yellowish brown (10 YR 3/0), highly indurated, moist	
			Variation 1	SM	GB	Silly Sand (SM) -black [10 YR 2/1], trace graver, trace yellow nodules, moist, slightly indurated	
		**	28-29			Sity Sand (SM) -dark yelliwinish brown (10 YR 3tb), trace yellow nodules, fine sand, slightly indurated, strongly indurated at 14.6°, very dense.	refusal at 13.0' with split spoons, auger to 14.0'
5 +		++	23-50/4	SM	НВ		Februari at 14 8" with spirit appoints, sugar to 15.0"
		++	50/3	SWI	НВ		refusal at 15.3' with split spoons, suger to 10.0'
Ī		++	50/4				refusal at 16.3' with solid spooris, awger to 17.0'
t				SM	GB	Silly Sand (SM) -bank yellowish brown (16 VR 316), trace verious holdings, time saind, highly indurated, very	
			I	I	I	Sirty Sand (SM) -dark yellowish brown (10 YR 3/6), trace yellow nodules, the sand, highly indurated, yery -dense	



Site: Dundalk Marine Terminal

Boring No: INC-27VW

Northing: 574193.2

Easting: 1447924.8

Elevation: 20.4

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Mary Velasquez

Total Depth: 36.0 Ft

GW Depth: 12.0 Ft Date: 01/24/2010

Locus

1 1						
Rec	DPC	Blow	uscs		Stratum Description	Sample Comments
			SM	НВ	Sifty Sand (SM) -dark yellowith brown (10 YR 3/5), trace yellow nodules, fine sand, highly indurated, very dense	
	**	18-62-23-7	SM	GB	Sity Sand (SM) -black (10 YR 3/11, yellów nodules, green nodules, moist, particulate	driller notes strata change
					Lean Clay (CL) moisied with red (2.5 YR 4/8), yellowish red (5YR 5/8) and light brownish gray (10 YR 6/2), sifly clay with fine sand, slightly moist, very stiff	
	*	3-5-13-17	CL			
			11			
	+	6-4-7-6				
ш			CL		Lean Clay (CL) -yellowish red (5 YR 5/8), high plasticity, shiff	
		WOR	CL		Lean Clay (CL) -yellowish red (5 YR 5/8), mottled with red (2.5 YR 4/8) and light brownish gray (10 YR 6/2), silty clay with fine sand, moist, very soft	
		2-3-4-5	SP		Poorly Graded Sand (SP) -dark olive brown (2.5 Y 3/3), time to medium sand, moist, loose	
	Rec		++ 18-62-23-7 + 3-5-13-17 + 6-4-7-6	DPC Count USCS SM ++ 18-62-23-7 SM + 6-4-7-6 - WOR CL	DPC Count USCS Code SM HB ++ 18-62-23-7 SM GB + 6-4-7-6 - WOR CL	DPC Count USCS Code Stry Sand (SM) -dark yellowah brown (10 YR 38), trace yellow nodules, fine sand, highly indurated, very denser ++ 18-62-23-7 SM GB Sity Sand (SM) -black (10 YR 3/11, yellow nodules, green nodules, moist, particulate Lisrar Clay (CL) -motision with red (2.5 YR 4/8), yellowish red (3YR 5/8) and light brownish gray (10 YR 6/2), sitly disk with fine sand, slightly moist, refly stit! + 6-4-7-6 CL Lean Clay (CL) -yellowish red (5 YR 5/8), high plasticity, stif! Lean Clay (CL) -yellowish red (5 YR 5/8), motiod with red (2 5 YR 4/8), and light brownish gray (10 YR 6/2), sitly clay with fine sand, moist, very soft - WOR CL Poorty Graded Sand (SP) -dark plive brown (2 5 Y 3/3), fine to imposure sand, moist, bode

Site: Dundalk Marine Terminal

Boring No: INC-27VW

Northing: 574193.2

Easting: 1447924.8 Elevation: 20.4 Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Mary Velasquez

Total Depth: 36.0 Ft

GW Depth: 12.0 Ft Date: 01/24/2010

Depth Ft	Rec	DPC	Blow	uscs	Soil Code	Stratum Description	Sample Comments
27		e	2-3-4-5	SP		Poorly Graded Sand (SP) -dark offve brown (2.5 Y 3/3), fine to medium sand, moist, loose	
Ì			1-2-2-3	SP		Poorty Graded Sand (SP) -gray (2.5 V firt), very fine to fine sand, wet, loose	
80 +		(4)	1-2-2-4	SP		Poorty Graded Sand (SP) - sycred black (2 % Y 2/1) and gray (2.5 Y 5/1), fine sand, loose, wet	
			5-3-3-2	ML		Clayey Sit (ML) -gray (2.5 Y 5/1), medium stiff, wet, low plassicity	
				sc		Clayey Sit (SC) -gray (2.5 Y 5/1), wet, sand clay mixture, fine samz	
35		-	4-2-11	SM		Sity Sand (SM) -dark gray (2.5 Y 4/1), medium sand, trane shell fragments, wall, medium dense	Vibrating wire piezometer installed at 7, 19 3 and 34 bgs. Inclinometer installed at 35 bgs.



Site: Dundalk Marine Terminal

Boring No: INC-30VW

Northing: 574482.2

Easting: 1448146.9

Elevation: 21.562

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Mary Velasquez

Total Depth: 36.0 Ft

GW Depth: 33.0 Ft Date: 01/25/2010

epth Ft	Rec	DPC	Blow	uscs	Soil Code	Stratum Description	Sample Comments
			15-9-8	sc		Asphalt and road base Clayey Sand with Gravel (SC) -large subangular gravel fragments, (fill)	
		4	5-6-18-14	SM		Sity Sand (5M) -dark brown (10 VH 3/3), magain dense, dry, 1.6-2' fine sand, trace fine subangular gravel, (fill)	geotextile fabric liner encountered
		-	2-2-3-4	ML SM		Clayey Sit (ML) -brown (10 YR 4/3), medium stiff, damp, slight plasticity, some fine sand Sity Sand (SM) -dark brown (10 YR 3/3), damp, few coarse angular gravel Road base with gravel, coarse sand, asphalt	geolestile fabric liner encountered
						No Recovery	retusel tel 7.5' with spirt spoons, auger to 6.0'



Site: Dundalk Marine Terminal

Boring No: INC-30VW

Northing: 574482.2

Easting: 1448146.9

Elevation: 21.562

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc. Geologist: Mary Velasquez GW Depth: 33.0 Ft Date: 01/25/2010

Total Depth: 36.0 Ft

epth Ft	Rec	DPC	Blow	uscs	Soil Code	Stratum Description	Sample Comments
		3	10-17-14-14			Road bese, sand-gravef sitt mix, with coarse subangular gravel, asphalt	
0 +	2	-+-		SM	HR	Sity Sand (SM) dark yellowes brown (10 YR 38), booth indurated, dry medium dense. Well Graded Sand (SW) -otive yellow (2.5 Y 6/9), horits, fine to medium sand, coarse subrounded gravel, medium dense.	
ŀ	ζ	4	8-11-8-5	sw			
			4-11-25-23	CL		Lean Clay (CL) -yellowish red (5 YR 5/8). modiled with red (2.5 YR 4/6), fine sand, moist, hard, sitly clay 12-13.2	
		++		SM	НВ	Sity Sand (SM) -dark yellowish brown (10 YR 3/6), highly indurated, dry, yellow nodules. (dense) Sity Sand (SM) -black (10 YR 2/1), dry, loose, particulate, fine to medium sand	
t				SM	HB/GB		
5 +		++	27-34-50/5	SM	НВ	Sity Sand (SM) -dark yellowish brown (10 YR 5/6), yellow nodules, dry, very dense	
				SM		Sity Sand (SM) -dark brown (10 YR 3/3) mottled with gray (10 VH 5/3) and brownish yellow (10 YR 6/5), most trace small angular gravel	refusel at 15.4" with split spoons, auger to 18.0"



BORING LOG Page 3 of 5
Honeywell

Site: Dundalk Marine Terminal

Boring No: INC-30VW

Northing: 574482.2

Easting: 1448146.9 Elevation: 21.562 Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Mary Velasquez

Total Depth: 36.0 Ft

GW Depth: 33.0 Ft Date: 01/25/2010

Depth Soil Blow Rec DPC USCS Stratum Description Sample Comments Ft Count Code Silty Sand (SM) -dark brown (10 YR 3/3) mottled with gray (10 YR 5/3) and brownish yellow (10 YR 6/6), moist, 16 race simali angular gravel SM No Recovery refusal at 16.8' with split spoons, auger to 18.0' 32-50/3 Sity Sand (EM) -olack (10 YR 2/1), more: signify indurated, medium dense, fine to medium sand SM HB/GB WOR-4-10-17 20 Sity Sand (SM) -dark yellowish brown (10 YR 3/3), lightly indurated, stage hard brown with yellow nodules: 5-4-7-13 SM HB/GB Lean Clay (CL) -red (2.5 YR 4/8), moist, fine sand, (very 站街 CL SC 5-6-5-7 Lean Clay (CL) -red (2.5 YR 4/8), moist, medium plasticity, stiff CL Clayey Sand (SC)-dark-brown (10 YR 3/3), moist, loost lane sand: - - - -Locus

Page 4 of 5 **BORING LOG** Honeywell

Site: Dundalk Marine Terminal

Boring No: INC-30VW

Northing: 574482.2

Easting: 1448146.9

Elevation: 21.562

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Mary Velasquez

Total Depth: 36.0 Ft

GW Depth: 33.0 Ft Date: 01/25/2010

Depth Ft	Rec	DPC	Blow	uscs	Soil Code	Stratum Description	Sample Comments
25 —		*				Clayery Sand (SC) -dark brown (10 YR 3/3), moigt, loose, fine sand	
		-	3-3-5-5	sc			
						Poorly Graded Sand (SP) -dark graytin brown (2.5 Y 3/2) with trace black (2.5 Y 2.5/1), moist, loose, coarse subrounded graves fine to madium sand.	
				SP			
1			4-4-5-5	ML		Sandy Sin (ML) -very stars grayer treems (2.5 Y 3/2), fine sand, moist, soft, medium stiff, layers of sandy will and clayery will	•
			11=1			Sit (ML) very dark graysh prown (2.5 ¥ 3/2), clayey sit, slight plasticity, moist, stiff	
				ML			
†		-	7-11-4-6	sw		Well Graded Sand with Gravel (SW) -dark grayish brown (2.5 Y 3/2), shell tragments line to medium subrounded gravel, medium dense, moist	
100						Sity Clay (CL) -dark greenish gray mottled with bluish black (GLEY 2 4/1 and 2.8/1), moist, medium to high plasticity, stiff	
				CL			
			2-3-5-12				
				SP		Poorly Graded Sand with Gravel (SP) -dark greenish gray (GLEY 2 4/1), 32.5-34' greenish gray (GLEY 1 5/1), moist dense fine to medium sand, trace well reunded fine project.	
₃₂ ⊥	Ш			SP		Prooffy Graped Same with Gravet (SP) - cark greening gray (GLET 2 w/1), 36.5-34 greening gray (GLET 1-37). Indist, depse, fine to medium sand, trace well-rounded line gar/el-	*Loor



Site: Dundalk Marine Terminal

Boring No: INC-30VW

Northing: 574482.2

Easting: 1448146.9

Elevation: 21.562

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Mary Velasquez

Total Depth: 36.0 Ft

GW Depth: 33.0 Ft

Date: 01/25/2010

Yec I	DPC	Count	USCS	Soil Code	Stratum Description	Sample Comments
		5-14-20-15	SP		Poorly Graded Sand with Cravel (SP) -dark greenish gray (GLEY 2 4/1), 32.5-34" greenish mast, dense, fine to medium sand, trace well reunded fine gravel	gray (GLEY 1 5/1),
		JI-12-15-24	SP		Poorly Graded Sand (SP) -greenen gray (GLEY 1 SF), fine to medium sand, wet, medium rounded gravet	dense, trace fine Vibrating wires installed at 7', 18' and 28' bgs. Inclinometer installed to 35'.
				8-12-15-24 SP	7-12-15-24 SP	Pointy Graded Sand (SP) -greensh gray (GLEY 1'SF), fine to medium sand, wet, medium (conded grave)



Site: Dundalk Marine Terminal

Boring No: INC-49



Driller: Boart Longyear Consultant: CH2M Hill

Method: Rotosonic Geologist: E. Curbo

Total Depth: 40.0 Ft

Start Date: 07/13/2012

End Date: 07/13/2012

						End Date: 07/13/2012
Depth	Rec	DPC	Stratum	USCS Code	Stratum Description	<u>I</u>
10				CH	FAT CLAY with GRAVEL (CH), reddish brown (2.5YR 4/4), dry, with subangular gravel,	stiff.
				СП	POORLY GRADED SAND with GRAVEL (SP), very dark brown (10YR 2/2), dry, fine to gravel and trace silt, weakly indurated, loose.	coarse grained sand with subangular
			НВ	SP		
†			GB	SP	POORLY GRADED SAND (SP), very dark gray (10YR 3/1), fine to medium grained sar loose.	d with trace rounded gravel,
			НВ	SP	POORLY GRADED SAND with GRAVEL (SP), very dark brown (10YR 2/2), dry, fine to gravel and trace silt, weakly indurated, loose.	coarse grained sand with subangular
15 +		++		СН	FAT CLAY with GRAVEL (CH), reddish brown (2.5YR 4/4), dry, with some subrounded	gravel, stiff.
			GB	SP	POORLY GRADED SAND (SP), very dark gray (10YR 3/1), moist, fine to medium grain weakly indurated, medium dense.	ed sand with trace rounded gravel,
			НВ	SP	POORLY GRADED SAND (SP), brown (7.5Y 4/4), moist to dry, fine to medium grained indurated, medium dense.	sand with trace silt, strongly
†			GB	SP	POORLY GRADED SAND (SP), very dark gray (10YR 3/1), moist, fine to medium grain weakly indurated, medium dense.	ed sand with trace rounded gravel,

Site: Dundalk Marine Terminal

Boring No: INC-49

Driller: Boart Longyear

Method: Rotosonic

Consultant: CH2M Hill

Geologist: E. Curbo

Total Depth: 40.0 Ft

Start Date: 07/13/2012

End Date: 07/13/2012

					Life Date. 07/13/2012
			LISCS		
3ec	DPC	Stratum		Stratum Description	
				FAT CLAY with SAND (CH), reddish brown (2.5YR 4/4), moist, with medium to fine grain	ed sand, low plasticity, firm.
			СН		
			СН	SANDY LEAN CLAY (CL), reddish brown (2.5YR 4/4), moist, with fine to medium graine soft.	d sand, low plasticity, very
				FAT CLAY (CH), red (2.5YR 4/6), moist, high plasticity, firm.	
			СН		
			SP		
	-		SP	POORLY GRADED SAND (SP), reddish brown (2.5YR 4/4), wet, medium to coarse grai soft, very loose.	ned sand with trace clay, very
			SP	POORLY GRADED SAND (SP), brown (7.5YR 4/4), wet, medium to coarse grained san very loose.	d with trace clay, very soft,
			SP	POORLY GRADED SAND (SP), grayish brown (10YR 5/2), wet, medium to coarse grain soft, very loose.	ed sand with trace clay, very
	Rec	PPC DPC	DPC Stratum	CH CH SP SP SP	SANDY LEAN CLAY (CL), reddish brown (2.5YR 4/4), moist, with medium to fine grain soft. CH SANDY LEAN CLAY (CL), reddish brown (2.5YR 4/4), moist, with fine to medium graines soft. CH FAT CLAY (CH), red (2.5YR 4/6), moist, high plasticity, firm. CH SP POORLY GRADED SAND (SP), light brownish gray (10YR 6/2), wet, medium to coarse grainsoft, very loose. SP POORLY GRADED SAND (SP), brown (7.5YR 4/4), wet, medium to coarse grainsoft, very loose. SP POORLY GRADED SAND (SP), brown (7.5YR 4/4), wet, medium to coarse grainsoft, very loose. SP

BORING H(ey		Page 1 of 3	Site: Dundalk Marine Terminal Boring No: INC-1501-C CPT No:	Northing: 574233.5 Easting: 1448048.8 Elevation: 21.53	Method:	'arratt Wolff, Inc. NA ant: Mira Abdelaziz (CH2M Hi	ll, Inc.)		pth: 27.0 oth: 0.0 Fi	t
Depth oo	Sample	DPC	Soil Code	Asphall and grave	Soil Description	i	uscs	Comments	w	Gs	PL	LL	P200
5					-brown (7.5 YR 4/3), some clay and	l gravel, damp, stiff, silty	SM						
		-		SAND (SP) -with o	concrete, some gravel, dry, dense sa	nd.	SP						
9											11	OC	us

BORING LOG Page 2 of 3 Site: Dundalk Marine Terminal Northing: 574233.5 Driller: Parratt Wolff, Inc. Total Depth: 27.0 Ft Honeywell Boring No: INC-1501-C Easting: 1448048.8 Method: NA GW Depth: 0.0 Ft CPT No: Consultant: Mira Abdelaziz (CH2M Hill, Inc.) Date: 01/26/2007 Elevation: 21.53 Depth 3 Sample Soil DPC Soil Description USCS Comments W Gs PL LL P200 ID Code SAND (SP) with concrete, some gravel, dry, dense sand SP 10 SILTY CLAY (CL) -red (2.5 YR 5/6), trace fine sand, stiff, moist sity clay. CL refusal -no recovery. SILTY SAND (SM) -reddish brown (5 YR 4/4), weakly indurated, some to little gravel, moist, medium stiff to stiff, silty sand. SM 4+ SILTY SAND (SM) -black (GLEY 1 2.5), green particles, yellow particles, moist SM to wet, medium stiff to stiff, silty sand. 15 SILTY SAND (SM) -reddish brown (5 YR 4/4), weakly indurated, some to little gravel; SM moist, medium stiff to stiff, silty sand. SILTY SAND (SM) -black (GLEY 1 2.5), green particles, yellow particles, moist GB SM to wet, medium stiff to stiff, silty sand. refusal -no recovery.

Locus

H		ey		Page 3 of 3	Site: Dundalk Marine Terminal Boring No: INC-1501-C CPT No:	Northing: 574233.5 Easting: 1448048.8 Elevation: 21.53	Method:	Parratt Wolff, Inc. NA ant: Mira Abdelaziz ((CH2M Hi	l, Inc.)	GW Dep	pth: 27.0 oth: 0.0 Fi	t
Depth 30	Sample	DPC	Soil Code		Soil Description	n	uscs	Comments	w	Gs	PL	LL	P200
18	75.1	**		refusal -no recove	ry.								
		7		SILTY SAND (SM sity sand	-yellowish brown (10 YR 5/4), some	e gravel, wet, medium stiffness,	SM						
20 —	SILTY SAND (SM) -yellowish brown (10 YR 5/4) some gravel, wet, medium stiffner sity sand. SILTY SAND (SM) -ned (2.5 YR 4/5), little clay, trace gravel, medium stiff to stiff, wet to moist, sity-sand.				ravel, medium stiff to	SM							
25				CLAYEY SILT (ML soft, clayey silt.	.) -reddish brown (2.5 YR 4/4), some	e fine sand, trace gravel, wet,	ML						
27.0											:1,	OCI	us

Site: Dundalk Marine Terminal Boring No: INC-1501-D

CPT No: Elevation: 21.77

Driller: Parratt Wolff, Inc. Method: NA

Consultant: Mira Abdelaziz (CH2M Hill, Inc.)

Total Depth: 28.0 Ft GW Depth: 0.0 Ft

Date: 01/23/2007

epth Ft	Recov	Sample	Blow	DPC	Soil Code	Soil Description	uscs	Comments	w	Gs	PL	LL	P20
	Ī				5000	Aschait gravel successe							
			3-30-9	×		SAND (SP) -tark yellowish brown (10 YR 4/4), musum graved, some gravel, trace sit, damp, medium density, sand	SP						
			10-7-11-14										
5			46.13.13			SILTY SAND (SM) dark yellowish brown (10 YR 4/4), some gravel, damp, medium dense, trace day, sity sand	SM						
			12-12-37-50/3	-		GRAVEL AND SAND (SW) -medium grained, dry, hard, with concrete gravel and seind very dense.	sw						
			35-25-17-20	**		SAND (SP) -bluish gray (GLEY 2 5/1), medium grained, damp, dense sand.	SP						
					GB	SILTY SAND (SM) -red (10 YR 4/6), little gravel and clay, very dense, setly sand.	SM						

Northing: 574248.6

Easting: 1448085.7



Site: Dundalk Marine Terminal Boring No: INC-1501-D CPT No: Northing: 574248.6 Easting: 1448085.7

Elevation: 21.77

Driller: Parratt Wolff, Inc.

Method: NA

Consultant: Mira Abdelaziz (CH2M Hill, Inc.)

Total Depth: 28.0 Ft GW Depth: 0.0 Ft

Date: 01/23/2007

	Recov	Sample ID	Blow Count	DPC	Soil Code	Soil Description	uscs	Comments	w	Gs	PL	LL	P200
10	I		4-50/4	4	GB	SILTY SAND (SM) -red (10 YR +6), hits gravet and day, very dense, sity sand.	SM						
					НВ	SILTY SAND (SM) -dark redden brown (5 YR 3/2), indurated trace gravel, very dense, sity send.	SM						
			50/2	J		no recovery		Classified as HB based on descriptions and high blow counts through intervals 12-14 and 14-16.					
15 -			50/4		НВ	SILTY SAND (SM) -dark reddish brown (5 YR 3/2), indurated, trace servel, very demail sity sand.	SM						
			28-50/5	**		SILTY SAND (SM) -black (GLEY 2.5/1), little gravel, green particulaises semin very dense, sitty sand.	SM						
			50/4	**		SAND (SP) -red (20 YR 5/3), some clay, trace gravel, wet to moiste very dense, sity sand.	SP						
ю Ш							+				71	OCI	us

Site: Dundalk Marine Terminal Boring No: INC-1501-D CPT No: Northing: 574248.6 Easting: 1448085.7

Elevation: 21.77

Driller: Parratt Wolff, Inc.

Method: NA

Consultant: Mira Abdelaziz (CH2M Hill, Inc.)

Total Depth: 28.0 Ft

GW Depth: 0.0 Ft Date: 01/23/2007

Depth Ft	Recov.	Sample	Blow	DPC	Soil Code	Soil Description	uscs	Comments	w	Gs	PL	LL	P200
20		H	33-40-47-12			SILTY SAND (SM) red (20 R 4/II), some days, track graves, well to moise, very dense, effly send.	SM						
			Q-9-10-16			no recovery							
25 -			3-4-6-7			SILTY SAND (SM) -red (20 R 4/6), some clay, trace gravel, wet to moist, loose, sity sand.	SM						
28.0			3-4-6-9			SAND (SP) -black (GLEY 2.5/1), medium grained, trace silt, wet, loose, skind.	SP						



	on On	e,	y V	Page 1 of 3	Site: Dundalk Marine Terminal Boring No: INC-1501-I CPT No:	Northing: 574030.1 Easting: 1447951.8 Elevation: 19.71	Method:	Parratt Wolff, Inc. NA ant: Mira Abdelaziz ((CH2M Hi	l, Inc.)	Total De GW Dep Date: 01	th: 0.0 Ft	1
Depth S	Sample	DP	Soil Code		Soil Description	n	uscs	Comments	w	Gs	PL	LL	P200
5 —				Asphalt / gravél su SILTY SAND (SM sility sand) -dark grayish brown (10 YR 4/2), se	ome gravel and clay, moist,	SM						
				SAND (SP) -ligh g	ray (10 YR 7/1), medium grained, or	oncrete, some gravel, dry, sand	SP						
-				SILTY SAND (SM) silty sand.) -dark grayish brown (10 YR 4/2), so	ome gravel and clay, moist,	SM						
10 —			_L	L						_	11,	oci	us

BORING LOG Page 2 of 3 Site: Dundalk Marine Terminal Northing: 574030.1 Driller: Parratt Wolff, Inc. Total Depth: 27.0 Ft Honeywell Boring No: INC-1501-I Easting: 1447951.8 Method: NA GW Depth: 0.0 Ft CPT No: Elevation: 19.71 Consultant: Mira Abdelaziz (CH2M Hill, Inc.) Date: 01/24/2007 Depth 20 Sample Soil DPC Soil Description USCS Comments W Gs PL LL P200 ID Code 10 SILTY SAND (SM) -dark grayish brown (10 YR 4/2), some gravel and clay, moist, SM silty sand CL SILTY CLAY (CL) -red (10 YR 5/6), moist, cohesive, stiff, silty clay. no recovery 15 SILTY SAND (SM) -yellowish red (5 YR 4/6), very hard, dry, some gravel, sitty HB SM SILTY SAND (SM) -black (GLEY 1 2.5/-), green particulates, moist to dry, some SM SILTY CLAY (CL) -red (10 YR 5/6), moist, stiff, silty clay. CL SILTY SAND (SM) -red (10 YR 5/6), molted with yellow and white, some clay and SM gravel, silt pockets, wet to moist, stiff, silty clay. Locus

H	on	ey	V	vell	Boring No: INC-1501-I CPT No:	Easting: 1447951.8 Elevation: 19.71	Method: Consulta	NA ant: Mira Abdelaziz (CH2M Hil	l, Inc.)	GW Dep Date: 01	th: 0.0 Ft /24/2007	
Pepth 800 Pt 20	Sample ID	DPC	Soil Code		Soil Descriptio	n	uscs	Comments	w	Gs	PL	LL	P200
				SILTY SAND (SM gravel, set pockets) -red (10 YR 5/5), molled with yellow s, wet to moist, stiff, sitty clay.	w and white, some clay and	SM						
25 —		201		CLAYEY SILT (CL	.) -black (5 YR 2,5/1), trace medium	to fine sand, soft, clayey	ML						
				SAND (SP) -black	s (5 YR 2.5/1), wet, loose, medium s	and.	SP						

)n(ey		Page 1 of 5	Site: Dundalk Marine Terminal Boring No: INC-1501-K CPT No:	Northing: 573868.8 Easting: 1448175.1 Elevation: 19.8	Metho	A. Chapel (Parratt Wo d: Hollow Stem Auger litant: J. Rogers (CH2))	Total Dep GW Dep Date: 06	th: 0.0 Ft	1
Depth	Recov.	Sample	DPC	Soil Code		Soil Description	n	uscs	Comments	w	Gs	PL	LL	P200
0					Precut asphalt and	d gravel pack			Location is ~ 8' from a storm drain					
					GRAVELLY SAND sand with some m) (SW) -gray (7.5YR 5/1), brown (7.5 edium sand, fine gravel	YR 4/3), moist, (loose), coarse	sw						
					SANDY SILT (ML) silt, fine to mediun	-dark brown (7.5YR 3/3), brown (7.5 n sand with some gravel, trace organ	5YR 4/3), medium stiff, moist, nics, trace debris (brick)	ML						
5 _			-									11	oci	us

BORING LOG Page 2 of 5 Site: Dundalk Marine Terminal Northing: 573868.8 Driller: A. Chapel (Parratt Wolff, Inc.) Total Depth: 24.0 Ft Honeywell Boring No: INC-1501-K Easting: 1448175.1 Method: Hollow Stem Auger GW Depth: 0.0 Ft CPT No: Elevation: 19.8 Consultant: J. Rogers (CH2M Hill, Inc.) Date: 06/15/2007 Depth 8 Sample Soil DPC Soil Description USCS Comments W Gs PL LL P200 Ft ID Code 5 SANDY SILT (ML) -dark brown (7.5YR 3/3), brown (7.5YR 4/3), medium stiff, moist, ML sit, fine to medium sand with some gravel, trace organics, trace debris (brick) SANDY SILT (ML) -dark brown (7.5YR 3/3), brown (7.5YR 4/3), soft, moist, silt, fine to medium sand with some gravel, trace organics, trace debris (brick), low plasticity ML SANDY SILT (ML) -dark brown (7.5YR 3/3), brown (7.5YR 4/3), medium stiff, moist, Increased ML silt, fine to medium sand with some gravel, trace organics, trace debris (brick) hammering at ~7.0" POORLY-GRADED GRAVEL (GP) -light gray (7.5YR 7/1), damp, hard, gravel, GP coarse sand Appears: to be old concrete. mixed with asphall black (7.5VR 2.5/1) WELL-GRADED SAND (SW) -gray (7.5YR 6/1), moist, (loose), coarse sand, medium 5W HC-1801-X-5X3-08000 13 sand, some fine gravel FAT CLAY (CH) -red (2.5YR 5/8), moist, medium stiff to soft, high plasticity. CH Locus

BORING LOG Page 3 of 5 Site: Dundalk Marine Terminal Northing: 573868.8 Total Depth: 24.0 Ft Driller: A. Chapel (Parratt Wolff, Inc.) Honeywell Boring No: INC-1501-K Easting: 1448175.1 Method: Hollow Stem Auger GW Depth: 0.0 Ft CPT No: Elevation: 19.8 Consultant: J. Rogers (CH2M Hill, Inc.) Date: 06/15/2007 Depth 3 Sample Soil DPC Soil Description uscs Comments W Gs PL LL P200 ID Code 10 FAT CLAY (CH) -red (2.5YR 5/8), maist, medium stiff to soft, high plasticity CH 10.5 -10.8 WELL-GRADED SAND with GRAVEL (SW) -strong brown (7.5YR 5/8), damp, (dense), lightly to moderately indurated, coarse sand, fine gravel, trace yellow nodules, GB COPR present from 10.5-10.8 feet HB/GB mix GB very dark gray (7.5YR SW HB 13 NO 1981 IN ACTIVISATION ++ 3/1), fine sand, lightly indurated Retains No Recovery at 11.5; yery hard Locus

BORING LOG Page 4 of 5 Site: Dundalk Marine Terminal Northing: 573868.8 Total Depth: 24.0 Ft Driller: A. Chapel (Parratt Wolff, Inc.) Honeywell Boring No: INC-1501-K Easting: 1448175.1 Method: Hollow Stem Auger GW Depth: 0.0 Ft CPT No: Elevation: 19.8 Consultant: J. Rogers (CH2M Hill, Inc.) Date: 06/15/2007 Recov. Sample Soil DPC Soil Description uscs Comments W Gs PL LL P200 ID Code 15 High intensity WELL-GRADED SAND with GRAVEL (SW) -strong brown (7.5YR 5/8), damp, (dense), HB SW and downward pressure ICHINASO GIE ++ 19 2.86 lightly to moderately indurated, coarse sand, fine gravel, trace yellow nodules on hammer No Recovery SILTY SAND (SM) -very dark gray (7.5YR 3/1), wel. (loose), particulate, fine sand, GB SM 3.05 NC-1805-K-BOX (1016) some yellow sand FAT CLAY (CH) -red (2.5YR 5/8), reddish yellow (5YR 6/8), moist, medium stiff to

NC-HO+K-BO HO+N

soft, high plasticity

SILTY CLAY (CL) -red (2.5YR 5/8), reddish yellow (5YR 6/8), moist, soft, medium plasticity, trace sand

CH

CL

Locus

BORING LOG Page 5 of 5 Site: Dundalk Marine Terminal Northing: 573868.8 Driller: A. Chapel (Parratt Wolff, Inc.) Total Depth: 24.0 Ft Honeywell Boring No: INC-1501-K Easting: 1448175.1 Method: Hollow Stem Auger GW Depth: 0.0 Ft CPT No: Elevation: 19.8 Consultant: J. Rogers (CH2M Hill, Inc.) Date: 06/15/2007 Depth 3 Sample Soil DPC Soil Description uscs Comments W Gs PL LL P200 Code ID 20 SILTY CLAY (CL) -red (2.5YR 5/8), reddish yellow (5YR 6/8), moist, soft, medium plasticity, frace sand CL inclinometer installed at this location

)n(2)		Page 1 of 6	Site: Dundalk Marine Terminal Boring No: INC-1501-M CPT No:	Northing: 573913.5 Easting: 1448225.7 Elevation: 20.2	Metho	A. Chapel (Parratt Wo d: Hollow Stem Auger litant: J. Rogers (CH2)		.)	Total De GW Dep Date: 06	-	t
Depth Ft	Recov.	Sample ID	DPC	Soil Code		Soil Description	n	uscs	Comments	w	Gs	PL	LL	P200
					Asphalt, concrete.	gravel pack removed by Parratt Wo	itt							
		e⊂ 4001 (4 60)+100000			SANDY SILT (ML) 5/8), damp to mois	-dark grayish brown (10YF 4/2), gr. st. hard, s.d. fine sand, medium sand	ay (10YR 5/1), trace red (2.5YR I, trace gravel, trace debris (brick)	ML	perched water was encountered at 5' ogs	11				
4						*************		l				11,	oci	us

BORING H(ey	/V	Page 2 of 6	Site: Dundalk Marine Terminal Boring No: INC-1501-M CPT No:	Northing: 573913.5 Easting: 1448225.7 Elevation: 20.2	Metho	: A. Chapel (Parratt Wo d: Hollow Stem Auger ultant: J. Rogers (CH2N)	Total Dep GW Dep Date: 06	th: 6.0 Ft	
Depth 000	Sample ID	DPC	Soil Code		Soil Description	1-	uscs	Comments	w	Gs	PL	LL	P200
5 —		-		SANDY SILT (ML) 5/8), damp to mois	-dark grayish brown (10YR 4/2), gra tt, hard, set, fine sand, medium sand	ay (10YR 5/1), trace red (2.5YR , trace gravel, trace debris (brick)	ML	perched water was encountered at 6' bgs					
Ţ				SANDY SILT (ML) 5/8), wet to satura (brick)	-dark grayish brown (10YR 4/2), gra led, hard, silt, fine sand, medium sar	ay (10YR 5/1), trace red (2.5YR and, trace gravel, trace debris	ML						
-		-		WELL-GRADED S damp, (very dense	AND with GRAVEL (SW) -black (10' e), coarse sand, fine gravel, coarse g	YR 2/1), light gray (10YR 7/1), gravel, medium sand	sw	increased hammer intensity at 7.0°, sample tooks like asphalit and concrete					
8 1		⊥		l			1				11,	OCI	us

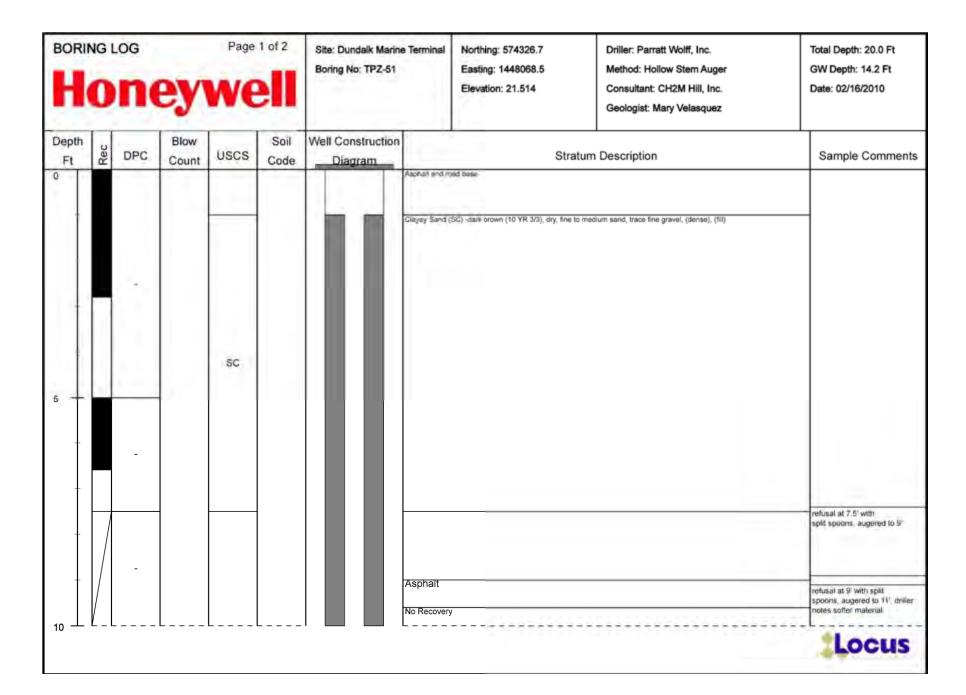
		o LOG	2)	/V	Page 3 of 6	Site: Dundalk Marine Terminal Boring No: INC-1501-M CPT No:	Northing: 573913.5 Easting: 1448225.7 Elevation: 20.2	Method	A. Chapel (Parratt Wo d: Hollow Stem Auger litant: J. Rogers (CH2N		.)	GW Dep	pth: 22.0 oth: 6.0 Fi	t
Depth Ft	Recov.	Sample ID	DPC	Soil Code		Soil Description	n	uscs	Comments	w	Gs	PL	LL	P200
8		MC1991 Mes accord			POORLY-GRADE coarse sand	D SAND (SP) -gray (10YR 5/1), dam	np, (dense), medium sand,	SP		8.5				
					POORLY-GRADE 8/3), damp, (dense	D SAND (SP) -very pale brown (10Y e), fine sand, medium sand	R 7/4), pale brown (10YR	SP						
10 -		+C 1901 ₩ 508 100100			FAT CLAY (CH) -n plasticity	eddish yellow (5YR 6局), yellowish re	ed (5YR 5/8), damp, stiff, medium	СН		26				
					No Recovery				hammer intensity near maximum pressure, yery slow downerd movement, refuse -12.0' bgs					
12				L	L.,,,,,,,			1,				:1	OCI	us

		o LOG	2)		Page 4 of 6	Site: Dundalk Marine Terminal Boring No: INC-1501-M CPT No:	Northing: 573913.5 Easting: 1448225.7 Elevation: 20.2	Metho	A. Chapel (Parratt Wol d: Hollow Stem Auger litant: J. Rogers (CH2M)	GW Dep	pth: 22.0 oth: 6.0 Ft 1/29/2007	
Depth Ft	Recov.	Sample ID	DPC	Soil Code		Soil Description	n	uscs	Comments	w	Gs	PL	LL	P200
					No Recovery				COPR in soil cuttings when auger is advanced to 14', appears to be HB					
15		NC 1884 B NO 140 NO	**	GB	[(7.5YR 5/8), wel. (-very dark gray (7.5YR 3/1), black (very dense), lightly indurated to part el, trace bright yellow nodules, mixe	ticulate, fine sand, medium sand.	SM	hammer interestly high, weight of rig depited, could only advance approximately It's marrefusal near 15.0"	26				
					No Recavery									
16 -	TI			L-6-	L	~~~~~~~		1			_	11,	OCI	us

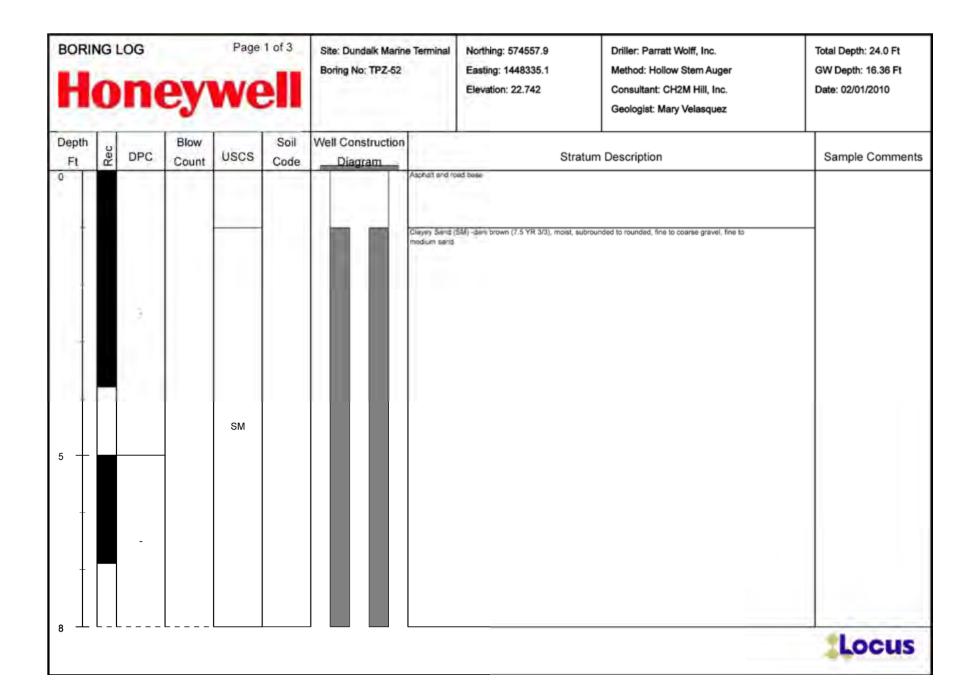
BORING LOG Page 5 of 6 Site: Dundalk Marine Terminal Northing: 573913.5 Total Depth: 22.0 Ft Driller: A. Chapel (Parratt Wolff, Inc.) Honeywell Boring No: INC-1501-M Easting: 1448225.7 Method: Hollow Stem Auger GW Depth: 6.0 Ft CPT No: Elevation: 20.2 Consultant: J. Rogers (CH2M Hill, Inc.) Date: 06/29/2007 Depth Seco Sample Soil DPC Soil Description USCS Comments W Gs PL LL P200 ID Code 16 No Recovery during retrieval of sampler threads sheared off top WELL-GRADED SAND with GRAVEL (SW) very dark gray (7 5YR 3/1), trace yellowish red (5YR 5/8), trace gray (10YR 5/1), wet, (very dense), moderately indurated to particulate (some pieces of gravel strongly indurated), coarse sand, coarse gravel, medium sand, of sampler, sampler GB SW 19 SECTION ASSESSMENT stuck in hole. fine gravel, trace bright yellow sampler retrievest. by overdrilling SILTY CLAY (CL) -reddish yellow (5YR 5/8), yellowish red (5YR 6/8), moist to damp, hard to medium stiff, clay, sill, trace sand, medium plasticity. CL Locus

Gs PL LL	\top
$\overline{}$	P2
_	



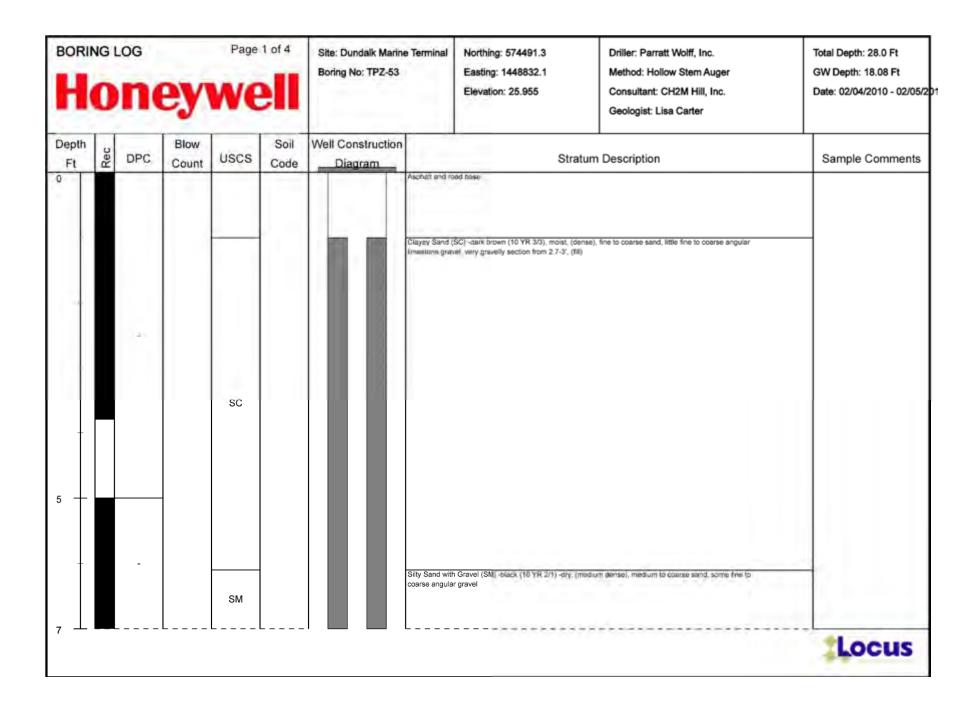


Page 2 of 2 **BORING LOG** Site: Dundalk Marine Terminal Northing: 574326.7 Driller: Parratt Wolff, Inc. Total Depth: 20.0 Ft Easting: 1448068.5 GW Depth: 14.2 Ft Boring No: TPZ-51 Method: Hollow Stem Auger Honeywell Elevation: 21.514 Consultant: CH2M Hill, Inc. Date: 02/16/2010 Geologist: Mary Velasquez Well Construction Depth Blow Soil Rec DPC USCS Stratum Description Sample Comments Ft Count Code Diagram No Recovery 10 refusal at 9' with split spoons, augered to 11', driller notes softer material Sity Sand (SM) stark brown (7.5 YR 4/4), dry, (medium dense), lightly to strongly indurated, fine to medium and, fine angular gravel SM HB Clayey Sand (SC) klark grayish brown (2.5 Y 4/2), dry, fine to medium sand, (very dense), (fill) SC DPT refusal at 14" Sity Sairid (SM) -strong trown (7.5 YR 5/6), dry, (medium dense), indurated, fine to medium sand, fine angular synoptic water levels gravel collected 3-11-10 15 SM HB Clayey Sand (SC) -dark yellowish brown (10 YR 4/3), dry, fine to medium sand, (very dense), trace fine gravel SC Sity Sand (SM) -dark yellowen brown (10 YR 6/3), dry, particulate to slightly indurated. (fill) SM HB. Sity Send (5M) -black (10 YR 2/1), green nodules, fine to medium send, lightly indurated, dry. (medium dense). SM GB Sity Sand (SM) dark yellowsh brown (10 YR 4/3), yellow nodules, slightly incurated, dry. fine to medium SM HB chiller notes softer. Siny Sand (SM) -dark gray (7.5 YR 3/1), fine to medium hand, particulate (medium dense), trace green nodules material SM GB ean Clay (CL) -red (2.5 YR 4/6), damp, hard; medium plasticity, trace fine sand CL 20.0 Boring GW Depth Locus



Page 2 of 3 **BORING LOG** Site: Dundalk Marine Terminal Northing: 574557.9 Driller: Parratt Wolff, Inc. Total Depth: 24.0 Ft Boring No: TPZ-62 Easting: 1448335.1 Method: Hollow Stem Auger GW Depth: 16.36 Ft Honeywell Elevation: 22.742 Consultant: CH2M Hill, Inc. Date: 02/01/2010 Geologist: Mary Velasquez Well Construction Depth Blow Soil Rec USCS Stratum Description Sample Comments DPC Ft Diagram Count Code Sity Sand (SM) -black (10 YR 2/1) and very dark grayish brown (10 YR 3/2), moist, coarse sand, trace mica, 8 race subangular grave SM 10 Lewi City (CL) -yeriowish red (5 YR 4/6), moist, medium to no plasticity, (very stiff) CL Sity-Sand (SM) -yellowish red (5 YR 5/8), fine to medium sand, moist, slightly indurated, trace light green nodules. (medium dense) driller notes DPT refusal at 11', auger to 15.6', attemp DPT at 15.6" HB SM 15 driller notes difficult drilling at 15' using 15 tons drawn pressure, synoptic writer,

Page 3 of 3 **BORING LOG** Site: Dundalk Marine Terminal Driller: Parratt Wolff, Inc. Northing: 574557.9 Total Depth: 24.0 Ft Boring No: TPZ-52 Easting: 1448335.1 Method: Hollow Stem Auger GW Depth: 16.36 Ft Honeywell Elevation: 22,742 Consultant: CH2M Hill, Inc. Date: 02/01/2010 Geologist: Mary Velasquez Well Construction Depth Blow Soil Rec DPC USCS Stratum Description Sample Comments Ft Count Code Diagram Sity Sand (SM) -yellowish red (5 YR 5/8), fine to medium sand, moist, slightly indurated, trace light green driller notes difficult 16 nodules. (medium dense) drilling at 15' using 15 tons down pressure, synoptic water levels collected 3-11-10 SM HB driller notes out of HB at approximately 17.3', refusa No Recovery at 17.0' with DPT, augered to 20.0', drill to 18' and DPT to 20" Skity Sand (SM) -yellowith brown (10 YR 5/6), moist, (medium dense), trace fine subangular gravel, fine to medum sand (M) SM Clayey-Sand (SC) -black (10 YR 2/1), moist, fine to medium sand, trace fine rounded gravel, trace shell fragmeints. 20 SC encountered lightly indurated gray black nodule approximately 3 cm at 21.6 Lean Clay (CL) -yellowish red (5 VR R/S), most, measure to no plasticity, very stiff CL Glayery 511 (ML) -dark grayian brown (2.5 Y 472), damp, fine sand, ach ML Wei Grades Sans with Gravet (SW) love brown (2.5 Y MA), damp, (bose), measure to course sand SW 24.0 Boring GW Depth Locus



BORING LOG Page 2 of 4

Honeywell

Site: Dundalk Marine Terminal

Boring No: TPZ-53

Northing: 574491.3

Easting: 1448832.1

Elevation: 25.955

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Lisa Carter

Total Depth: 28.0 Ft

GW Depth: 18.08 Ft

Date: 02/04/2010 - 02/05/201

Depth Ft	Rec	DPC	Blow	uscs	Soil Code	Well Construction Diagram	Stratum Description	Sample Comment
				SM			Sity Sand with Gravel (SM) -black (10 YR 2/1) -dry, (medium dense), medium to coarse sand, some fine to coarse angular gravel	offset ~10' from original location (east) due to concret pad encountered while drillin to 7.3', auger to 8' and collect DPT sample
		Ε,						
10 +		***		SM			Sirty-Saird (SM)-pale trown (10 YR 6/3) and dark gray (2.5 Y 4/1), slightly moist, (dense), fine to coarse angular grayet, coarse sand Lean City (CL)-yellowser red (5 YR 5/6), most, medium plasticity, (stiff to very stiff), fine sand below	
		-					11.3	
		++		CL				
14		***		SM	GB	-	Silty Sand (SM) -black (10 YH 2/1), well (medium dense), particulate, little green nocules, medium sand	-



Page 3 of 4 **BORING LOG** Site: Dundalk Marine Terminal Northing: 574491.3 Driller: Parratt Wolff, Inc. Total Depth: 28.0 Ft Boring No: TPZ-53 Easting: 1448832.1 Method: Hollow Stem Auger GW Depth: 18.08 Ft Honeywell Elevation: 25.955 Consultant: CH2M Hill, Inc. Date: 02/04/2010 - 02/05/201 Geologist: Lisa Carter Depth Well Construction Soil Blow Rec DPC USCS Stratum Description Sample Comments Ft Code Diagram Count Sity Sand (SM) -black (10 YR 2/1), wet, (medium dense), particulate, little green nodules, medium sand 14. SM GB Sity Sand (SM) -brownish yellow (10 YR 6/1), very moist, lighly indurated, (medium dense), pockets of clay 15 SM HB Sity Sand (SM)-black (10 YR 2/1), intermittant layers of concentrated green nodules at 15.6', 17.1-17.2', 20 5-20 9, thin layer of HB at 17.9-18', wet, (medium dense), particulate, medium sand synoptic water levels SM GB collected 3-11-10

Locus

20

21

Boring GW Depth

Site: Dundalk Marine Terminal

Boring No: TPZ-53

Northing: 574491.3

Easting: 1448832.1

Elevation: 25.955

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Lisa Carter

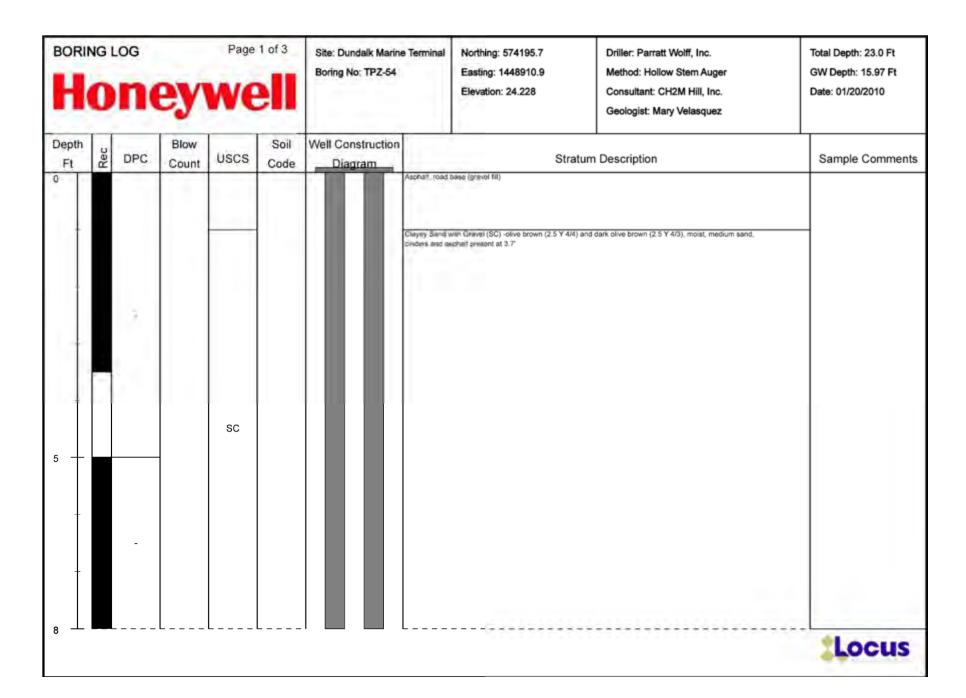
Total Depth: 28.0 Ft

GW Depth: 18.08 Ft

Date: 02/04/2010 - 02/05/201

Depth Ft	Rec	DPC	Blow	uscs	Soil Code	Well Construction Diagram	Stratum Description	Sample Comment
21		**		SM	GB		Sety Seind (SM)-black (16 YR 2/1), intermittant layers of concentrated green nodules at 15.6°, 17.1-17.2°, 20 s-20 9°, thun layer of HB at 17.9-18°, wet, (medium dense), particulate, medium sand	
-				SP			Poorly Graded Sand (SP)-dark olive brown (2.5 Y 3/3) and olive brown, medium to coarse sand, wet, (loose) Poorly Graded Sand (SP)-light olive brown (2.5 Y 5/5), wet, (loose), coarse sand, fine to coarse angular gravel.	
.5 —				SP				
		++						





Page 2 of 3 **BORING LOG** Site: Dundalk Marine Terminal Northing: 574195.7 Driller: Parratt Wolff, Inc. Total Depth: 23.0 Ft Boring No: TPZ-54 Easting: 1448910.9 Method: Hollow Stem Auger GW Depth: 15.97 Ft Honeywell Elevation: 24,228 Consultant: CH2M Hill, Inc. Date: 01/20/2010 Geologist: Mary Velasquez Well Construction Depth Blow Soil Rec USCS Stratum Description DPC Sample Comments Diagram Ft Count Code Dayley Sand with Gravel (SC) -olive brown (2.5 Y 4/4) and dark olive brown (2.5 Y 4/3), moist, medium sand, 8 SC driller notes DPT refusal Clayey Gravel with Sand (SC) -dark olive brown (2.5 Y 4/3), sand/clay/silt mixture, well graded angular at 8.6' gravel (2cm and 3-5cm gravel) 10 GC Lean Clay (CL)-yellowish red (5 YR 5/6), slightly moist, medium to no plasticity, trace angular gravel (1-2cm); CL

15

**

Boring GW Depth

HB

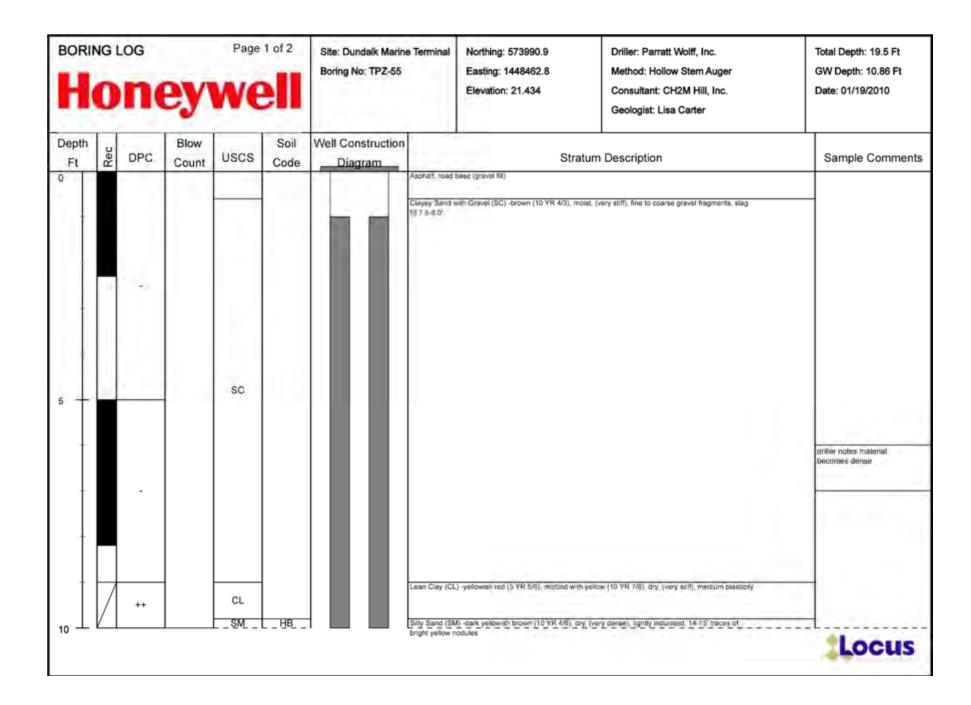
Sity-Send (SM)-dark yellowish brown (10 YR 4/6), bright yellow modules; dry, lightly inclurated, (very dense).

driller notes DPT refusal

synoptic water levels collected 3-11-10

Locus

Page 3 of 3 **BORING LOG** Site: Dundalk Marine Terminal Northing: 574195.7 Driller: Parratt Wolff, Inc. Total Depth: 23.0 Ft Honeywell Boring No: TPZ-54 Easting: 1448910.9 Method: Hollow Stem Auger GW Depth: 15.97 Ft Elevation: 24,228 Consultant: CH2M Hill, Inc. Date: 01/20/2010 Geologist: Mary Velasquez Well Construction Depth Soil Blow Rec DPC USCS Stratum Description Sample Comments Ft Diagram Count Code Sity Sand (SM) -dark yellowish brown (10 YR 4/6), bright yellow nodules, dry, lightly indurated, (very dense) 16 driller notes DPT refusal HB Slify Sand (SM) -gray black/greenish black (GLEY 1.2.5/1) very moist, fine sand, bright yellow nodules, particulate medium dense) attempted sample at 19.5' to 21' but hit DPT refusal at 20.5° 20 SM GB Lean Clay (CL) -yellowish red (5 YR 5f6), most, medium to low pleshory, trace very fine angular gravel CL (1-2cm) Locus



BORING LOG Page 2 of 2

Honeywell

Site: Dundalk Marine Terminal

Boring No: TPZ-55

Northing: 573990.9

Easting: 1448462.8

Elevation: 21.434

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Lisa Carter

Total Depth: 19.5 Ft

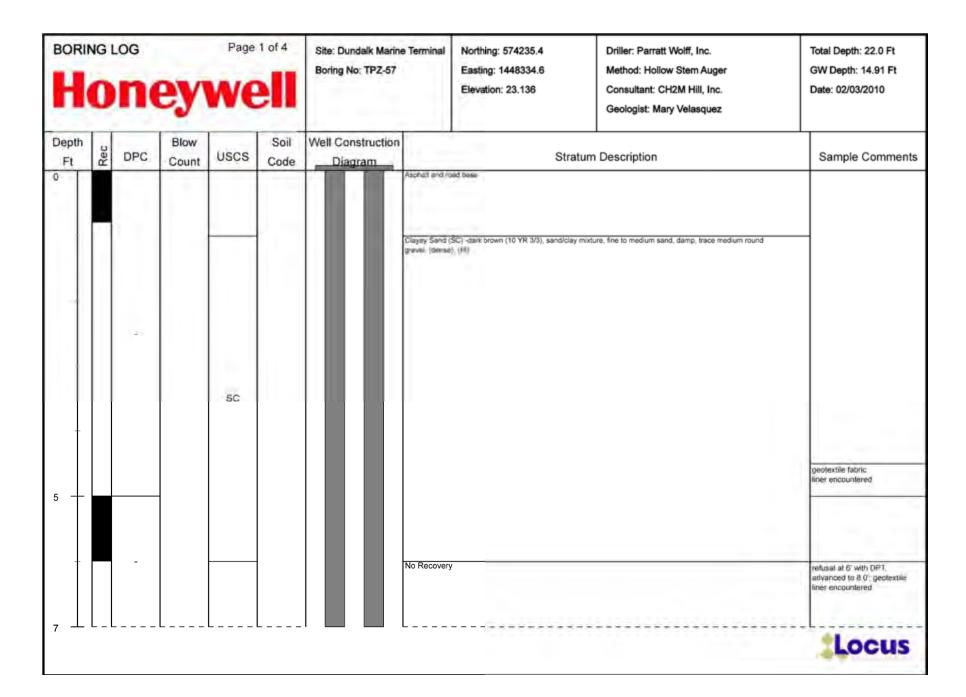
GW Depth: 10.86 Ft

Date: 01/19/2010

Depth Ft	Rec	DPC	Blow	uscs	Soil Code	Well Construction Diagram	Stratum Description	Sample Commen				
10				Count	Count	Count	Oddin	SM	НВ		Siity Siind (5M) -dark yellowish brown (10 YR 4/6), dry, (very dense), lightly indurated, 14-15' traces of bright yellow ncclules	driller notes DPT refusal at 10" synoptic water levels collected 3-11-10
5 —		SM	om	ria .	400		attempted sample at 14' but hit DPT refusal at 1					
		**										
		++ SM GB	GB		Skity Sand (SM) -very dark-gray (GLEY 1 3/1), moint, moderately particulate to indurated, some bright yellow nodules. Nerd brown fragments	water observed above clar layer						
	1	_		CL			Lean Clay (CL) (yellowish red (5 YR 5/6), most, medium plasboty, (stiff), trace the sand	-				







Page 2 of 4 **BORING LOG** Site: Dundalk Marine Terminal Northing: 574235.4 Driller: Parratt Wolff, Inc. Total Depth: 22.0 Ft Boring No: TPZ-57 Easting: 1448334.6 Method: Hollow Stem Auger GW Depth: 14.91 Ft Honeywell Elevation: 23.136 Consultant: CH2M Hill, Inc. Date: 02/03/2010 Geologist: Mary Velasquez Well Construction Depth Blow Soil Rec DPC USCS Stratum Description Sample Comments Ft Diagram Count Code No Recovery refusal at 6' with DPT, advanced to 8.0°; geotextile liner encountered Asphalt Lean Clay (CL) -yulkiwish red (5 YR 5/6), medium to low plasticity, dry. (very stiff) driller notes soft material CL Sity Sand (SM) dark yellowish brown (10 YR 4/6), bright yellow nodules, dry, indurated, (medium dense), ine to medium sand 10 ++ SM HB refusal at 13' with DPT advanced to 15.0, synoptic water levels collected 3-11-10 Locus

BORING LOG Page 3 of 4 Site: Dundalk Marine Terminal Driller: Parratt Wolff, Inc. Northing: 574235.4 Total Depth: 22.0 Ft Boring No: TPZ-57 Easting: 1448334.6 Method: Hollow Stem Auger GW Depth: 14.91 Ft Honeywell Elevation: 23,136 Consultant: CH2M Hill, Inc. Date: 02/03/2010 Geologist: Mary Velasquez Well Construction Depth Blow Soil Rec DPC USCS Stratum Description Sample Comments Ft Count Code Diagram Sity Sand (SM) dark yellowish brown (10 YR 4/6), bright yellow nodules, dry, indurated, (medium dense), refusal at 13' with 14. DPT, advanced to 15.0', synoptic water levels collected 3-11-10 15 HB SM Sity Sand (SM)-yellowish brown (5 YR 5/8), bright yellow nodules, dry, moderately indurated, fine to medium HB and GB alternate at 3-6" wand, (medium dense) intervals SM HB Sirty Sand (SM) -black (10 YR 2/1), light green and yellow nodules, dry to damp, particulate, (loose), fine SM HB/GB Sity Sand (SM) -black (10 YR 2/1), light green rodules, damp, particulars, (loose), line to medium sand DPT refusal at 18.5' SM GB 20 Lean Clay (CL) -yellowish red (5 YR 5/6), medium to low plasticity, hard, dry CL Sity Sand (SM) -black (10 YR 2/1), slightly morel (dense), the to reaction sand, little angular line gravel-SM 21 Boring GW Depth Locus

BORING LOG Page 4 of 4 Site: Dundalk Marine Terminal Northing: 574235.4 Driller: Parratt Wolff, Inc. Total Depth: 22.0 Ft Boring No: TPZ-57 Easting: 1448334.6 Method: Hollow Stem Auger GW Depth: 14.91 Ft Honeywell Elevation: 23.136 Consultant: CH2M Hill, Inc. Date: 02/03/2010 Geologist: Mary Velasquez Well Construction Depth Soil Blow Rec DPC USCS Stratum Description Sample Comments Diagram Ft Count Code Sity Send (SM) -black (10 YR 2/1), slightly moist, (dense), fine to medium sand, little angular fine gravel 21 SM Clayey 5it (ML) dark gray (2.5 Y 4/2), (stiff), some angular fine gravel, damp, trace fine sand ML



Page 1 of 1

Site: Dundalk Marine Terminal

Boring No: TPZ-64



Northing: 574028.744 Easting: 1448100.31

Driller: Parratt Wolff, Inc. Method: Direct Push Total Depth: 8.0 Ft

Elevation: 18.68

Consultant: CH2M Hill Geologist: J. Myers Start Date: 07/13/2011

						· · · · · · · · · · · · · · · · · · ·		
Depth Ft	Rec	DPC	PID	USCS	Soil Code	Stratum Description		
0			0			Asphelt and road base (sand and gravel).		
-				sc		CLAYEY SAND with GRAVEL (SC), very dark grayish brown (10YR 3/2), most, with coers	e gravel, (very dense).	
5 —					sc		CLAYEY SAND with GRAVEL (SC), very dark grayleh brown (10YR 3/2), wat, with coarse	
		-	o	SC		CLAYEY SAND with GRAVEL (SC), very dark grayish brown (10YR 3/2), most, with coers	e graver, (very dense).	
8.0						Asphalt.		

Page 1 of 2

Site: Dundalk Marine Terminal

Total Depth: 12.0 Ft

Start Date: 07/14/2011

Boring No: TPZ-67



Northing: 573889.398 Driller: Parratt Wolff, Inc.
Easting: 1447945.441 Method: Direct Push

Elevation: 12 Consultant: CH2M Hill Geologist: J. Myers Depth Soil DPC PID USCS Stratum Description Code Ft Asphalt and road base. CLAYEY SAND with GRAVEL (SC), brown (10YR 4/3), moist, with fine to medium sand and some gravel, (dense), fill. 0 SC CLAY (CL), reddish brown (2.5YR), molet, (stiff), medium plasticity. CL 0 SILTY SAND (SM), yelowish red (5YR 4H), dry, friable, nonplastic. SM HB 0 CLAY (CL), reddish brown (2.5YR), molet, (stiff), medium plasticity. CL 0 0 WELL GRADED GRAVEL with SAND and SILT (GW-SM), dark yellowish brown (10YR 3/4), well, angular granitic gravel, GW-SM 0 WELL GRADED GRAVEL with SAND and SILT (GW-SM), dark yellowish brown (10YR 3/4), wet, with angular granibe GW-SM

Page 2 of 2

Site: Dundalk Marine Terminal

Boring No: TPZ-67



CH2MHILL

Northing: 573889.398 Driller: Parratt Wolff, Inc.
Easting: 1447945.441 Method: Direct Push

Elevation: 12 Consultant: CH2M Hill

Geologist: J. Myers

Total Depth: 12.0 Ft

Start Date: 07/14/2011

Depth Ft	י	Rec	DPC	PID	USCS	Soil Code	Stratum Description
10			+		GW-SM		WELL GRADED GRAVEL with SAND and SILT (GW-SM), dark yellowish brown (10YR 3H), wet, with angular granitic gravel, (madium dense).
12.0			++	0	sw	GB	WELL GRADED SAND (SW), clark yellowish brown (10YR 34), wet, fine to coarse sand with little fine gravel, (very dense).

Appendix B Soil Boring Logs for Wells To Be Retained



BORING NUMBER: BRP-1

Sheet: |

SOIL BORING LOG

PROJECT: DUNDALK MARING TERMINAL

LOCATION: SEE PLAN

ELEVATION: ~ +14

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTITORILL XL MAX 170 WATER LEVELS:

START:

FINISH:

LOGGER: L. LINCOLN

	Т.	SAMP		STANDADD		LUGGER: L. LINICOLN
W (#		AIVIP	Т	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 -				-	0-0.4 ASPHALT 0.4-1.5 GRY, BLK SILTY F.C SAND, SM GVL (FILL)	NO RXN
2 - 3	5-1	SONIC CORE	3'	- - - -	1.5-2.0 GRY C.M SAND, SM GVL (ROAD BASE AGGREGATE) 2.0-2.3 GRY-GREEN C.F SAND, SUN GIVL (ROAD BASE) 2.3.3.0 RED BRN, BRN SILTY F.C SAND, SM GVL (FILL) 3-4.3 SOFT RED COMY, TR GVL	NO RXW
4 5 V	\$\mathcal{L}		3'	1 A	1.3 - U. O BEN, RED BRN C-F SAND, SM SILT, YELLOW FLAKES/PARTICLES, TR. GNL (COPP)	DPC+



BORING NUMBER: BRP-1

Sheet: Z

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ ㅋ(니

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAY 170

WATER LEVELS:

START: 1450 FINISH: LÓGGER: L.LINCOLN

(£)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
\ - -				- - -	6-7.3 RED CLAY, SM DK GRY F. M SAND, TR GIVL (WET)	DPC+
7 8	5.3	SOMIC CORE	3'	N/A -	7.3-9.0 RED BRN, DK GRY, BRN SILTY F.C. SAND, SM GVL, MOD LITHIFIED	DPCT
- 9 - - -		2" 0.0		- او -	9-9.0 DK GRY SILTY F.M SAND, WET	DPC+
- 10 –	55.4	6.6	12"	7 -	9.6.10 RED BRN, BRN CLANEY F. M. SAND, TR. GVL	DPC1
	55 5			ام - - ار	10-11 DK BRN MOD-HIGHLY LITHIFIED SILTY F.M SAND, SM GVL (NON-COPR FILL)	HO DPC RXN
- 11				-		EDB @ 11.0'
				_		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122

T: 91	7 10fK, N1 17 339-930 w.mrce.cor	00 F: 917 339	9-9400			BORING NO.	BRP-1	
PROJECT	7	NTALK	MARINE	E TERMI	NAL	FILE NO.	587	
LOCATION			LE MARY			SURFACE ELEV.	~ +1	4
BORING LOC		SEE	PLAN			DATUM		
		9//						
TEST/INSPEC REFERENCE							ng gan sa kang digina an ang kang digina di kang digina di kang digina di kang di kang digina di kang digina d Mang digina di digina di kang digina di kang digina di kang digina di kang di kang di kang di kang di kang dig	
BORING EQL	UIPMENT A		OF STABILIZING BOR	EHOLE				
TYPE OF BORI	NC DIG	TYPE OF FE		CASING USED)	YES	✓ NO	
TRUCK	110 1110	MECHANIC		DIA., IN.		DEPTH, FT. FROM	<u> </u>	то
SKID		HYDRAULI		DIA., IN.		DEPTH, FT. FROM		то
BARGE		OTHER		DIA., IN.		DEPTH, FT. FROM	20010000	то
OTHER								
TYPE AND SI D-SAMPLER U-SAMPLER		0.D. SPL17	SPOON	DRILLING MU DIAMETER OF TYPE OF DRIL	F ROTARY BIT, IN.	YES	✓ NO	
S-SAMPLER						YES	√ NO	
CORE BARREL	3"	SONIC		AUGER USED				
CORE BIT				TYPE AND DI	AMELER, IN.			
DRILL RODS			ggggganii (18 Gillian)			AVEE	ACE CALL IN	
				CASING HAM		destructes described a mixinal confess.	IAGE FALL, IN.	30
				SAMPLER HA		140 AVER	RAGE FALL, IN.	
				TYPE OF HAN	AIMIEK			
			51101 F					
WATER LEV	EL OBSERV	<u>ATIONS IN BOR</u> I	EHULE		<u> </u>			
DATE	TIME	DEPTH OF HOL	E DEPTH OF CASING	DEPTH TO WATER	quantitate A	CONDITIONS	OF OBSERVATION	
	<u> </u>							
PIEZOMETE	ER INSTALLE	<u>:D</u>	VES	NO SKET	CH SHOWN ON	P. L	1	erstegde in veger planning von som en vertreit de de vertilet de
STANDPIPE:		TYPE	2	ID, IN.	LEN	NGTH, FT.	TOP ELEV.	
INTAKE ELEM	AFNT.	TYPE	<u> </u>	OD, IN.		NGTH, FT.	TIP ELEV.	
FILTER:	ri L14 1.	MATERIAL		OD, IN.		NGTH, FT.	BOT, ELEV	*
FILTEN.		(VI) CITIFE	turing a specify polytic program of the control of	and the second second	WASSER OF CHARGE PARKETS AND AND AND AND AND AND AND AND AND AND	. A Secretarial	man property of the materials for the	apara ana, ny apara-basa 3 ao a Garana ao amin'ny fa
PAY QUAN	TITIES							
	Y SAMPLE BO	RING	UN. FT.		NO. OF 3" SHELE	BY TUBE SAMPLES		
	Y SAMPLE BU SAMPLE BORI		LIN, FT.			STURBED SAMPLES		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
		and .	UN, FT.		OTHER:		***************************************	
CORE DRILLI	NG IN KUCK		MIT. I I .		J.,		· · · · · · · · · · · · · · · · · · ·	
BOBING CO	OMTP ACTO)	AQUIFER :	TPU IN	win .	TESTING		
	ONTRACTOR				HELPERS		en en en en en en en en en en en en en e	ar ayyar 1994 (1997 - S. Perlandonia, Prinsiperla de
DRILLER		PHY 1	PALOMEQUE	S			THE RESERVE AND ADDRESS OF THE PARTY OF THE PARTY.	خانا دخه ببين و بده ساعنا ساعاه بين بالويا الذوق ويزان به على ويند
REMARKS RESIDENT	ENGINEED		ANDRA LIN	ICOLN	anang salah milah ini kalabahan menangga me r	DA'	TE W/9	1/15
NESIDEITI (FIAGNACEU	_120	-11-11-11-11-11-11-11-11-11-11-11-11-11	100K13				1
							BORING NO.	BRP-1



BORING NUMBER: BRP-2

Sheet: |

SOIL BORING LOG

PROJECT: Honeywell DMT SSZ

LOCATION:

DRILLING CONTRACTOR: Aquifer Drilling and Testing , The

T	SA	MPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
			2Y (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
DEL III CETTO	NUMBER	TYPE	RECOVERY	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
	-			-	3' Asphalt	Novanced the 3" ID SON'C Macro core from o'-10 bgs
					0.25-1.0'- Poorly graded GENTEL with Sand (GP), gray brown (2.54R s/2), dry mixed with cousted Arphalt	DP:-
-					in a gooded soul with gravel	(SP),
-	1				Ped. Brown 17.54R 4/4), dry, medium	DPC-
	5-1	CORE	W84		to coarse grouned sand,	
	1	PICEO C	100	2		
1-	-	F			-	
	1	Sovic				
0	1				-3.01. woven geotextile	
3					- Silty SANTO with graves (SM), Brown- Red gray (1:5-1R 4/2), moist, fine to medium grained sand	bpc -
y	1		4		hid-same as above, wet	
	-				1	Drc -
	1	CORE		/	1	
	+	MACRO	3	भ	- 1 P. Call de la land	
5	-	2			- sol- Polyethylene liner - sol- same as above impost	PPC-



BORING NUMBER: BRP-2

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	rype	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
_	Z	1	22			
6	52	SONIC HACRO	-	-	6.81- Pieces of Crushed asphalt	DPC-
-					70'- same as above, higher fines content, wet	DPC-
-						
2 -	53		36"	-	8.0'-8.5'- Poorly graded sand with Silt and gravel ISP-SHO, Red-Brown (7.54R 4/4) moist, medium to care grainds and.	DPC-
3 - 1 - 1		SOMIC MACRO			8.5' - Crushed consult	DPL -
10-	м	SS		8	10 0 - Poorly graded sand with graved (SP), gray (7-54R S/,), wet, coanse grained sand	installed to a depth of 100's 100's switched to ss sampling 140-16 hammer: 30-in. drop 3"dia. Sampler.
11-	55.4	5	24"	13	dry, Low plasticity many shof	p12c-
-	. 1			10.	×	



BORING NUMBER: BR-P-2

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(tt)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
ELOW (RY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
12-				6 98 -	120'-125- Fat CLAY (CH), Red (1044/4), wet, highly plastic, very shiff.	120'-groundwater enlainter during drilling	
-	55.75	35	13"	50/10	12-5'- silty servo (SM). Red-Brown (7-5 yr ylu). moist, the to medium grained sand, particulate (AB COPR)	DPC A	
3 -		330		-	same as above bright green Partiales, slightly indusated,	DPC-1	
14-	5-6	最	23				
		SONNE A					
15-				8	Same as above	bpc+	
	22.3	,		25			
16-		55	20"	19 -			
				14			
13-	55.8	ss	12"	10	Same as above, wet	DPC →	
10	22	2-		16	- 17.7' same as above growith brown (2-5-18.413), trace grave)	DPC+	



BORING NUMBER: BRP-Z

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH:

(#)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
ELOW			RY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
18 -	55-9	22	6"	5	18.0' - Fal CLAY (COH), Red (1074/4), wet, very shift, highly plashe	DPC- 18.0-6" DO OUTER CONT O
19					Bottom of boring @ 18.5 logs	installed to 18-0' by

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET DUNDALK MARINE TERMINAL FILE NO. 1058 PROJECT SURFACE ELEV. BALTIMORE, MARYLAND LOCATION DATUM BORING LOCATION TEST/INSPECTION EQUIPMENT REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED CASING USED DURING CORING TYPE OF BORING RIG 64 TO DEPTH, FT. FROM DIA., IN. TRUCK MECHANICAL TO DEPTH, FT. FROM DIA, IN. HYDRAULIC SKID DEPTH, FT. FROM DIA., IN. OTHER BARGE OTHER YES DRILLING MUD USED TYPE AND SIZE OF: DIAMETER OF ROTARY BIT, IN. D-SAMPLER TYPE OF DRILLING MUD U-SAMPLER S-SAMPLER NO YES AUGER USED CORE BARREL TYPE AND DIAMETER, IN. CORE BIT DRILL RODS AVERAGE FALL, IN. CASING HAMMER, LBS. Sonie Macro Care -30 AVERAGE FALL, IN. SAMPLER HAMMER, LBS. 14-0 utowakt TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE CONDITIONS OF OBSERVATION DEPTH OF CASING DEPTH TO WATER DEPTH OF HOLE DATE TIME 14-0 well that all a High SKETCH SHOWN ON NO PIEZOMETER INSTALLED TOP ELEV. LENGTH, FT. ID, IN. PVC TYPE STANDPIPE: LENGTH, FT. TIP ELEV. OD, IN. TYPE INTAKE ELEMENT: BOT. ELEV. LENGTH, FT. 00, IN. MATERIAL FILTER: PAY QUANTITIES NO. OF 3" SHELBY TUBE SAMPLES UN. FT. 3.5" DIA. DRY SAMPLE BORING NO. OF 3" UNDISTURBED SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. OTHER: UN. FT. CORE DRILLING IN ROCK and tegting lave . BORING CONTRACTOR Philten HELPERS DRILLER

sippometer installed

atury soluta

1217115

DATE

REMARKS

RESIDENT ENGINEER



BORING NUMBER: BRF6

Sheet:

SOIL BORING LOG

PROJECT: Dundalk Marine Terminal

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Sonic Samp Drill May 170; SPT Samping LOGGER: Moll Charry

START: Nov. - FINISH: LOGGER: Moll Charry

START: 08:45

€	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 	51	3:1 Sant C	31	NA	Roudbase / Asphalt Brown to bleck	DPC-none
3 -	5.2	3:1 Son'l	3	NA -	3.0-4.0 Ory red brayon f-C Sitty send, trace grave! (SM) 4.0-5.5 Damp red brown f-m sitty sand, trace grave!, trace clay (SM)	geomembrane \$ 3.0 DPC - hone DPC-
6-				-	5.5-6.0 Deg sed brown with white and green partities for in silty sand, trace gravel (6M)	geomembrane \$25' DFC-



BORING NUMBER: BEP-6

Sheet: 2/6

SOIL BORING LOG

PROJECT: Dundalk Marine Terminal ELEVATION: ~+70

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Frask Mali Drill Max 170; Sonit Sampling LOGGER: Mark Change

	£	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
	DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
,	b —	5-2	311 Sonil	3'	NA -	6.0-7.0 Dry red brown with white and green particles f-M sixty sand, trace graved (SM)	DPC- VORC
	- - 7 —				-		
	, -	5-3				7.0-8'9" Damp Red brown with white and prange particles f-M silty sand, trace gravel, trace clay (SM-CL)	PPC-none
	{ -			٠.	<u>-</u>		·
	j				-	8'9"-9.0' Road base Jasphalt	
	9 -				•	9.0-10.0 Dry white f-Mgand, concrete	ppc-nonc
	- - - 10		5 H			16-11'2" D - 11	
	-	55-l	2" 60 51mphr		19-15 . 22-26 .	16-11'3" Dry red brown with white particles f-Msiltysand trangeneral (GM) Medium danse	DPC - hone
	- - -				-		
	_					11'3"-11'6" Camp red brown f- m silty sand (SM) 11'6"-12' Damp and brown f-H silty sand (SM)	OPC-hone
	12-	1	Į .		-	1 .	



BORING NUMBER: BRP. L Sheet: 3/6

SOIL BORING LOG

PROJECT: Dundally Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: ~+20

DRILLING METHOD AND EQUIPMENT: Frash Multi Drill Max 170; Sonic Sampling START: FINISH:

LOGGER: Mark Chancy

€	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТУРЕ	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12 -	55-Z - -	211 OD sampler	1	36-41 <u> </u>	12-13'8" Brown moist f-m Silty Sand with green particles (SM) medilunts medilum dense	DPC-none
13 -				- - -	13'8"-14 Moist and brown film silty sand (GM)	DPC- none
14 -	_ 55 ⁻³	2" OB SOMPles		34-32 32-34	14-15 Moist clease (edbrown to black f-m sitysand; trak gravel, trak quartz, trakellay (SM-CL)	DPC—none
15.			1.5	- - -	15-16' Dry Dense sed f-Hsilty sund, trace Clay (SM-CL)	DPL-none
16.	-554 -	260 Surthy		48-39	16.0-17.0 Dry Dense Red brown f-M silty sand transgravel, quartz (SM)	DPC - none
17 .	- 55-5 55-5	sembr.	1.5	49-52	17.0-18.0 Dry dense sed brown f-M sitty sand trace gravel trace clay (SM-CL)	OPC-
18	1					



PROJECT NUMBER: 10587

BORING NUMBER: BRP-6
Sheet: 411

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH: 12:35 LOGGER:

€	s	AMPL	Ε	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18-	55-b	200 Sampler	6"	64 _	18.0-18'6" Moist Dense redbrown f.m sandy day travegravel, silt (CL-SM)	DPC + faint
- - !9 -	55-7	ľ,	3'	61	18'6"-19 moist dinse red clay liner Boring terminated a) 19' per K. Chatervedula	PPC-none 6" OD C45/hz tol9
20 -			5-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	- - - -	Vwp installed @ 181 1518415	Thermometer Air temp 9.0 thermistor Air temp 7.0 Vup Air temp 14.3 Po 9629
- 21 - -				- - -	1 thermistor installed a 9,51	Vmp depth T. 144 P. 9632
22-				-		Vmp Slumy Tz PZ
23-			,	- - -	* · · ·	Thermister slurry temp
- 24 -				<u>.</u>		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400

Www.	inrce.cc	m r:91/	339-9400				BORING NO.	BRP-6	
PROJECT		D /.1	V M also T	ex walnut			SHEET	OF	
LOCATION		Baltimar	L Marine To Maryland	Transq;	,		FILE NO. SURFACE ELEV.	10569 ~ +20	
BORING LOCA	ATION	1241 1 1101	See local	ion plan			DATUM		
TEST/INSPECT REFERENCE C	TION EQU	IPMENT			amp Orill Max	170; Sunic Sau			
BORING EQUI	PMENT A	ND METHO	S OF STABI	LIZING BO	REHOLE				
		TÝPE Ó							
TYPE OF BORING	G RIG	DURING	CORING		CASING USE	D	YES	NO	1.01
TRUCK		MECHA	NICAL _		DIA., IN.	6"	DEPTH, FT. FROM	O TO	0 19
SKID		HYDRA	JLIC	•	DIA., IN.		DEPTH, FT. FROM	T(o
BARGE		OTHER	-		DIA., IN.		DEPTH, FT. FROM	т	0
OTHER	Track							_	
TYPE AND SIZ D-SAMPLER U-SAMPLER	. 					IUD USED OF ROTARY BIT, IN. ILLING MUD	YES	NO	
S-SAMPLER CORE BARREL CORE BIT DRILL RODS		Sampler)	2" Diam	Sampler	AUGER USE TYPE AND D	D NAMETER, IN.	YES	NO	
·	3"]	D Soult	Marstore		CASING HAN SAMPLER HA TYPE OF HA	AMMER, LBS.	A1. 0	GE FALL, IN.	30
WATER LEVEL	OBSERV/	ATIONS IN BO	DREHOLE			T			
DATE	TIME	DEPTH OF H	DEE DEPTH	OF CASING	DEPTH TO WATER		CONDITIONS OF	OBSERVATION	
					·				
						-		· · · · · · · · · · · · · · · · · · ·	
					· · · · · · · · · · · · · · · · · · ·				
								· · · · · · · · · · · · · · · · · · ·	
							····		u: :
PIEZOMETER I	INSTALLE	<u>D</u>	YES		NO SKET	CH SHOWN ON	See V	mp installation lea	rord
STANDPIPE:		TYPE			ID, IN.	1 ENZ	ЭТН, FT.	TOREITY	
NTAKE ELEMEN		TYPE			OD, IN.		STH, FT.	TOP ELEV. TIP ELEV.	
ILTER:		MATERIAL			OD, IN.		STH, FT.	BOT. ELEV.	*****
NAV OLIANITIT	IEC	***	*****						
PAY QUANTIT 3.5" DIA. DRY SA		ung '	LIN. FT.			NO OF SHELLING	TUBECASABLES		
3.5" DIA. U-SAM			LIN. FT.	***************************************		NO. OF 3" SHELBY			
ORE DRILLING			LIN. FT.			NO. OF 3" UNDIST OTHER:	ORBED SAIVIPLES		
BORING CONT	RACTOR		1 £	Dellu	c. 1 Techton				
ORILLER	Tuni	, _	nguller	צמינוזינו	and Testing	HELPERS			
REMARKS			er mister th			_HELFERS			~
RESIDENT ENG			ark Chang				DATE	12-8-	- 15
			7		· · · · · · · · · · · · · · · · · · ·				

BORING NO.

BR P-6

Page 1 of 2

Site: Dundalk Marine Terminal

Boring No: BRP-6A

Northing: 574484.781

Easting: 1448090.619

Driller: Parratt Wolff, Inc.

Method:

Total Depth: 18.5 feet

GW Depth:

	L	U	1e	y • •		•		Geologist: Jacobs		
pth et	Rec	DPC	Blow Count	USCS	Soil Code		Stratum Description		Sample	e Comments
				Asphalt		Asphalt				
		+					very coarse sand, some gravel, trace silt, 10 YR	4/3 (brown),		
		+	27-34-14	SP-SM		dry, well-graded, dense, possible CC	OPR impacted fill			
		+								
		+]							
↓		+	11-12-14-15							
		-		SP-SM		Coarse SAND (SP-SM); medium to	very coarse sand, some gravel, trace silt, 10 YR	4/3 (brown),		
↓		-		CL		Sity CLAY (CE); 10 PR 4/2 (Sale gr	OPER impacted fill, contains 3 of red brick fill			
	_	-								
\downarrow		-	2-3-4-6			Clayey SILT (ML); trace f-m gravel,	10 YR 4/2 (dark grayish brown), gravel subangu	lar, dry,		
	-	-	-	ML		loose				
$\downarrow \mid$		-				Sandy SILT (ML): trace fine to media	um gravel, trace clay, 10 YR 4/2 (dark grayish bi	rown), drv.		
		-				med dense				
+		-	8-10-11-11	ML						
	-	-	-							
+		-		ML			um gravel, trace clay, 10 YR 4/2 (dark grayish bi	rown), dry,		
			-	Asphalt		Weathered Asphalt; GLEY 1 2.5/N (I	black), f-m sand, trace fine gravel, dry, dense			
+			13-100/4							
		-								

Page 2 of 2

Site: Dundalk Marine Terminal

Honeywell

Boring No: BRP-6A

Northing: 574484.781

Easting: 1448090.619

Elevation: NA

Driller: Parratt Wolff, Inc.

Method:

Consultant: JACOBS

Geologist: Jacobs

Total Depth: 18.5 feet

GW Depth:

Date: 07/18/2018 - 07/19/201

Depth feet	Rec	DPC	Blow Count	USCS	Soil Code	;	Stratum Description		Sample	Comments
0		-								
		-	10-11-7-11			SAND (SP-SM); f-m sand, trace silt, trace f-n	n gravel 10 VP 5/6 (vellowish brown) moist i	med		
		-	10-11-7-11	SP-SM		dense, mixed with GLEY 1 2.5/N	rgiaver, to treate typical bloady, most,			
		-		SP-SM		SAND (SP-SM); f-m sand, trace slit, GLEY 1	2.5/N (black) moist, med dense, well graded			
		-								
1		-	3-6-8-7							
		-								
1		-		CL		Silty CLAY (CL); trace fine silt, 7.5 YR 9.5/1	(white) mixed with 10 YR 4/3 (brown), moist, s	stiff		
	_	-		SP-SM		SAND (SP-SM); f-m sand, trace silt, clay				
, \downarrow		-	3-5-7-24	ML		Clayey SILT (ML); trace fine sand, GLEY 1 4 gravel	/N (dark gray) and 2.5 YR 5/8 (red), dry, trace	rounded		
	_	-				SAND and Silt (SM); f-c sand, GLEY 1 2.5 3/	N (black), dry, med dense			
1		++		SM	GB					
		++				Silty SAND (SM); trace fine gravel, 10 YR 3/2	O (come doubt growtoh haveur) day, dono			
1		++	8-3-27-50/3		0.0	Siny SAND (SM); trace line gravel, 10 YR 3/2	z (very uark grayish brown), dry, dense			
		++		SM	GB					
1		++		SM	GB	Silty SAND (SM); Gley 1 10/3 Y (very dark gi Silty SAND (SM); Gley 1 10/3 Y (very dark gi	reenish gray), dry, dense	with country la		
3.5		++	11	SM CL	GB GB	Silty SAND (SM); Gley 1 10/3 Y (very dark gi	eenish gray), dry, dense, Gley 1 3/N (very da	rk gray),		



BORING NUMBER: BRP-7

Sheet:

SOIL BORING LOG

PROJECT: Honeywell DATEST

V (ft)		AMP	LE	STANDARD PENETRATION	START: 12 915 FINISH: 121915	LOGGER. K-chaturvedu
ELOW			r (ft)	TEST	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (#)	6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLIN RATE, DRILLING FLUID LOS TESTS AND INTRUMENTATION
]	4' Asphalt 0.21 - Poorly graded gravel with Sand (GP), gray-black (N2), dry, Road based aggregate	Advanced Ro borehold using 3"ED sonit make core from 0-10"
1	1000	O CONTRACTOR OF THE CONTRACTOR	269	_ ;	Brown-group (7.54R4/2), dry, medium to coase grained sand	DPC-
	21300					DPC-
-	1				01-Same as above, het	
5-1	SOME MATER CORE			5.0	-woven geolephia -same ao above	bpe-



BORING NUMBER: BRP-7

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6-1-1-1	5-V	SOULC MACED		7	Gol-Same as above, moist	DPC -
1111				-	7.01-8.51. Same as above	DP/ -
	2	SOUTH MARKS CARG	36		8.5'-10' Poorly graded gravel with sand (GN), gray (7.54R 911), dry, coanegrained sand, Road based aggregate, mixed with cores	tre
11,11,1	35-14	55	21"	1 8	medium plackrist	10.0'- 6'00 outer casing advanced to 10' bgs. I 105'- Below to switched to 55 Sampling: 140-16 hammed 30 to drop; 3' dia. sample
1111				10 -	(7.54 R 6/4), dry, medium to coane grained cand, parkenate (copp and non-copp)	DPC+



BORING NUMBER: BRP-7

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMPL	LE .	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
3	SS-S	55	21"	2.2	120-125- same as above. 12-5- same as above, topp. Slightly to moderately indicated	DPC-4
4-1-1	1000	tore tore	IZ,,	13	1381- Same an above,	135'-55 refusal', scrice macroccore advanced to 150' bgs DPC+
	5	conic rough	214	50/2"	150'-16.3'- same as abone	150-6" op outer casing installed to 150 bys. 150-55 refusal; conce main core advanced to 140 bys
1111	28	55	\$! ¹		16.3'- Poorly graded sand with sixt (SRSM), gray-black (N2), wet, basked (BBCOPK) 6.8'. Fat CLAY (CM), Red (104414), moich, very shift, highly blastic same as above	tot, DPC+
-				1-	BoHom of the borning @ 17.5' bgs.	194

Mueser Rutledge Consulting Engineers 14 Penn Plaza • 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 www.mrce.com BORING NO. SHEET PROJECT DUNDALK MARINE TERMINAL FILE NO. 1058 LOCATION BALTIMORE, MARYLAND SURFACE ELEV. BORING LOCATION DATUM Sonie Some DrillaL Mam 170 TEST/INSPECTION EQUIPMENT TOP REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG DURING CORING CASING USED TRUCK MECHANICAL DIA., IN. DEPTH, FT. FROM SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER From TYPE AND SIZE OF: DRILLING MUD USED YES D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER CORE BARREL AUGER USED NO YES CORE BIT TYPE AND DIAMETER, IN. DRILL RODS CASING HAMMER, LBS. Conic MARIO CONO -AVERAGE FALL, IN. SAMPLER HAMMER, LBS. 145 AVERAGE FALL, IN. 20 TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION PIEZOMETER INSTALLED **SKETCH SHOWN ON** installation STANOPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL OD, IN. LENGTH, FT. BOT, ELEV. PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING UN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. NO. OF 3" UNDISTURBED SAMPLES

OTHER:

Loshie HELPERS ill pre installed.

> 12 9 BRP-3 BORING NO.

DATE

CORE DRILLING IN ROCK

BORING CONTRACTOR

RESIDENT ENGINEER

UN. FT.

Ch2m

PROJECT NUMBER:

BORING NUMBER: BPP 8

PPC-

PPK-

Sheet: t

SOIL BORING LOG

PROJECT: Horayard DM7 SSE

ELEVATION:

3%

5

LOCATION:

DRILLING CONTRACTOR: A quiter Dilling and testing, Enc.

DRILLING METHOD AND EQUIPMENT: Jonic Samp Driv Mar 170; SATSampling

CTADT: 12/A/15 FINISH: 12/5/15 LOGGER: K. Chaturvedula 07:40 09.30 **STANDARD** COMMENTS SAMPLE SOIL DESCRIPTION £ PENETRATION DEPTH BELOW TEST **RESULTS** DEPTH OF CASING, DRILLING RECOVERY SOIL NAME, USCS GROUP SYMBOL, RATE, DRILLING FLUID LOSS, COLOR, MOISTURE CONTENT, RELATIVE NUMBER **DENSITY OR CONSISTENCY, SOIL TESTS AND** TYPE INTRUMENTATION 6" - 6" - 6" (N) STRUCTURE, MINERALOGY Advanced he bombole 3" rostalt 0 from of 16' wing 3' 20'
Sonic maero core 255' - sithy some wint grownel (sm), gray-Brown (7.5/RU12), day, DPC medium to coerse grained sand 9. Source Meets cold 26 DPC -

> 5.0'- (layey Sand (90), gray - brown (7-57k 012), wet, the to medium grained sand

60 - Polachyleas lines

601- same as above.



BORING NUMBER: BRP-8

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -	57	Source macino		-	60'- Silly SAND with gravel (SM), gray-Brown (75 to 467, dry, medium to coarse grained sand	De-
7 - 8	5-3	Some MACRO CARR	3.1°	- - -	7-01-8-01_same as above 8-01-3" thick Astphalt 8-25'- Crushed conencte 8-25'- Sithy sand (SM), Red Brown C3-54R ale), day, the formedium grained sand, trace graves 8-41- Poorty graded sand with graves CSP1, Red-white (7-54R 714), day, coarse grained sand	DPC-
- 01 11 11	ي دي	Ş	วำ	21 2b 18	10.01-silks spand with graver (sp.). gray-Brown (7.548412), dry' medium to coane grained sand 11.01-Pieces of wood, decomposed organies 11.21-tal (241), Red (104414), dry, very stiff, medium plastics by	10.01-6"00 outer casing installed to 10'5gs. 10.51- Below 10'switched to 55 sampling; 140-16 hamn 30+0 drop; 3" dia sampler PPC -



BORING NUMBER: BRP-8

Sheet: 3

SOIL BORING LOG

PROJECT:

ELEVATION:

LOCATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

		- 7 LL			SIAKI. FINISH.	LOGGER:
(£)	S	AMPI	1	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DERTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12-		જ હતું	•	_	12:01- Poorly graded cond (SP), greenish-Brown (104R 4/3), dry, time grained Sand, trace gravel	DPC-
13 -	55-5	MMC Ro	24"	z, -	13.0 - Fall LANGEN Red-white (1044)4 moist. vegether high planticity	DPE-
		SOMIC		_	13.61-5114 smup (SN), Red-Boon-gree CA-54R WW, dry, fine to medium	PPC+
14-	55-6 5-7	zor;	NP.	50/j" _	grained sand particulate (HB copp)	14.01. Sis refusal, 3" Disonde macio core advanced to 151 bgs.
15 -	•	O. T.	12 .			150'-6" DID Sonic macro core
				\$ 5 5 - 1	15-5'- Same as about dry.	ground 15.0'- water encountered
16 -	55.5	\$5	2W.	17		during drilling
1.5-	Ä.			13	1671 Poorly graded Smo with Sit (SR Cark groy black (N2), wet, medium to Coarsegrained Sand Forticular (GB	D6C3+1
	55.9	SS	6	20	Cope libright green-vellow particles. Some as above	
18-			·	-		



BORING NUMBER: BRP-8

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

-			VELO	-			
ſ	(£)	. S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
	DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
	18-	S5-10	<u>.</u> \$\$	ς''	, 6	120'. Same as above, bright Obven and yellow particles	
	- - 19 _	25-N	\$5	6		18.5'-18.8'. Some as above 18.8'. Silysonio (sm), Red-Brown (15/124/6) day fine to medicin grained sand stighty to moderately industed (AB (0PE)). DEC 4
	-	(1)	د و	Ŋü	٠ ١٠ ٠	Fat CLAY (CAY), Red-white (104 4/4). Most, very stiff, highly plastic	DP(-
	10 -					Thermometer her temp=10.0 Thermistra A.r Temp 9.3 Thermistra Slarry Temp 10.3	I'' PVC Installed @ 19' Vwp installed @ 18' 1518427 thermisterinstalled at 16.5' Interface of HB& OBCOPT AN Vwp To12.7 Po9736 dipth T.13.7 P. 9710.
	, 44 -	-					Sluny T2133 P2 8336 .

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET DUNDALK MARINE TERMINAL **PROJECT** FILE NO. LOCATION SURFACE ELEV. BALTIMORE, MARYLAND **BORING LOCATION** DATUM **TEST/INSPECTION EQUIPMENT** SOME SAMP DRILL MAY 170; SPT Sampling REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING** CASING USED 6" TRUCK MECHANICAL DIA., IN. DEPTH, FT. FROM TO SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM TO BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER Track TYPE AND SIZE OF: DRILLING MUD USED YES **D-SAMPLER** DIAMETER OF ROTARY BIT. IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER Sampler: NO CORE BARREL YES AUGER USED CORE BIT TYPE AND DIAMETER, IN, **DRILL RODS** CASING HAMMER, LBS. SOMIC MACROCORD. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. AVERAGE FALL, IN. 30 TYPE OF HAMMER Automa WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE **DEPTH OF CASING** DEPTH TO WATER CONDITIONS OF OBSERVATION 12/9/15 encountered during diffing 08:40 120 12.0; 15.01 VWP PIEZOMETER INSTALLED YES NO **SKETCH SHOWN ON** STANOPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: LENGTH, FT. MATERIAL OD, IN. BOT. ELEV. **PAY QUANTITIES** 3.5" DIA, DRY SAMPLE BORING UN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK UN, FT. OTHER: **BORING CONTRACTOR** The en DRILLER HELPERS Chris Installed Thermistor and 1" pre REMARKS RESIDENT ENGINEER DATE 12/9/15

BORING NO. COCY-S



BORING NUMBER: BLP -3

Sheet: |

SOIL BORING LOG

PROJECT: Honeywell DMT SSI

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: A quiter Drilling and Testing, Price

DRILLING METHOD AND EQUIPMENT: Some samp Dell XL MAX 170; SPT Sampling

WATER LEVELS:

START: 12/10/15

FINISH: 12/10/15 LOGGER: Kichatur vedula

Œ	s	AMPL	.E	STANDARD	15:00 16:30 SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0				-	2" Asphalt. 0-11-Poorly graded Sard with GRAVEL (SP), gray-brown (7-5 YR 4/1), most, Coarse grained Sard	Advanced the Borchole h lot bys using 3th DD sonic macro cover DPC-
2-	د ^ی	SONIC MAREO WAS	₩	- - - - - - -	2-0'- Sity cano with gravel (SM), Dank Red Brown (7:54R 4/2), Moist,	DPC-
-				-	3.5'- same as above, higher fines conject	DPC-
\(\frac{\zeta}{1} \\ \zeta	, X	aces es fre		-	hio'-same as above, higher fines can book 50'-same as above	·
- - - - م		พราพอร		- - -	6.0'- two layers of geoterfile	- 1 m m m m m m m m m m m m m m m m m m



BORING NUMBER: BRP-9

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

			···			
(£)	S	AMPL	T	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
рертн весом	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -	5 ⁻⁷	More work			6.0' same as above.	
7		Q		-	7-01. Clayey SAND (SC) with gravel, gray-brown (7:5 VR 4/2), wet, time grained sand	Pre-
3 -	دی	sowu marko lo Re	36	- - - -	-8-5'- Crushed converte and Asphalt	DPC-
- ol	ملا .	77		_	9-5'- Poolly graded some with gravel (Str., gray (SN), moist, Loane grained Eand, Road based aggregate.	PPC - 10-0'- 6"op outer casing mutalled to 10' bags
 	; .		5"	[50] _{1,1}	10:0'- Clayey SAND (SC), Red (104414) dry, fine to medium grained sand, verydense, (cope and non-cope) 10:4'-Silty SAND (SM), Red-Brown- greensh (7:51 R A/S), dry, fine to medium grained sand, Particulate	10:51 - Below 10' switched to SS sampling, 140-16 hammer 30-in-drop; 1314 to Samplar DPC+1-
 - - -	5.5	24 4 CA 4 CA 4 CA 4 CA 4 CA 4 CA 4 CA 4	18°	- - -	medium grained sand, farticulate CHBCOPE) 11:12- same as above	SS refusal. Advanced the 3" DD Sonic matrocore to 12:0's
12						



BORING NUMBER: BRP. 9

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£	s	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12-	55-p	دي	Ŋü	29 -	12:0' - Sarus as abow, (lope and non-	DPC+/-
]\r\5 =). 	के प्रति		21/1	12-51- Same as above, HB COPR	12-5'-25 refusal; some macro core advanced to 14-0'bgs
13-	5- 2	DNIC MANZOC		- -	13.01. Same as above, greenish 13.21. Same as above, greenish	DPC-F
	55-8	G	,		13.9'. Poorly graded SAND with silt (sp. gray-black (N2), moist, coase grained Sand, Particulate (GB copp) 14.0'. 14.2'. same as above 14.2'. 14.5'. Silty SAND with gravel (SH),	sm). DPC+ 14-6-groundwater encountered OPC+ during drilling
- 21			ヾ	50 ° ' -	lu 3- lu 4. Sity sandwith glavel (sh), gray-brown (2-5 ve 4/2), moist, fine to meaning grained sand (copperation coff) lust-15-5). Silty same (sh), gray. Brown (25 ve 4/2), moist, fine to media grained sand (copp)	
1452 <u>-</u>		07		-	12.5-16.6- cope and non-cope Ail	Ss-refusal; Sonir macro fore advanced to 1766 bgs.
1 -	55	SINIC MAC CORE	184	,	16.0'- Silty CARVID (SM), Red-Brown - greenish. (454R 413), moist, time to medium grained sand, particulate CHBCOPR), trace gravel	DPC+
17- - -	55 ¹⁰	ζS	ιż	ν - 13 -	(75 YR. 4/2), wet, for R and non-cap	r) DPC+1-
18-				- -		



BORING NUMBER: BRF-9

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

	£	s	AMPL	.E	STANDARD	SOIL DESCRIPTION	COMMENTS
-	DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
		·(1.22	, ÇS	3 ^{\(\)}	9 -	18.0' - Same as above	DPC +/_
٠	-	S5- ⁷³ S5-73		4" 6"	9	Fat clay (cet). Red (1074/u), wet, very stiff, highly plastic Bothon of boing @ 20.0' bgs	DPC- DPC- 20:01-6"00 outer casing installed to a depth of 20:01 base
	υ - -				 - - -	I vwp 1517448 installed & 19' I thermister installed & 13.5' depth Thermometer air temp: 7.2'	Vmp Air To 10.9 Po 9769 Opply T. 12.1 P. 9466
	* -					Thermister Air Temp 7.8 Thermister Sturry Temp 13.2	Sluy T219.9P2 8542
	, -	100 kg 100 kg			- -		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400

	BORING NO. BRP-9
PROJECT DUNDALK MARINE TE	SHEET OF
LOCATION BALTIMORE MARYLANI	
BORING LOCATION BARYLANT	
	DATUM
TEST/INSPECTION EQUIPMENT	
REFERENCE CODES/STANDARDS	
Service of the servic	THE PROPERTY OF THE PROPERTY O
BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE	
TYPE OF FEED	•
TYPE OF BORING RIG DURING CORING CASI	NG USED YES NO
TRUCK MECHANICAL DIA.	IN 6"
SKID HYDRAULIC DIA.	IN DESTRUCTION
DIA.,	IN DESTRUCTION
OTHER FLOCK	TO
TUDE AND SING A	1
TYPE AND SIZE OF: DRILL	LING MUD USED YES NO
DIAMPLER DIAM	METER OF ROTARY BIT, IN.
TYPE	OF DRILLING MUD
SAMPLER CS Complex - \$3/0 dia.	
ORF RIT AUGI	R USED YES NO
ORILL RODS TYPE	AND DIAMETER, IN.
Part A A A A A A A A A A A A A A A A A A A	VG HAMMER, LBS. AVERAGE FALL IN
	OFHAMMER Automatic
VATER LEVEL OBSERVATIONS IN BOREHOLE	
ATER LEVEL OBSERVATIONS IN BOREHOLE	
VATER LEVEL OBSERVATIONS IN BOREHOLE	
VATER LEVEL OBSERVATIONS IN BOREHOLE	
VATER LEVEL OBSERVATIONS IN BOREHOLE	
VATER LEVEL OBSERVATIONS IN BOREHOLE	
VATER LEVEL OBSERVATIONS IN BOREHOLE	
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V	
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED YES NO	SKETCH SHOWN ON VMP Installation log
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED ANDPIPE: TYPE ID, IN.	SKETCH SHOWN ON VMP Installation log
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TAKE ELEMENT: TYPE OD, IN	SKETCH SHOWN ON VMP Installa hom log LENGTH, FT. TOP ELEV. LENGTH, FT. TIP ELEV.
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TAKE ELEMENT: TYPE OD, IN. TER MATERIAL OD, IN.	SKETCH SHOWN ON VMP Installa hom log LENGTH, FT. TOP ELEV. LENGTH, FT. TIP ELEV.
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TAKE ELEMENT: TYPE OD, IN. TAKE ELEMENT: TYPE OD, IN. TAKE OD, IN. TYPE OD, IN. TYPE OD, IN. TY QUANTITIES	SKETCH SHOWN ON VMP Installa hom log LENGTH, FT. TOP ELEV. LENGTH, FT. TIP ELEV.
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TAKE ELEMENT: TYPE OD, IN. TER. MATERIAL OD, IN. AY QUANTITIES "DIA. DRY SAMPLE BORING LIN. FT.	SKETCH SHOWN ON VER TOPELEV. LENGTH, FT. LENGTH, FT. LENGTH, FT. LENGTH, FT. LENGTH, FT. LENGTH, FT. BOT. ELEV.
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TAKE ELEMENT: TYPE OD, IN THE MATERIAL OD, IN Y QUANTITIES TO IA. U-SAMPLE BORING LIN. FT. TO IMPLEMENT OF HOLE DEPTH OF CASING DEPTH TO V DEPTH TO V DEPTH TO V DEPTH OF CASING DEPTH TO V TYPE OD, IN OD, IN TO IA. U-SAMPLE BORING LIN. FT. TO IA. U-SAMPLE BORING LIN. FT.	SKETCH SHOWN ON VMP Tristalia hom log LENGTH, FT. TOP ELEV. LENGTH, FT. TIP ELEV. LENGTH, FT. BOT. ELEV NO. OF 3" SHELBY TUBE SAMPLES
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TAKE ELEMENT: TYPE OD, IN THE MATERIAL OD, IN Y QUANTITIES TO IA. U-SAMPLE BORING LIN. FT. TO IMPLEMENT OF HOLE DEPTH OF CASING DEPTH TO V DEPTH TO V DEPTH TO V DEPTH OF CASING DEPTH TO V TYPE OD, IN OD, IN TO IA. U-SAMPLE BORING LIN. FT. TO IA. U-SAMPLE BORING LIN. FT.	CONDITIONS OF OBSERVATION SKETCH SHOWN ON VMP Tostalia Non 109 LENGTH, FT. TOP ELEV. LENGTH, FT. TIP ELEV. LENGTH, FT. BOT. ELEV NO. OF 3" SHELBY TUBE SAMPLES NO. OF 3" UNDISTURBED SAMPLES
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TAKE ELEMENT: TYPE OD, IN OD, IN OD, IN OD, IN OTHER DEPTH OF HOLE ID PTH OF CASING DEPTH TO V ID, IN. OD, IN OD	CONDITIONS OF OBSERVATION SKETCH SHOWN ON VER TINSTALLA DOS 109 LENGTH, FT. TOP ELEV. LENGTH, FT. TIP ELEV. LENGTH, FT. BOT. ELEV NO. OF 3" SHELBY TUBE SAMPLES NO. OF 3" UNDISTURBED SAMPLES OTHER:
EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TER. MATERIAL OD, IN Y QUANTITIES "DIA. DRY SAMPLE BORING LIN. FT. IRING CONTRACTOR DEPTH OF HOLE DEPTH OF CASING DEPTH TO V DEPTH OF CASING DEPTH TO V DEPTH OF CASING DEPTH TO V TYPE DEPTH OF CASING DEPTH TO V DEPTH OF CASING DEPTH TO V ID, IN. I	CONDITIONS OF OBSERVATION SKETCH SHOWN ON VER TINSTALLA DOS 109 LENGTH, FT. TOP ELEV. LENGTH, FT. TIP ELEV. LENGTH, FT. BOT. ELEV NO. OF 3" SHELBY TUBE SAMPLES NO. OF 3" UNDISTURBED SAMPLES OTHER:
EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TER MATERIAL OD, IN. Y QUANTITIES "DIA. DRY SAMPLE BORING LIN. FT. WI COLONTRACTOR RE DRIVE DRIVE ACTOR OF THE COLONTRACTOR IN FT. PRING CONTRACTOR AQUITED ACTOR OF THE COLONTRACTOR ILLER	CONDITIONS OF OBSERVATION SKETCH SHOWN ON VER TINSTALLA DON 109 LENGTH, FT. TOP ELEV. LENGTH, FT. TIP ELEV. BOT. ELEV NO. OF 3" SHELBY TUBE SAMPLES NO. OF 3" UNDISTURBED SAMPLES OTHER:
DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO V EZOMETER INSTALLED ANDPIPE: TYPE ID, IN. TAKE ELEMENT: TYPE OD, IN THE MATERIAL OD, IN AND QUANTITIES TO DIA. DRY SAMPLE BORING LIN. FT. THE RE DRILLING IN ROCK LIN. FT. RE DRILLING IN ROCK LIN. FT.	CONDITIONS OF OBSERVATION SKETCH SHOWN ON VMP Installa how log LENGTH, FT. TOP ELEV. LENGTH, FT. TIP ELEV. LENGTH, FT. BOT. ELEV NO. OF 3" SHELBY TUBE SAMPLES NO. OF 3" UNDISTURBED SAMPLES OTHER:



BORING NUMBER: BER 11

Sheet: 1

SOIL BORING LOG

PROJECT: Honeywell DMY SST

LOCATION:

DRILLING CONTRACTOR: Aquifer Drilling and testing, Inc. DRILLING METHOD AND EQUIPMENT: Sonic Samp AL Drill Max 170, SPT Sampling

WATER LEVELS:

START: 12/10/15 FINISH: 12/10/15 LOGGER: Kchaturedule

NUMBER	CORE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY 3" Asphalt 0.25'- 1.0'. Poorly graded gravel with Sound (GP), gray-black (W2), dry 170000 based aggregate	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION Advanced the borehole between o'- 10' bgs: With 3" Ito Ionic macrocar
5-1	eve:			0.25'- 1.0'. Poorly graded gravel with sand (GP), gray-black (W2), dry troad	between o'- 10 bas +
5-1	300				
	PCDs Co	36		hel-sitty sourd Withgraver (SM), Red-Brown (754R U16), moist, fineto Medium grained sand	DPC-
	SONIC P.F			201. when geolephile 201. Samme at above	
					DP C-
	0			Gol-Same as above	- Hard drilling
! 5~V	NAPERO	36		5 6'- Gob'. Reintweet concrete	
5	, N	SOME MACES CARS	NAPCE	Water -	3 0'- G-D'. Reinforced concrete



BORING NUMBER: BRP-11

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(tr)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	BER		RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND
DEPT	NUMBER	TYPE	REC	6" - 6" - 6" (N)	STRUCTURE, MINERALOGY	INTRUMENTATION
C -	SN	Fonic Macro con			G.o'_ Silty SAND with groved (SH), Brown (2-5 4516), Moist, fine to midlion qualities Jand,	pbe-
					7:01-8:01- Pourly graded sand with gravel (SP), gray (5N), coarse graind sand. Road based aggregate	DPC-
3 -	53	MARK	36"		80-8.5'. clayer sond (sic) with gravel, Red-Brown (2.54R 4/3), wet, time to medium grained sand 8.5'-30'-6" thick layers of Asphalt	OPE-
0-		Source				1000'- 6"00 outer casing installed to a depth of
	g, v	55	22"	The second second second	gravel (er), group brown (25) yr 112), mout, medium to coase graves sourd.	100' bgs. 10x'- below 10' switched to 55 Sampling; 1 unto hamme 30 in . drop: 3"dia. sampler
				23	11-5'-11-5'. Bentonite mix	PPC-
17-					11-9'- Piece of convete encountered	



BORING NUMBER: BER-11

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£	S	AMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (#)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
2-	523	SS	24"	23	1201-1301- Same as above Red- white (1-54R 714), moist, coone grains sand	Dre-
3 -				19	120. Sondy CLAY (SC); Red (104414), Moist. Low planticity, Stiff, trace Junes	DPc -
15-	<u>5</u> 5-6	55	24 ^a	19 -	The - Fat CLAY CELT , Red (104 414); dry , Low plackery , very shiff, troce gravel	PPC-
	_S X	55	'n	3 11.87	16.0'- Same as above well 16.5'- Same as above haces of 1.	16.0'-6" so outer caving installed to a depth of 15' bgs
>-	23-82	ÇS	6"	10	12.0% raying Stood (SC), Red Tellows (2.18 Kblo), moist, finely medium	pre-
1-	25-9	ςS	44	8	No recovery	

ch2m

PROJECT NUMBER:

BORING NUMBER: BRP-11

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

E	\$	AMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	ER		RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING	
	NUMBER	TYPE	RECOV	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
18-	55-10	SS	6"	10	180'. Pat cony (CLI), Red (104 blu), moist. Stiff, medium plasteity.	DPC-	
	12-11	52	("	17	18.51 - same as above		
9-5-	5-12	22	Ĉ,	17	Pooly graded sond with Sild (sp. sn) gray-brown (7.54 R4/3), moist, cine to measur grained sono, particulate (9500	actions of the spoon,	
100	55-13	22	G"	19	Same as above	opc-	
-0	55-M	SS.	5 ¹⁴	3 -	fat CLAT (CCH), Red (1044/4), Moist, Shiff, medium plashiby, traces of COPR material (COPR and non-COPR)	200'-6' ob outercasing install	
< 1	sç-15	دد	NR	5	No Recovery		
.1 -	55-16	\$\$	6"	3	Fat (LAY (CH), Red (1044) W), moist, Shof, high plashicity.	DPC-	
The state of the state of					Bottom of boring @ 21.5' bgs. I'PUC histalled to 21' depth Thermometer Air reading = 13.2 thermister metalled @ 16.5 depth Thermister Air 13.4	Vwp To 20.3 Po 9619 Dopth T. 20.4 P. 9608 Slowy T2 17.5 P2 8184	
1.4.1				- 4	Thermotor slurry		

ROJECT	N B	UNDALK	MARIN E, MARY	E TERM	INAL	SHEET FILE NO. SURFACE ELEV. DATUM	BR1-11 5 OF 5 0587
	PECTION EQU CE CODES/ST		Sonic sou	ar Word gu	Max 170;	sprsampli	9
ORING E	QUIPMENT	AND METHODS O	F STABILIZING BO	REHOLE			
VPE OF BE	ORING RIG	TYPE OF FEE	The same of the sa	CASING USE		Ves	- Jun
RUCK	Jilling Rid	MECHANICA	700	DIA, IN.	60	DEPTH, FT. FROM	A 10 21
KID		HYDRAULIC		DIA. IN.		DEPTH, FT. FROM	TO
ARGE	Mack	OTHER	-	DIA, IN		DEPTH, FT. FROM	to
THER	2.00	_					
	SIZE OF:			DRILLING M	UD USED	YES	OND
-SAMPLER	· ·				OF ROTARY BIT, IN		
-SAMPLER -SAMPLER		Sausplay.	30 00	TYPE OF OR	LUNG MUD	-	
ORE BARR			2.00.	AUGER USE	0	YES	VNO.
ORE BIT				TYPE AND D	MAMETER, IN.		
RILL RODS	-	no cone :	3" I.D	CASING HAM	20 Day 14 D	2100	RAGE FALL, IN.
				TYPE OF HA	AMMER, LBS. MMER AN	140 AVE	NGE FALL, IN. 30
VATER LE	EVEL OBSERV	ATTONS IN BORE	HOLE				
VATER LE	TIME	ATIONS IN BORE	DEPTH OF CASING	DEPTH TO WATER		CONDITIONS	DF OBSERVATION
	15			DEPTH TO WATER		CONDITIONS	OF OBSERVATION
	15			DEPTH TO WATER		CONDITIONS	DF OBSERVATION
	15			DEPTH TO WATER		CONDITIONS	OF OBSERVATION
	15			DEPTH TO WATER		CONDITIONS	DF OBSERVATION
	15			DEPTH TO WATER		CONDITIONS	OF OBSERVATION
	15			DEPTH TO WATER			
DATE	15	DEPTH OF HOLE	DEPTH OF CASING		CH SHOWN ON		Tastallation log
DATE	TER INSTALLS	DEPTH OF HOLE	DEPTH OF CASING	NO SKET		ywy	enstallation log
DATE	TER INSTALLS	DEPTH OF HOLE	DEPTH OF CASING		LENK		Enstallation log
DATE JEZOMET TANOPIPE JTAKE ELE	TER INSTALLS	DEPTH OF HOLE	DEPTH OF CASING	NO SKET	LENK	√W≯ sin.es.	enstallation log
DATE TANOPIPE TTAKE ELE LTER	TER INSTALLS	DEPTH OF HOLE TYPE TYPE	DEPTH OF CASING	NO SKET	LENK	VWY STM, FT. STM, FT.	Enstallation log TOPELEV. TIPELEV.
DATE SEZOMET TANOPIPE STAKE ELE LITER AY QUAN	TER INSTALLS	DEPTH OF HOLE TYPE TYPE MATERIAL	DEPTH OF CASING	NO SKET	LENG		Enstallation log TOPELEV. TIPELEV.
DATE IEZOMET TANOPIPE ITAKÉ ÉLÉ ILTER AY QUAN 5" DIA DE	TIME TER INSTALLS MENT:	DEPTH OF HOLE ED TYPE TYPE MATERIAL RING	DEPTH OF CASING	NO SKET	LENK	VWP STH, FT. STH, FT. TUBE SAMPLES	Enstallation log TOPELEV. TIPELEV.
DATE IEZOMET TANOPIPE ITAKÉ ÉLÉ ILTER AY QUAN 5" DIA. DI	TER INSTALLS MENT: NTITIES RY SAMPLE BO	DEPTH OF HOLE ED TYPE TYPE MATERIAL RING	VES LIN FT.	NO SKET	LENG LENG NO. OF 3" SHELBY	VWP STH, FT. STH, FT. TUBE SAMPLES	Enstallation log TOPELEV. TIPELEV.
TANOPIPE STAKE ELE LITER AY QUAN 5" DIA, U- ORE DRILL	TER INSTALLS MENT: MITTIES RY SAMPLE BORING SAM	DEPTH OF HOLE ED TYPE TYPE MATERIAL RING NG	VES LIN FT.	NO SKET	LENG LENG NO. OF 3" SHELBY NO. OF 3" UNDIST	VWP STH, FT. STH, FT. TUBE SAMPLES	Enstallation log TOPELEV. TIPELEV.
DATE STANDPIPE TAKE ELE LTER AY QUAN 5" DIA, DI ORE DRILL	TIME TER INSTALLS MENT: MILTIES RYSAMPLE BORI JING IN ROCK CONTRACTOR RYSAM CONTRACTOR	TYPE TYPE MATERIAL RING NG AQU	VES UN FT. UN FT. UN FT.	NO SKET	NO. OF 3" SHELBY NO. OF 3" UNDIST OTHER:	TUBE SAMPLES URBEO SAMPLES	Topeley. TIPELEY. BOT. ELEY
EZOMET ANOPIPE TAKE ELE TER AY QUAN 5" DIA. U- DRE DRILL	TIME TER INSTALLS MENT: MILTIES RYSAMPLE BORI JING IN ROCK CONTRACTOR RYSAM CONTRACTOR	TYPE TYPE MATERIAL RING NG AQU	VES UN FT. UN FT. UN FT.	NO SKET	NO. OF 3" SHELBY NO. OF 3" UNDIST OTHER:	TUBE SAMPLES URBEO SAMPLES	Top ELEV. TIP ELEV. BOT. ELEV



BORING NUMBER: BRP-12

Sheet: 1/6

SOIL BORING LOG

PROJECT: Oundalk Marine From Mal

LOCATION: Balthmore, Maryland

ELEVATION: ~+20

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Fruste Multi Orill XL Max 170: Sonic Sampler WATER LEVELS:

START: 2:70 FINISH: LOC

LOGGER: Mark change

₽	5	AMP	LE	STANDARD		T John I Charle
3		T		PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 -	51 ·	311 XVn1L	3'	-	Asphalf Rocalbase	
/ -				-		
2 -						
3 -					3.0-4.0 Dark Oil Brown f-Msitty sand, trace stavel (SM)	PPC-none
4 -	5-2	311 Sonic			4.0-7.0	DPC - none
<i>5</i> –						
6-						grominibrani a 6



BORING NUMBER: BRP-12

Sheet: 2/6

SOIL BORING LOG

PROJECT: Dundalk Marine Terminal

ELEVATION: ~+ 20

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Frust Multi Duil Max 170, Sanie Samples

WATER LEVELS:

START: FINISH: LOGGER:

LOGGER: May Chavey

				67440455		Trade Chancy
(£)	s	AMPL	T	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -	5-2	311 SouiL	31	NA -	Damp led brown f-Msilty sand frace gravel	DRC - None goodwan brano 00 6.
7 -	S-3	311 Sonil	3'	NA -	7.0-7.5 damp sed brown f. 11 silly sand trave gravel, trace clay (SMCL)	
8 -				-	7.5-9.0 Asphalt/road Sase Concrete gray toward to block gravelly f-(sand (s)	
9 _					9.0-10.0 Camp Red Fat clay	DR-none focket pen 3.75
-				35-6//3"		OR-none pocket pen 3.75 3 readings 4.0 taken a mid punt of 4.25 Fat Clay layer
	33 -1			-	(3M)(HB) 10'9"-17'	DPC++ Very strang
17	54				Dry Red brown f-M mode attly und rated Silty send, some gravel (SM) (HB)	DRTstrong



BORING NUMBER: BER 12

Sheet: 3/6

SOIL BORING LOG

PROJECT: Duntalk Marine Terminal LOCATION: Baltimore, Maryland
ELEVATION: ~ 120 DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Frask Multipall Max 170: Sanic Sampling
WATER LEVELS: START: FINISH: LOGGER: Mark Chancy

		SAMP	F	STANDARD		
₩ Ж		7	T	PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12	55-2	211 OD Sample	20"	28-31 - 21-31	12-14' Dry midium dense, moderately inducated f-m silty sand (SM) (I+B)	DPC+ Strong
/3						
14.	- 55-3	21' 00 Sampler	7"	32 100/3	14-14'9" wet dense to very dense highly Inducated f. Mgiltysand (SM) (HB)	refusul to 14'9" PPCts from g advanced with somic to 16'
15-	5-5	311 Son((4'	NA -	14'4"-15' Moist brown with purple particles f-M sitty sand RM > (GB) 15.0-16.0 Dry 1 of brown f-M sitty sand Somegravel (SM) (HB)	DAC+ strong Sould streeted DAC+ strong
11 -	55-4	2" 00 b				DPC++ verystrong
n-	555		2	7	(SM) (HB)	DPC+strong
[8 -					17.0-18 Wit med demie - dense Ved brown f-M Silty sand trace graves (GB) (SM)	VICI SHOOTY



BORING NUMBER:

Sheet: 4/6

SOIL BORING LOG

PROJECT: Durdalk Merchet (crusted)

ELEVATION: ~ 120

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Froste Multi Drill May 170; Sonic Scapler

WATER LEVELS:

START: FINISH: 6:00 pm LOGGER: Mark Charry

€	S	AMPL	.E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS.
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL , STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18 -	4-6	211 00 sarpler	4"	100/5 -	18-18-4" Wet very dense grapish brown with green particles Bilty sand GM)(GB)	refusal 2 18'5" DRC ++ very strong advanced with some to 19'
-	5-b	311 Sohi(4'	NA :	18'4"-19' moist Red brown with green Particles f-M silty sand (GB)(SM)	DPC+ strong
19 -	45-7	2:1 00 54mple/]	13 -	19'0-19'. 6" moist Red mottled u. 44 white Fat clay CCH)	OPC-none
-					Borehola terminated w 19.5	Poulet pen 1.75 1.25
20 -					•	6"00 casing to 19"
21 -					Thermometer Airtemp=12,2	AIK. Vwp To 15.9 Po 9668
22-					themselv air reading 12,8	depty T, 24.4 P, 9172
23-				•	Vwp 1517441 :	Slumy T2 29.9
- 24 -					thermistiv installed @ 15' Thermister slurry 24.7	P ₂ 8348,

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET Oundalk Marine Terminal FILE NO. **PROJECT** Baltimore, Maryland Ser lose SURFACE ELEV. LOCATION **DATUM BORING LOCATION** Soull Samp Drill Mux 170; SPT Sampling **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED CASING USED TYPE OF BORING RIG **DURING CORING** DIA., IN. DEPTH, FT. FROM TO TRUCK **MECHANICAL** DEPTH, FT. FROM TO **HYDRAULIC** DIA., IN. SKID DIA., IN. DEPTH, FT. FROM OTHER BARGE **OTHER** TACK YES DRILLING MUD USED TYPE AND SIZE OF: **D-SAMPLER** DIAMETER OF ROTARY BIT, IN. TYPE OF DRILLING MUD U-SAMPLER SS-sampler > 2" Dia Sampler S-SAMPLER YES **CORE BARREL** AUGER USED **CORE BIT** TYPE AND DIAMETER, IN. **DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. 140 30 AVERAGE FALL, IN. SAMPLER HAMMER, LBS. Automati TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DEPTH OF CASING | DEPTH TO WATER CONDITIONS OF OBSERVATION DATE DEPTH OF HOLE TIME PIEZOMETER INSTALLED NO SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. OD, IN. LENGTH, FT. TIP ELEV. INTAKE ELEMENT: TYPE BOT, ELEV. OD, IN. LENGTH, FT. FILTER: MATERIAL **PAY QUANTITIES** 3.5" DIA, DRY SAMPLE BORING LIN. FT. NO. OF 3" SHELBY TUBE SAMPLES LIN. FT. NO. OF 3" UNDISTURBED SAMPLES

Agrifer Willing and Testing **BORING CONTRACTOR** HELPERS Jo siph DRILLER

OTHER:

LIN. FT.

VWP and I theremister installed REMARKS DATE RESIDENT ENGINEER

> BRP- 12 BORING NO.

3.5" DIA. U-SAMPLE BORING

CORE DRILLING IN ROCK

BORING LOG

Page 1 of 2

Site: Dundalk Marine Terminal

Boring No: BRP-12A

Northing: 574337.694

Easting: 1448099.493

Driller: Parratt Wolff, Inc.

Method:

Total Depth: 20.0 feet

GW Depth:

	L	U	10	y W	16	•		Geologist: Jacobs		
pth et	Rec	DPC	Blow Count	USCS	Soil Code		Stratum Description		Sample	Comments
Ť			Count		Oode	Asphalt				
		-								
Ī		-	12-13-6				ravel, 10 YR 3/2 (very dark grayish brown), inc	reasing gravel		
		-		SP-SM		content with depth, angular to subangu	ular, dry, well graded, medium dense			
		-				Clayey SILT (ML); trace fine sand, trac mottled with GLEY 1 2.5/N (black), mo	ce medium gravel, 10 YR 3/2 (very dark grayis	n brown),		
+		-	2-1-3-4	ML						
+		_								
	-	-		ML		mottled with GLEY 1 2.5/N (black), mo	ee medium gravel, 10 YR 3/2 (very dark grayis sist	n brown),		
T		-	2-4-6-11			SAND (SP-SM); f-m sand, trace silt, 10	0 YR 4/3 (brown), dry, loose, poorly graded			
		-		SP-SM						
		-		SP-SM		SANB (SW), I m sand, treep silt, 10	0 VR 4/2 (brown), dry loces, poorty graded il, weathered asphalt, GLEY 1 2.5/N (black), d	y, very dense,		
1	_	-	28-18-50-65	SW		well-graded				
				011						
+										
		-		SW		SAND (SW); f-m sand, trace silt, grave জ্ঞান ক্রমান্ত (SM); 2.5 YR 5/8 (red), gra	el, weathered asphalt, GLEY 1 2.5/N (black), d	y, very dense,		
†		++	14-7-31-10/6	SM		SAND (SP-SM); f-m sand, trace silt. 7.	.5 YR 6/8 (reddish yellow), poorly graded, dry,	med. Dense		
		++	1	SP-SM		_ (== ===,, = ==== === === === === === ==	- (), Fee-ily Grandal aily			

BORING LOG

Page 2 of 2

Site: Dundalk Marine Terminal

Honeywell

Boring No: BRP-12A

Northing: 574337.694

Easting: 1448099.493

Elevation: NA

Driller: Parratt Wolff, Inc.

Method:

Consultant: JACOBS

Geologist: Jacobs

Total Depth: 20.0 feet

GW Depth:

Date: 07/19/2018 - 07/20/201

Depth feet	Rec	DPC	Blow Count	USCS	Soil Code	Stratum Description	Sample Comments
10		++				SAND (SP-SM); f-m sand, trace silt, 10 YR 6/4 (yellowish brown) and 5 YR 4/6 (yellowish red),	
		++				dry, very dense, weakly indurated	
†		++	34-39-52-41	SP-SM	НВ		
		++					
Ť		++]
		++				SAND (SP-SM); f-m sand, trace fine gravel, silt, 2.5 Y 6/2 to 6/4, dry to moist	
Ť		++	13-7-8-13	SP-SM	НВ		
		++	1	SP-SM	НВ	SAND (SP-SM); f-m sand, trace fine gravel, silt, 5 YR 4/6 (yellowish red)	-
†		++					1
		++				SAND (SP-SM); f-m sand, trace siit, GLEY 1 2.5/N (black), wet, poorly graded, very dense	
5 十		++	24-66-50/1	SP-SM	GB		
		++	-	SP-SM	НВ	SAND (SP-SM); f-m sand, trace silt, 7.5 YR 6/8 (yellowish brown), weakly to strongly indurated, poorly graded, dry, dense	
t		++					1
	-	++	-	SP-SM	НВ	SAND (SP-SM); f-m sand, trace silt, trace gravel, grading to silty f-m sand, 7.5 YR 5/4 (brown), moist, well-graded, very dense	
		++	27-26-49-39			SAND (SP-SM); f-m sand, trace gravel and silt, GLEY 1 3/N (very dark gray) intermixed with 2.5	1
		++		SP-SM	GB	Y 5/3 (olive brown), dry, very dense, poorly graded, weakly indurated	
Ť		++		SM	GB	SAND with silt (SM); f-c sand, trace gravel, GLEY 1 3/N (very dark gray), moist, poorly graded,	
		++	14-16-13-5	SM	GB	SAND with silt (SM); f-c sand, trace gravel, GLEY 1 3/N (very dark gray), moist, poorly graded,	
		++	1.4-10-10-0	SM	GB	SAND with silt (SM); f-c sand, trace gravel, GLEY 1 3/N (very dark gray), moist, poorly graded,	1
		++	1	SM CL	GB	TYPE W. (1995 M.): f c sand, trace gravel, CLEY 1 3/N (very dark gray), moist, poorly graded, CLLY 1 3/N (very dark gray), moist, poorly graded, weakly indurated	7



BORING NUMBER: BRP-13 Sheet: 1/5

SOIL BORING LOG

PROJECT: Dundalk Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: ~ 20

DRILLING METHOD AND EQUIPMENT: Fruste Multi Orill Mak 170: Sourie Sampler WATER LEVELS: START: 10:45 FINISH:

LOGGER: Mark Chancy

					10.45	I HAVE CHANGY
(E)	s	AMPL		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 -	5-1	3 ¹¹ Soni(1.5	NA -	Asphal +/Road base	
1			/	- - - - -		
4-5-	5-2	311 Sonik			Bott 6" Dry red brown Silly f-M Gund Frace gravel SM. 4.0-6.0 Dry red brown Silly f-M Gund Frace grave 1 SM	PPC - geo numbrane 25'



BORING NUMBER: BLP-13

Sheet: 2/5

SOIL BORING LOG

PROJECT: Dundalle Marine Terminal ELEVATION: ~ 20

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Fraste MultiDrill Max 170: Sonic Sampler WATER LEVELS: START: FINISH: LOGGER: Mail Change

(E)	S	AMPL	E .	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -				NA -	6.0-7.0 lood bose/Asphot with Ind brown f-M silty sand trace gravel (SM)	PPC-nonc
7-	5-3	311 Souic	3	NA -	7.0-8.0 Dry Rid modfled with white Fad clay (CH)	DRC-Gone
8					8.0-9.0 Dry led brown f-M silly sand frake gravel with concrete (SM)	DPC-none
7 - 10 -					9.0-10-D Dry 1 brown folgitly sand with white particles (SM)	OPC-more
	55- 1	OD		20-10 - 71-1043 - -	Dry red with orange particles mod-highly indurated for scitty sand trace gravel (SM) (HB) Medium dense to very dense	DPC + Strang
12-	s-4	311 SoniC		NA _	Day red with reguse particles f-M scity sandwith gravel (SM) (HB)	(clusal at 11'3" Advanced with sonic to 13' DPC+ Strong



BORING NUMBER: BEP-13

Sheet:

SOIL BORING LOG

3/5

PROJECT: Rundal & Harry Terenhai

ELEVATION: 120

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Frask Molfi drill max 170; Shall Saupler WATER LEVELS:

START: FINISH: LOGGER: Mark Change

	6	AMPL	E	STANDARD		1 1912 Charles
(ft)	- 3	CHAIL P		PENETRATION	SOIL DESCRIPTION	COMMENTS
ОЕРТН ВЕLOW	NUMBER	TYPE	RECOVERY (#)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12 -	54	3" Sinil	2.5	NA -	Dry rid brown f-M silty sand with gravel (SM) (HB)	Sonic from 11/3"-13' DPL +t. Verystrong
13-	55-Z	211 00 5auplu	1.51	10-6 -	Tup 4" Dry red brown f- Mailtysand with gravel (SM)(HB)	DPC+ strong
<u> </u>			n in the second		bottly" Moist dark brown with purple and green f-M si'lty sund (GB) (SM)	DPC HT verystrong
15	11.7	211		36-100/3	Moist red brown f-M Silty Co.	DPC+ Strong
	55-3	Stupler	311		Moist red brown for M Silty Sund Moderate to higaly under ated (SM) (HB)	refusal @ 15'9"
16-	5-5	311 Son14	231	NA -	15'a"-17.6' Dry red Grown f-(sillly sand with grant (SM) (HB)	advanul OPC++ very strong
- - 77-						
1 20 20 20 20 20	55 ⁴	211 ODIV Sam	21	44-44 -	W.O-180 Moist Medium dense moderately inducated f-M silty sand, trace gravel	DPC H-very string
18-				•		



BORING NUMBER: BRP-13

Sheet:

SOIL BORING LOG

PROJECT: Dundalk Marin, Terminal

LOCATION: By Hymer, May land DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Fresh Mart I drill may 170; Sonic Saupler START: FINISH: 2:10pm LOGGER: Mark Chancy

€	s	AMPL	.E	STANDARD	SOIL DESCRIPTION	- Mark (Many
DEPTH BELOW (NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
/8 -	1 55-5	2" OP for sample	21	22-11 -	- 18.0-14.0 Moist red brown Med deusc f-Msiltygand, trace clay SM (GB)	OPC+strong
-		211 Sampler 211 Sampler		27	ol9-14.5' Moist and brown Med dose f-M Gitty Sand trace clay (SM)(GB) 19.5'-19.75' Dry Very dense and mattle with white Fat clay (Clt)	Sampled from 19-195' DPC- none
20					CHAI CIAY (CIT)	Borny terminated a
- 22-						1"PVC sleve installed 2 195'
- - - 23,-						Vwp installed & 18.5' thermister installed & 15' 6"OD casing to 19.5'
24 -						, , , , , , , , , , , , , , , , , , , ,

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET Dundalk Murine Terespinal Baltimore Maryland Sec location Plan FILE NO. **PROJECT SURFACE ELEV.** LOCATION DATUM **BORING LOCATION** Frust Multi drill may 170; Sonic Sampler **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED CASING USED TYPE OF BORING RIG **DURING CORING** MECHANICAL DIA., IN. **TRUCK** то DEPTH, FT. FROM DIA., IN. SKID **HYDRAULIC** DEPTH, FT. FROM OTHER DIA., IN. BARGE **OTHER** YES TYPE AND SIZE OF: **DRILLING MUD USED** DIAMETER OF ROTARY BIT, IN. **D-SAMPLER** TYPE OF DRILLING MUD U-SAMPLER S-SAMPLER YES CORE BARREL AUGER USED TYPE AND DIAMETER, IN. CORE BIT **DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION DEPTH OF HOLE DATE TIME SKETCH SHOWN ON PIEZOMETER INSTALLED LENGTH, FT. TOP ELEV. STANDPIPE: TYPE ID, IN. TIP ELEV. OD, IN. LENGTH, FT. INTAKE ELEMENT: TYPE BOT. ELEV. OD, IN. LENGTH, FT. FILTER: MATERIAL **PAY QUANTITIES** NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. DRY SAMPLE BORING LIN. FT. NO. OF 3" UNDISTURBED SAMPLES 3.5" DIA. U-SAMPLE BORING LIN. FT.

BORING NO.

DATE

BPP-13

DRILLER

REMARKS

CORE DRILLING IN ROCK

BORING CONTRACTOR

RESIDENT ENGINEER

LIN. FT.

Ywp & I thermister installed, graved in place



BORING NUMBER: BRAIN

Sheet: |

SOIL BORING LOG

PROJECT: Honeywell DMT SSD

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Aquifer drilling and testing, The-

DRILLING METHOD AND EQUIPMENT: Sonic Samp Drill Mare 170; Spr Sampling
WATER LEVELS:

START: 12/9/15 FINISH: LOGGER: K.chaturvedula

£	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (#)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
2	S-1	Sovic macra corr	uo"		3" Asphalt 0-15!-Silty SAND with graved (SM), gray-brown (2-54R 412), dry, medium to (oarse grained Sand	Ardvanced the hole from object using 3° PD soric macro Core. DPC-
	ςN	SONI C MPLEO CURE	36"		4.0'-5.6'-same as above 5.3'-clayey s AND (SC) with grave), gray-brown (7.54R4)z), wet, fine to medium grained sand 6.0'-Polyethylene liner	DPC-



BORING NUMBER: BRP-14

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

1 + M Sisampling your 1	- 							
Solve Silly sand with growth (can), Red Brown (134 & 4/2), dry, fine to medium DRC- Brown (134 & 4/2), dry, fine to medium DRC- Gastr (4/2), wet, fine to medium DRC- grained sand, trace grovel 7-81. 2" Asphalt 800'- Poorly graded GRANET with Sand Cop, dry, coaste grained sand mined with largen of crushed as phalt. 9 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	€	S	AMPL			SOIL DESCRIPTION	COMMENTS	
To'- clayey s ANT (SE), gray-brown (3-SYR 4/2), wet, fine to midlom grained sand, trace grovel 7-8'- 2" Asphalt 80'- Poorly graded GRANEL with Sand (GP), dry, cooke grained sond mixed with layen of Crushed asphalt. 100'- Foorly graded Sand sand (SP) with graves, gray (4-SYR 5/1), dry, medium grained Sand 105'- below to Switched for SI Sand (ON), Red (1074/4), moist, thre to medium grained Sand 102'- 112'- 131'- Fat CLAY (CH), Red (1074/4) most	DEPTH BELOW	NUMBER	ТҮРЕ		RESULTS	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL	RATE, DRILLING FLUID LOSS, TESTS AND	
10 25 No. 14 12 Stry SAND (SM), Red (10 Tyly) and Sand sand bpc- 10 25 No. 14 12 14 14 15 16 16 metion bpc- 10 25 No. 14 10 25 112 - Stry SAND (SM), Red (10 Tyly) and Sand bpc- 10 25 No. 16 12 - Stry SAND (SM), Red (10 Tyly) and Sand bpc- 10 25 No. 16 12 - 14 - Fat CAP (CH), Red (10 Tyly) and sand bpc-	ِ لَى - -	5-2	Moon		-	6:0'- 5ilty sand with gravel (SN), Red- Brown (154R41a), dry, fine to medium grained sand	DPC-	
10.25- below to switched for SIN SIN Sampler 10.75'- 11.2'- SIN SAMPLER 30-in. drop: 3° did Sampler grained Sand 10.25- below to switched for SI Sampler 30-in. drop: 3° did Sampler grained Sand 10.25- below to switched for SI Sampler 30-in. drop: 3° did Sampler grained Sand 10.25- below to switched for SI Sampler 30-in. drop: 3° did	8 -	5->	MACKO	28				
177 HB COPR encountered at	- - -	Z57 _{XV}	5	2\"	9 :	medium grained sand 10.75'-11-2'- Silky SAND (SM), Red (10 44/4), Moist, thre to medium grained Sand [1-2'-149'- Fat CLAY (CH), Red (1044).)	10:51- below 10' switched for SI sampling 11011.16 hamm 30-in. drop: 30 dia sampler DPC- moist pp	

ch2m:

PROJECT NUMBER:

BORING NUMBER: BRP-f = Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12-	らう	SONIC MACRO	\z`*	- - -	120'- SIL, Street (SN), Red-Brown. greenish (ASIR 9/3), dry, fine to medium grained (and Tarkiculate to moderately, induvated, bught green parkites (HB (050))	17:0'- Js refusal; Advanced the Borehale to 13'bg: coing 3°20 Paris macro som. Dye 4
13-	823	ζS	NR	50/40	NO Recovery	13.0° - SS refusal. Advanced the bordice to 15'b ga cum 3" ID some macro core.
14	s7	SOMIC MACE CORE	2 ¹ 1	-	Same as above	
15 -	s58	ss	2"	50/2"	same a above , wet	15:01-6" op outer casing installed to 15! bgs.
	S-9	long Long	23	-	158'-15-0'-Poorly gradul SAND with sitt SP-5M2, wet, medium to coarse grained Sand, particulate, bright green particle (GBCOPR) 16-0'-SiltyCANTO(SM), Rod-Browngreenish (2-51R 413), dry, fine to medium grain-ch Sand, particulate to mederated indurated (HBCOPR)	15:0'-SS refusal; Advanced the borchole to 17' bgs using 3' 2D sonic macrocon15:0'- ground wakes encountered at 15:0' bgs during drilling.
-	55-10	SS	611	22 - 56/2" -	1701-GB COPR layer; wet 173- HB COPR, moderately to highly Indurated, dry	
18-	5-11	Sonic Hacro			Same as above	SS refusal; Advanced the bornhole to 191 bgs using 3"ID sonic macro core

ch2m:

PROJECT NUMBER:

BORING NUMBER: BRP-14 Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

			-			LOGGER.
V (ft)	S	AMPL		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТУРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
-	Sall	rose ,	184			DPC-F
19 -	55-17		Чн	20/4"	19:01-Same as above, highly indurated	19.0'- SS refusal', some sample to 20' bgs
"	5,13	Meny Sovic	6"	-	19.5'- same as above.	DPC-1
20-	23-JM	Ç?	6"	۱ų - -	20.0'-lean CLAYCCUT, Red-white - Croyalar, dry, very SKPF, Low blasking	20.0'-6" or outer casing lastalled to 20'bg.s
				ار	Bottom of boring @ 20.5' bays Thermoneter Air temp: 12.6° Thermister Air temp: 17.9 Thermister Slong temp	1" PVC installed to 20. 1 Vwp installed at 19.5' Thermister installed at 1911 Interface of HBIGB CORR Vwp Air To 17.6 Po9773 depth T, 20.5 P.9672 Sloving T, P.
				_		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET DUNDALK MARINE TERMINAL FILE NO. **PROJECT** LOCATION SURFACE ELEV. BALTIMORE, MARYLAND BORING LOCATION DATUM SONIC SAMP DRILL MAY 170; SPT Sumpling **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED CASING USED TYPE OF BORING RIG **DURING CORING** NO DIA., IN. DEPTH, FT. FROM TO TRUCK MECHANICAL HYDRAULIC DIA., IN. DEPTH, FT. FROM TO SKID BARGE OTHER DIA., IN. DEPTH, FT. FROM TO OTHER tracis DRILLING MUD USED TYPE AND SIZE OF: YES D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER CORE BARREL AUGER USED CORE BIT TYPE AND DIAMETER, IN. **DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. SONIC MACRO CORP - 3" P.D SAMPLER HAMMER, LBS. 14 0 AVERAGE FALL, IN. 30 TYPE OF HAMMER Automak WATER LEVEL OBSERVATIONS IN BOREHOLE **DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER** CONDITIONS OF OBSERVATION 12915 15.2 10.01 encountered during drilling 11:40 120 vwp inetallation log **SKETCH SHOWN ON** PIEZOMETER INSTALLED NO STANOPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: BOT. ELEV. MATERIAL OD, IN, LENGTH, FT. **PAY QUANTITIES** 3.5" DIA. DRY SAMPLE BORING UN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING UN, FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK OTHER: UN. FT. Aquifer Drilling and testina **BORING CONTRACTOR** HELPERS DRILLER Brian Pholen Instrumentation Onstalled **REMARKS** DATE **RESIDENT ENGINEER** K. Chaturvedu 12/91

BORING NO. BRP-14



BORING NUMBER: BRP-16

Sheet:

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: EEE PLAN ... ELEVATION: ~ +13 DRILLING CONTRACTOR: SDS

WATER LEVELS: START: 1040 FINISH: 113115 LOGGER: L.LINCOLN

Œ	S	AMPL	-E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0	S. S.	SOME MACRO CORE	14"	A th	4" ABPHALT 4" CONCRETE BRN, DK GRY F-M SAND, SVM SILT, GVL (FILL)	NO DPC
	55-7	35	5"	7 4	2-3' RED BRN. DK GRY C-F SAND, SM GVL, TR SILT, PLANT FIBERS (FILL)	
	55-3	55		lo	3 3 2 GRY C SAND (FILL) 3.2-3.7 RED CLY, MOST WHITE	
+ =	Soul	SS	NR	4 5	TR GVL (4B)	DPC+
5	SSS	ss.	62,	13	Fig. E. of Fat ELAY (CH), Red (1044) highly plastic, soft to shift	WYNE DPC



BORING NUMBER: BRP-16

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH:

3	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	~		RY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
DEPTH	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -	55-6	55	12"	31	Silty SAND (SM), Reddish-Brown and grean Clove 4/4), moist, Particulate do to slightly Inducated (HB COPE)	DPC-1
7	557	ss	7"	17 2)	Same as above	
070	\$ 8	S	12"	20	8.81- silty sand (SH), dark gray . black (N2) . Wet , Porticulate (GBCD	8.0 : water encountered during drilling at 8 0' by s
9	55.9	55	6"	31	gol_ silly sanorsul, brown-greenesh gray (love 4/3), well for ficulate (core)	+7
	25-10		6"	35	same as about g.g Poorly graded samo with sills (sp) dark gray - black (No. Wes, Tarhenlah	SM)
10-	St.41	2.2	60		Same at Above	
-	85-17	55	611	33	samo as above	
11 -	g.13	55	6"	28	Silty SMID (SM), brown - great ich group Clove U/3), wet, Particulate (FOPE)	
12	55-14	S	631	13	same as above	



BORING NUMBER: BRP-14

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

SAMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS	
TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
SS	G"	11	Poorly graded SAND with SILI (SP.SM), doing grayand black (NZ), wet; Particulate (QB COPP)		
22	NR	13	No Recovery		
23	611	18 -	13-31-13-51 Sandy CLAY (CL),		
	SS TYPE	TYPE SS TYPE	PENETRATION TEST RESULTS 6"-6"-6" (N) SS NP 13	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY SS (4) PROPRIED SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY SS (4) Propried Graded SAND with SIA (SP. SM), do it grayant black (N2), wet; Particulate (GB EDP E) NO Reforeign	



BORING NUMBER: 889-17

Sheet:

SOIL BORING LOG

PROJECT: Horeywell DMT SST

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Aquifer drilling and testing the .

DRILLING METHOD AND EQUIPMENT: SOME SHALF DIVILL MAKE TO, SPT SAMPLING

WATER LEVELS:

START: 1915 FINISH: 113/15 LOGGER: Kichaduryedula

Œ	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	~		RY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
DEPIH	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
	s-1	SONI & MACROLORE	12"		4" Asphalt 0:3'-1:0'- corente 1:0'-3:0'- Alternating layers of Silty shows cons. Fray-black nove to dry and Asphalt.	Advanced the top 3' with the 3" 20 some macro corp
	التوع	55	₹ ^v	27	3:0'-3-4'- Poorly graded sano(st) tan-gray (10 yr 7/2), moist, Middium to coarse grained sand 34'- Well graded Geaver (with Sound and clay (Sp), Red and Black, 6 moist, fine In medium graded sand 40'- Poorly graded sano with graves (geotextile cheaunted so the spean,
4 4 4 4	\$1:3	83	120	17	medium to coarse growned sound. 4.3' - los CLAY (CH), Red (1044), most, highly plastic	
1	SEM	ÇS	7	24 21	Errown . Roddish - green (104R414), day, time to medium grained sand,	Hested the sample with dighteryl contracted. Installed the 6"00 out rasing to dapth of 5 bgs



BORING NUMBER: BRP-17

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	~		ERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
DEPTH	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6-	55.5	55	12"	24	Poorly graded SAND EWHY SILT (SP-SM), dark gray and black (ND), wet, particulate (GE COPR)	
7-	72-6	22	2"	50/211	And - Sorty Samon (Sh). Rod Brown and greenish (104R 4)4), day, fine to medium Sard, Portrodate (ing come	
8-	SCA	55	12"	50 100	same as about, wet, slightly to moderately indusated	est ground water into working all 201 togs during all 1654
9 -	55-8	55	F/F	100/4"	Sanceratedas No Preovery	Advanced the sonic money core 3"12 to 100 bgs
	59	S	6"		Some on ploone; day (55-27	. Also advanted the 6000 outer lasing down to 10' bgs
0-	52.10	65	6"	89	Same at above	
	55-11	3.2	NR	50/2"	tvo Pecovery	Advanced some macrocome to 10 0' legs
11-	55/12	SS	6"	53	come at above (25-11)	*
12-	s/B		60	UI .	same as above	



BORING NUMBER: 38 14

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY No Recovery Sample hereadded Sample hereadded Sample hereadded Sample hereadded Sample hereadded Sample hereadded Sample hereadded Sample hereadded Sample hereadded	(£)	S	SAMPLE		STANDARD	SOIL DESCRIPTION	COMMENTS	
Sample beganded Soll SS NR 10 No Recovery Sample beganded Soll SS G' 9 Slep Poorly ground same with silt (spring dark groung black charlest, misdly ground sand, Particulat, (GBC mi, dark groung black charlest, (GBC mi, dark groung black) (GBC mi, dark groung black misdly ground sand, Particulat, (GBC mi, dark groung black) (GBC mi, dark groung black misdly ground sand proving black (GBC mi, dark groung black misdly plack) (GBC mi, dark groung black misdly black misdly (GBC mi, dark groung black misdly misdly misdly (GBC mi, dark groung black misdly misd	DEPTH BELOW	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND	
Ssen Foorly gended sang with sile (sp. con, dark gray - black (not bear, medium grained sand, Particulate, (GB cope) 125-122 Same as above 18512-120 - Fall CLATCON, Bed Mortulate (bottom of Boring of 18 0 by 5	12-	314	\$	Pl.	18	Same as above, lott		
SSAR SS GU Y Solling of the control		5015	\$\$	NR	10	No Recovery	sample beyondolad	
SSAT SS GV Y 185'-183' Same as above 185'-183' Same as above 185'-183' Same as above 186'-183	3-	5-16	SS.	6"	9	ester Poorly ground some with sill (sp. ton, dark grany - black (N2) we medium grained sand, Particular	6	
	4	SS-A-74	53	Ş	Ч	(GB COPR) 1851-1891 Same as above 1851-1901 - Fall CLATCON, Red That wet highly plantic		
	5-1-1							
	6-							
	-							
	=							



BORING NUMBER: BRP-18

Sheet: 1

SOIL BORING LOG

PROJECT: Honeywell DMT SST

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Aquiter Drilling and testing Inc.

DRILLING METHOD AND EQUIPMENT: SOME SHIMP DIZLLE MAY 170; SPT_SAMPLING

WATER LEVELS:

START: 11/16/15

FINISH: 11/1/15

LOGGER: Kichaturiedula

3	S	AMPL	E.	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
-	ž	7	8	C 10 10 (N)		Advanced the between
7 -					5" As phalt sity sanut with gravel (SM), gray-brown (2.54R 4/4), moid, tint to medium ground Sand	0-10 using some meers love
1 1 1	5-1	WALES CORE	36"			
11111		SOFIET MA			20 - Inemase in gravel content	DPC-
1 1 1 1						
1					4.0'-5.0'- Same as above, dry	DPC -
	57	WALES CORE	36		sol- sitty some (sol , red (1074/u), moist, fine grained sand,	PPE -
-		SOMIC N			575 - sordy CLAY (CL), dark gray, brown (J-SYR 3/2), moist, SKFF	



BORING NUMBER: BRP-18

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(£)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)			RY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
	512				65' - same as above wet with gravel	DPC-
					7-01-7-751- same as above, pockets of cope material	DPC +1-
	\$3	SORIL MAJORD CORCE	36		7-75'. Poorly graded SAND with Silt (SP-SN), dark gray - blace (UZ), n bright green particles, (aB copp) particulate	noist DPC+
	-			9	10-01 - same as almore, wet	100' 6"00 outer casing advanced to 10' bgs
	- 54	SE	24"	18	- 11.0'-11.8' Same as above, bright green and dark grow	bret
2	-			26	gravet (SP), ten-Brown (104R 6/4), moist, medium grained cand	DPC+



BORING NUMBER: BRP-18

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

DRILLING CONTRACTOR:

5	SA	MPLE	-	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	rype	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
-	ž	-	œ	6 -	1200 - same as above trace gravel .	bpc -
	S\$-5	ئى	12"	7	12-5'- Poorly graded sond with clay (SP-SC), gray-Brown (1048614), moist, medium to coarse grained sond	
3 -	-SS-6	22	6"	9	same as above, wet	DPC-
	-SS-7	22	6"	11	gray Grown (1048 (14), moist, medium to coarse grained sand, thate day	Drc-
14.	· s-8	55	5"	7	- Same as above, increase in times content	DPC-
	-55-9	3.5	6"	15	- Same es above	DPC - 1501 - Advanced the 6" OD
15	351		150	WOH 5	- Poorly graded start with site (SP-SM), darkgray-black (NZ). Wet, Pockets of soft clay (GB COPE)	the second of the second
16	+				15-751- Taf CLAY (CH), gray (GLEY)	
1/0	55	يا دد	6	1 8	Poorly graded sand care (SP), dark gray (104 k 4/1), wet, Pockets of soft	A DEC-
	-		5 (8	- Same as above	ppc-
40	×	12 5	5 5	10	Same as above, contains rook	during drilling.
	+	AM 5	5	0/ 1/0	- same as above, contains roots, - trace clay	



BORING NUMBER: BRP-18

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ii)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
10 -	52-15	ss	24"	5 12 9	19.0'. Same as above, trace gravel, wet 19.5'-20.0' - Fatcher (ru), 7.04 (CIETI Sliot), wet, highly plastic, soft, seam of Red-Brown, clayey	PP = 1.5-2.5 fsf 20.0'- Advanced the 6" DD
21-	5.14	SOMIC WALROCORES	24		Sand. (LI" thick) 20-4 Poorly graded SAND with gravel (SP), gray (GLEY2 Shoy), wet, medium gravined Sand.	Duter casing to 200 bgs. 20'-22' collected an indistribed Picton Sample. - No Recovery in the Sampler Pressure increased to asso the Sample extended but no recovery. - A sonic make core sample collected between 20'-12' 6
23.	SKR	Ş	Sta	A (0	22.0'- Poorly graded SAND (SP), gray (aley show), wet, medium grainted sand 23.0'- same as above, Red-brow (3.54R 6/8)	n-Yellow
24.]			14		



BORING NUMBER: BRP-18

Sheet: 5

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(£)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER		RY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
		TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
1-				10	same as above	
	9518	55	"	8 -		
-			24"	1 -	460	
				5	25-5'- seam of clay, & Nown-Red	
				-	- tone tan-aran	
_			-		25.75' - Jame as above, tan-gray 26.0' - Same as above, Reddish	
			-	Mott	Brown- Lellon.	-
	55-19	55	245	Man	*	
				1 7	270'- Fat CLAY with Sand (CH), gray (GLEYZ 5/104), wet, medium	
				,	Hasticity	-
				1		
				1	Same as above	
	· N	53	244	3		
1-	5	2	1			
	1			3	29.25' - same as above, trace sand	(0)
				4		lasing to 30 bgs.



BORING NUMBER: BRP-18

Sheet: 6

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	рертн весом	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
D -				-	Top- Fat CLAY CCU, gray GLEY Stroy wet, highly plastic,	PP: 0.9+st Piston tube sample collecte	
	PTZ	PISTON TUBE	24"	-		between 30-32.	
32-		-		Moti	Bottom - same as above 32.0-32.5- Sandy clay (CU), - gray(cueris(104), web, solt	pr. 0.5-0.644	
3-	5520	55	54,	7	(GLEVI Strot), wet highly plastic,		
4-				2	34.0-35.5'- Sandy CLAY (CL), gray (GLEY 15/104), wet, soft.		
35.	51-12	ري ري	24"	3			
36.				ч	35:5'- Fat (LAY (CH), gray (GUEY) 5/104), wet, firm, highly plashir.		



BORING NUMBER: BEP-18

Sheet: 7

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
1-	PYZZY	PISTON TUBE	NR	-	NO Recovery	the sampler could not be retrieved from the hale.
309					Bottom of borng @ 38 o' 1gs.	



BORING NUMBER: BRP-20

Sheet: 1

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION: ~ +26

DRILLING CONTRACTOR: ATT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: ~ +U

START: (645

FINISH: 1030 LOGGER: L.LINCOLN

11/12/15 11/14/15

. €		SAMPI	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
3			€	PENETRATION TEST		O MINICITI O
DEPTH BELOW	NUMBER	TYPE	RECOVERY (RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0				-	4" ASPHALT CONCRETE GRY BRN GVLY C'M SAND (ROADBASE	DPC-
2	-55	CORE	3	N/A	2.5-4.0 BRN, DK GRY SILTY F.M SAND, SM GM (FILD)	FABRIC GEOTEXTILE ~ 2-5' TOPC-
3 -		3" SONIC		-	4-7 BRN, RED BRN, DK GRN CLAYEY FM SAND, SM BN	
5	52		3'	H/A	Jan on	



100 1/12/15

PROJECT NUMBER:

BORING NUMBER: BRP. Zo

Sheet: 2

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +26 DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XLMAX 170

WATER LEVELS: ~ +6 START: 1645 FINISH: 1030 LOGGER: L. LIN COLN

€		SAMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	IYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6-	s-2 cont		CE.	N/R		
8 9 9	5.3	S" SONIC	33	7/4	7-7.4 DO S.Z 7.4-9 GRY, BLUE: GRY C.F SAND, SM. GN. (RUAD BASE AGGREGATE) 9.9.5 RED SILTY F SAND 9.5-10 TAM, DK GRY SILTY F.M. SAND, SM. GVL (COPR)	DPC+ 4" CASING TO 10.
" -	55. ⁴	2" 0.D. 5.5,	24"	28 - 52 -	10-11.5 STRONGLY INDURATED BRN, DK GRY, TAM SILTY P.M SAND, SM QV (HB) 11.5-12 BLK, GREEN M-F SAND, WEAKLY-MOD, INDURATED (GB)	DPC+



BORING NUMBER: BRP. 20

Sheet: 3

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAM

ELEVATION: ~ + 26

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: ~ +6

START: 1645 FINISH: 1030 LOGGER: L.LIN COLN

	1	SAMPI	_	STANDARD	L.LINCOLIN	
= =====================================	3	AWIP	-	PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12	- - - -	2" 0.D. 5.S.		34	12-14' BIK, GREEN M.F SAND, WEAKLY MOD. INDURATED (GB)	DPC+
13 -	55.5		21"	47 51		
14 -	55.7	3" SONJIC	v	42 50/2"	14.14.3 DO SE 5 (GB) 14.3.14.0 DK BRN, RED, DK GRY SICH FM SAND, TR. GM. STRONGLY INDURATED (HB) 14.0-15 DO SS-U (HB) 15.15.0 BRN, TAN, YELLOW BUTY FM SAND SM GML (HB) 15.0.10.0 MOD. INDURATED BLK SILTY FM SAND (GB)	DPC+ DPC+ DPC+
17 -	ઽકઃપૈ	2° 0.0, 6.5.	[] la	15	GREEN, TAN F.M SAND, SM SILT, GM, MOD-INDURATED (HB/GB MIX) 17.4' RUBBER GEOSINTHETIC	= N.



BORING NUMBER: BRP-20

Sheet: 4

SOIL BORING LOG

PROJECT: DUNDAUK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION: ~ +ZU

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRAGTE MINITIDEIUL XI MAY 170

WATER LEVELS: ~ + 6

START: 1445 11/12/15

113/15

FINISH: 1030 LOGGER: L.LINCOLN

€	S	SAMPLE		STANDARD	SOIL DESCRIPTION	COMMENTS
) N			£	PENETRATION	COLDEGUAL FION	COMMENTS
Ĭ				RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
DEPTH BELOW	NUMBER	TYPE	RECOVERY	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18	- 55-A	2" 0.17 5.5	3"		18-18.5 DO SS-9 (HB/GB)	4" CASING TO
- 101-	55-10		4"		TANIDE GRY SILTY FIN SAND,	DPC1
	- 551 ¹¹		6"	27	19. 19.5 MOD INDURATED RED. BRN, DK GRY, GREEN SILTY FM SAND, TR GYL (HB)	
	55.12			-	19.5. 19.9 DO SS 11 (HB)	DPC+
20 -	25.		6"	35 -	19 9- 20 BLK M F SAND, TR YELLOW 20-20-2 DK REN PARTICLES (GB) -	DPC1
	55·13		6"	35	20.20.2 DK BRN, DK GRY SILTY F.M. 20.2.2.20.3 BLK, GREEN M. F SAND, MAD. INDURATED (GB)	
21 -	SS:H		4"	25	SILTY FM SAND TR GNL, LELLOW/ GREEN PARTICLES /MD. LELLOW/	DPC1 SAMPLE @
1	55.15		(₀ "	22	WEAKLY INDUKATED, MOIST (AB)	11" CASINIG TO
-					21,5-21,7 DO SS 15 (AB)	DPC+ 20'
22-	55.1V		6"	30 -	21.7- ZZ BRN C-M SAND, SATURATED	WHETHER DUE TO
-				22 -	22-23 DO SS-16	CONTAMINATION IN GW
-	SS M		12"	26	· ·	
23-				-	-	
-				51	23-24 TAN, BRN C.M SAND, SATURATED	U
-	5516			-		
24-				12	,	



BORING NUMBER: BRP.20

Sheet: 5

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ 126

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: ~ + 6

START: 1045 FINISH: 1030 LOGGER: L.LINCOLN

	- 1	SAMP	1 =	STANDARD	1/12/16 1/14/16			
_ B	5		-	PENETRATION	SOIL DESCRIPTION	COMMENTS		
S ind broad	NUMBER	ТУРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION		
24	55.	2"	· q"	77 - 50/3" -	24-24-8 BEN CM SAND, SATURATED	DPC+ - UNCLEAR IF REACTING TO CONTAMINATED WATER		
25		3" SOULC			24.8.2L BRN, TAN CM SAND, TR. GVL, SATURATED	4" CASING TO 25'		
	5.78		15"	N/A				
24	-	2.0.0.	=	19 -	26.27.5 BRN, TAN CF SAND,			
27	652	5.5		21	27.5.28 TAN, GRY IN-F SAND,	DPCT -> UNICLEAR IF REACTING TO CONTAMINATED WATER DPC-		
15	-				20-28.7 BRN CIM SAND, SATURATED	DPC+		
29.	55-12	2,		71 -	SLTY FIM SAND, SAT.	DPC- 4" CASING TD 30"		
<i>79</i> -	>			70 -	1	27		



BORING NUMBER: BER ZO

Sheet:

SOIL BORING LOG

PROJECT: DUNDAUK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION: ~ +26

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

	-	SAMP	S: _{~ +}	STANDARD	START: 1645 FINISH: 1030	LOGGER: L. LINCOLN
W (ft		AWIP		PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
200		2" 0.D. 5.S.		lu -	30-30.5 BRN GVLY M.F SAND, MOIST	DPC-
3	55.23	24	-400	13 -	SAND, MOIST	DPC-
		To a	24"	12	31.5-32 TAN M-F SAND, SATURATED	DPC-
32			-	25	M-C SAND, EATURATED	
33 -	55:24	Tr	24'	2u - 42 -		
ا ا	_			51 -	33.7-40 BRN MI-FSAND, SM SILT, MOIST	DPC-
-				(0 -	M. F SAND, TR. GVL, MUST	**************************************
5 -	SS 25	33 38		 	e: 100 m	4
30			\$210	- 18 -	35.7-34 DK GRY CLAYEY F.M	20



BORING NUMBER: BRP-20

Sheet: 7

SOIL BORING LOG

PROJECT: DUNDACK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ 4 26

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: ~ -+-

200 /13/15 START: 1045 FINISH: 1030 LOGGER: 1. 1-INCOLN

	2 70				1/12/15 MAII 1030	LOGGER: L. LINCOLA
E	5	SAMP	_	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
34		2.1 0.D. 5.S.		Wu	36.37.5 DO BOT SS.25	
31-	95.7h	L	24"	\ -		
-				3 - 4 -	37.5-38 DK GRY CLAY, MOTH	PP = 1.0
39-	U-21	PISTON	0"	P=24" R=0"	NO RECOVERY IN TUBE, SONIC TO 40'- DK GRY CLAY, TR SILT, V. SOFT	PISTON SAMPLER 38 40'
40		NRBED		11	**	
41		VNDIS	13.5	P:24" R:13.5"	DO SS-24 BOT	a ×
42	J - 28					



BORING NUMBER: BRP- 20

Sheet: 8

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +26

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MIULTIDRILL XL MAY 170

WATER LEVELS: START: 1045 LOGGER: L.LINCOLN FINISH: 1080 11/12/15 11/14/15 SAMPLE STANDARD Ξ SOIL DESCRIPTION COMMENTS PENETRATION DEPTH BELOW **TEST** RESULTS RECOVERY SOIL NAME, USCS GROUP SYMBOL, DEPTH OF CASING, DRILLING NUMBER COLOR, MOISTURE CONTENT, RELATIVE RATE, DRILLING FLUID LOSS, TYPE DENSITY OR CONSISTENCY, SOIL **TESTS AND** 6" - 6" - 6" (N) STRUCTURE, MINERALOGY INTRUMENTATION 42-PO 55.26 BOT V. SOFT DK GRY SAMPLER CLAY P= 24" R: 7.5" PISTON SAN 7.5" 43 44 EOB@44'

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400

w.	ww.mrce.co	om				BORING NO.	BRP-20	100
						SHEET	9 OF	11 "
PROJECT	<u> D</u>	INDALK	MARIN	E TERM	INAL	and the contract of the contra	587	
LOCATION			E MARY	LAND		SURFACE ELEV.	4 + 26	1172
BORING LO	CATION	<u> 555</u>	PLAN	and the second	The second second	DATUM	to Mile	81
SHILLING	CTION EQU							
			XIII XIII XX			The property of		Station
BORING EQ	UIPMENT A		F STABILIZING BO	REHOLE				
DWC OC 808	MC DIC	TYPE OF FEE			Annual Property			
TYPE OF BOR	ING KIG	DURING CO		CASING US	The state of the s	YES	NO	4.
TRUCK	-025.23	MECHANICA		DIA., IN.	4	DEPTH, FT. FROM		0 40
SKID .	/	HYDRAULIC		DIA., IN.		DEPTH, FT. FROM		0
BARGE		OTHER		DIA., IN.	Y	OEPTH, FT. FROM	1.,	0
OTHER		34						
TYPE AND S	IZE OE-			DBIII (AIC A	11011550	[
D-SAMPLER		D.D. SPLAT	· · · · · · · · · · · · · · · · · · ·	DRILLING M		YES	NO	
U-SAMPLER		STON	SHUN		OF ROTARY BIT, IN. ILLING MUD	_	200	- 50,1
S-SAMPLER	r Ic	SIDING	5 h	TIFEOFOR	ICTIAG IAIOD	-	At a second	E
CORE BARREI	2."	SOLIC		AUGER USE		YES	NO	
CORE BIT	-	SOKIO		1.70	HAMETER, IN.		_ V _ NO	
DRILL RODS	-			THEMIDE	MONTH CETT, DV.		2000	
				CASING HA	MMFR. LRS.	AVERA	GE FALL, IN.	
					AMMER, LBS.		GE FALL, IN.	30
		, KE		TYPE OF HA		AUTOM AT	- 1 A A A A	80
				11		AU JUIOI A		20
WATER LEV	EL OBSERVA	ATIONS IN BORE	HOLE					
50	E 10	Tu lla				1200		
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER		CONDITIONS OF	OBSERVATION	
500						10		
	100	2-11	11 252 50	1000000			<u> </u>	115
		200 E2 THE				78		1 "" (1
2	- 11			177 (0)		1 10 20	<u> </u>	
			3011			-1 n ==		<u> </u>
11,		92			E IIG	100 256 0	7 24	<u> </u>
	0.2				22 3			
PIEZOMETE	R INSTALLEI		YES	NO SKET	CH SHOWN ON			
	111	. 111		INO SKEI	CH SHOWN ON			
TANDPIPE:		TYPE	VWPS	ID, IN.	I FN	STH, FT.	TOP ELEV.	
NTAKE ELEM		TYPE		OD, IN.		этн, гт.	TIP ELEV.	es The
ILTER:		MATERIAL		OD, IN.		TH, FT.	BOT. ELEV	
	,						BOT. ELEV.	1
AY QUANT	ITIES							
	SAMPLE BOR	ING	UN. FT.		NO. OF 3" SHELBY	THE CAMBIEC		
	MPLE BORIN		UN. FT.		NO. OF 3" UNDIST			
ORE DRILLIN		20	UN. FT.		OTHER:	OUDED SHALLES		
							i	
ORING COL	NTRACTOR	.Δ.	QUIFER	DRILLIA	IG ALI	> TESTIN	6	
RILLER	-17		OMEQUE		HELPERS	DYLAN	JEWELL	110
EMARKS						12714	200000	
ESIDENT EI	NGINEER	WKA	NDRA LI	NCON		DATE	11/14/	15
						7.0		
	7.1							

BORING NO.



BORING NUMBER: BRP-21

Sheet: |

SOIL BORING LOG

PROJECT: Horseput II DAT SIT

LOCATION:

DRILLING CONTRACTOR: Aquifer Drilling and testing inc.
DRILLING METHOD AND EQUIPMENT: Sonic Samp drill max 170; SPI Sampling

WATER LEVELS:

START: 11 14 15 FINISH: 11/14/15 LOGGER: Kehaturvedula

3	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (R)	w.		ERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
DEPTH	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
	5-1	SONIT MARCED CARE	30		u" Asphalt Poorly graded some with gravel (SP), grow (CLEY2 7/104), dry, fire to medium growed sond, layor of Isphald I.S Silfy share with gravel (SM) door brown - grow (254 Sfy), dry, fire to me dium grained sand,	Advanced the hale from o'-10 with sonic moiro core 3'10 DPC.
	şa	SONIC WASEA	36		4.0'-6.0'- Same as above, moist 5.0'-5.0'- tower fines content	DPC-
1		9		3	30'- Polyethulane layer	BPC -



BORING NUMBER: PR (2)

Sheet: >

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

LOGGER:

611	S	AMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEL IN BELOW (II)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
1 1 1 1	5-7				6.0'- Poorly graded Sand with graves (SP), dart gray-black (N2), dry, Medium to Coarse grained Sand, Road based aggregate	DPC -
1 1 1 1					7.0'- 9.5'- Same as above	DP+-
A STATE OF THE PARTY OF THE PARTY.	5/2	SOWIC MATRO CORE	36		9.5°- S. Hyspinoist, medium grained (2.5484), moist, medium grained Sand	DPC -
1 1 1 1 1 1 1 1 1 1	gg 4	ø	ð _{n,}	71 12	10.0 - 10.25 - Same as above, gray-light red, higher finer context 10.75 - 11.5' - Same as above, light red-yellow(2.54R 618) 11.5! Fatelat (CH), Red-white (104416), moist, very sett, highly plastic	100'- Advanced 6"00 outer Casing to 10'bgs. 105'- Below 1 8' switched to 55 sampling 1,110-16 hamme 34th drop; 2"diansamples DPC-



BORING NUMBER: BRP-21

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH:

LOGGER:

(11)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (R)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
2	555	22	20"	14	12-0'-13-0' same as above	bpc-	
4 1 1 1 1			Žů.	50/2	130' - Silty Strice (SM) - Reddish Brown - arrenish (DITTE 4/3), dry. particulate to slightly indurated (UB cork)	DPC +	
15		LORGE MACRO	IZ,	-	same as above	Advanced to 15' bgs using 3"79 some marrianes beca	
	-			12	15:0'-15:75' - Same as above	15.0' - Advanced the co	
	55.7	25	12,	11-	15:25 -16.0 - Fet CLAY (CAI), Red (1074/4), moist, highly plates	DPC -	
					Bottom of boring @ 16.0 bys		



BORING NUMBER: BRP 23

Sheet: ////

SOIL BORING LOG

PROJECT: Oundalk Marine Terminal

LOCATION: Boltimore Maryland

ELEVATION: 126 DRILLING CONTRACTOR.

DRILLING METHOD AND EQUIPMENT: Frest South Somp Prill Max 170; SPT sampling

START: THE FINISH: LOGGER: Mak Chancy

					12-14-15 2:40 FINISH:	LOGGER: Mak Chancy
 2€	- 5	SAMP	T	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 - -	5-	31. Sonic	3'	NA -	Asphalt/Road base Well graded gravel	Estimated depth: 44' DPC-none
- / - - -			÷	- - - -		
2 -				- - - - - - -		
3 -						
4 - 5	5-2 5	311. SourC	3'	NA _ 5	to-65 Dry brown with white, red and pranse particles form sitty send trace grave 1(5M)	Pomen brune & 4'6"
5-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1						



BORING NUMBER: BRP 23

Sheet: 2/11

SOIL BORING LOG

PROJECT: Oundal's Marine Terminal

LOCATION: Baltmore, Maryland

DRILLING METHOD AND EQUIPMENT: Frest Soni Sampling Drill Max 170; SPT sampling START: FINISH: LOGGER: Mark Change

€	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	~		:RY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
DEPTH	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -	5-2	311 SoulL	31	NA -		
					6.5-7.0 Dry grayish blue poorly graded gravel with f-l sand	gromembrane & 6.5' PPC-none
7-	5-3	311 SONIL	31	NA _	7.0-7.5 same as above	DPC-none
- 				- -	7.5-10 Dry Brawn with blue f-Msand With bits of renerate	DPC-none
	,		•	· -		
9 -				-		
- -						•
10 -	45-1	311 OD Sampler		1b-17 25-12	10-10.5 Same as above	DPC-none
-				-	10'5"-10'10" Dry medium brown f-l sand trace grant	DPC-none
(1)				- - -	10'6' 11'6" Dry hadison 2nd f-14 Silly sund	Ofc-nanc
12_				- -	11'9"-12' Dry Blackf- (Sandwith gravel 11'9"-12' Dry medium fed (lay(CL)	PPC- Hone PP 1.25

Ch2m:

PROJECT NUMBER: 10587

BORING NUMBER: BRP-23

Sheet: 3///

SOIL BORING LOG

PROJECT: Dundall Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: 4+26

DRILLING METHOD AND EQUIPMENT: Frust Mit Dail Max 170, Sonic Sampling LOGGER: Mark Change

(£)	s	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12 -	55-Z	Biom Salple	9"	42-50/1 -	12-12'7" Red mottled with white Hard clay(CH)	DR-none
						refusal@ 12'7"
 /3	5-4	311 SUN!	21	NH -	127"-13' Dry (1d brown f-1 silty sand (SM) (HB)	OPE + strong.
7					13-14 Bluck f-Cs and with gravel	DRd strong
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\						
	543	saufur BD	27 ¹¹	11-18 <u> </u>	14-15 Dry sed brown moderally industi f-bn silty send trace graves (517)(++B)	PPC+ stong
15-				-		
-				-	15-11 Drymedium Red brown with black and series particles f- M silty sand (SM) (GB)	Ofet Stong
16-					16-17 Dry loose Red Whotled with white	Doi
	55-4	31' Sunfly	1'	1-6 - -		PP1.25 DR-hone
101						
	55-5	300	15"	7-37	17-18 Dry loose to dense Red brown Worange for Sandy Clay, trace SIIt (CL)	DPC-none
18-						PP-2.25



BORING NUMBER: BLP-23

Sheet:

SOIL BORING LOG

PROJECT: Oundalk Marcheterminal ELEVATION: ~ 126

DRILLING CONTRACTOR: ADT

W (ft)	£ SAN			STANDARD PENETRATION TEST	DRILLING CONTRACTOR: ADT NT: Frost M. H. Drill Max 170 Con Conplex START: FINISH: SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (#)	RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
8 - 	55-1	300 somple		25-27	16.0-18'2" Dry andian cell train for the Sandy (Clay (CL) 18'2" 19' Dry widing Blell with arease for yven for tides for M silly; and (SM) (GB)	PPC-nonc DPC+ Strong
/9 - - -	55-7	311 OD Sample	-1		19.0-20.0 December Black	APC+ strong
20 — - -	55-8	311 00 5amph	Mo Pic	2-6 5-10		
21 -				-		
22-	559	311 00 5arple	21			DPC + Shons Enlainfield Water
23-5	510	300 sample	21	20-75	17 410 >111	DPC+j- DPC+j-
24						



BORING NUMBER: BRP

Sheet: 5///

SOIL BORING LOG

PROJECT: Dundolly Marine Terminal LOCATION: Baltimore, Maryland ELEVATION: ~ +26 DRILLING CONTRACTOR: ADT DRILLING METHOD AND EQUIPMENT: Frast Mort Dail Max 170; Sanic Sampling WATER LEVELS: START: 12-14-15 FINISH: 50pm LOGGER: Maill Change

	1 6	SAMP		STANDARD	127975	Maill Chavey
> ±	<u> </u>	AIVIP	T	PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
24-	5510	300 Sample	21	75-50/2"	24-24'1" maist mid Stalk wygeen particles f-M Silty Sand (SM)(GB) 24'1"-24'8" Moist Very dense brown f-C Sand	OR+
-				-	trace silt	OPC-none
25-	<u> </u>	1				refusal 2248" advenced with somic to 26' Holfed work at 5pm 12-14-15
	55-11	311 OD Sample	ر پ	46-63 -	25.0-26'3" Wet dense to very dense light brown f-L Sand trave gravel	Started 5-5 at 8,20 am 12-15-15
-			26"	50 3" - -	DR+ shong	Cleaned and hole from 24'8"-25 Smobtingrame
,]						
-	55	Sinil	4	NA .	26'3"-27 moist light brownf-csand trace smooth gravel transitioning to f-Csandy gray Silt	refusal at 26'3" advanced with some to 28' PPC - none
27- -				- <u> </u>	27-28 muist gray clayey filt (ML)	DPC-none
28				4		
4)-l'			-		Shell she of asing 25'
29				-		Shelly type scape from 28'-30'
				-		24" penetration, 30" tube
30-				-	i de la companya de la companya de la companya de la companya de la companya de la companya de la companya de	F. Both



BORING NUMBER: BRP-23

Sheet:

SOIL BORING LOG

PROJECT: Dundalk Marin: Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: \sim †26

DRILLING METHOD AND EQUIPMENT: Fraste Multidail Max 170; Sonic Sampling WATER LEVELS: START: FINISH: LOGGER: Mark Change

	T 6	SAMP		STANDARD		- Mark Many
- · Ĕ	-	PANIE		PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (#)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
30-	4	30 pe	11	12-18 -	30-31 Wet medium light brown f-C sand	Sample Saturated
-	55-12,	sample		-	(S)	OPC-hone
31 -		711		-		
-	<i>55</i> -13	311 00 54mple	21	26-28	31-33 wet medium light brown f-C sand (S)	DPL-hone
31-				- - - -		
-	55-14	311 Cample	1.5	33-34 - 45-31 -	33-35 wet Dense light brown f-c send (S)	2' recovered in spoon top 6'wash DPC-none
34 -				7-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1		
35 -	15	311 Sample	21	24-19	35-36 Same as 55-14	DPC-nonc



10587

BORING NUMBER: BRP-23

Sheet: 7///

SOIL BORING LOG

PROJECT: Dund, 1/ Maring Terminal ELEVATION: ~+26

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Frysk Multidail Max 170, Sourc Sampling WATER LEVELS: START: FINISH:

LOGGER: Made Chang

	Т.	SAMPI	<u>-</u>	STANDARD		LOGGER: Mark Chancy
. (fr		AIVIP	T	PENETRATION TEST	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТУРЕ	RECOVERY (ft)	RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
36 -	¥15	311 00 Sample	2'	21-24 -	36-37 moist medium light brown f-(send (S)	DPC-none
37-	- - - - - - - - - - - - - - - - - - -	3:1 OD Seewifte	2'	1-1 8-11	37-38.5 maist very lowse to loose brown f-m squal (6)	DPC-none
38-				-		
39-	V-2			-	38.5-39 moist loose sed brown f-m sand : trace silt (3M)	pre-none advanced cosms to 391 before advancing shelby tube from 39-41'
40-						Renchation = 24" 11 covery = 27" 30" shelby
4/ -	V-3					T4 1 3.5"
42-				-		PP= 0.25



BORING NUMBER: BEP-23

Sheet: 8///

SOIL BORING LOG

PROJECT: Dundally Maying Terminal

LOCATION: Baltman, Maryland

ELEVATION: ~ 126

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Fosk Multipli may 170 Some Sampling WATER LEVELS:

START: FINISH: 1:30 START:

LOGGER: Mark Chang

	T ,			T 071110100	12-15-15	LOGGER: Mark Chang
(F)		SAMPL	T	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
42	VB			F-L→ 7	Vwe 1/534629 - 50' Air To196 Pose Installed at 43' depth 2) 1517395 - 30' Air To197 Pogs Installed at 24' depth thermister 1) 50' Installed at 15' depth Air 18. Thermometer Air temp - 15,6	719 T. 19.0 P. 9522 Sluny T. # 7828480

14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET Dundalk Marine Terminal FILE NO. **PROJECT** Baltimore, Maryland **SURFACE ELEV.** LOCATION DATUM **BORING LOCATION** Sonic Samp Orill Max 170; SPT Sampling **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED **DURING CORING** CASING USED TYPE OF BORING RIG DEPTH, FT. FROM TRUCK **MECHANICAL** DIA., IN. DEPTH, FT. FROM SKID HYDRAULIC DIA., IN. BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER TYPE AND SIZE OF: YES DRILLING MUD USED D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD SS-Sampler; 3" dia Sampler S-SAMPLER **CORE BARREL** YES AUGER USED CORE BIT TYPE AND DIAMETER, IN. **DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. 3'ID Spric Macrococc 140 AVERAGE FALL, IN. 3<u>a</u> SAMPLER HAMMER, LBS. TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE | DEPTH OF CASING | DEPTH TO WATER CONDITIONS OF OBSERVATION PIEZOMETER INSTALLED SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. TIP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. FILTER: MATERIAL BOT. ELEV. OD, IN. LENGTH, FT. **PAY QUANTITIES** 3.5" DIA. DRY SAMPLE BORING LIN. FT. NO. OF 3" SHELBY TUBE SAMPLES LIN. FT. 3.5" DIA. U-SAMPLE BORING NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK LIN. FT. OTHER: **BORING CONTRACTOR** Chris Phelan DRILLER 1 thermister 50 **REMARKS** RESIDENT ENGINEER DATE **BORING NO.**

Mueser Rutledge Consulting Engineers

BOR-4_JAN2013



BORING NUMBER: SRP 25

Sheet:

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION: ~ +26

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MIVETIDEILL XL MAX 170

WATER LEVELS:

START: 1230 FINISH: 0930 LOGGER: LINCOLN

3	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
11111					4" ASPHALT GRY, BRN GVLY C F SAND (FILL)	DPC-
	51		4'	NA	Z-3.7 DK BRN SILTY F. M - SAND; SM GVL (FILL)	DPC:
		S. SONIC			3:7.4 LT BRN M.C SAND, TR GVL (FILL) 4-5:7 BRN, GRY SILTY FM SAND, SM GVL (GILL)	DPC-
11111	5-2		3	N/A	5.7-4 LT BRN, TAN M.C SAND, SM GVL (FILL)	DPC-



BORING NUMBER: BRP - 25

Sheet: 2

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION: 4726

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAY 170

WATER LEVELS:

START: 1230

11/15/15

FINISH: 0930 LOGGER: L LINCOLN

3	5	SAMP	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	R		RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING
ОЕРТН	NUMBER	TYPE	RECOV	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
1 1 1 1	S-2 COUT		3'	NA	(FILL) GULY C.F.SAND	DPC-
1111111	2	3" SONIC	Z.6		7-7,5 GRY C F SAND, SM GVL (FILL) 7.5.9.0 GRY GREEN CF SAND, SM GVL (FILL)	DPC
111111	5.3			12/4	9-10 RED-BRN, BRN, GRY SILTY F.M SAND, SM GVL (FILL	DPC-
	55.4	2" 0.D 5.5	24	24 157 57	10-16.5 GRY SILTY F.M. SAND, TR GYL (FILL) 10.15.11 DX BRN. GRY GVLY CIF SAND, TR SILT (COPR) 11-12 RED. BRN. BRY, GREEN SILTY F.M. SAND, TR. GYL MUD. INDUR ATED (HB)	GEDTEXTILE @ 10.0' DPC+ DPC+



BORING NUMBER: BRP-25

Sheet: 3

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMUNAL LOCATION: SEE PLAN

ELEVATION: 4-26 DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASE MULTIPELL XL MAK 170

WATER LEVELS: START: 1230 FINISH: 0930 LOGGER: L LINCOLN

DEPTH BELOW (ft)	S	AMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12 -	55.5	2"P 5.5	4"	50/4"	12.12 21 : EL EKN, DE BRY SICTY F.M. SATID, TR BYL (HB) 12.2-12.3 TAN M FSAND (FILL)	DPC 1
3-	SU	S" SONIC	12"	N/A	12.3. IH ERN, DK BRY F.C SOND, SM SILT, GIVL STRONGLY INDURATED	DPC+
11111		2" 5.5		13 -	14.14.8 TAN, YELLOW, BRN SILTY F.M. SAND, TR GIVL	DPC+
-	55.7		201	9 -	M SAND	DPC-
1 1 1				13 -	15.5-15.8 GRY, GREEN, BRN SILTY F.M SAND, TR GYL 15.8-14 YELLOW-LT. BRN M. SAND	DPC-
-				17	IV-16.3 BRN, BRY, TAN SILTY FIM SAND IV-3-16.8 YELLON- LT BEN M SAND	DPC-
-	550		24"	23 -	16.8-17.3 ELK, BREEN M-FSAND WEAKLY INDIRATED	DPC+
-	300			1	SAND, SM CILT, MOD. INDURATED	DPCT
1				40 -	WEAKLY INDURATED (GB)	DPC+



BORING NUMBER: BRP 25

Sheet: 4

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION: ~+26

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS:

11/15/15

START: 1280 FINISH: 0930 LOGGER: LILINCOLN

(ft)	S	AMP	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18 — -		2" 0.D. 5.S		51	18-19:3 BLK, GREEN IN-FSAND	DPC1
19 -	45		24"	74		
-				78.	19.3 20 LT BRN SILT, SM F SAND, TR WUOD FIRERS	DPC-
0-				52	20.21.4 DK 61RY, BRN WAY	
, -	5510		24"	72 -		
,]				84	SILT, TR F SAND	
2 -				15	22 22.9 DK BRN, ARY SILTY FM SAND, MOD. INDURATED (HB)	DPC+ WATER APPROX.
3-			24.	23	72 8-24 TAN, GRY M SAND, TR. SILT	DPC-
-				2니 -		



BORING NUMBER: 8R.P. 25

Sheet: 5

SOIL BORING LOG

PROJECT: DUNDALIE MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +26 DRILLING CONTRACTOR: ACT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS:

START: 1230 FINISH: 0930 LOGGER: L LINCOLM

(2)	S	AMP	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
24 -		2" U.D. S.S		17	24. ZU TAM, ORANGE CLAYEY M-F SAMD	DPC-
25	55-12		24"	25		
24 -				22	NR	DRILLED THROUGH WHEN ADVANCED CASING
8		2°, D. 5.5		27	28-28.8 DE BRN, GRY, YELLOW SILTY FIM SAND	DPC+
7	5.13		24.	27 _	28.8.30 Do 55 12	DPC-
30 -				24 -		



BORING NUMBER: BEP 25

Sheet:

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +26

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS:

START: 1230 FINISH: 0930 LOGGER: L. LINICOLN

	3		AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
	DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
3	0 -		2" 0.D. 5.5		30	30-31.7 TAN, GRY, ORANGE CLAY, SM F-M SAND	DPC-
3	-	55 14		24"	28		
32					57 -	31.7 - 32 ORANGE M SAND 32-33.5 DO SS 14 BOT.	
33	1 - 5	55.15			15		
24	1				33 -	33.5-34 TAN M SAND,	
	-				30	34.35.5 GRANGE M SAND, MOIST	
35	-5.				38 -		
36	-				29	35.5- BU TAN M SAND, MOIST	



BORING NUMBER: BRP 25

Sheet: 7

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE DLAN

ELEVATION: - +26 DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: START: 1230 FINISH: 0930 LOGGER: LLINCOLN

E	S	AMP	E_	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
U-		2" 0.0, 5.5		2	36.36.5 ORANGEM SAND,	
7-	5517		24"	٧	BL.5.38 TAN M SAND, MOIST	
-				5		
, -				8 _	38.39.5 ORANGE, LT. BRN	
-				υ - -	M SAND, MOIST	
-	55.16		24"	12 -		
				17	39.5.40 DK GRY SANDY CLAY	4" CASING TO
				8	40.42 LT BRN M SAND	4D:
- 45	5:19			4		
-				8		



BORING NUMBER: BRP.25

Sheet: 3

SOIL BORING LOG

PROJECT: DUNDALK MARING TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +20

DRILLING CONTRACTOR: ATT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIPRILL XL MAY 170

WATER LEVELS:

START: 1230 FINISH: 0930 LOGGER: L LINCOLN

(1)	S	AMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
12 -		2" 0.0. S.S.		9	42-44 LT BRN M. F SAND	
13-	45.70		246	12		
4 -				15		EOB @ 44'
				-		
,						
				-		
5-						

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400

- Contract	v.mrce.co					SHEET	
ROJECT	D	MIDALY	MARINI	E TERM	NAL		05 B7
CATION			E MARY			SURFACE ELEV.	1+26
ORING LOC		SEE	PLAN	FUIAIS		DATUM	
Omito Loc		-1366	PURIT			-	
EST/INSPEC	TION EQU	IPMENT					
EFERENCE							
	4.10-4.10						
ORING EQU	JIPMENT A	ND METHODS OF	F STABILIZING BO	REHOLE			
		TYPE OF FEE					
PE OF BORI	NG RIG	DURING COR	RING	CASING USED		YES	NO
RUCK	222	MECHANICA	L	DIA., IN.		DEPTH, FT. FROM	TO
ID O	V	HYDRAULIC		DIA., IN.		DEPTH, FT. FROM	то
RGE		OTHER		DIA., IN.		DEPTH, FT. FROM	
HER							
						_	
YPE AND SI	ZE OF:			DRILLING MI	S.P. DOSCOV., M. CO.	YES	NO
SAMPLER	2"0	D. SPLIT S	POUN		F ROTARY BIT, IN.	_	
SAMPLER	_			TYPE OF DRI	LUNG MUD	-	
SAMPLER	-						()
RE BARREL	_	_		AUGER USED		YES	✓ NO
RE BIT			_	TYPE AND DI	AMETER, IN.	-	
RILL RODS	-		_	CASING HAM	*****	AVE	RAGE FALL, IN.
				CASING HAM			
				EARADIED UA	DOL COLLEGE	ILIP AND	
				SAMPLER HA		-	RAGE FALL, IN. 30
				SAMPLER HA		140 AVE	
ATER LEVI	EI OBSERV	ATIONS IN BODE	WOLE.			-	
ATER LEVI	EL OBSERV	ATIONS IN BORE	HOLE			-	
ATER LEVI	EL OBSERV					UTOMATIC	
				TYPE OF HAN		UTOMATIC	
				TYPE OF HAN		UTOMATIC	
				TYPE OF HAN		UTOMATIC	
				TYPE OF HAN		UTOMATIC	
				TYPE OF HAN		UTOMATIC	
				TYPE OF HAN		UTOMATIC	
				TYPE OF HAN		UTOMATIC	
				DEPTH TO WATER	MMER A	UTOMATIC	
DATE		DEPTH OF HOLE		DEPTH TO WATER		UTOMATIC	
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER	CH SHOWN ON	CONDITIONS	OF OBSERVATION
DATE EZOMETE ANOPIPE:	TIME R INSTALLE	DEPTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER NO SKET	CH SHOWN ON LENG	CONDITIONS STH, FT.	OF OBSERVATION TOP ELEV.
EZOMETE ANOPIPE: TAKE ELEM	TIME R INSTALLE	DEPTH OF HOLE D TYPE TYPE	DEPTH OF CASING	DEPTH TO WATER DEPTH TO WATER NO SKET ID, IN. OD, IN.	CH SHOWN ON LENG	CONDITIONS STH, FT.	TOP ELEV.
EZOMETE! ANOPIPE:	TIME R INSTALLE	DEPTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER NO SKET	CH SHOWN ON LENG	CONDITIONS STH, FT.	OF OBSERVATION TOP ELEV.
EZOMETE: ANOPIPE: TAKE ELEMI	TIME R INSTALLE	DEPTH OF HOLE D TYPE TYPE	DEPTH OF CASING	DEPTH TO WATER DEPTH TO WATER NO SKET ID, IN. OD, IN.	CH SHOWN ON LENG	CONDITIONS STH, FT.	TOP ELEV.
EZOMETE! ANOPIPE: TAKE ELEMILTER:	TIME R INSTALLE ENT:	DEPTH OF HOLE D TYPE TYPE MATERIAL	DEPTH OF CASING	DEPTH TO WATER DEPTH TO WATER NO SKET ID, IN. OD, IN.	CH SHOWN ON LENG LENG LENG	CONDITIONS STH, FT. STH, FT.	TOP ELEV.
EZOMETE! ANOPIPE: TAKE ELEMILTER: AY QUANT 5" DIA. DRY	TIME R INSTALLE ENT: ITIES SAMPLE BO	DEPTH OF HOLE D TYPE TYPE MATERIAL RING	YES	DEPTH TO WATER DEPTH TO WATER NO SKET ID, IN. OD, IN.	CH SHOWN ON LENG LENG NO. OF 3" SHELBY	CONDITIONS STH, FT. STH, FT. TUBE SAMPLES	TOP ELEV.
EZOMETE! ANOPIPE: TAKE ELEMI LTER: AY QUANT 5" DIA. DRY 5" DIA. U-S/	R INSTALLE ENT: SAMPLE BORN	DEPTH OF HOLE D TYPE TYPE MATERIAL RING	YES LIN. FT.	DEPTH TO WATER DEPTH TO WATER NO SKET ID, IN. OD, IN.	CH SHOWN ON LENG LENG NO. OF 3" SHELBY NO. OF 3" UNDIST	CONDITIONS STH, FT. STH, FT. TUBE SAMPLES	TOP ELEV.
EZOMETEI TAKE ELEMI LTER: AY QUANT 5" DIA. DRY 5" DIA. U-SA	R INSTALLE ENT: SAMPLE BORN	DEPTH OF HOLE D TYPE TYPE MATERIAL RING	YES	DEPTH TO WATER DEPTH TO WATER NO SKET ID, IN. OD, IN.	CH SHOWN ON LENG LENG NO. OF 3" SHELBY	CONDITIONS STH, FT. STH, FT. TUBE SAMPLES	TOP ELEV.
DATE DEZOMETE TANOPIPE: TAKE ELEMI LTER: AY QUANT 5" DIA. DRY ORE DRILLIN	R INSTALLE ENT: SAMPLE BORING IN ROCK	DEPTH OF HOLE D TYPE TYPE MATERIAL RING NG	YES LIN. FT. LIN. FT. LIN. FT.	DEPTH TO WATER DEPTH TO WATER ID, IN. OD, IN. OD, IN.	CH SHOWN ON LENG LENG NO. OF 3" SHELBY NO. OF 3" UNDIST OTHER:	CONDITIONS CONDITIONS STH, FT. STH, FT. STH, FT. TUBE SAMPLES URBEO SAMPLES	TOP ELEV.
EZOMETE! CANOPIPE: TAKE ELEMILITER: AY QUANT 5" DIA. DRY DRE DRILLIN ORING CO	TIME R INSTALLE ENT: SAMPLE BORN IG IN ROCK NTRACTOR	DEPTH OF HOLE D TYPE TYPE MATERIAL RING NG	VES UN. FT. UN. FT. UN. FT.	DEPTH TO WATER DEPTH TO WATER NO SKET ID, IN. OD, IN.	CH SHOWN ON LENG LENG NO. OF 3" SHELBY NO. OF 3" UNDIST OTHER:	CONDITIONS STH, FT. STH, FT. TUBE SAMPLES TURBED SAMPLES	TOP ELEV. TIP ELEV. BOT. ELEV.
DATE IEZOMETE TANOPIPE: ITAKE ELEMI LTER: AY QUANT 5" DIA. DRY ORE DRILLIN	R INSTALLE ENT: SAMPLE BORING IN ROCK	DEPTH OF HOLE D TYPE TYPE MATERIAL RING NG	YES LIN. FT. LIN. FT. LIN. FT.	DEPTH TO WATER DEPTH TO WATER ID, IN. OD, IN. OD, IN.	CH SHOWN ON LENG LENG NO. OF 3" SHELBY NO. OF 3" UNDIST OTHER:	CONDITIONS CONDITIONS STH, FT. STH, FT. STH, FT. TUBE SAMPLES URBEO SAMPLES	TOP ELEV. TIP ELEV. BOT. ELEV.



BORING NUMBER: BRP- 28

Sheet:

SOIL BORING LOG

PROJECT: HONEYINEU DATISSI

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: MQUIFER DRILLING FART TESTING FOR.

DRILLING METHOD AND EQUIPMENT: SOM'C SPAN P DELL PREY YORD I SPE SAMPLING

WATER LEVELS:

START: 12 16 15

FINISH: 12/15/15

LOGGER: K. Charlovedula

£	S	AMPL	E	STANDARD	0구: Un SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТҮРЕ	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 - 2 - 2 2	r .	Some Herricorre	w	- - -	U" Asphalt 0.3. 0.75. Sincy groded Iffered with sand (GP), gray (GIBH Glory), dry mod based aggregate. 0.75!- Poorly graded sand with sittand graded (SPSN), Red-Brown (#51R4[6], moist, coarse grained Sand	Advanced the horehole from o'-10' using stop conic major cove DPC-1MF528.5" DPC-
	97	SONIC FIREROCOPUS			a.ol. silly serve with gravel (sou), brown (54312), well, fine to medican grained so a.zsl. woven geoterhio-liker a.zsl. some as a bore, wet	host per



BORING NUMBER: BRENS

Sheet: >

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

LOGGER:

WATER LEVELS.						
£	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТҮРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
<i>f</i> ₀ =	SS				6.01-6.91. Same as above, most, Red-through supplied. 6.91. Poorly graded grand with same (6.91. Poorly graded grand based aggreent dry, read based aggreent	PPC- 11, DPC-1, MPL= 0.73" gadr.
7	20	Sour Rivered	30'		3.25-3.25- Crushed Concrete 3.25-3.25- Crushed Concrete 3.25-3.25- Crushed Concrete 3.25- Poorly growled sand mills growel (SP), gray (activity), moist, coarse grained sand, Road based aggregate. 8.21- Same as above, lighted (3.54R 716)	phe.
		\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	31/1	3 18 44	10.0'-10.25'- Sandy Clay (SC), Red-Yellow (2.5 YR 516), dry, very SHCF, LOW plasticity. 10.25'-51ly SAND (SM), Red-Brown- grewit (7.54 Ruly), dry, the to medium grained Sand, Particulate to Moderately inducated (HBCOPP)	10-0-6"00 outercasing instato a depth of 10 bys. 10-5-50 on 10 switched for confirmation 19-0-15 have 30-to-drop; 30 dia samples. DFC-
	1				_	



BORING NUMBER: 57 12 18

Sheet: 7

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

LOGGER:

VATE	RLE	VELS	:		START: FINISH:	LOGGER:
(f)	S	AMPL	E £	STANDARD PENETRATION TEST	SOIL DESCRIPTION	COMMENTS
BELO	~		ERY (RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
ュ_	<i>و</i> وځ	4.5	io'	80°	12.0'- some as above	Dpc+
8/_				20/5	12.8°- same as above	35 Refugal; Sonic maero co- advanced to two jogs.
- 3 -	ماري	fra _{ac} Conge	18"			
-		(ŋ* ^A		-		
- - ۲				-	luid - some as above	1.000
	5-7	NW 1 42 Ceres	5 N,i	50/3"		14.01-Ss refusal; Some macro core advanced to 16'bo SS sample - No ve co very
> 		Muos		-		
ь -	SI-8				some as above	150 - 5" on Outer casing installed to adepth of 15' bg
	5.8	6000 10000 20000	12"			15.9' - se requal . Some macrocon advanced to 12
F				7.7	170-50 mp 25 about	
ļ.S.	25°	35	£.,	20 14		Ss refusal) sonic to 19/69:
T .	5,70				125'-38 we as above	



BORING NUMBER: BRP. W

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal **ELEVATION:**

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Frist Moth Drill Max 170; Sonic Sampling WATER LEVELS: START: FINISH: LOGGER:

WATER LEVELS:

£	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТҮРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18-	SNO	Soule MACRO	18"	-	18-21- same as about, moist, contains grove	tyr gut
19-	-55-23	30	MB-	हुंग्ड [े] ट	NO Recovery	SS refusal; sonic to 21'tys
7/0 -	520	Soul MACICO	181.	-	laring show white ground land. Red-Brown (2-5-112 Alb.) moish, fine to medium growned sound, green and white particles (corp. and man-copp.)	BRC4 CANDS 8 ""
60 %	-اکری _ک	-		6 .	21:01- same as above a bright green barbeles	installed to adopting 2010 by
ه ک			24	15		8PC+1-;mps=1.8"
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		5>	4.4	17	23.0'-23.5'- same as above 23.5'- 74.0'- Pooling graded sord with gravel (SP), gray(GEP) (Glay	D8C+1-



BORING NUMBER: \$29 28 Sheet: 5

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION:

DRILLING METHOD AND EQUIPMENT: Frist Moth Drill Mar 170; Sonic Sampling WATER LEVELS: START: FINISH: LOGGER:

FINISH:	.)	LOGGER: XA

	€ SAMPLE		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS		
•	DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
	24-	2,	り	247	2 -	wol-foolygradedsard with sitand grown (sp.sn), granish-gray, wet 100me aromed 22nd 100mp and run 100mp	bre +1_ ', mps 215"
	2 <u>5</u>				8	25 j- same as above, wet (copp and non-copp)	25:0-6"00 outer cacing installed to 25:0'bgs.) 25:0'-groundwater at 25' encountered alumng drilling
	2 6 -	55 ⁷¹⁵	J)	5 Na	9 12		DPC +/_
	5.7-				17	27.0'.72.2'. Poorly graded sand (SP), gray_brown (1-51R4/1), wet, (some grained sand	DPI -
	ავ- -	56-10	ςς	ŊŊ [*]	1 15	27.81. Fat CLAY (CH), gray (GLEY/L 6/104) rever, stiff to very stiff, medium to high plasticity	DP:-
	2 n		55	12"	24.	29.1-29.75 Foody graded sand (gray-brown (7.5124), wet, coar	sp) regrained DPC-
	30-	>' -		1	24	29.751_ Silty SAND(SM)grays not rellow 125-16/3), wet, timegraine Sand	n- d PPC-



BORING NUMBER: BR P-28 Sheet: 6

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

LOGGER:

<u></u>	_					LOGGER.
W (ft)	- *	SAMPL		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
30-	53-18	SÈ	20	3 -	30.0'- 51.0'- same as albona.	300-6"00 outer radicy installed in 30 bgs.
31 -				. •	31.0'- Fat CLAY (Cit), gray (aley) 6/m), wet. Shift to very shift, high plashing	DPE-
32-	51-1ª	shelby tube	24 ¹		Tog-Poorlygraded and doruguy- black	37:0'- Shelby tube pushed to 33:75'-resistance penetrating further. -tull ceronery may be
33-328		41945		- - -	Bothom- Fat CLAY and Stilly savel	because of the tube Pushing through wash.
<u></u>	520	55	2i) - - - - -	sto loving graded sand with	34:0'- Cleaned the hole to 34'bgs and ss sample was collected from 3435 bgs.
(A)	>7			9		
Sb _				_		advanced to 85' bas



BORING NUMBER: BRP-28

Sheet: 🗇

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

LOGGER:

£		AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
36 -	<u>ડ</u> કરો	SS	100	39 20/4"	360. same as above	36'- Sample full with wash.
38	ટ્રોપ	SONIC MACROLURES	w	-	38.01 Same as above.	380. Before taking the sample from 381-40! the bottom of the hole was on-as used to be 36. Fo minitethe heaving of the samel into the borehole, the outer casing was installed adjugs and a some sample was obtained from 381-40!
(4) - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	la,	C)	16	2 6	41.51- silly sand (sn), gray (acey Glory), wet, time to medium	uo-o'-Cleaned the Lole prior to taking new Comple



BORING NUMBER: BRP-28 Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

€		SAMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
43 43	- C.	\$	12"	10 -	same as above.	
	35.255	55	۱2"	9 -	same as above, higher times Content	
Th'	35-26	55	12"	12 =	44-44'4'Same as above 44'4"-45' moist stiff pat clay trace form 5 and	PP=1.25 +sf measured the
Us-	- 57-7°	shadby trake	Nŝ		950- Foorly graded sandwith Sit (St. SM, gray, wet, coase grained sous 45.5'- Fat CLAY CCHI, gray (GLOI 6/1047), wet, medium Shiff, highly plastic	Bottom of the hole-qs'bgs No Recovery in the Sheling tube some more core advanced in the and some sample collected - Description based on Some sample. IP- 0.5 to 1.5 tof
47-	35-78	65	י ניצ"	3	420'-Same as above	Pr= 2.0+ch



BORING NUMBER: BRP-28

Sheet:

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

Œ	s	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
78-	\$\f\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	shelbytube	2 W	-	Top-Fat CLAY Bothom - Silly SAND	Cleaned the hole and measured the bottom of
-	\$\$ ¹ 0	S	· · · · · · · · · · · · · · · · · · ·	y 7 0	Soil Sith Sand Cominaray, had, cooling growing to not to not the state of the state	hole - Soibes before SS Coruptins 50 - 1500 poles casing installa to Solibers
		sheetby for be	5 N		Short bighty place. Top-Sith Some	The over the second



BORING NUMBER: \$ \$ 1.3 9 Sheet: 10

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

	SAMPLE			STANDARD		
(f)	S	AMPL	<u> </u>	PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
55	81-3J	*	\$	wot -	The Test Clay (ch), gray, wet, sighty	Clar (88-50)
5,					Softom of boring @ 56.0 bgs.	1
57- - - - - -						
- 50_ -						

PROJECT NUMBER
BORING NUMBER
BORING SHEET | OF 6

12-2-15 SOIL BORING LOG

	D	da IK A	i lante	Terminal	LOCATION Bel	timore
ELEVATI	ON	$\mathcal{L}\mathcal{U}$			DRILLING CONTRACTOR	
DAILLIN	G METH	od and	EQUIP	MENT TO	FMULT DILLI XL Max 170 FENSH 12.15	LOGGER M. Chancy
WATER	-	درو و الراب و الراب		and the state of	SOIL DESCRIPTION	COMMENTS.
WO HER HILD OWN STREET COM	NTERWAL.	NUMBER AND TWEE	RECOVERY (FT)	PENETRATION PENETRATION RESULTS B*6*:6*	SOIL NAME, USES CHOUP SYMBOL. COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DÉPTH OF CASING, BHILLING HATE, DHILLING FLUID LOSS, TESTS AND INSTRUMENTATION
0	5-1	311 Conil			Top 2 * food buse asphalt	DPC-Nonc
1 -	1				<u>-</u>	
	1					_
2						
						geotextile membrane 225'
3					Red Motthed Clayer mont f-M sand Some gravel	-
·						-
4	+				Red Mottled with grayish brown clayer sand, trace gravel	DPC-None
	- - 5	2 3'	3	1	-	-
)		Sur				-
	1					- ggs membrane & b.00

PROJECT NUMBER BORING NUMBER BRPZ9SHEET Z OF 6
SOIL BORING LOG

) EQUIPI		minutes 4	LOGGER M. Chancy
HEVEL	8			START FINISH	COMMENTS.
INTERVAL	NUMBER AND TYPE	RECOVERY (FT)	STANDARD PENETRATION TEST ARSULTS 6"-6"-6"	SQIL DESCRIPTION SOIL NAME, USOS GROUP SYMBOL, GOLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INSTRUMENTATION
12	三 菱	正學	44	Red nottled with brown & white	gro mem brane @ 6.00
1 5-	2 3" Sonk			But A sandy cky, trace gravel	DPC - None
1	Sone			· /	
+	1			7.0-8.5 Rid wolfford Brown F.M.	DPC-None
+				8.5-9.25 Road base/asphalt	
15	.3			9.25- 9.5 Which to gray gravel -	
-			·	9.5-10 Ted clay hardened almost rocklike	
4	ļ			Ted clay hardened 4 mb> + to CE II R.C.	
4					
0	3"	Nove	4"/50	35 from 10-104" No recovery 10-11.5 Red brown f.M sand	Advance with source from 10,4"-12!
+	5-L 3			Some gravel (HB) DPL+	- 12
4	's4 Son	l l			
1				11.5-12 Red-Stown & Sand Massome gravel trace clay DPC-	

PROJECT NUMBER

10587

BORING NUMBER

BR P 29 SHEET 3 OF 6

PE	OJECT	r	Undal	K Ma	rihe Termil	407	Honc
EL	EVATIO)N	~2	<u>l</u>		gste Multi Drill XL Max 170	
			OD AND	EQUIP	VENT	START FINISH	LOGGER Mchang
٢	1	EVELS	<u>۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔۔</u>	1	OTANISADIT.	SOIL DESCRIPTION	COMMENTS.
	DEPTH BELOW SUBFACE (FT)	NTERVAL	NUMBER AND TYPE AND TYPE	RECOVERY (FT)	STANDARD PENETRATION TEST RESULTS 6'-6'-6'	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR OONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING HATE, DRILLING FLUID LOSS, TESTS AND INSTRUMENTATION
	13 -	35-7	31 0D 55 31 Soull	14"	18-30 2/50	Red Clay chy mothed with green Particles 12- Mix" 12.5 Particles 12- Mix" 12.5 Particles 12- Mix" 12.5 Particles 12- Mix" 12.5 Particles 12- Mix" 12.5 Particles 12- Mix" 12.5 Particles 12- Mix and trace gravel For 12 Red brown M-Lsand, Some Gravel, trace clay (HB)	OPC+ strong Sonic from 13.0-14 DPC+ strong DPC+ strong
	14 -	- 55-3	3'1' OD 55	16"	20-44 86- ¹ /50	160 12 Day Roll Somm mothled clay	OPC + strong
	16	35-6	311 Sunt		10-11	Top 12" Brown with white the green specks f-M white the greent Bott 12 Red fC sand with grave (GHB Top 12" Red brown lightly inducted M-C sand some gravel Bot FM 17-17.5 Mottled sandy clay treets gravel 175-18 Red mottled Fat Clay Possible clay liner	PPC - Your PPC - Your PPC - Your REV 12/01 FORM D1588

PROJECT NUMBER BORING NUMBER BRP29 SHEET 4 OF 6

SOIL BORING LOG

PROJEC	r1)vnda	LK Me	mine Termi	10001014	Sultimore
ELEVATION DRILLING	NC NETH	OD AND) EQUIP	MENT F	prilling contractor AD gstc Multi Orill XL Max 170	
WATER					STAFIT FINISH	LOGGER M. Chancy
36	S	AMPLE		STANDARD PENETRATION	SQIL DESCRIPTION	COMMENTS.
DEPTH BELOW SUBFACE (FT)	NTERVAL.	NUMBER AND TYRE	RECOVERY (FT)	RESULTS 6"-6"-6"	SOIL NAME, USOS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR-CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DÉPTH OF CASING, DRILLING HATE, DRILLING FLUID LOSS, TESTS AND INSTRUMENTATION
18				11/6"	18-185 Red Clay mothed with white	AC
-	545	30	0.5'	.	Brown particles to M Sand, trace	Burny terminated 2 18.51
					Clayiy Sand	- -
19-					-	Thermistor Installed @ 13!
						VWP institled o 16' -
_	1				-	Thermomet 11,6° story
	-				·	Therm To= 11.7 14.3 -
						- γωγιο 15.8 - - ρ _{ο 97/3.5} -
					-	
						Patdoph 97.9.2 T14.3
	1	•				Rshing 85 5 T19.9
	-			72		- A. O. Marie Co.
				, ess		- VWP 60000 - 1518429 -
					-	-
	<u>.</u>					-
	1					
	+					

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET Dundalk Marine Terminal **PROJECT** FILE NO. Baltimore, Manyland See boring location Plan LOCATION SURFACE ELEV. **BORING LOCATION** DATUM Soull Samp Drill Max 170; SPT Sampling **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED CASING USED TYPE OF BORING RIG **DURING CORING TRUCK** MECHANICAL DIA., IN. SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM OTHER **BARGE** DIA., IN. DEPTH, FT. FROM **OTHER** TYPE AND SIZE OF: **DRILLING MUD USED D-SAMPLER** DIAMETER OF ROTARY BIT, IN. **U-SAMPLER** TYPE OF DRILLING MUD 35- sampler 5 3" Diam sample S-SAMPLER CORE BARREL AUGER USED YES **CORE BIT** TYPE AND DIAMETER, IN. **DRILL RODS** AVERAGE FALL, IN. CASING HAMMER, LBS. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. 30 Automatic TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE CONDITIONS OF OBSERVATION DEPTH OF HOLE DEPTH OF CASING **DEPTH TO WATER** DATE TIME See VW Piczonde-Indallation Cound PIEZOMETER INSTALLED SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. OD, IN. LENGTH, FT. TIP ELEV. **INTAKE ELEMENT:** TYPE BOT. ELEV. 3and OD, IN. LENGTH, FT. FILTER: MATERIAL

PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING LIN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING LIN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK OTHER: LIN. FT. Aquifer Drilling and Testing
ick
Thermistors Installed
Mark Chancy BORING CONTRACTOR Phelan Chris DRILLER HELPERS REMARKS DATE

BORING NO.

BRP-29



BORING NUMBER: BRP-31

Sheet: /

SOIL BORING LOG

PROJECT: Honogwell DACT SIT

LOCATION: Moved of prox . 23 towards the guad rail

ELEVATION: DRILLING CONTRACTOR: AQUIFER DRILLING AND TESTING, INC.

DRILLING METHOD AND EQUIPMENT: SONIC SAMP DRIVE MAY XC170; SPT SOMPTING

WATER LEVELS:

START: 12/21/15 FINISH: LOGGER: K. Chatur vedula

€		SAMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
0-				-	3' Asphalt 0.75'-Poorly graded GRAVEL with sond (GP), gray-Brown (7.54R4/2), dry, medium to lookse grained sond iRoad based aggregate.	Advanced the borehole from 0'-16' using 3"ID Sonic ma ero core. DPC-; mps = 0.25"
2-	; -1	SONIE MACRO COPLE	IJΟ [™]		2:01_ woven g cotuble. 2:01_ silty sand with ground (SM), Red-Brown and gray(7:51R 4/6), moist. fine to medium grained, sond	DPC-
		SONIL MALED LO RE	W2.		4.0'-6.0'- Same as above, wet	mps = 1.0"



BORING NUMBER: BRP-3/

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH:

£	S	SAMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE RECOVERY (ft)	TEST RESULTS 6"-6"-6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
	5-2	SOLUTE NACION CORE			6.01_same as above, moist	DPt-
8		SOISIC MITTER CORE	e M		8-0'- 6" thick A-sphall, beparated into 4 layers. 8-5'- Poorly, graded sand with ground (Sp1, gray (CIGII 6/104), wet, Road based aggregate.	8:0'- Relatively difficult drilling conditions. - Difficulty in cutracting the sonic maeno core - Low recovery; part of the sample lost due to the presence release in the sonic maenocore.



BORING NUMBER: BRP-3

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

€		SAMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
13-14-1	5	MACRO CORES	U8"	-	12.0-13.5'-Poorly graded I and with ground [spl, light brown (10'12 5/3), dry, have to medium grained sond, road based fill 13:5' well growled growed with sond (GW), brown (10'12 5/3), dry, mixed with pieces of asphalt	DPC-; mps=0.5"
5	5	SONIE		11	(15 of - Si Ity Smup (SM), gray brown (15 of R 4/2), moist, trace grower trace copp - vellow particles (copp and non-copp) contains roots / organic fibes.	155'- At 16.0' ficus of wood encountered in the matro co while cleaning the hole. 160'. 6" op outer casing installed to a depth of 15.0' bgs. 16.5'- Below 16.0' switched to se sampling; 140-16 hammer: 30 in. drop; 8" dia. Sampler D1(+/- DPE-



BORING NUMBER: BRP-31

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

3	,	SAMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТУРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18.		5.5	1211	8	some as abone, wet, contains roots.	18:01. ground water Enwunteredding drilling.
20.	55-7	35	1211	34	19.0'-Same as above, ontain Pieces of wood, net 19.71- Poorly graded sand with silt (SP) dork gray (NZ), wel, parse grained Send, contain praces and	700- 6 01) DOIG COSING
	53.2	2.2	ک _م .	-75°	20.0' same as above Red - Brown	instabled to 200 bgs. 20.5'. ss refusal Macro core advanced to 221 bgs. DP(+
21 -	S5 ⁻⁷	95	દુપ	8	22-0'- Poorly graded Sand with Silt (SP-SN), dork gray back ND) Wet, medium to Loance grained Sand, Particulate to highly indurated (GB COPR)	DPC+
23-	SSNO	55	Su	6	23.01. wer graded sand wingravel (SW), gray (HIDY), wet, fine to Coance grained sand	DPC+1-; mps=0.75" - water in the spoon



BORING NUMBER: BRF-3

Sheet: 5

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

_						rinion.	LOGGER:
43	W (ft)	S	SAMP	7	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
	DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
	. Y	55-41	٤٤	120	13	24.0'- Same as above, higher growd content	DPC- -waterin the spoon
		55.12	Şİ	5 M	24	25.0' - well graded gravel with. sand (GW), gray (4/104), wet, coarse grained sand	DP (- ;mps =0.5"
2		\$513	\$5	21"	30 5 5	27.01- elastic silt (MH), group (GLEY 1 6/104), wet, very stiff	
25	' 4		55	6	9	29.0'- same as above, higher Sand Content	; iii



BORING NUMBER: BRP-31

Sheet: 6

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

æ	T 5	SAMPLE		STANDARD		
) W(T	€	PENETRATION TEST	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
30 -				2-6 40	30.01 - Poorly-graded sand with Siff (SP-SM), gray (G/10y), but, medium grained sand	30-0'-6" or outer casing installed to 30-0' bgs.
31 -				3 .	31.01- Same as above	
32-	55,15	54	w.	10		
33-				17	33:0 - Same as above	
34-	SS-16	\$	24	23 -		
35-	ssa ^a	SS	1811	33	35.0'- Poorly-graded sand (SP), group (GLEY, 6/104), wet, coarse growned sand, frace STUT	35.0. 6"00 outer casing installed to 35.0' 6 gs



BORING NUMBER: BRP-3 |

Sheet:

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH:

3	S	AMPI	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТҮРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
31 -		32		25 -		
20	55-78	55	zy ⁿ	20 48 60	370 - some as above	
19-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-		SONT L MACEO CORES	241		40.01—same as abore	u.o.ol-6"op outer casing advanced to 40' bgs - Bottom of casing at 42.0' bgs - outer casing sinking down the hole



BORING NUMBER: BR4-3

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

3	5	AMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
43-	C,	MACRO WRE	36		430- Fat CLAY (CH), gray (GLEY) 6/104), wet, soft to step, highly plastic	-6" of outer casing Sinking downtho hole about 2' logs. Hence, 5' outer casing section adolptional hole advanced to 45' Leg. 19-0.25-0.5+5P
44-	5.21	1 tube Sonie	241		TOP-FAT CLAY	PP = 0.25-0.5458
43-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	5522	tube.	20			PP = 0.5.075+5P- 47.01. cleaned the hole and confirmed the depth of the hole to be 47thy before Collecting the Shelly from 47+491



BORING NUMBER: BRP-31

Sheet:

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

\$	3	SAMPLE		STANDARD	SOIL DESCRIPTION	
3			Ê	PENETRATION TEST	SOIL DESCRIPTION	COMMENTS
DEPTH RELOW (#)	NUMBER	NUMBER		6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
48	-			- - -	Bottom-Poorly groded sond with	
50	- 55.7	S's	22"	H0H	Clay Poorly graded sard withclay (SPSC dark gray (CLEY 4/101), wet, COOME grained sand,	
51	· -			-	50.3- Fat CLAY (CHI, dorkgray (GLEYI 4/104), wet, Veryshiff, highly plastic.	PP=1.5-1-75fsf
			29	- - -	709- Poorly graded sandwith	Shelby discorded_
52.	- (x)	tube	10	-		. P
\$	5.5	Sonic Shalby marm core	511/1	1	Bottom - Poorly graded sand with Clay. 53.6- Poorly graded sand with Clay, gray (GUET 4/101), wet, coarse grained sand.	6" of outer cost of installed to 45.016gs.
54		3 12				



BORING NUMBER: BRP-31

Sheet: 10

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

LOGGER: K Chetrocodyla /M Cherry

E E	5	AMP		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	rype	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
54 - 55 -					Fod CLAY (CCH), group (GLOT) 4/104) Wet, Stiff, highly plastic.	55.01-6" on outercosing installed to 55.0' bgs	
56-	5.7	Shelby tube.	22.		Potton Tat Clay	PP. = 0.25-0.5+sp.	
57 -	ys 71	311 OD Sample	24"	3		PP = 0.5-0.75tsf 57'-Stoppedat 17:00 On 12/21/15 57'Stated at 6:32 12-22-13 Will sample from 57-59 With 6" Split-spoon PP-0.5 tsf	
59-	5T-78	halby Noc	24"	4 -	59-61 Top-Fat Clay (Clt):	Shuby tube from 59-61 PP= 0.5	



BORING NUMBER: BEF-31

Sheet: //

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH: LOGGER: Mall ()

	9	AMPL	E	STANDARD	08:45	Mark Chancy
. S	7	CONTRACTOR OF THE PARTY OF THE	1.77	PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
60-	57-78	Sidby	24"	-	Both Fat Clay (CH) Boschole terminated @ 61'	PP: 0.5 24" penetration Full recovery
62-				-	Per Ramzi Khuri (CH2MHill)	2 Vwp's & I themister This talked and graved in place P-1 depth = 48'
63 -				1 1		P-1 depth = 48' P-2 depth = 23' Thermistrolepth = 22'
64-						· · · · · · · · · · · · · · · · · · ·
65						

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BRP- 21 www.mrce.com BORING NO. SHEET **PROJECT** Dundalk Marike Ferminal FILE NO. 10587 Baltimore Maryland LOCATION SURFACE ELEV. **BORING LOCATION DATUM** Souic Samp Orill Max 170; SPT Sampling **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS **BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE** TYPE OF FEED TYPE OF BORING RIG **DURING CORING** CASING USED TRUCK MECHANICAL DIA., IN. DEPTH, FT. FROM 60 TO SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM TO BARGE OTHER DIA., IN. DEPTH, FT. FROM Truck **OTHER** TYPE AND SIZE OF: DRILLING MUD USED **D-SAMPLER** DIAMETER OF ROTARY BIT, IN. **U-SAMPLER** TYPE OF DRILLING MUD 65-Sampler; 3"dia Sampler S-SAMPLER **CORE BARREL AUGER USED CORE BIT** TYPE AND DIAMETER, IN. **DRILL RODS** 3" ID Son' (Macroroge CASING HAMMER, LBS. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. 140 AVERAGE FALL, IN. 30 TYPE OF HAMMER Home til WATER LEVEL OBSERVATIONS IN BOREHOLE DATE DEPTH OF HOLE TIME DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION PIEZOMETER INSTALLED NO SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL OD, IN. LENGTH, FT. BOT. ELEV. **PAY QUANTITIES** 3.5" DIA. DRY SAMPLE BORING LIN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING HN FT NO. OF 3" UNDISTURBED SAMPLES

BORING CONTRACTOR

Acostic Doilling and tisting

HELPERS

Chris Philan

REMARKS

2 Vap's and 1 through installed

RESIDENT ENGINEER

K. (haterocolda (CH2MHill) | Mark Chaucy

DATE

1/2-21-1/2-2

OTHER:

LIN. FT.

BORING NO.

3RP-31

CORE DRILLING IN ROCK

ch2m:

PROJECT NUMBER: 10587

BORING NUMBER: BRP-32

Sheet: 1/6

SOIL BORING LOG

PROJECT: Honeywell DNT SED ELEVATION: ~+23

DRILLING CONTRACTOR: Aquifer Drilling and Testing, Ene

		EVELS SAMPL		STANDARD	START: 1219115 FINISH: 1219115 15:40 SOIL DESCRIPTION	LOGGER: K-chaturredula)
ЭЕРТН ВЕLOW (ft)	~		RY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	COMMENTS
DEPTH E	NUMBER	TYPE	RECOVERY	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 -				 - -	4"- Aspealt 0.25'- Poorly graded Gener win and (GP), gray-black-white (N2), dry, coarse grained sand, Road based aggregate	Advanced the 3°DB sonic macro core from 0'- 10' DIC-
1 -	51	CoRE			1.0'- silty samp with Gravel (SM), gray-brown (7.51R4h), dry, medium to coarsegrained same	
2-		ONIC MACPE	36,			XI
3) S		-	20'- woven geolextile 3:0'- Same as above, Red-Brown (757R414), moist	DPC-
4-	52	Mrceu Co Ro			45'-55'- clayey sand with gravel (SC), gray -brown (3.54R412), wet, fine to medium grained sand.	DPC-
6		Some M			55- Polyethylene liner 55-Silty Smro with gravel (CM), gray-brown-Raddish (754R413), moist, time to medium grained sand	DPC-

ch2m:

PROJECT NUMBER: 10587

BORING NUMBER: BRP-32

Sheet: 2/6

SOIL BORING LOG

PROJECT: Dundalk Marke Terminal

ELEVATION: ~+23

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Frast MultiDrill Max 170; Sonic Sampling START: FINISH:

LOGGER: M. Chary / W. Chatursed (9

	-	AND	po	STANDARD	START: FINISH:	LOGGER: M. Chany/ K. Chatwoodla
W (ft)	-	SAMP	_	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
-	5-2	sonic mare				DPC-
7				-	70'-3' thick Asphalt 725'- P.25' - Poorly graded SAND with graves (SP), dry, gray (7.5425), Coarse grained sand), DPC-
9		MACLO COPUE	28	_	8.25-9.0'- Silty sawp with graved (SN), Red-Brown (7.5-18.4) 4) dry, time to medium grained sand	DPC-
-		Same			9.0'- Sity SAND (IM), Red-Brown- greenish (7.5 yr 4/3), dry, fineto medium grainel Sand, particulate to moderately indurated (HBCOPP)	DPC 7
10-5	3-1 3	200 mapler	6"	56/3	moderately udraftd (BM) HB *	10 0'-6"01) outercasing
- 5	4	3" suric	3'	NA -N	13-11 Dr/fed brown f-m silty sand moderately indirected BM) (HB) trace gravel OPE4 strong	installed to 100 bgs 10.5- Below 10' switched to SS Sampling; 140-15 hammer
1 - 4	5-2	200	311		'-11'3" SAMZ 95 S5-1	30 in drop, 13/4" dia sample
5		311 -			311-12 Sam as 5-4	DPC+ strong
					1-1 1-1 1-2-0	



BORING NUMBER: BRP-32

Sheet: 3/6

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: ~+23

DRILLING METHOD AND EQUIPMENT: Frost Moth Drill Max 170; Sonic Sampling WATER LEVELS: START: FINISH: LOGGER: Mark Chancy

(£)	S	AMPL	_	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
12 -	53	200 Sample	14"	50/3" -	12.0-12'6" Same as 55-2 12'6"-13' Ory Hard Red clay with f-m sand, trace	OPC + Strong	
13 -	3-6	311 SINIL	1.5'	NA	13-13'2" Pry very dense f-m siltysand 13'2"-14 Same as 5-5	OPC+ strong	
- /y - -	354	200 Sample 311	6"	56/311	141-143" same as 55-3	DPC+ strong DPC+ strong	
15-	545	Sonic 211 00 xup4-	1.5	4-9		11 that	
16	S. S. C.				Water Dry sed brown f-m silty sand Moderately Indurated (SM) (+3)	OPO-foint	
17-	446	21100 Sample			166-17 Dry medium dense brown to black f-m 6/11/56rd with grant I (6 M/4B)	DPC+strong DPC+	
18-			12		17.3'- Poorly graded. SAND with silt (SP-SN), gray-black (N2), coan- grained sand, particulate (GB COPE) DPC+	

ch2m:

PROJECT NUMBER: 10587

BORING NUMBER: BRP-32

Sheet: 46

SOIL BORING LOG

1	SAMPI		STANDARD PENETRATION TEST RESULTS 6" - 6" - 6" (N)	DRILLING CONTRACTOR: ADT NT: Frost Moth Drill Max 170; Som START: FINISH: 12/9/15 13:20 SOIL DESCRIPTION	COMMENTS	
NUMBER	TYPE	RECOVERY (ft)		SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION	
- 553			DPC+ 180'= Groundwaks encounter during drilling			
35-8	55	611	47	18:51- same es above, dry	Drc 4	
-SS-9	22	Cal	35 -	19.01- same as above, wet		
32-10	35	411	33	19.3'-lean CLAY (CL), Red (107414), dry, very stiff, Low plasticity 19.5'-Fat CLAT (CH), Red (104414), moi Very, Stiff, high plasticity	DPC- T, DPC-PP-3 otsc	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			-	Bottom of borng @ 20.01 bgs	20.0'-6' of outer casing installed to 20.0' bgs. 2 thermistors installed Alt depth	
	2 P			thermometer air reading 11.6	T, 17.5' 11.8 38.6 Drillers damaged both thermie while drepping stand pope into borel I had to replace a 15825' thermistor	

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122

	ww.mrce.c	UNDALK	MARIN		INAL	BORING NO SHEET FILE NO.	10587	F6
LOCATION		ALTIMOR	E MARY			SURFACE EL	EV.	t23
BORING L	OCATION		See location p	Jan		DATUM		
	ECTION EQ		Sonic &	amp Dril	Man XL	170',	spisampl	ing
BORING E	QUIPMENT	AND METHODS O	F STABIUZING BO	REHOLE				
	9	TYPE OF FEI	112					
TYPE OF BO	RING RIG	DURING CO		CASING US	64		res No	- 1
TRUCK	-	MECHANIC		DIA., IN.	6	DEPTH, FT. FR		TO 20'
BARGE		HYDRAUUC OTHER		DIA., IN.		DEPTH, FT. FR		
1000	traci			DIA., IN.	\	DEPTH, FT. FR	OM	
TYPE AND D-SAMPLER U-SAMPLER S-SAMPLER		analira -	Zw dia		AUD USED OF ROTARY BIT, IN. RILLING MUD	ليسسا	VES NO)
CORE BARRE		7.8		AUGER USE TYPE AND I	ED DIAMETER, IN,		ES NO)
		VATIONS IN BORE		SAMPLER H	AMMER, LBS.		AVERAGE FALL, IN.	30
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	DEPTH TO WATE		The second second second	NS OF OBSERVATION	
12/9/15	12:10	18.5	10.0	18-01	encounte	red dus	ing drilling	V
	-						0 (2
PIEZOMETO	ER INSTALLE		ves	NO SKE	TCH SHOWN ON	Star	rol pipe inst	albhonlog
STANOPIPE:		TYPE		ID, IN.	IFN	GTH, FT.	TOP ELEV	U
NTAKE ELEM	MENT:	TYPE		OD, IN.		GTH, FT.	TIP ELEV.	•
FILTER;		MATERIAL		00, IN,		GTH, FT.	BOT. ELEV	
PAY QUAN	TITIES Y SAMPLE BO	BIAIC	UN. FT.					
	AMPLE BORI		UN. FT.	9	NO. OF 3" SHELBY			
ORE DRILLI			UN. FT.		OTHER:	· AUDITO SHIALLES		
BORING CO	NTRACTOR	A	quifer Dr	illing au	nd tes him	g too		
DRILLER	To	DY COLD			HELPERS	1 Jose Dr		
REMARKS	-	Standp	pe piezome		thermusian	installed		
RESIDENT E	NGINEER	K:Cha	Furviolula	M. Chan	cen		ATE	
			*		9			BB0 22



BORING NUMBER: BAP-33

Sheet: 1/6

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: ~ + 24

DRILLING METHOD AND EQUIPMENT: Frest Moth Drill Max 170 Sonic Sompling WATER LEVELS: START: 09:00 14 FINISH: 121015 LOGGER: Mark Chancy

Œ	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
0-	5-1	711 SOLIL		NA -	Roadbese/Asphalt	DPC - none
1			4	-		
1			17	-		
-						
2 -				-		
3-				-	2.5-4.0 Dry and brown f. m sitty said trace graves (SM)	of omembrane 2 2.51
-				-		
	5-2	311 Sonil		NA -	4.0-5.5 Dry sed brown f-m silty sand trace gravel, (SM)	DPC-
-			4	-		
-				1	·	
1				-	5.5-6.0 Road bac/applet	geomembrane at 5.5°



BORING NUMBER: BRP-37

Sheet:

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: ~ + 24

DRILLING METHOD AND EQUIPMENT: Frist Moth Drill Mar 170; Sonic Sampling WATER LEVELS: START: FINISH: LOGGER: Mark Chancy

(£)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	- TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6 -	5-1	311 SoniL	4	NA -	6.0-6.5 Pry grayish green f-m Sandwith gravel, trove silt (5)	PPC - none
7					6.5-7.0 Dry brown f- (sand with grave)	DPC - runc
	5-3	311 Sonic	31	NA -	7-8.0 Concrete with f-m sands frace sitt	OPC- None
6-			·		8.0-9.0 Dry grapish While f-m Sand with gravel (6)	PPC-none
9 -					9.0-10.0 Dry light hrown f- Mgond willhiwhite and and Perticks, true gravel, tracesilt	DPC-none
10-	<u> </u>	21' 00 562 ph	1.51	22-56/4"	10.0'-10'3" Dry medium dense red brown f- Asithy Sand trace gravel (SM) DRC-none 10'3'-10'6" Ory medium to very dense f-c sand with 4 VACTE 10'6'-10'10" Dry fed brown very dense f-M silty	refusel à 10'10" advanced re/sonie to 12' ORC-none PRC-Mone
	44	311 Sonk	21	NA _	10'10"-11'3" Dry red Grown very dense +-11 sisty sound (SM) 10'10"-11'3" Dry white f-L sand with concrete 11'3-12' Dry reddish brown f-11 Silty sand with grave I modrately undurate (SM) HB	PPC-none PPC+ strong
12-						



BORING NUMBER: BLP-33

Sheet: 3/6

SOIL BORING LOG

	S	AMPL	E	STANDARD PENETRATION	DRILLING CONTRACTOR: ADT NT: Frost Moth Da' (1 Mar 170; Soni START: FINISH: SOIL DESCRIPTION	COMMENTS
	ER		VERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
	NUMBER	TYPE	RECOVERY	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
9	55.2	2" 00 394PE	8"	22-50/5'-	12-12:11" Day medium to very other Reddish / f-Msilly sand trou gravel moduately industed (SM) HB	pri + strang
-5	5	311 SoniL	2'	NA =	19'11-13' Dry red brown from silty sond	refosol @ 12'11" advanced wisovied. +0 14" DPC + Stoms
- 55	43	2 (1 00 ple	13"	15-16 - 52 3'' -	14-15/3" Damp Midium dense to very dense red brown - black f. M silty sand (SM)(GB) true green particles	DPC+strong
5-	-b	311 SuniL	2		15'3"-16.0 Damp red brown to black with green forticles formsity sand (SM) GB) 16.0-17.0 Same as S-5	Ichosal D 15'3" advanced with Source, to 17 DPC + 5 thora
5.	7	211 UD .	4" 7.1	50/2" NA	17.0-17'2" wet very dusc red brown 17'2"-181 Same 95 5-5	advanced 6"00 casing to 17' wet sample DRC + Strong

ch2m:

PROJECT NUMBER: 10587

BORING NUMBER: BPP-33 Sheet:

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: 124

DRILLING METHOD AND EQUIPMENT: First Moth Drill Max 170; Sonic Sampling WATER LEVELS: START: FINISH: LOGGER: Mark Chancy

3	1 8	SAMPL	.E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE .	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18-	115	211 00 sample	11	21 -	18-18'3" Same as SS-2 (SM)(HB)	OPE + Strong
	555	Sample	1		18'3"-18'6" Same \$5 55-3 (SM)(GB)	OPC+ strong
	55-6	211 Sample	8"	54	18:6'-19 Moist very dense black with seen & brown fartlicks from silty sind (SM)(CB)	OPC tistrong
19 -	557	200 B	6"	65	19-19'6" Noy dense deemp led brown with Organic particles from silly sand BM) (HB)	OPC + Strong
20.	55-8	21' 00 20 21'	Чн .	62	79'6"-20' damp very dense red brown with orange and white particles f-M sitty food + trace Clay &M-CL) HB	DPC + Strong
20 -	55-9	500 Sample	10	67 -	16'-20,5' Same 95 55-8	DPC+ strong
	55-10	z'op sample	14	50/11	No recovery	DPC+
21-	5-8	Sou; (15,		Same as above	Advanced the borehole to 21.5' using somic macrocon DPC+
	55-11	Soulye 5,00	311	9	Fat CLAY CCHT, Red -Gray (1074/4), wet, shiff, highly plashie.	PPC-
22-					Buttom of buring @ 22-0' bgs. Vwf # 1517443	Vmp Air To9815 P. 22.6
				1	1" PUC Installed to 22	Depth T 9812 P. 22.6
23-				-	Vwp metalled @ 21 Thermister installed @/y'	Slurry Tz 9346 Pz 257
					Thermister Aur. 24.3	
-			• 1	-	Thermistan depth = 23.6	
24-					Themometer Ar temp = 17.3	1

throughorshory = 21.1

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 www.mrce.com BORING NO. SHEET Dondalk Marine Terminal **PROJECT** FILE NO. 0587 LOCATION Baltimore Many land SURFACE ELEV. **BORING LOCATION** DATUM Son'L Samp Drill Max 170; SPT Sampling **TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING** CASING USED TRUCK **MECHANICAL** DIA., IN. DEPTH, FT. FROM TO SKID **HYDRAULIC** DIA., IN. DEPTH, FT. FROM TO BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER TYPE AND SIZE OF: DRILLING MUD USED D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD 55-Sompler: 2" Dig Semples S-SAMPLER CORE BARREL AUGER USED YES **CORE BIT** TYPE AND DIAMETER, IN. **DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. 140 SAMPLER HAMMER, LBS. AVERAGE FALL, IN. 30 Automatic TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE DEPTH OF CASING | DEPTH TO WATER CONDITIONS OF OBSERVATION PIEZOMETER INSTALLED NO SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL OD, IN. LENGTH, FT. BOT, ELEV. **PAY QUANTITIES** 3.5" DIA. DRY SAMPLE BORING LIN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING LIN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK LIN. FT. OTHER: **BORING CONTRACTOR** DRILLER Joseph Arrayo HELPERS REMARKS RESIDENT ENGINEER DATE 12-10-15

BORING NO.

BRP-33



BORING NUMBER: BRP-36

Sheet: //6

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: ~ † 23

DRILLING METHOD AND EQUIPMENT: Frest Moth Drill Max 170; Sonic Sampling WATER LEVELS: START: 0800 FINISH: LOGGER: Mark Chancy

€	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
2-	5-1	3.1 5001:L	21	NA -	Road buse asphalt	
y - 5 -	5-2	311 Souil	31	NA -	4.0-5.0 Damp red brown f-m sitty sand trace gravel, trace Clay (SM-CL) 5.0-6.0 concrete with gravel	DPC -



BORING NUMBER: BLP-36 Sheet: 2/6

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal ELEVATION: ~ +23

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Frest Moth Drill Max 170; Sonic Sampling
WATER LEVELS: START: FINISH: LOGGER: Mark Chancy

2		SAMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (#)	NUMBER	ТУРЕ	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6	5-2	311 SONIC	31	N4 -	6.0-7.0 Asphalt with concrete	PPC - none
7	3-3	311 South	31	NA	7.0-8.8 Doup Red brown & Maily Sand truce Clay trace gravel (SM-CL)	DPC-none
8			,	-	8.0- 98" Dry gray to light brown f-C sand with gravel, trace silt, trace (oncrete chips BM)	DPC-none
7				- - -	98'- 10' Dry Red clay with while streeks	Olc-none
13	55-	21' 00 5117)L	21	41-44 50/3"	10-11.0 Pry (ed brown sitty f-m soind (SM)	Dect story advanced 6"50 casing to 10' Dect story Open story
11		311 Sonil	1.51	NA	11-11'3" light brown to green for sand trace grave! 11'3' 12' Dry green to black gravelly for M Sand, trace silt (GB)	ope 1/2 refusal of 11/3" advanced systems to 12' DPC+ strong



BORING NUMBER: BRP-36

Sheet: 3/6

SOIL BORING LOG

PROJECT: Dundal & Marine Terminal

DRILLING CONTRACTOR: ADT

ELEVATION: ~ 123

DRILLING METHOD AND EQUIPMENT: Froste Moth Drill Max 170; Sonic Sampling WATER LEVELS: START: FINISH: LOGGER: Mark Chancy

£	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (#)	NUMBER	ТҮРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
J2 -	55-2	21.00 Samp's	9"	/0 - 50/3" - - -	112-12'9" Day loose to very chose real brown moderately indicated f-missify Sand (5M) (1+B) travegreen particles	OPC+ strong
/3 -	5-5	311 Sonič	lb"	NA =	12-9'-14' Day damp fed brown f-m Silty sand, trace black (GB (OPR) trace Mostly (HB)	OPC+ Strong
14 -	65-3	211 00 Saufter	1'3"	45-50/5"	14-7" Dry dunse to very duse red from f-m silty shad trace grand (BM) (HB) M'7"-14"1" Damp viv., dunse red to black f-m silty Sand (SM) GB	DPC+strong
15-	5-6	311 Sonil	2'	NA -	14'11'-16' Dampred brown to black f-MSI'lty Sand (ITB)COPRIE Mixed together, trace clay SM	PPC+ strong
16 - - - - 17 -	5-7	211 op sampler 3!! Sovii	-	50/4" NA	16-164" Dry very dense ced brown with green and which puticises in situation (SM) (HB) trace grave! 164-18 Dry with orange highly indicated f-M silty sand, trace grave! (SM) HB	DPC+ Strong Lefusel at 16 4" 5 anic to 181 Driller says the sonic meets to advance and passes for very hard dilling lighter Sample under pressure, lost sample
18-			, :			in air

Ch2m:

PROJECT NUMBER: 10587

BORING NUMBER: BAR

Sheet: 🕜

SOIL BORING LOG

PROJECT: Dindal & Marine Terminal ELEVATION: ~ +23

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Frost Muth Drill Max 170; Sonic Sampling
WATER LEVELS:

START: FINISH: 12:45 LOGGER: Mark Chancy

	SAMPLE STAND		STANDARD	12.95	<u> </u>	
(¥)	5	AMPL		PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18 - 20 - 21 - 22 - 23 - 23 - 23 - 23 - 23 - 23	-	TOMP TO SO NI	1	50 4" NA	18-182" Dryvery dense ted brown f-M still count GMXHAS 182-189 Dryvery dense blacks instity south to GD 184' Ory Black moderately inderested commentations Ministrial almost usphalt like with white Straks and trace claystrace f-M silty sand (Matriol boyed for testing) Thermoreter air temp 15.50 I vul installed at 18 1518414 2 thermisters Ti 14.5' Mb 24.2	OPE ++ Very strong OPE ++ Very strong Cops al al 184" Drill says count advance with sonic to 19" ho Sample will be obtained, advanced With conicto 20"
24				- -		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET Dundalk Marine Terminal **PROJECT** FILE NO. 0587 LOCATION Baltimon Many land SURFACE ELEV. **BORING LOCATION** DATUM Sonic Somp Drill Max 170; SPT Sampling TEST/INSPECTION EQUIPMENT REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING** CASING USED TRUCK **MECHANICAL** DIA., IN. TO SKID HYDRAULIC DIA., IN. TO DEPTH, FT. FROM BARGE OTHER DIA., IN. DEPTH, FT. FROM TΩ OTHER TYPE AND SIZE OF: DRILLING MUD USED YES **D-SAMPLER** DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD 55-50mpler: 2" Octy Sam S-SAMPLER **CORE BARREL** AUGER USED YES **CORE BIT** TYPE AND DIAMETER, IN. **DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. 140 SAMPLER HAMMER, LBS. AVERAGE FALL, IN. 30 Automatic TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION PIEZOMETER INSTALLED SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL OD, IN. LENGTH, FT. BOT. ELEV. **PAY QUANTITIES** 3.5" DIA. DRY SAMPLE BORING LIN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING LIN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK LIN. FT. OTHER: **BORING CONTRACTOR** Joseph Arrayo HELPERS

DATE

12-9-15

BORING NO.

BOR-4 JAN2013

RESIDENT ENGINEER

DRILLER REMARKS



BORING NUMBER: BRP 39

Sheet: //7

SOIL BORING LOG

PROJECT: Dundalk Marine Terminal

LOCATION: Bolthwore, MD

ELEVATION: ~21

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Freste M. Hi drill XL Max 170

WATER LEVELS:

START: 12-1-15 08-20

FINISH:

LOGGER: M. Chancy

					12-1-15 08:20 12-1-15 11:45	
V (ft)	S	AMPL		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТҮРЕ	RECOVERY (#)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 -				-	Asphall Road Base	6" 00 casing installed to5'
1 1 1 1	5-1	3" sonic	3'	NA -		DPC - noue
2 -				- - -		
- 3 - -				-	3-4 Red brown C-M Silty sand; some gravel, trace clay (SM)	
4 - - - - 5 -	5-2	311 Sonic	31	NA -	Brown M-C Sand, some gravel trace clay (S)	PPC - none
6-		·			·	



BORING NUMBER: BRP39

Sheet: 2/7

SOIL BORING LOG

PROJECT: Dundalk Marine Terminal

LOCATION: Baltmore, MD DRILLING CONTRACTOR: ADT

				larine Termina	the state of the s		
DRIL	LING	N: ~ METH	2 / 100 A	ND FOLIPMEN	DRILLING CONTRACTOR: ADT NT: Fouste Molts Brill 72 Mar 170		
WATE	R LE	VELS): 	TO EQUIT INC.	START: FINISH: 12-1-15 11:45	LOGGER: M. Chancy	
€	S	AMPL	Ε	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	VUMBER	гүре	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
6 -	52	311 Sonic	31	- N4 -	-6.5'-7.0' Lood base asphalt	DR-none Membrane a 6.0'	
7 8 - 9 - 9	53	3/ Souric	2'	NA -	Top 17" Red brown M-L sand trace clay (S)	DPC - none	
-	<i>5</i> 51	3 th	2'	6-82-50-15	Post-10' light brown to white gravely m-c sand (s) Top & Red brown silly f-M sand some grave (HB) (SM) Mod-highly indurated Bolf 18 Red brown this by indurated f-Mailty sand, some grave (CHB) (SM)	6"00 casing installed to lo' PPC + strong	



BORING NUMBER: BLP 39

Sheet: 3/7

SOIL BORING LOG

PROJECT: Dundalk Marine Firminal

ELEVATION: ~21

LOCATION: Baltimore, MD DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Fraste Multi Drill XL Max 170

WATER LEVELS:

START:

FINISH: LOGGER: M Chancy

		ANDI		STANDARD	12-1-15 08:20 12-1-15 11:45	7
(ft) v	3	AMPL	1	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
13-	4 47	31 ¹	2'	27-34 - 45-56 - -	Top 18" Red Silty sand f-M sand (Highly some gravel trace white specks Indurated) trace clay (+B) (5M) Bott 6" Pad brance Highly well and with	DPC+ strong
 				28-26	Red brown Highly Industed with grafish brown specks M-C sifty sand trace grane (HB)(511)	
15-	<i>4</i> -3	3'' 55	20	26-44 <u> </u>	Red brown Mudindwated M-C sand some grave 1 (HB)	OPCT strong 6"00 casing installed to 15'
14 -	5-4	3" \$\$	8"	45-2/100 -	16-168" Led brown Highly andrated M-C sand som gravel true chy HB	
17-	54	3 ¹¹ Sonic	ľY"	NA -	168"-18' advanced with somic Led brown M-C sand, gravel HB	Picture not taken



BORING NUMBER: BEP 39

Sheet: 4/7

SOIL BORING LOG

PROJECT: Pundalk Marine Terrainal

ELEVATION: ~2/

LOCATION: Baltmore, MD DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: Fraste Multidaill XL Mar 170

WATER LEVELS:

START:

FINISH:

LOGGER: M. Chancy

	_				12-1-15 08:20 12-1-15 11:45	LOGGER. 171. Chancy
) (E) A	S	AMPI	1	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18 - - -	355		12	97-58 - - -	Red brown Nerydense Highly= Midwated M-L Sand, some gravel	OPC + Strong
19 -	55-b	3" \$\$	6"	_	19.0-19.25 Mod industry for Redbourn M-C Sand, trace clay 19.25-19.5' Bott led clay-liner	OPC+ strong Boring terminated 2 19.5'
						2 thermisters installed 2 13.5 '-T2 16.5 '-T1 Thermometer= 12.0°dec Celcil Air reading T, 11.7° T2 11.2° (eading D installation depth T, 16.5° T2 13.5'

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET Dundalk Marine Terminal **PROJECT** FILE NO. 10587 Baltimore, Maryland See boring location Plan LOCATION **SURFACE ELEV. BORING LOCATION** DATUM Sonic Samp Orill Max 170; SPT Sampling TEST/INSPECTION EQUIPMENT REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING CASING USED** TRUCK **MECHANICAL** DIA., IN. DEPTH, FT. FROM SKID **HYDRAULIC** DIA., IN. DEPTH, FT. FROM **BARGE** OTHER DIA., IN. DEPTH, FT. FROM **OTHER** TYPE AND SIZE OF: DRILLING MUD USED D-SAMPLER DIAMETER OF ROTARY BIT, IN. **U-SAMPLER** TYPE OF DRILLING MUD 35-Sampler; 3" Dia. Sampler S-SAMPLER **CORE BARREL** AUGER USED YES CORE BIT TYPE AND DIAMETER, IN. **DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. 140 SAMPLER HAMMER, LBS. AVERAGE FALL, IN. 30 TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE DEPTH OF CASING | DEPTH TO WATER CONDITIONS OF OBSERVATION PIEZOMETER INSTALLED NO SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. **INTAKE ELEMENT:** TYPE OD, IN. TIP ELEV. LENGTH, FT. FILTER: MATER!AL OD, IN. LENGTH, FT. BOT, ELEV. **PAY QUANTITIES** 3.5" DIA. DRY SAMPLE BORING LIN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING LIN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK LIN. FT. OTHER: **BORING CONTRACTOR** Chris Phelan **DRILLER**

BORING NO.

12-1-15

DATE

BRP-39

REMARKS

RESIDENT ENGINEER

2 thermistors installed



BORING NUMBER: BRE 47

Sheet: \

SOIL BORING LOG

PROJECT: Honeywell DMT IST

LOCATION: Mored lofeet to the South of Stoked lobbe

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT: SONIC SOME DRILL MAK HO; SET SAMPING

WATER LEVELS:

START: 11/19115 FINISH: 1/12/15 LOGGER: & Chadaryedula

(£)	5	SAMPI	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	ER		RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS
DEPT	NUMBER	TYPE	RECO	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
0-					" Asphalt 075-20' Poorly graded gravel with Sand (UP), Gray white [25485]2). Lry + 200d bosed aggregate	Advanced the hole from s't to bys using 2" 210 sonic macro core
1111	5-1	6 PACKACOF	36"	= 1		brc -
2		Seldie Baral		-	20 - woven geolerfile 20 - sity sans with gravel (en), dark red - Brown (love sty); olyg, three to medium grained sand	DPC-
		Q.	0		Mol-seme a databet, eset	DPC -
	53	MALLE COPE			501-Polyethyltne liner enterestered	
	7. 9	509314			50-60 - same by ahour, wet	Þ9 c ÷



BORING NUMBER: BR R. 42

Sheet:

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(£	S	AMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
6 - 1 1 1	52				1.0'- Poorly graded armet with sand (ap), gray-White (2.54 8/1), dry, Tood bared aggregate.	DPC -
111111					68-3" of Asphalt 10'-775'- Billy crows with grown (sm) dark red-Brown (sweet), months to medium grained sand	+ DPC+
1 1 1 1	5-3	- WALRESCALE	36		dry root barrel as segate modium	DPC -
11111		2017			30' - Roomy graded small of the sitt of and graved [30 shi, light red as 12 th (75 y 2 76), moi it is medium to coone grained soud	DPc-
1 1 1 1	55-11	53		46 79	g gl-horin geokette 10'- 10:5'- some to above odore 0:04 (7:51/2 411), moist 105-110'- Fot CLAY (CM), Roll	to a sampling; hather harmon; samples . But a samples .
1	*	*			(10 Y 6/4), maist, the medican Halfury My - Silvy Shorts (5m), Endarch Roma Principality (1991), dry, parket for the Singular restor and a landquist 1 HG (60%)	



BORING NUMBER: BRILLING

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH:

(ft)	S	AMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12 -	S.I	3000 to 0000 KH	12"		same as above i	bPc -4
13-	524	Swite	24	12	134'-145'- Poorty graded and with the (Sp. 211), danigous there (NZ), but, bush green parkiles, Portralate (OB (OPR)	130- ground water encountered during double at 26 bgs
15-				34	165 - STRY EMMES (SIL) Red-Proch (2548 414) mort . Perticulate to slightly induce test (HE CORE)	Pricel
-	4.72	55	1.1	100 3"	carre as about	Core to 14 bys wary
0					test - as core layer, but	Drd +)
+-	43	22	M	19	Bright green for liter, wet	Alternating larger of Giland to CVTD
	A.C.	d2	59	62	12.5- HB COPE lingue, Hellins Particles, slightly lindus and its	



BORING NUMBER: BKP 1/2

Sheet:

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH:

(ft)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18 _	55-10	SŠ	ñ,	(00/30	same as above porticulate, any	180- 6 or outer laining
-	11-2	SON Ze	2"		some as above dry	Advanted to hide to 15th g Litting so 300 poor moun
19 -		58	6	10	(19-0' - sand from Elmi recipled - white	DPc-
7.0					Borry tereseable at 1915/bgs	
21-						
11-						
3-1-1						
1				-		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 www.mrce.com BORING NO. SHEET PROJECT DUNDALK MARINE TERMINAL FILE NO. 1058 LOCATION BALTIMORE, MARYLAND SURFACE ELEV. **BORING LOCATION** DATUM TEST/INSPECTION EQUIPMENT Sonic Samp Drill Man 170; SP REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG DURING CORING CASING USED TRUCK MECHANICAL 6" OD DIA., IN. DEPTH, FT. FROM TO SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER Track TYPE AND SIZE OF: DRILLING MUD USED NO YES D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER Sampling CORE BARREL AUGER USED YES NO CORE BIT TYPE AND DIAMETER, IN. DRILL RODS 3" ID Sonic Hacrocore CASING HAMMER, LBS. AVERAGE FALL IN. SAMPLER HAMMER, LBS. 140 30 AVERAGE FALL, IN. TYPE OF HAMMER Automatic WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION 21/21/11 13:00 13.0 10.0 13.0 Dunna milling PIEZOMETER INSTALLED YES See Standpipe Installation SKETCH SHOWN ON STANOPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE CO, IN LENGTH, FT. TIP ELEV. FILTER: MATERIAL 00, IN. LENGTH, FT. BOT. ELEV. **PAY QUANTITIES** 3.5" DIA. DRY SAMPLE BORING UN. FT. NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK UN. FT. OTHER:

and Itshing

BORING NO. BRP-42

11 12 15

phelen

DATE

DRILLER

REMARKS

BORING CONTRACTOR

RESIDENT ENGINEER

K. Chaturvedula



BORING NUMBER: PARTE LAND

Sheet:

SOIL BORING LOG

PROJECT: Homeword Trut 15

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: AQUITETE DELLENG AND TENTAGEN -

WATER LEVELS: START: 12 15 15 FINISH: 12 16 15 LOGGE

LOGGER: 12-1/2 turnedula

STANDARD SAMPLE COMMENTS SOIL DESCRIPTION DEPTH BELOW (ft) PENETRATION RECOVERY (ft) TEST RESULTS SOIL NAME, USCS GROUP SYMBOL, DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, NUMBER COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL TESTS AND INTRUMENTATION STRUCTURE, MINERALOGY 6" - 6" - 6" (N) Advanced the borehole 3" Asphalt from play using 2° 21 ORF), gray - brown (754P 4/1), dry some warm core mixed with Churced precessory DOC-Barton U3" 2.81-51ty SPATO with grown (SII), Brown (F-51/1 4 2), dry, time to DREmedium sand 3-2'- trecomposed preses of wood Brown (7500 Uno, wet, fine to DPC medium grained stand. 36" 55- Polyethylene liner. Brown-Red (#51/R 4/4) dry this to medium grained sord (4)



BORING NUMBER: 374- 44

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
, , , , ,	52	MARCRO CORES		-	6.8- 2" thick dephate and contract acres	P97
4-1-1-					70'- Poorly grooted sord with grown) (CP), grown (25-18 721), dry, coarse grained sound, Roadbased aggregate	DPL-1-5"
8	5/2	SONIE HIMFO COLC	30		8.3'- sity smnt with gravel (sn), Red Brown (2.5 YRUID), moist. fine to medium grained said, pockets of clay	Dec-
						100-6"00 outs casing
10	35.24	55	g v	3 10.5	10.5'- 11.0'- Sily Spino (SN), gray-	installed to 10 by: . 10.5'- helowed suitched by 15 sampling 140-16 Nomes 2 30in disp. 3'dis Sampler
11	5.5	to rid	15	>	- greenich; dry tine to medium ground road bright green portiles (corr) 11.21 - same as above, Reddish - Brown (154R 414), slightly to moderately indurated (HS copr)	



BORING NUMBER: BER- IN O

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(£)	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	туре	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
3-	g.b	22	الآء	18 13	Poorly graded sard with sittlessell, daring ray . Wall (1021, moiet. medium to rooms grained sand, Parsiculate, bright green particles (GIZ COPPS)	Drif
- Z	57	Ind.	100	50 2"	14.0-Siling Smart (Stall Laddith-Lower (7-5-18 46), dry, fine to midlium grained sord, bright green particles, porticulate to slightly indurated (HB COPR)	the 3° 10 sonis macro con in the 3° 10 sonis macro con in 16' bgs. - No recovery in the 3° Soni DPC-1
6-		FOR C	12	50 2"	ts.o'-same as above	16.0-6"00 ooks lasing installed to 15.0" bgs 16.0'-Sirgual: Advanced to 3"10 sons maio conto
>-	5.0	toric mare	111	20/3"	12-01- same as above, forhulate to moderately indurated	, , , , , , , , , , , , , , , , , , , ,



BORING NUMBER: 37-4 W

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW	NUMBER	турЕ	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18-	59	22	ŗ*	34	18.1 - Poorly ground sand with SHSMI. group-greenity, moist, coarse grained some forticulate (GIZ COPP) Some as above	· OPE-F
19-	2570	25.	7*	20)54	some or weave, wet	19. 1.9 mind water Incorntered during drilling
	517	morn morn	10"	- 1	some as above, but, doingray and greens he particles	Care advanced in 200 ligs.
20	22:13	22	3"	7.4 1	20.0'- same anabow. 20.2'- Silty s AND (SM), Roddis h Brown	installed to 200' bgs.
ten	32-17	67	3"		12.5 12 Ului, moist, fine tamedium grained sand, green partiles (HECOPR) 20-25 - SERTA AL above	Sonic maro core advanced to 21-5 bys
1)	5-12	Sonit	9"	-	21-01- GB COPR layer, well	
	55.16	55	511	10	21.5 - Charoly Clay (CLI, Red-Tellows moist, mixed with Fax CLAY sparme	Croyalus, DPC-
72				8	Bothom of boring @ 22-0 bgs. To 18 13.2 To 13 12.9 6'send 15' bentwee	Thermometer and temp=13 22' 2" Standpile installed



BORING NUMBER: BREAT

Sheet: \

SOIL BORING LOG

PROJECT: Honeywell DIMT SIT

LOCATION: Moved 10' South from the Stated location

ELEVATION: DRILLING CONTRACTOR: Aquifer drilling at it testing. Inc.
DRILLING METHOD AND EQUIPMENT: SOME SAME DELL MAY 130; SPI SAMPLING

WATER LEVELS:

START: 11 11/15 FINISH: 11/12/15

LOGGER: K. Chadrie Vedula

£	S	AMPL	E	STANDARD PENETRATION TEST RESULTS CO 6" - 6" - 6" (N)	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)		SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 -	5-1	SORVIE B MAPLED CORTE	36	-	4" Asphalt 0.25'-2.0'- Poorly ground GRAVEL with Sand (GIP). Gray (25tesh), dry. Road based aggregate 20'- waven geotestile encountered 20'- silty SAND (SM with gravel, dark brown-gray (by KSH2), dry, fine to medium grained Sand	Advanced the help from dhe to be using 2000 sonic motor core DPC -
3 5 5	51	Bance be the Can Can Can Can Can Can Can Can Can Can	36		40'-65'- sandy lean CLATCLES, dank gray (1042411), wet soft - 50'- There of wood	DPC —



BORING NUMBER: BRP-45

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

E	8	SAMP	.E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
ELOW			RY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING	
DEPTH BELOW (ft)	NUMBER	TYPE	TYPE	RECOVERY	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
1111	5-2	Source Merbig			bst- encountered Polyethylere lines bst-to- Poorly graded some (ON), Black - Asphall odor, moist	bPe =	
-					30'-2-75' - Sandy CLATE(CL), dark gray (1048 4117, Wet, Soft	DPC-	
	ça.	CARE.	rs. Vi		From the structure of courses arphase	Dpr -	
111111		ZONIC MACRO			901-11/14 SAND (SM), Red-Brown (25485/6), dry, Red-Brown (25485/6), dry, Red-Brown Slightly indurated, laminated (148 core)	DAC 4	
0				2 -	10-V-1071 - SAME AS ABOVE	DP t -4	
11.	1.0	g5	24	13	1075'-115'- Poorty graded growel with sand (ap), ten-write, moist medium grained sand 115'- same as whose, dark gray (104R 411)	100' - 6" Op outer cost y advance 10-0' bgs. DIC - 11.0' - Below 10' switched to 55 Sampling: 11:0-th house. 30 in 1107 , 3" DI - Sampler	



BORING NUMBER: BRP-MS

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH: LOGGER:

(#)	5	SAMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12-				12 -	12'-12:5' tot ILEY (UH), Red (104414) moid, respectff, highly placks	bPt-
13-	55"	55	20	18	Red-vellow (104R 7/6) + morely	thre-
15-				14	125' 125' - Fot Eler (CH), Red (10711)u) moist, Shiff, highly Plastic	DPC-
19		\$5	4.	46 50/m	14.0- Silly sprin (chi, Red Torown Castr 576), dry, slightly to Moderation industred, landinated (HB COPIZ)	bys Wing 3" In some mater Low .
5 -	55%	5			green ish-Israus	outer rasing to 15/695
14-					16-67-Some at allowe, Red-Brown	16.6". No receivers in the
	5.7	55	-9 <i>t</i> f	50 30	moderately individual	Spirit spinon Inaple. Indiversal the Late living I'm strict mass courts Adepth of the' bye
7	558	95	7	3 -	same as above, bright yellow particles	12-1 - Consord water
18-	55.00	çç	31	100/11,	Particulate, bright green justiles	Describe Read of



BORING NUMBER: BRE45

Sheet: 4

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

3	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
18-	5-10	Tany Marco	6"		sitysmio (su), Red-Brown (7:57R516) dry, farticulate to moderately Endurated, laminated (MB COPE)	Advanced the some macro
-	511	Swell or prototo	6"	20/00	Same as above, bright green and Yellow particles	matro to 1901 bgs.
19 -	55-12	55	3"	100/311	Poorly graded sause with \$14 (59-514), dark gray - black (N2), wet, green farticles (GB LOPE)	-
-	5-13	Sonic motors	6"	1	HB topk, particulate, dry	Advanced to 20' bgs using senic macro core
20-	55-14	SS	2"	60 -	some as about	-
	2545	15	16	50/1"	same arabone, destres gray	Advanced the sonic mount core to 21 bgs
11-	55-16	12	41	66	same as above, wet	
12	5514	\$2	sll	54	same as about, wet	
	\$14,2	22,	211	12	22.4 - Same as about 22.4 - Far cut red noyy wet, highly plants Bottom of borning at 22-5 bgs	(u), -
					the tra sale	1

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BRP-45 www.mrce.com BORING NO. SHEET PROJECT DUNDALK MARINE TERMINAL FILE NO. LOCATION BALTIMORE, MARYLAND SURFACE ELEV. **BORING LOCATION** DATUM Sonic Samp Dail Man 170: Spr sampling TEST/INSPECTION EQUIPMENT REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING** CASING USED TRUCK 6"00 MECHANICAL DIA., IN. DEPTH, FT. FROM SKID HYDRAUUC DIA., IN. DEPTH, FT. FROM TO BARGE OTHER DIA., IN. DEPTH, FT. FROM rack OTHER TYPE AND SIZE OF: DRILLING MUD USED YES V/NO D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER CORE BARREL AUGER USED NO CORE BIT TYPE AND DIAMETER, IN. DRILL RODS 2" LD Sonic Macro core CASING HAMMER, LBS. AVERAGE FALL, IN. 40 SAMPLER HAMMER, LBS. AVERAGE FALL, IN. 30 TYPE OF HAMMER ntomake WATER LEVEL OBSERVATIONS IN BOREHOLE DATE DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION 11112115 08:00 17-5 15 17.5 Standpipe Installation PIEZOMETER INSTALLED YES SKETCH SHOWN ON STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE 00, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL 00, IN. LENGTH, FT. BOT, ELEV. PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING LIN FT NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA, U-SAMPLE BORING UN. FT.

NO. OF 3" UNDISTURBED SAMPLES

OTHER:

HELPERS

Drilling

and testing

BRP-US BORING NO.

Phelen

DATE

DRILLER

REMARKS

CORE DRILLING IN ROCK

BORING CONTRACTOR

RESIDENT ENGINEER

UN. FT.

K. Charuredula



BORING NUMBER:

Sheet:

SOIL BORING LOG

PROJECT: Horeywell DMT 557

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Agents and the send decided the

DRILLING METHOD AND EQUIPMENT: Sales Sales Date House that the sales

WATER LEVELS:

START: 11/12/15

FINISH: 11/12/15

LOGGER: K-Chaduruedula

(H)	S	AMPI	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
1 1 1 1		370000	30		2" Asphalt 0.25'-1.25'- Poorly grand decrease entry Sornord sit (Gran) , dar gray (astri) Tour best aggregat, day 510)	Advanced has bording from S-1 work and the source of the s
11111	2.1	Spint who			128'-1735' - Sity senserus), Tercon- Gray (548 5/2) - most	byc -
11111					his'- Parily modern gravel with Sound and still correction, ground reply of well, Paril based aggregate	a) tre -
1 1 1 1					20'- 100'- Eldy GRANEL with Soul (Co Brown LL 24 (SYK5/8), ever,) DP(-
11111	3/2	Sept Called	36	-	4.0'- SILLY SAND (SH), group-brown (548 31)) , wet, fine to medium ground Sand	DFC-
1111		27480				
-						



BORING NUMBER: BRP-46

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

E SAMPLE		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
NUMBER STAPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
3 -			6-0'+2-0' - Same as about the	PRC-
SAMM MARIES CAN	24		2101. 7-25" - Arphald 2-25" - 85". Fromy graded and Corl. Prong (GISTI SIN 1", day, Recombined aggregate.	TW-
			fullum of boning a live ligs.	

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 www.mrce.com BORING NO. SHEET PROJECT DUNDALK MARINE TERMINAL FILE NO. LOCATION BALTIMORE, MARYLAND SURFACE ELEV. BORING LOCATION DATUM Sonie Samp Drill Max 170; Spi Sampling TEST/INSPECTION EQUIPMENT REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG **DURING CORING** CASING USED TRUCK MECHANICAL DIA., IN. DEPTH, FT. FROM SKID HYDRAULIC DIA., IN. DEPTH, FT. FROM BARGE OTHER DIA., IN. DEPTH, FT. FROM OTHER Trouble TYPE AND SIZE OF: DRILLING MUDUSED YES D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER CORE BARREL AUGER USED YES NO CORE BIT TYPE AND DIAMETER, IN. DRILL RODS 3" 20 Sovie Maero core CASING HAMMER, LBS. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. AVERAGE FALL, IN. TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER CONDITIONS OF OBSERVATION PIEZOMETER INSTALLED YES SKETCH SHOWN ON See Standpipe Installation STANDPIPE: TYPE ID, IN. LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE OD, IN. LENGTH, FT. TIP ELEV. FILTER: MATERIAL OD, IN. LENGTH, FT. BOT. ELEV PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING LIN. FT. NO. OF 3" SHELBY TUBE SAMPLES

NO. OF 3" UNDISTURBED SAMPLES

OTHER:

HELPERS

rilling and testing. The.

BORING NO. BRP-46

11/12/1

thelen

DATE

DRILLER

REMARKS

3.5" DIA. U-SAMPLE BORING

CORE DRILLING IN ROCK

BORING CONTRACTOR

RESIDENT ENGINEER

UN. FT.

UN. FT.

K. Chahurredula



BORING NUMBER: GP - IA

Sheet:

SOIL BORING LOG

PROJECT: Honeywell DAT SIZ

ELEVATION:

LOCATION:

DRILLING CONTRACTOR: AQUITER DRILLE NG PHOTESTING A KO DRILLING METHOD AND EQUIPMENT: Some Some Drill & Like to Set sampling -

3	SAI	MPLE	STANDARD		LOGGER: Kicket wedne
LOW		(m)	PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	RECOVERY	RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
1	Souls Parkers	36		1. Asphalt 0.3. Poorly graded gravel with Sand (Est gray (Olove) blood dry, Poord based oggingate. 0.75- Polyethylene liner	Advanced in boreholi from of it using sovie mass core 1 mps = 0.25" Low recovery
				210'- Poorty graded sand with grown (SP), dang ray (GLEY) 4/104), moist, read based aggregate LS'- Sitty Sand with grown (SN), Leel-Brown (J-5-12 4/4), moist, fire to medium grained sand	DPC - 1, mps=0.75"
	NAC PO	42		10 - same as above, dark brown- long black (7:59 R 3/3) Moist, the to medium grained sand,	DEC -



BORING NUMBER: GP- 14

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft.)		SAME	LE	STANDARD PENETRATION	SOIL DESCRIPTION	700.000			
DEPTH BELOW (ft)	NUMBER	NUMBER	NUMBER	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
111111	51	MACEG SORG	1/2"	-	Same as whome	RPC-			
		CONE				Below of switched to si sampling: 140-16 hammer 30 in drap; 3 dia sampler.			
111111	3	ILE MELLES	2U ^d]	8-11- crushed controle. 6-21- Sity sand with gravel (Sui). Gray. black (NI). dry. fine Granned Sand, proces of work mesh	8.0'- SS Spoon refusal? Advanced son't main core to 16' bgs.			
-		CE/SOMIC]	(GD, Red-Brown (7-51 R416), moisin	Mps= 1.25", pp1-			
	14	55	2M ⁰	- 1	10.01- Sility SAND (SAT with grave). Brown (2.5+R4/e), moist, fire to nedium grained sand, intermined with layer of grown (road baser)	9			
19				2)	12'- same as above. Red-Brown (I-54 R5/6)	DFC-			
1				14-					



BORING NUMBER: GP-IA

Sheet: 3

SOIL BORING LOG

PROJECT:

ELEVATION:

LOCATION: DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

	SAMP	LE	STANDARD	SOIL DESCRIPTION	
NUMBER	YPE ECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND	
-	-	611	q	12:01 - Same as a boro -	INTRUMENTATION DPc -
ççlı	55	3	1.5	Same as above , trace copp	DPC +1_
Sert	SS	2"	7 -	same as above, piece of grown	mps=1-25" ppc-
55-8	دې	44	9	135'- saraly CLAY (CLI, Red-Yellow Brown (250RS/8), moist, SOR	DFC-
559	52	60,	8	same as above	DPC-
25-10	22	Gu	6	Same as about	PPC -
				Bottom of boring @ 150' bgs.	installed to 15/bgs
֡֡֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜֜	SSS NUMBER	25 25 25 25 25 25 25 25 25 25 25 25 25 2	NUMBER 19 55 55 55 17 17 18 18 18 18 18 18 18 18 18 18 18 18 18	PENETRATION TEST RESULTS 6"-6"-6"(N) 555 55 611 9 557 55 2" 7 557 55 41 9 557 55 41 9 557 55 61 6	SOIL DESCRIPTION TEST RESULTS SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY SS S S S S S S S S S S S S S S S S S

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 www.mrce.com BORING NO. SHEET PROJECT DUNDALK MARINE TERMINAL FILE NO. LOCATION BALTIMORE, MARYLAND SURFACE ELEV. BORING LOCATION DATUM TEST/INSPECTION EQUIPMENT Sonic Some Doll At May 170: SPT REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED TYPE OF BORING RIG DURING CORING CASING USED NO TRUCK MECHANICAL DIA., IN DEPTH, FT. FROM TO SKID HYDRAUUC DIA., IN DEPTH, FT. FROM TO BARGE OTHER DIA., IN DEPTH, FT. FROM OTHER 1000 TYPE AND SIZE OF: DRILLING MUD USED YES D-SAMPLER DIAMETER OF ROTARY BIT, IN. U-SAMPLER TYPE OF DRILLING MUD S-SAMPLER CORE BARREL AUGER USED YES NO CORE BIT TYPE AND DIAMETER, IN. DRILL RODS CASING HAMMER, LBS. AVERAGE FALL, IN. SAMPLER HAMMER, LBS. WO AVERAGE FALL, IN. 30 TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE DATE TIME DEPTH OF CASING DEPTH TO WATER DEPTH OF HOLE CONDITIONS OF OBSERVATION NE PIEZOMETER INSTALLED VWF Installation NO SKETCH SHOWN ON STANOPIPE: ID, IN LENGTH, FT. TOP ELEV. INTAKE ELEMENT: TYPE 00, IN LENGTH, FT. TIP ELEV. FILTER. MATERIAL 00, IN. LENGTH, FT. BOT. ELEV PAY QUANTITIES 3.5" DIA. DRY SAMPLE BORING LIN FT NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA. U-SAMPLE BORING UN. FT. NO. OF 3" UNDISTURBED SAMPLES CORE DRILLING IN ROCK UN. FT. OTHER: BORING CONTRACTOR TOCHY A DRILLER HELPERS REMARKS RESIDENT ENGINEER DATE

BOR-4_JAN2013

GP-IA

BORING NO.



BORING NUMBER: SP-2A

Sheet: |

SOIL BORING LOG

PROJECT: Horaywell DMTSI

ELEVATION:

LOCATION:

DRILLING METHOD AND EQUIPMENT: SONIC SAMP DRILLYCHAP 170.

DRILLING CONTRACTOR: AQUIFER DRILLING AND TESTING, EN

WATER LEVELS:

START: 12/19/15

FINISH:

LOGGER: Kchaturveduk

.			SAMP		STANDARD	08:00	LUGGER: Kehaturvedula
Ì	V. (π.)	<u> </u>	/ NIF		PENETRATION	SOIL DESCRIPTION	COMMENTS
•	DEPTH BELOW	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, ° TESTS AND INTRUMENTATION
	3-	5-1	SOUTH MACRO CAPLES	ůч		u" Asphalt 0.3! Poorly graded sandwith gravel (SP), gray-brown, (GLEYIG/row), dr Road based aggregate 0.8! - Silty SAND with gravel (SN), Red-Brown, moist, medium to Coane grained Sand 2.0! - 2.3! - Same as above, dance gray-tlack 2.3! - 2.8! - Same as above, gray, Road based aggregate 2.8! - Silty SAND (SN), Red-Brown (Z-SYR UB), moist-time to medium grainer! Sand (COPR and non-(opr mix)	Advanced the borchole from 0 L81 using 3 PD Sonic maero core. DPC
1.	5-		ADVIC MACKS CARE	U8"		gravel (SC), Red. Brown Gravel (SC), Red. Brown (7.54R 4/6), Moist, fine to Medium grained same 4.3'-5.8'- Fat (LAY (C4), Red (107 4/4), Moist, Stiff to Very Shiff, highly plashe. 5.2!- Silty SAND (SM), Reddish 5.70WN - Greenish (7-54R 4/4),	DP1-



BORING NUMBER GP-ZA

Sheet: 2

SOIL BORING LOG

PROJECT

ELEVATION:

LUCATIONS

DRILLING CONFRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START

FANISH:

roegen:

						And 63 24 6 2 7	L. MADONE.	The Control of the Co
		-	AMPI T	70.00	STANDARD PENETRATION	SOILDE	ECRIPTION.	COMMENTS
	NO THE HADDE	NUMBER	ACAL	REGOVERY (#)	6"-6"-6" (N)	COLOR, MOISTURE DENSITY OR CO	S CROUP SYMBOL, CONTENT, RELATIVE DISSITENCY, SOIL WINERALOGY	DEPTH OF CASING, ORILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
entere en 11 e mandy et is winderen enterprisent januarie	6-	an san sine production states to the states and the	CORE			moist, fine to mo sand, particulal Indurated (HB	e to moderately	DPC+
	7-	sn	SOMIC MACRUCO	480		681. Same as all slightly to mod	now, dry, erately linduated	TR F
The same of the sa	- B		V		1.		•	80'-switched to ss
	o -21 -	513	gs.	170	50 4"	No Recovery 8.31- Same aso	ubove (HB (OPR)	Sampling below 80%; 140-16hamner; 307ndrp 3-india Sampler. Ss-refusal at 8-25/696
والمدودة والمتعارض والمتعارض والمتعارض والمتعارض والمتحاطية والمحاج كالمتحاص والمتعارض والمتعارض والمتعارض	9		core	4	The second secon			Advanced the hole to 10'bgs using sonic mach core— No recovery in the SS Sampler
leased and the second second	10			-	-			de de la companya de
mandandia and the second of th	~		SS.	newis	27 - 84	10.01-10.5'-same moderately ande 10.5'-same as a brown, gray	e as above; wrated bove, graenish-	Bedding patturn observed in the spoon [1:0] - 6" op outer casing
Separation and a separation of the second se	The state of the s	55-6	\$5	The state of the s	ł,	tho'-same as a brown-gray, particles ecope	hatalit a sa	of 10. d bgs.
1	12			-	and the second s	mandamang kili - a tana langa a ta 1924 ta p <u>aga ang ang ang ang ang ang ang ang ang </u>		-



BORING NUMBER: GP-ZA

Sheet: 3

SOL BORNOLIGE

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

BRILLING METHOD AND EQUIPMENT:
WATER LEVELS:

START:

FINISH:

LOCKER:

			White both course when the		The same of the sa	Circle. Press.	
	E	-	SAMPI	-	STANDARD PENETHATION	SOIL DESCRIPTION	COMMENTS
***************************************	DEPTH BELOW (#)	NUMBER		RECOVERY (#)	7EST RESULTS 6"-6"-6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SCAL STRUCTURE, MINERALOGY	TEPTH OF CASING, MILLING TATE, DRILLING FLUID LOSS, TASTS AND MIRLING FLUID LOSS,
b	12	3556	SS.	6	25	Same as abone. (GRCOPE)	DY(+
		2-7	SŞ	4.11	!	12.5'- same as above	DPC
	13	22-8	<i>5.</i> 5	NR	7	No recovery	
7	- 14 -	55-9	דצ י י	3 ^u	3	Pat CEAY COST, Red (1074/4), o Soft to medium shiff, highly plash's	vet DPC-
al-Basin wed in communication	-	-			en en en en en en en en en en en en en e	Bottom of Boring @ 14.01695	
الشهيسة ازن سرية أسرها استسهاب	*** ***		and in the party of the party o				
and a residence of the second		To debut the second sec	The state of the s	e er testina en está en especia de la companya de l		The state of the s	
and Associations Confirm the major	en der en en en en en en en en en en en en en	According to the second	- The section of the	weth definitional and designations of the		The second secon	
en hannagan against de	247	ghalberg very increase in consequents	i belep (fink) de valente un verbf as as planes	والمراقبة المراقبة المراقبة المراقبة المراقبة المراقبة المراقبة المراقبة المراقبة المراقبة المراقبة المراقبة ا	estremy	Control of the Contro	
remondational descriptions	the section of the se	and viry (river) decima partematic trace	n enderson in taken pane	فاعدوه واسترقه ميانيوسة	agendington and a		and the second s
·	Lumburgham	York M. Wildigs mayble and female.	رافشاده وواليطونية الادامة	is any study aparents of Agrees pages	shows a monopolity of the state		The state of the s
				ried in manufacture of the latest	and the second s		Transaction products



BORING NUMBER: GP2C

, SOIL BORING LOG

PROJECT:

ELEVATION:

LOCATION: DRILLING CONTRACTOR

TAILLING METHOD AND EQUIPMENT: WATER LEVELS:

	(F)		HAN	I.E	STANDARD	STAR 12 17 15 FINISH: 12 17/15	Chaturnd.
Allen Blanch	NE OS		•	Ē	PERETRATION TEST RESULTS	SQIL DESCRIPTION	COMMENTS
- C	HLATE	NUMBER	TYPE		6"-6"-6"(N)	COLOR, MONETURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
Andread restant is between the secretarity of the second secretarity is the second sec		endrigiji mendasyn servinon nyazisehi juginasya i internas	: ¥		The second secon	4 b A sphalt	Advanced the borchole two o'-go bgs wing 3' 20 Sonic maens core No recovery in the
a made de marche administrativos de propries consume		5)	der Co	7	The state of the s	e de la companya de l	trist sonic spoon - Hard drilling conditions - Concrete duet
2	and the same franchischer of the same of t	Gridentiantidaniamiamiamiamiamiamiamiamiamiamiamiamiami		eterminativa andeza esta esta esta esta esta esta esta est	The second secon		emilted when the drillers opened the Ex Sonic maero core and lock the samplo clown the auto.
3 -	and the state of t	e de la companya de Companya de la c	Offers any advicables of the grant state of the state of	ti di manung kuluan semeri di magamak kemesi kuman kuta kati kati	of were under make many the second has so	Andrew or deliver many company and a contract many and a contract	
The state of the s	5-2	Are transmission and a second and a second and a second as a secon	4	the state of the s	Jr Iv	3 7 F- 9 / L / 300 m / L + 3	pp(+1) mp(=1.0"
Lamenteigen		FONC N	3	Gerballer van Seine van Valle van Valle van Van Van Van Van Van Van Van Van Van V		- Stily spron (sn). red-brown	



BORING NUMBER: GP-2C

Sheet: 2

SOIL BORING LOG

PROJECT:

: ELEVATION:

LOCATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START: FINISH:

<u> </u>			Sum de Pari		#TANDARD		
***	11.			tereterres tendese	PENETRATION	SOIL DESCRIPTION	COMMENTS
A COTE PER CAR		M. C. S. C. C. C. C. C. C. C. C. C. C. C. C. C.		RECOVERY (M)	TEST RESULTS 6"-6"-6" (N)	SOIL NAME, USES GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
6	7	λ	coric			Cars NR 4/16), dry, fine to medium grainel sand, particulate to slightly indurated (HB corr)	DPC+
7	7	tek yile yer amalikana wan asayikir iddira.	whe		•		
C	and the second has seen and and and	er kitteteres hypi Vila omespopeles (s. 1897-bland		- - - - -			JJ-Below 8' switched SS sampling 1100-16 he Boindrop: 3"dia sam
CXO	A Second Second	C C C C C C C C C C C C C C C C C C C	([8"	12	Erol same as above, greenish. gray, polar wet (COPR)	Bedding Structure observed in the spoon
4	Contraction of the second			The state of the s	21 - 29 -	g.o'- bright yellow and green particles.	DPC-F
	المكسفونات المسادمين	er web by the Homestate of Company of the Selections	d taining ability special steps, at speps, we should	A STATE OF THE STA			100'-6' op outer casing
The state of the s	- T	A	55	O statement of the stat	22 - 50/4" -	10.0'_ clayey sand (sc), but to ted- white (10441a), dry, fine to medium) sand, trace growed coope and non-work 106'-to-silty s AND (sM), greenish gray, moist, time to medium	installed to lo bgr. PPCfl— 10.8'- ss refusal; some macro core a drenud to
Parties of the State of Language of State of Sta		- A Company of the Co		12 by		sand, particulate (core) from some as above, moderately	12' 695 , BPC+ focalding of different layers of Corr observed
	2		Co.				greenish gray,



BORING NUMBER: GP-70

Sficet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

DAYLLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

	Andrew With Street	2	Ţį.	•	FINION.	LWGGEK:
NUMBER NUMBER TYPE SALOW(ff) SALOSTER SALOW(ff) SALOSTER		-		SOIL DESCRIPTION	COMMENTS	
		RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION 12'0' - Groundwater		
7.	sslo	35	6	13	12.0. Poorly gradeds and with Silf (SP-SN), gray-black- greensh, wet. Parkenlate to Blightly inducated (GBCOPF)	ppc+ encountered during drill
- 1	55-J	55	6''	16.	same as above	DPC+
the state of the s	gs-8	SS	2"	16	13.0'. Sand clay (CL), Red (101) 4/4), time moist, time to medium grained sand	<u> </u>
	55-10	SS	N' 1	6	grained sand 13.5- Fat CLAY (CH), Red (1044), wet, shiff, reachighly plashic	Dre -
3		The state of the s	discheritation and language (Bottom of boring at 14.0 bgs.	
	a para significant para para para para para para para par		Elaster biogram majoriso, que de jos	State of participation of the state of the s		
4	and of the second second second second second second second second second second second second second second se		en en en en en en en en en en en en en e		Diaghia an 2 4' Bott COPL 12.5	SAA 92889
mental and makes	agureno de arrumo como de acesto de acesto de acesto de acesto de acesto de acesto de acesto de acesto de aces		ed ill be assert of them are two time the	and in the control of	thurmistor 28	Installed at 13 dept
The second secon	rider trialler i permeganyaj inervojve este	bergeles dispersuas as designer	ee symmem Dig sal etympyca Ukramagadi	The state of the s		
Andrew Section -	18 alie f réidheis-afficach Kenry is	ottennen er dinnen general keinno ist	iek Lamen, so iek czająci na so aską feminoski na so so iek czająci na so aską feminoski na so so iek czająci na so aską feminoski na so so iek czająci na so aską feminoski na so so iek czająci na so aską feminoski na so so iek czająci na so aską feminoski na so so iek czająci na so aską feminoski na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so so iek czająci na so iek	ming or a common of the common	Extraording from a final back.	
A CT SECTION AND A COMPANY OF THE PARTY OF T	od decembratische (2 best decembratische 1	Section (Assessment Season and Assessment Season and Assessment Season and Assessment Season and Assessment Se	mentinging arang design on personal	der de la companya de		
7-7-7	raind sales on Mongai chet sales.	edajari alberralasi menege e Di Bali dan	e en en en en en en en en en en en en en	all the second of the second o	And the state of t	



BORING NUMBER: GP-3A

Sheet:

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION: ~ +13

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS: ~ 19.2

START: 0745 FINISH:

LOGGER: L.LINCOLN

				1. 2	LINCOCK		
£	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
0	5.1	Sowic	20"	14/4	0.0.4 ASPHALT 2" CONCRETE BRN, GRY SILTY C-F SAND, SM GUL (FILL)	NO DPC RXN	
3 -					3-3.9 RED CLAY, MOTT. WHITE, TR GVL 3.9-6.0 BRN, GRY, GREEN MOD. LITHIFIED SILTY F.C. SAND, SM GVL (HB)	DPC+	
5 -	S·2			N/A			



BORING NUMBER: GP-3A

Sheet: 2

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ + 13

DRILLING CONTRACTOR: 5D5

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS: 3.8' 695 ~ + 9.2

START: 0745 FINISH: 0900 LOGGER: L.LINCOLN

£			STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS		
DEPTH BELOW (ft)	NUMBER	гурЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
4 -	5.3	SONIC	12"		TOP 3" RED BRN GRY MOD LITH SILTY SAND, SM GVL, PLANT FIBERS (HB) BOT 9" TAN, BRN, GRY, GREEN SILTY F.C SAND, SM GVL (HB)		
	55-4	2000	9"	17	TOP 3" DO S-3 BOT (HB) BOT 6" RED BRN, BUL, GRM, GREEN SILTY F M SAND, TR GUL, RED CLAY (HB)		
\\\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\	455		li ⁱⁱ	5	FIN SAND, SM SILT, WET		
9 -	4,10		5°	4 7	9-9.5 DO SS-5 BOT. (GB)		
10 -	55		6" 6"	33	9.5-10 DO SS.5 BOT. (GB) 10-10,1 DO SS.5 BOT. (GB) 10.1-10.5 RED. BRN, GRY, YELLOW SILTY F.M. SAND, TR. GVL (HB)		
u	- 50		U"	29	10.5-10.8 BLK F.M SAND, SM SILT (GB) 10.8-11.0 RED. BRN, GRY SILM F.M SAND, TR GVL (HB) 11-11.5 DO SS-9 (HB)		
12	S			3 -	11.5-12.0 RED WAY MOTH	EOB @12-0'	

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET FILE NO. 10587 MARINE TERMINAL PROJECT DUNDALK **SURFACE ELEV.** -113 LOCATION MARYLAND **DATUM BORING LOCATION TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED ✓ NO YES **CASING USED DURING CORING** TYPE OF BORING RIG DEPTH, FT. FROM TO DIA., IN. MECHANICAL TRUCK TO DEPTH, FT. FROM DIA., IN. / HYDRAULIC SKID DEPTH, FT. FROM TO OTHER DIA., IN. BARGE OTHER YES DRILLING MUD USED TYPE AND SIZE OF: DIAMETER OF ROTARY BIT, IN. 2" O.D. SPLIT SPOON D-SAMPLER TYPE OF DRILLING MUD **U-SAMPLER** S-SAMPLER J NO AUGER USED YES CORE BARREL TYPE AND DIAMETER, IN. **CORE BIT DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. 40 AVERAGE FALL, IN. 30 SAMPLER HAMMER, LBS. TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE CONDITIONS OF OBSERVATION DEPTH OF HOLE DEPTH OF CASING DEPTH TO WATER TIME 3.8 0900 11/3/15 **SKETCH SHOWN ON** YES NO PIEZOMETER INSTALLED TOP ELEV. ID, IN. LENGTH, FT. STANDPIPE: TYPE TIP ELEV. LENGTH, FT. TYPE OD, IN. INTAKE ELEMENT: OD, IN. LENGTH, FT. BOT, ELEV. MATERIAL FILTER: PAY QUANTITIES NO. OF 3" SHELBY TUBE SAMPLES 3.5" DIA, DRY SAMPLE BORING UN. FT.

NO. OF 3" UNDISTURBED SAMPLES 3.5" DIA. U-SAMPLE BORING UN, FT.

OTHER: CORE DRILLING IN ROCK UN, FT.

BORING CONTRACTOR	SONIC	DRILLING	SERVICES		NAMES OF TAXABLE PARTY.
DRILLER 1	PHA	HELP	ERS WARRE	<u> </u>	
REMARKS				THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER.	
RESIDENT ENGINEER	UNSANDRA	UNCOM	DATE	11/3/19	5

GP-3A BORING NO.



BORING NUMBER: GP-36

Sheet: Charge 1

SOIL BORING LOG

PROJECT: Honeywell THAT ISI

LOCATION:

DRILLING CONTRACTOR: Aquifer drilling and teching Inc.
DRILLING METHOD AND EQUIPMENT: SONIC STALL DRILLING FINISH: 170 SET SAMPING
WATER LEVELS:

DRILLING CONTRACTOR: Aquifer drilling and teching Inc.

START: 11/2/15 FINISH: 1/2/15 LOGGER: K-Chaturvedula

3	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
11111111	5-1	icko colue	2)"		2" Asphalt 02'-1.2' - Poorly graded Genvel with sand (ap), black and gray (10 yr 411), dry, coarrigrained sand, road based aggregate 1.2'-2.0' - Poorly graded Shoo (sp) moist, tan-white I medium grained sand, trass grave		
1 1 1 1 1 1		SONTE MA			2.01-3.01- silky card with gravel Red-gray (1048 714), moist, time to medium graned sand.	(SH)	
3 -					3161- Stity Emili with growel (SN), darkgroy (NZ), dry, midrum to char sand.	Advanced from 3-61 using sonir macro core 3'20	
1-	5,7	SONIC MACRO CORE	13,,		3:51-4:0'- Fat CLAY (CH), Red (1014)4), movered, highly Plastic- 4:0'- silty cand (SN), Red-Brown and greenich (1044)4), dry, tine tomedown sand, Particulate (HB LOPR)	TOTAL STORES OF THE STORES	
5 -							



BORING NUMBER: GP-3C

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

Œ	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER		RY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING	
DEPTH		TYPE	RECOVERY	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
1111	5.3	SOMIC MINERO	12"		Sity smoots M), Red-Brown and green's W (10 YR 414), dry 1 Hing grained soud, particulate (HB COPR)	paranceal from 61-91 using 31 20 son in mocro	
1	SELL		124	12	7-8-7-3- same as above 7-31-7-61- sandy lean clay (EL) Red-edite (1044/4), moist.	70'- ground water so bys during drilling Below 70' Switched to	
	s ^S		q!	9	76-80-SILY SHUD (CM), Brown and gray (1048 4/3), greenith. Particles, & most, the grained sand Particulate (HB copp.) 801-Poorly graded SAND with SIH (SPLSM), dairingray and black (N2), wet, Particulate (ABCOPE)	SS Sampling : 140-16 hamm 30 m drop : 13/2 de song	
-	ه خې	55	311	ta 8	sam as above	Installed the 6" ab	
4	5.7	SS	3"	6	wet, highly plastic) outer casing to 10' bgs	
111111111					Botton of Bonny @ 10' has		



BORING NUMBER: 91-44

Sheet: \

SOIL BORING LOG

PROJECT: Honeywell DMT SSI

ELEVATION:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

LOCATION:

DRILLING CONTRACTOR: Aquifer Drilling and testing Inc.

START: 114/15

FINISH: 114 15

LOGGER: K. Chasturvedula

	T^{-}	241401	_	OTANDADO	67:40 lo:04		
W (ff.)	—	SAMPI		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
2-3-	2-1	SOMIC MATERS COLE	24	- - - -	2" Asphalt 0.2'-1.0' _Pooly graded GRAVEL With Sand (GP), darkgray (104R3/1) dry, coarse grained sand, road based aggregate. 10'-2.9'-Poorly graded sprip with clay and granel (Sp. sc), tanwhite (104R8/1), moist, medium to coarse grained sand 2.5'-3.0'- sandy lean CLATCCL), Red- white (1044/4), dry. Silty SAND (SM), Red-Brown (104R5/4) dry, fine to Coarse grained sand,	-Portion of the Sample last while	
4	S-V	SONIC MACED CORE	Š		Particulate (HB COPK)	pulling up the Sonic. Macro cure. - Dipheny Carbazide. Used to test the presence of cope.	



BORING NUMBER: GP-4A

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

			EVEL	J .		START: FINISH:	LOGGER:	
ſ	(£)/	5	SAMP	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
	DEPTH BELOW (ft)	ER		RECOVERY (#)	VERY (ft	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
	DEPTH	NUMBER	TYPE	RECO	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION	
	6 -					same as above, slightly to moderate indurated	ey	
	-	,	3700	26"				
	7 - -	5-3	SONIC MACRO		_			
	-	·	Some		-			
	8 -				_			
	1				. -			
	3 -	4	Q			Same as above, Particulate to slightly induvated		
	-	يدي	SONIC MACEO CORE	12"	-	and and a surder sea		
+	0 -		Sor		, "	same as above, wet	Below 10' Switched to	
	- - - -	5.5	ζς	n'	62 41	Source out of the second	SS sampling; 140-16 hanner 30-in dop, 134" dia Samplex -Installed the 6"ob outer	
,	<u> </u>					Ponds and I gave with all form	Casing to 10' bgs-	
	-	مري	ري	5"	(9	Poorly graded samp with sill (cm), lark-gray and black (N2), wet, Particulate (aB copp)	during drilling at 10 bgs.	
1	ν				19			



BORING NUMBER: GP- 4A

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

	Т-	SAMPLE STANDA			THUSH.	LOGGER:		
. (₹) (₹)		SAMPL	т——	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS		
DEPTH BELOW (ft)	NUMBER	ТҮРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION		
12-	-557	çŞ	12"	35 34	1 Silty SAND with gravel (SM), Brown and gray (7.54R 413), moist, Moderately to highly indurated (HB: COPR)			
13-	55-8	22	Į V	50/24 -	Clayey SAND (SC), dark gray-black(N wet, fine grained Sand, Particulate (GB COPR)	2),		
14-	55-9	S.S	£°	6	13.5'- same as above 13.9'- 14.0' - Fat clay(cH) mixed with abcope, wet			
-					Bottom of boring @ 14.0' bgs	•		
15 -								
-				-	en en en en en en en en en en en en en e			
-		·		-				
_		نو ا			•			



BORING NUMBER: GP-4C

Sheet: 1 oF

SOIL BORING LOG

LOCATION: SEE PLAN

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE P ELEVATION: ~ +1+ DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRICL XL MAX 170

WATER LEVELS: ~ 15

START: 1015

FINISH: 1120 LOGGER: L. LINCOLN

					· -	
€	s	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТҮРЕ	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 - - - - -	5-1		24"	N/A	4" ASPHANT CONCRETE, GRY BRN C AGGREGIATE	NO DPC RXN
2 -		CORE		- - -	2-2.7 BRN, TAN GVLY C.F SAND, SM SILT (FILL)	NO DPC RXH
3 -		ONIC			2.7.3.0 RED CLAY MOTT WHITE, SM GVL	FABRIC GEOTEXTILE @2.7'
- - - - -		SON	26	- A/L4	3.3.5 TAN, GRY SILTY F.M. GAND, SM. GVL (FILL) 3.5.5.0 RED.BRN, GRY SANDY CLAY, TR. GVL, YELLOW PARTICLES	NO DPC RXN
5 -	5.2			-	5.0 - 4.0 RED. BRN, DK GRY WHITE V. STIFF CLAY, SM F.M SANID, TR. GVL, YELOW STREAKS	DPC7



BORING NUMBER: GP-4C

Sheet: 2 or

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PELEVATION: ~ +14 DRILLING CONTRACTOR: ADT LOCATION: SEE PLAN

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MIAX 170

WATER LEVELS: ~ 45

START: 1015 FINISH: 1120 LOGGER: L.LINCOLN

£	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
لر — - -				-	6.6.8 BRN, LT, GRY CLAYEY F.M SAND, TR GVL MOD. LITHIFIED	DPC+
7 -				- - -	U.S. 7.0 GREEN, GRY, BRN F.M SAND, SM SILT, GVL WEAKLY LITH	DPC+
-	5.3	١.,	3'	H/A -	7.0.8.5 RED.BRM, DKGRY SILTY F.M SAND, SMGV, YELLOW STREAKS, MOD, LITHIFIED (HB)	DPC+
8 -		CORE		-		
- - 9 -		JINAS		-	8.5-9 BLK, YELLOW, GREEN F.M. SAND, SOME SILT (MOD LITHIFIED) (GB COPR)	DPC 1
-	5-4		12"	N/A	9-10 RED. BRM SILTY F.C SAND, SM GVL, TR. GREEN PARTICLES, WEAKLY-MOD. LITH, MOIST (HB)	DPC+ SAMPLE 5-4
10 -	a. 6	2" 0.D. 5.S		-	NO RECOVERY	·
11 -	7			- - - -	11-12 BRN, DK GRY, YOLOW. ORANGE SILTY F.C SAND, TR. GVL (HB) MOD. HIGHLY LITHIFIED, MOST	SPOON PUSHED TO 12', SAMPLE SS-5 12'2" TO 13'2"



BORING NUMBER: GP-4C

Sheet: 3 of

SOIL BORING LOG

7

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +1나

DRILLING CONTRACTOR: ADT

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: ~ +5

START: 1015 FINISH: 1120 LOGGER: L. LINI COLM

E	s	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТҮРЕ	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12 - 13 - 14 - 15 - 14 - 17 - 17 - 17 - 17 - 17 - 17 - 17	SS-5	NO S	12"	19 -	12-12.7 DK GRY, GREEN F.M SAND, TR EILT, MUIST (AB) 12.7-13.0 RED CLAY MOTT WHITE 13-13.1 RED CLAY MIXED WY DK GRY GWY SAND, SMELL	EOB @13.1'
18-	-			_		

Mueser Rutledge Consulting Engineers 14 Penn Plaza • 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 BORING NO. www.mrce.com SHEET TERMINAL FILE NO. 105B MARINE **PROJECT SURFACE ELEV.** +14 LOCATION DATUM **BORING LOCATION TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED **CASING USED DURING CORING** TYPE OF BORING RIG TO DEPTH, FT. FROM DIA., IN. MECHANICAL TRUCK то DEPTH, FT. FROM DIA., IN. HYDRAULIC SKID TO DEPTH, FT. FROM DIA., IN. OTHER BARGE OTHER NO YES DRILLING MUD USED TYPE AND SIZE OF: DIAMETER OF ROTARY BIT, IN. D-SAMPLER TYPE OF DRILLING MUD **U-SAMPLER** S-SAMPLER ✓ NO YES AUGER USED SOLIC CORE BARREL TYPE AND DIAMETER, IN. **CORE BIT** DRILL RODS CASING HAMMER, LBS. AVERAGE FALL, IN. 140 AVERAGE FALL, IN. <u>30</u> SAMPLER HAMMER, LBS. TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE CONDITIONS OF OBSERVATION DEPTH TO WATER DEPTH OF CASING DEPTH OF HOLE TIME DATE INSTRUMENTATION 9.0 BEFORE 13,0 13.0 P6. 5 **SKETCH SHOWN ON** PIEZOMETER INSTALLED 2 VWPS TOP ELEV. ID, IN. LENGTH, FT. TYPE STANDPIPE: TIP ELEV. LENGTH, FT. OD, IN. TYPE INTAKE ELEMENT: BOT, ELEV. LENGTH, FT. OD, IN. MATERIAL FILTER: **PAY QUANTITIES** NO. OF 3" SHELBY TUBE SAMPLES LIN. FT. 3.5" DIA, DRY SAMPLE BORING NO. OF 3" UNDISTURBED SAMPLES LIN. FT. 3.5" DIA. U-SAMPLE BORING

OTHER:

HELPERS

DRILLING AND

LIN. FT.

AQUIFER

PALONIEQUE

LYSANDRA LINCOLN

BORING NO. GP-4C

DATE

DRILLER REMARKS

CORE DRILLING IN ROCK

BORING CONTRACTOR

RESIDENT ENGINEER



BORING NUMBER: 97-68 Sheet:

SOIL BORING LOG

PROJECT: Horogenell DMT SST

DRILLING CONTRACTOR: Aquifer driking and testing, Inc.

DRILLING METHOD AND EQUIPMENT: SONIC SAM P DRILL XL MAN 170, SPT Sampling

WATER LEVELS:

START: 11/15 FINISH: 15:30 LOGGER: K. Chaturredula

(£)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
ELOW			RY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY	6"-"6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0 -	1-2	SONE MAKED FORE	244		3"-Asphalt 0:25'-3' Poorly graded SAND with gravel (SP), gray-brown (254 4/3), moist, medium grained Sand.	Advanced the top3' with 3' ID Sonic macro
2-		105			3'-375' same as above	3.d-6.d. Advanced wring
3	5-7	SOME MACED CARE	30"		3.25'-3.8'-Poorly graded GRAVEL with tenshite (7.57K8/1), moist, coarse grains 3.8'-4.8'- Fat clayeous, Red (1044 moist, highly plastie. 6.3'- Silty Share (5W) Red-Brown (2548'4), dry. trace gravel, particulate (COPR)	-



BORING NUMBER: GP GA

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT: WATER LEVELS:

START:

FINISH:

3	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS	
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION	
	\$3	SCHOR MINISTERIE	33"		Gol-GIST Tooky graded GRAVEL with sand (GP) gray. brown (ISTR 5/3), wet (COPR) 6.75'-Pourly graded sampwith grove 1 (Red-Brown (ISTR 5/6). dry (HBCOPR)	poeto preparativa di Grani esperi	
3 -	4-ري	SS	[2"	46 50	(2.5 YR 4/3) 3.0 - Sity Short with graval (50), Red-Brownard greenishs dry, Slightly to moderately indurated. (HBCOPR)	g.o'- Below 9 o' switched to se sampling; 140-16 hammer; 30-in drop; 3" sampler 6" op casing indalled to a depth of 10 bgs	
0.	3-5	\$\$	12	[8 37	Some as above		
11	- 55	· 55		46	- 11 0'-115' same as above 11.5'-12-0' - 2:11 samb (sw), dark gray-black (N2), moist. on fire to incidivin grained sand. Porticulate (GBCOPR)		



BORING NUMBER: GP-6A

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(ft)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
2 -	55-7	22	611	40 .	same as above, moist	during drilling
	8-23	22	61	38	Same at above, wet, greenish. brown (75tr 4/3), slightly indurated.	
3	55-9	s.s	211	21	same as above, wet	
4-	55-10	55	64	8	13-6- Fat CLAT (CH), Red (107 4/4) highly plastic	
					Boring terminated at a depth of	



BORING NUMBER: GP-6C

Sheet:

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ +16 DRILLING CONTRACTOR: 505

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: ~ +5.3

START: 1100 FINISH:

LOGGER: L. LINCOLN

	SAMPLE			13.3		
(£)	S	AMPL		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (#)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
o _				-	0.0-0.2 ASPHALT 0.2-1.3' BRN SILTY F.C SAND SM GVL	4" DIAM, CASING 0-10"
l =	S-1		2'6"	- - - N/A -	1.3.3' GRY, BRN C.F SAND, SM SILT, GVL (FILL)	
2 -		CORE		- - - -		
3 -		" SOMIC		- - - -	3.0-4.0 GRY SILTY F.M SAND, TR GVL (FILL	ı
4 -	.5.2	<i>₹</i> Ω	3'	14/A -	4-5,5 RED CLAY MOTTLED YELLOW, WHITE, TR F-M SAND SEAMS	PP = 2.6
- 6 -		-	·	- -	5.5-6' BRN GVLY C.FSAND, SM SILT (HB)	Dec+



PROJECT NUMBER: BORING NUMBER: GP 6C

Sheet: 2

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ + 16 DRILLING CONTRACTOR: SDS

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: ~ + 5.3

START: 1100 FINISH: 1230 LOGGER: L. LIN COLN

(£)	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (#)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RÁTE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
7					6.0.6.5' RED CLAY MOTT YELLOW, WHITE, SM CF SAND 65-8.5 WEAKLY LITHIFIED BEN, GRY SANDY GVL,	NO RXH
7	S·3	SONIC	3	N/A -	SM SILT, GREEN PARTICLES (HB COPR)	DPC+
8 -						
9				- -	SILTY F. C. SAND, SM GVL, GREEN PARTICLES (41B)	DPC+
-	ss પ્	3" DIA, SS	12"	- - -	9-9.7 BRN, GRY, TAN SILTY F-M SAND, TR GVL (HB)	DPC*
lo -	,			30 <u> </u>	9.7-10 BLK, MADD F.C SAND, SM & GYL, TR. SILT, MIXED W/ RED CLAY (GB) 10-10.3-BRN, GRY SILTY F.M.	
-	S\$-6		12"	12 -	SAND, TR GVL (HB) FM,- 10.3-11' MOD LITH, DRGRY, GREEN F M SAND, SM SILT, TR GVL (GB)	
-				9 -	11-11 4 DO SS-5 BOT (GB)	DFC*
-	SS-V			24 _	GRY, GREEN GARRY F.C SAMD, SM GVL, SILT (HB)	DPC*



BORING NUMBER: GP-6C

Sheet: 3

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ 116 DRILLING CONTRACTOR: SDS

DRILLING METHOD AND EQUIPMENT: FRASTE MULTIDRILL XL MAX 170

WATER LEVELS: 10.7 bg 5

START: 1100 FINISH: 1230 LOGGER: L. LIN COLN

1	E L	S	AMPL	~ ₁ € E	STANDARD	SOIL DESCRIPTION	COMMENTS
	DEP IN BELOW (II)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
17	- 50	s-ๅ 	3" DIA SS	1"	<u>-</u>	MOIST WEAKLY LITH BRN, GRY F.C SAND, SM SILT, TR. GVL, RED CLAY (HB)	
13	1	s- 8		4"	4 -	MOIST MOD LITH DK GRY, GREEN SILTY F M SAND (GB)	
	5	55-9		6"	2 -	13-13.2 DOSS 8 13.2-13.5 RED CLAY, SM - GUL, F-M SAND SEAMS,	NO DPC RXM
14					- -		EOB@13.5'
	-				- -		
15							
ار	, <u>-</u>				- , . <u>-</u>		
1-					-		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400 **BORING NO.** www.mrce.com SHEET DUNDALK MARINE TERMINAL FILE NO. 105 PROJECT SURFACE ELEV. +110 LOCATION BALTIMORE MARYLAND DATUM **BORING LOCATION TEST/INSPECTION EQUIPMENT** REFERENCE CODES/STANDARDS BORING EQUIPMENT AND METHODS OF STABILIZING BOREHOLE TYPE OF FEED YES NO CASING USED TYPE OF BORING RIG **DURING CORING** 4" DEPTH, FT. FROM TO 10 MECHANICAL DIA., ÍN. TRUCK DEPTH, FT. FROM TO DIA., IN. **HYDRAULIC** SKID TO DEPTH, FT. FROM DIA., IN. OTHER BARGE OTHER YES DRILLING MUD USED TYPE AND SIZE OF: DIAMETER OF ROTARY BIT, IN. D-SAMPLER TYPE OF DRILLING MUD **U-SAMPLER** S-SAMPLER NO YES AUGER USED **CORE BARREL** TYPE AND DIAMETER, IN. **CORE BIT DRILL RODS** CASING HAMMER, LBS. AVERAGE FALL, IN. 30 SAMPLER HAMMER, LBS. 140 AVERAGE FALL, IN. TYPE OF HAMMER WATER LEVEL OBSERVATIONS IN BOREHOLE CONDITIONS OF OBSERVATION DEPTH TO WATER TIME DEPTH OF HOLE DEPTH OF CASING DATE BORING COMPLETION 13.5 10.7 AFTER BEFORE INSTRUMENTATION INSTALLATION **SKETCH SHOWN ON** YES NO PIEZOMETER INSTALLED TOP ELEV. LENGTH, FT. STANDPIPE: TYPE ID, IN. LENGTH, FT. TIP ELEV. TYPE OD, IN. INTAKE ELEMENT: BOT. ELEV. MATERIAL OD, IN, LENGTH, FT. FILTER: PAY QUANTITIES NO. OF 3" SHELBY TUBE SAMPLES LIN. FT. 3.5" DIA. DRY SAMPLE BORING NO. OF 3" UNDISTURBED SAMPLES 3.5" DIA. U-SAMPLE BORING UN, FT. OTHER: CORE DRILLING IN ROCK UN. FT. DRILLING SERVICES

HELPERS

LIMCOLN

CHRIS

DATE

PHELAN

DRILLER

REMARKS

BORING CONTRACTOR

RESIDENT ENGINEER

BRIAN

SONIC

MSANDRA

KARSHI

CH2MHILL

PROJECT NUMBER

BORING NUMBER

GP TA SHEET 1 OF

SOIL BORING LOG

PROJECT Homeywell DMT SST

ELEVATION DRILLING CONTRACTOR Agrifus Drilling Liesting Inc.

DRILLING METHOD AND EQUIPMENT Sonic Samp Birll XL Mass Life Value Drilling Solid Solid Samp Birll XL Mass Life Value Drilling Solid S

36		MPLE		STANDARD	SOIL DESCRIPTION	COMMENTS.
SUBFACE (FT)	INTERNAL	AND TYPE	RECOVERY (FT)	PENETRATION TEST RESULTS 6'-6'-6' (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	DEPTH OF CASING, DRILLING RATE DRILLING FLUID LOSS, TESTS AND INSTRUMENTATION
	0	5-1			4" Arsphalt 0.4-21 Road based aggregate and . Concrete/coment treated aggregate	Advanced the some core through the first 2!
	1		80			121
12	2	7-5	12"	35 29	Poorly graded GRAVEL (GP) - tan-gray (GLEY 2 7/108); dense, dry, road based aggregate.	Below 2 (cet, switched to 1' split spoon sampling: 134" sampler: 140-16 ham, 30" drop
3	3	(ç-3	.61	13 9	3.0'-3.5'- Same as above 3.0'- Well graded SAND with '- Silt ISN-SHO, gray (GIEY) 7/104), moict,	
3 1 1 1	5	5-4	7"	6 31	sand with clay and graved (sc) - clayey SMD with gravel (sc) - gray (CLEYA 4/104), wet	
5.	S	2-5	/o"	50/111	"45' - Silty SAND (SM) with gravel-	Advancial the core the remaining 5" to a depth

CH2MHILL

PROJECT NUMBER

BORING NUMBER

AP-JA SHEET 2 OF

SOIL BORING LOG

PROJECT			
ELEVATION	DRILLING CONTRACTOR	- LOCATION	
DRILLING METHOD AND EQUIPMENT	- HOLOWITHOLOH -		
WATER-LEVELS	START	FINIOL	

_		er kann		1	STARY FINISH	LOGGER —
SE.	-	SAMPL	-	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
SUBFACE (FT)	NTERIVAL	NUMBER AND TYPE	RECOVERY (PT)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS
SER	INTE	AND	PEC (FT)	666.	OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INSTRUMENTATION
0	6				6'-6.41 - same as above	
1 1	1	J-22		31	6. W-6.9 - Silty SAND (CM) gray black (NZ), moist, fine tomedium grained sand (CB COPR)	Alternating layer of GBand HTB COPP COPP Soil semples for
1	7				6.91-200 - SIHY SAND with gravel - (SM) : greenith - Brown (25 4 4/4) _ medium grained sand (HE cope)	Presence of cope with dyp diphenyl curbacide
1 1 1 1	8	55-3-	S"	31/2,,	7:0-7:31 - Same as above, increase in gravel context 7:31-7:21 - Silty SAND (SM) - Aray black (NZ), Wmoist, tire to medium grained sand (GB Co	Advanced the remaining all using the store, to a depth of 8 Hz bgs
	0			0 11	8-01-8-1	The same of
1 1 1 1	9	55-8	2"	50/2"	8.0- Silty SATUD (SM) with gravel. greenish brown (2.54 4/4), boxbody. medium grained sand (HB COPP)	sonic care advanced to 9.0 bys and sample collecte in bag for wash observed in the recovered sample
1	9					
+	1	55-9	4h	50/41	Same as above	
1	10			17		to minimize the warhow 10.0'- Fouter casing advanced to adepth of 10 feet bys -
-		55-10	5	50/5"	Same as above	
-	11					
]	1	\$5-11		14	of sitty sand (SM) Tray-black (N2), medium Trained sand, moist (GBCOPE)	
1	1			16	- 1	
-	12				-	

CH2MHILL

PROJECT NUMBER	BORING NUMBER	SHEET	3	OF
	SOIL BORING LOG			

PROJECT				LODITION		
ELEVATION	V		DRILLING CONTRACTOR	LOCATION	-	
	METHOD AND EQ	UIPMENT	The second second	tale tale part of the same of		-
WATER LE	VELS		START	- FINISH	LOGGER —	
30	SAMPLE	STANDARD	200 05555			

-	SAMPL	E	STANDARD	SOIL DESCRIPTION	LOGGER —
CE (F)	-		STANDARD PENETRATION TEST RESULTS	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY	COMMENTS. DEPTH OF CASING, DRILLING RATE,
SURFACE (FT) INTERVAL	NUMBER AND TYPE	RECOVERY (FT)	6*-6*-6* (N)	OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DRILLING FLUID LOSS, TESTS AND INSTRUMENTATION
2 12		12"	17 38	Same as aloove, greenish - (254 4/3) exchen	
-11	3			same as above -	
	25-13		31	13.8'- Fast CLAY (CH) - Red (10R 4/6), highly plastic	
14				Boving terminated at 14.0H	
-					
				-	



BORING NUMBER: 9 P-7 C

Sheet:]

SOIL BORING LOG

PROJECT: Honeywell DMT SSI

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Aquester drilling and testing , Ene DRILLING METHOD AND EQUIPMENT: SONIC SAMP DRILL XL MAY 170; SPT Sampling

WATER LEVELS:

START: 10/30/15 FINISH: 10/30/15 LOGGER: K-Chaturvedula

£		SAME	LE	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
ДЕРТН ВЕ СОW (ft)	NUMBER	ш	RECOVERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS
O DEF	NO	TYPE	REG	6" - 6" - 6" (N)	STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
1		CORE			2.5" Asphatt 0.25'-3-0' Concrete/commont treated aggregate	Advanced the top 3' with the 3' op some Macro con 2"- 2.5" concrete pieces Pieces observed in the macro core
	101-311	SONIC MACKED CO	16,			
3				. 1	2:0'-3:4' Proving grounded strip with growel (SP) gray-white!), moist, medium growned sand 3:4'-3:9'	Advanced from 3'-6' with the 3" on some Macro core
1	5-2-	MACRE CORE	25"		For clay (CH) Red (10R416), highly plastic, moist 3:9'-5:0' 5:11ty sand with gravel (SH), Bd Red-Brown (7578 414), moist,	
11111	(3)	200318			So' - 6.0' will graded sould with silt and grave (SW-SM), sidny, fine to coarse	Macro core sample feeted withdiphnyl carbazide for presence of copic



BORING NUMBER: 9P-7C

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(£)	3	SAME	LE	STANDARD	SOIL DESCRIPTION	
DEPTH BELOW (ft)	NUMBER	NUMBER TYPE TYPE RECOVERY (ft) BENETRATION TEST RESULTS 6" - 6" - 6" - 6" (N)	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION		
\$ - - 7 -	5-3	ACRO CORE	19"		Same as above, darkgray Particulate	Advanced from 6191 with the 3°00 sonic macro core.
8-		SONIC MI			2-0'- Clayey SAND with gravel (sc) Red-white (10 R 4/6), dry, fine to medium grained sand, (corrand non-copyfill)	9.0'- Switched to 1-foot Spill spoon sampling, 140-16 hammer: 13/6" I DEAN 30-in drop
	55-4	22	15,1	20	dark gray and black (NZ), wet, firs to medium grained, particulate (GB COPE)	Solbgs during dilling
1 1 1 1 1	51-5	12	12!	12	silty sand withgravel (SM) gray-greenish (544/4), dry time to medium grained sand, slightly to moderately indurated (copp)	10.0%. 6" of outer casing advanced to a depth of 10' .bgs
- 19	3-6	SS	12"	13 49	silty sand (SM), dark gray and black (NZ), day moist, slightly indurated (GB cotr)	



BORING NUMBER: GP. 7C

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

(£)	S	AMP	LE	STANDARD	SOIL DESCRIPTION	200000
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12-	27	22	ç u	22	same as above, brightgreen particles	Switched to 6 tim. Split spoon sample
3 -	8-22	22	4"	32	Red (lor46). highly plastic	
					Boning terminated at a depth of 13.0' bgs	



BORING NUMBER: GP-84

Sheet: 1 of

SOIL BORING LOG

LOCATION: SEE LOCATION PLAN

PROJECT: DUNDALK MARINE TERMINAL DRILLING CONTRACTOR: SDS

DRILLING METHOD AND EQUIPMENT: SOMIC MULTIDRILL XL MAX 170 WATER LEVELS: ~ 18.5 START: 0800 FINISH: 1200 LOGGER: L.LINCOLN

Œ	S	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (#)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
0_				-	0.0.5': BLK ASPHALT 0.5-2.5': LT GRY CEMENTED F.M SAND (ROAD BASE)	
2_	5-1		3	7/4		
3_		VIC CORE		-	2.5-3.0'; BRN CF SAND, SM SILT (FILL) 3.0-4.25'; GRY CLAY, SM F.M SAND, SILT, TR GVL	
4 _		ROTOSONI		- - - -	4.25.5.0': RED BRN, BLK	DCP *
5 -	5.2		3'		SILTY F.C SAID, SM GVL, SHB 5.0 5.75: WHITE, GREEN CEMENTED C-M SAND, SM GVL	DCD 11
լ					5.75 U' RED BRN MOD CEMENTED C-F SAND, SM GNU GREEN PARTICUES (HB COPR)	DCP11



BORING NUMBER: 6P- 8A

Sheet: 2 oF

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE LOCATION PLAN

ELEVATION: 4+17 DRILLING CONTRACTOR: 5DS

DRILLING METHOD AND EQUIPMENT: SOME MUCTIDELL YE MAY 170

WATER LEVELS: ~ +8.5 START: 0800 FINISH: 1200 LOGGER: L. LINCOLM

£	s	AMPL	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
<i>u</i> _					6.7' GRY, RED BRN SANDY SILT, SM GVL	DCP -
7 _				- - -		
-	5-3	ROTOSON IC	3'	H/A -	7-7.6' MOD HIGHLY LITHIFIED BE COTTE - BLK, GREEN SIETY SAND, SM BY	DCP++
8 _		Ron		- - - -	T. U - 91' RED'BRN HIGHLY LITHIFIED HB COPR-SILTY C-F SAND, SM GVL, GREEN PARTICLES	DCP**
- a _				- -	manufacture parameter about a constant and a consta	·
-	55.4	3° A.	12"	2 55	M.F SAND, TR SILT MOD 15-10.0 RED BEN GRY, GREEN	
- fra _	555		12"	5	HIGHLY LITHIFIED HE SAND TR CLAY, GREEN	ひくをす
16 _				43 - - 25 -	11-11-7 BRN. GRY	
- 12_	ss."			69	GREEN PARTICLES (HB COPR) 11.7-11.9 BLK, GREEN HIGHLY LITHIFIED M. FSAND (GB COPP) 11.9-12 BEN, GREY HAGILY LITHIFIED	
	1				PARTICLES (HE COPE-)	



BORING NUMBER: 48 8A

Sheet: 3 or

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE LEGISTION: ~ +17 DRILLING CONTRACTOR: SDS

LOCATION: SEE LOCATION PLAN

ELE/	/ATIO	N: ~	417		DRILLING CONTRACTOR:	THE PLANT
WATI	LING ER LE	METH EVELS	HOD A 5: ~	ND EQUIPME! + &、ら	NT: SONIC MULTIDRILL XL MAX START: 0800 FINISH: 1200	LOGGER: L. LINCOLN
V (ft)	S	AMPL	т	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	ER		VERY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS.
	NUMBER	TYPE	RECOVERY	6" - 6" - 6" (N)	DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
12_	\$ 5-7	3" D.A. SS	12"	20 - V4 -	12-13': BRN, GRY, YELLOW HIGHLY LITHIFIED F.M SAND, TR SILT, GVL (HB: COPR)	
13 -	∞ ತ್ತವರ್ಣ ಬೌಡಿ			21 -	13-13-4: MOIST MOD. EITHIFIED	
 	22-8		12"	24 1 1 1 1 1 1 1 1 1	BRN, GRY, GREEN M. FSAMD SM SILT (HB COFR) THIS MOD LITHIFIED DK GRY, BLK, GREEN F. M SAND, TR. SILT (GB COPR)	
-	SS·9		2"	ا ن	RED BRN, GRY, F M SAND,	
15 _	\$5.10		lo"	\ \	BRN M. F SAND, TR. SILT, GREEM PARTICLES (GB) 14.8. IS RED CLAY, MOTT. YELLOW	IB (OPR)
				-		EDB @ 15.0' DRILLER PUSHED 5" CASING TO 15', CLEANED OUT HOLE WITH SONIC CORE
- 17 _ 				- - - -		BEFORE INSTALLING INSTRUMENTATION
18						

Mueser Rutledge Consulting Engineers 14 Penn Plaza • 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400

10 VA ******	vv.mrce.coi	···					GP. BA	<u> </u>
						SHEET	<u> </u>	U
PROJECT			MARIN		NAL		0587	
LOCATION	-	LTIMOR				SURFACE ELEV.	<u>~ +17</u>	
BORING LO	CATION	SEE	LOCATION	PLAN		DATUM		
TEST/INSPE	CTION EQUI	IPMENT	SONIC I	MULTIDE	111_ 11	MAY IT	O	
10 M 3 M 19 M	CODES/STA							
				A STATE OF THE STA			erse ha and a ann ann a na a ann a an a an	
BORING EQ	UIPMENT A	ND METHODS O	F STABILIZING BOF	REHOLE				
		TYPE OF FEE	D					
TYPE OF BOR	ING RIG	DURING CO		CASING USED) 	YES	NO	**
TRUCK	 	MECHANICA	L	DIA., IN.	5"	DEPTH, FT. FROM		то
SKID		HYDRAULIC		DIA., IN.		DEPTH, FT. FROM		<u></u>
BARGE		OTHER	ata a Maraja da a	DIA., IN.		DEPTH, FT. FROM	<u> </u>	
OTHER								
TYPE AND S	NZE OE:			DRILLING MU	IDUSED	YES	NO	
D-SAMPLER		O.D. SPL	IT SPANN		F ROTARY BIT, IN.	J153	[]NO	
U-SAMPLER		<u> </u>	<u>,,,</u> ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	TYPE OF DRIL				
S-SAMPLER	1					-		
CORE BARRE	L 3"t	DIA. ROTOSO	MIC	AUGER USED	n de la companya de la companya de la companya de la companya de la companya de la companya de la companya de	YES	NO	
ORE BIT				TYPE AND DI	AMETER, IN.			
ORILL RODS						•		
				CASING HAM		140 AVE	RAGE FALL, IN.	30
				SAMPLER HA	MMFR IRS	AVE	RAGE FALL, IN.	
						***************************************		TO A WILLIAM TO THE STATE OF TH
, j. 1				TYPE OF HAN		CONTRACTOR CONTRACTOR		
WATER LEV	/EL OBSERV/	ATIONS IN BORF	HOLF			Octopolinumicamposa.aasti		CONTROL STATEMENT
WATER LEV	<u>'EL OBSERV/</u>	ATIONS IN BORE	HOLE			**************************************		i i
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	TYPE OF HAN	AMER	CONDITIONS	OF OBSERVATION	CONTRACTOR AND AND AND AND AND AND AND AND AND AND
				TYPE OF HAN		CONDITIONS	OF OBSERVATION	NIGHT
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	TYPE OF HAN	AMER	CONDITIONS		NIGHT
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	TYPE OF HAN	AMER	CONDITIONS		NIGHT
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	TYPE OF HAN	AMER	CONDITIONS		NIGHT
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	TYPE OF HAN	AMER	CONDITIONS		NIGHT
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	TYPE OF HAN	AMER	CONDITIONS		NIGHT
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	TYPE OF HAN	AMER	CONDITIONS		NIGHT
DATE 10/29	TIME	DEPTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER	AMER	CONDITIONS	REVIOUS	NIGHT
DATE 10/29 PIEZOMETE	TIME 12.00	DEPTH OF HOLE	DEPTH OF CASING	DEPTH TO WATER 7.5	HEAVY	CONDITIONS RAIN P	REVIOUS.	
DATE 10/29 PIEZOMETE STANDPIPE:	TIME 12.00	DEPTH OF HOLE 15.0	DEPTH OF CASING	DEPTH TO WATER 7:5	HEAVY CH SHOWN ON LENG	CONDITIONS RAVN P	REVIOUS TOP ELEV	
DATE 10/29 PIEZOMETE STANDPIPE: NTAKE ELEM	TIME 12.00 ER INSTALLE	DEPTH OF HOLE 15.0	DEPTH OF CASING	DEPTH TO WATER 7:5 NO SKETO ID, IN. OD, IN.	HEAVY CH SHOWN ON LENG LENG	CONDITIONS RAIN P PG. TH, FT. TH, FT.	FEVIOUS TOP ELEV.	
DATE 10/29 PIEZOMETE STANDPIPE: NTAKE ELEM	TIME 12.00 ER INSTALLE	DEPTH OF HOLE 15.0	DEPTH OF CASING	DEPTH TO WATER 7:5	HEAVY CH SHOWN ON LENG LENG	CONDITIONS RAVN P	REVIOUS TOP ELEV	
DATE 10/29 PIEZOMETE STANDPIPE: NTAKE ELEM	TIME 12.00 ER INSTALLE	DEPTH OF HOLE 15.0	DEPTH OF CASING	DEPTH TO WATER 7:5 NO SKETO ID, IN. OD, IN.	HEAVY CH SHOWN ON LENG LENG	CONDITIONS RAIN P PG. TH, FT. TH, FT.	FEVIOUS TOP ELEV.	
DATE 10/ 2-9 PIEZOMETE STANDPIPE: NTAKE ELEM FILTER: PAY QUANT	TIME 12.00 ER INSTALLE MENT:	DEPTH OF HOLE 15. O TYPE TYPE MATERIAL	DEPTH OF CASING	DEPTH TO WATER 7:5 NO SKETO ID, IN. OD, IN.	HEAVY CH SHOWN ON LENG LENG	CONDITIONS RAIN P RG. TH, FT. TH, FT. TH, FT.	FEVIOUS TOP ELEV.	
DATE 10/2-9 PIEZOMETE STANDPIPE: NTAKE ELEM LTER: PAY QUANT 1.5" DIA. DRV	TIME 12.00 ER INSTALLE MENT: ITTIES / SAMPLE BOF	DEPTH OF HOLE 15.0 TYPE TYPE MATERIAL	DEPTH OF CASING 15.0	DEPTH TO WATER 7:5 NO SKETO ID, IN. OD, IN.	CH SHOWN ON LENG LENG NO, OF 3" SHELBY	CONDITIONS RAIN P PAIN P IH, FT. IH, FT. IH, FT. IUBE SAMPLES	FEVIOUS TOP ELEV.	
DATE 10/2-9 PIEZOMETE STANDPIPE: NTAKE ELEM FILTER: PAY QUANT 3.5" DIA. DRY 3.5" DIA. U-S	TIME 12.00 ER INSTALLE SENT: SAMPLE BORIN	DEPTH OF HOLE 15.0 TYPE TYPE MATERIAL	DEPTH OF CASING 15. 0 res Lin. ft. Lin. ft.	DEPTH TO WATER 7:5 NO SKETO ID, IN. OD, IN.	CH SHOWN ON LENG LENG NO. OF 3* SHELBY NO. OF 3* UNDISTL	CONDITIONS RAIN P PAIN P IH, FT. IH, FT. IH, FT. IUBE SAMPLES	FEVIOUS TOP ELEV.	
DATE 10/2-9 PIEZOMETE STANDPIPE: NTAKE ELEM FILTER: PAY QUANT 3.5" DIA. DRY 3.5" DIA. U-S	TIME 12.00 ER INSTALLE SENT: SAMPLE BORIN	DEPTH OF HOLE 15.0 TYPE TYPE MATERIAL	DEPTH OF CASING 15.0	DEPTH TO WATER 7:5 NO SKETO ID, IN. OD, IN.	CH SHOWN ON LENG LENG NO, OF 3" SHELBY	CONDITIONS RAIN P PAIN P IH, FT. IH, FT. IH, FT. IUBE SAMPLES	FEVIOUS TOP ELEV.	
DATE 10/ 2-9 PIEZOMETE STANDPIPE: NTAKE ELEM FILTER: PAY QUANT 3.5" DIA. DR. 3.5" DIA. U-S CORE DRILLIN	TIME 12.00 ER INSTALLE SENT: SAMPLE BORIN	DEPTH OF HOLE 15.0 TYPE TYPE MATERIAL RING IG	DEPTH OF CASING 15. O VES LIN. FT. LIN. FT. LIN. FT.	DEPTH TO WATER 7.5 NO SKETO DD, IN. OD, IN. OD, IN.	CH SHOWN ON LENG LENG NO, OF 3* SHELBY NO, OF 3* UNDISTL OTHER:	CONDITIONS RAIN P PG. TH, FT. TH, FT. TH, FT. TUBE SAMPLES DRBED SAMPLES	FEVIOUS TOP ELEV.	
DATE 10/2-9 PIEZOMETE STANDPIPE: NTAKE ELEM FILTER: PAY QUANT 3.5" DIA. DR.Y 3.5" DIA. U-S CORE DRILLIN	TIME 12.00 ER INSTALLE MENT: SAMPLE BORIA NG IN ROCK ONTRACTOR	DEPTH OF HOLE 15.0 TYPE TYPE MATERIAL RING IG	DEPTH OF CASING 15. 0 VES LIN. FT. LIN. FT. LIN. FT. LIN. FT. DNIC DR	DEPTH TO WATER 7:5 NO SKETO DD, IN. OD, IN. OD, IN.	CH SHOWN ON LENG LENG NO, OF 3" SHELBY NO, OF 3" UNDISTL OTHER: SERVICES	CONDITIONS RAIN P PAIN P TH, FT. TH, FT. TH, FT. TUBE SAMPLES DIRBED SAMPLES	TOP ELEV. BOT, ELEV	
DATE 10/2-9 PIEZOMETE STANDPIPE: NTAKE ELEN FILTER: PAY QUANT 3.5" DIA. DRY 3.5" DIA. U-S CORE DRILLIE BORING CO	TIME 12.00 ER INSTALLE MENT: SAMPLE BORIA NG IN ROCK ONTRACTOR	DEPTH OF HOLE 15.0 TYPE TYPE MATERIAL RING IG	DEPTH OF CASING 15. O VES LIN. FT. LIN. FT. LIN. FT.	DEPTH TO WATER 7.5 NO SKETO DD, IN. OD, IN. OD, IN.	CH SHOWN ON LENG LENG NO, OF 3* SHELBY NO, OF 3* UNDISTL OTHER:	CONDITIONS RAIN P PG. TH, FT. TH, FT. TH, FT. TUBE SAMPLES DRBED SAMPLES	TOP ELEV. BOT, ELEV	



BORING NUMBER: GP-8C

Sheet:

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION: ~ + 17

DRILLING CONTRACTOR: 5DS

DRILLING METHOD AND EQUIPMENT: FRASTE MIVETIDE ILL XL MAX 170

WATER LEVELS: ~ + €

START: 0930

FINISH: 1200 LOGGER: L. LINCOLN

£	S	AMPI	E	STANDARD	SOIL DESCRIPTION	201117172
N N			£	PENETRATION TEST	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
-					0-0.3 BLK ASPHALT	5" DIA. CASING DRIVEN TO 10 '
-	S-1		12"	N/A		\
3_		ROTO SONIC			3-5' CONCRETE	
4 - 1 -		Ro		-		
5	S-2		2'3"	4	5.5.5' BRN SANDY QUL 5.5.0' MOD LITHFIED BRN, BLK, GRY SILTY C-F SAND, TR. GVL	DPC+



BORING NUMBER: GP-8C

Sheet: 2

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL

LOCATION: SEE PLAN

ELEVATION: ~ +17

DRILLING CONTRACTOR: 505

DRILLING METHOD AND EQUIPMENT: SONIC MULTIDRILL XL MAX 170

WATER LEVELS: ~ + B START: 0830

FINISH: 1200 LOGGER: L.LINCOLN

	Τ .			OTANDA DO	1 1200	EUGGER. L. EINE GEG
W (#)		SAMP		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	ТУРЕ	RECOVERY (#)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
\(\bullet_{-}\)					F.M SAND, TR. GUL (HB)	DPC+
1				-	6.5 - B.T WEAKLY LITHIFIED RED BRN F.C SAND, SM GVL, SILT (HB)	DPC+
	S-3	Sonic	3'	N/A		
8 _		ROTOS		- - - - -		
9 -	55-4	3" 55	12"	1	8.7-9.0 HIGHLY LITHIFIED RED BRN, GRN, GREEN SILTY F.M SAND (HB) 9-9.5 MOIST SOFT BRN. DKGRY SILTY F.M.SAND(W) 9.5-9.75 CONCRETE	COMA
10 -			12"	7	9.75-10 MOIST MOD LITHIFIED BRM, GRY SILTY F-M SAND TR GVL (HB) 10-10-5 MOD LITHIFIED DK GRY, YELLOW M. E SAND SM	DPC+ DRIVEN TO ID'
t1	Ss-5		12		M-F SAND (GB)	DPC 1
12 -	SS-10			3 82 -	RED BRN, GRY, GREEN SILTY F. M SAND (HB)	



BORING NUMBER: 97-80

Sheet: 3

SOIL BORING LOG

PROJECT: DUNDALK MARINE TERMINAL LOCATION: SEE PLAN

ELEVATION: ~ + 17

DRILLING CONTRACTOR: SPS

DRILLING METHOD AND EQUIPMENT: SONIC MULTIDRILL XL MAX 170

WATER LEVELS: ~ + 8

START: 0830 FINISH: 1200 LOGGER: L.LINCOLN

(#) A	5	SAMPL		STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (#)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12	- 55-7 58/7	SS S	4"	100/4" -	HIGHLY LITHIFIED RED BRN, GRY, GREEN M.F SAND, SM SILTITE. GVL (HB)	
13	-5.8	SONIC 3" DIA. SS	12"	N/A - 31 - 41 -	DO SS-7	
14				-		
	\$5-10		6 "	30	PO SS-7	
15	-55-11		ا "کا	17 -	HIGHLY LITHIFIED RED. BRN CLAYEY F.M SAND, TR GIVL (HB)	LOW DPC RXN SAMPLE SS-11 MAY BE MIXED WI RED CLAY LINER
	55 12		4"	9	DO SS-11.	Keb
16.	-S5-13		3"	7 -	V. SOFT RED CLAY	EOB @ 16.0'
	- - -			- - -		
17 -				-		
18 -				-		

Mueser Rutledge Consulting Engineers 14 Penn Plaza - 225 West 34th Street New York, NY 10122 T: 917 339-9300 F: 917 339-9400

						SHEET L		U
ROJECT	וכד	INDALK	MARIN	F. TEKMI	INAL .	FILE NO.	587	
CATION			E MARY			SURFACE ELEV.	~ +1	
ORING L	DCATION	SEE	PLAN			DATUM		
CCT /INICO	ECTION FOI	HDA AFAIT						
, 在近期時十	ECTION EQU	t William Fall and the first						
FEREIVE	E CODES/ST	ANDARDS	Water Control of the		T	** \$40 pts \$100 allowed black and but \$100 pts and \$100 p		
ORING EC	QUIPMENT	AND METHODS O	F STABILIZING BO	REHOLE				
		TYPE OF FEI	S. S. S. S. S. S. S. S. S. S. S. S. S. S					
PE OF BO	RING RIG	DURING CO	RING	CASING USED)	YES	NO	
UCK		MECHANIC	AL <u></u>	DIA., IN.	5"	DEPTH, FT. FROM		TO 10
ID		HYDRAULIC		DIA., IN.	4 "	DEPTH, FT. FROM		то
RGE		OTHER		DIA., IN.		DEPTH, FT. FROM		то
HER								
					and the second of the second o			
	SIZE OF:		I	DRILLING MU			NO	
SAMPLER SAMPLER		U.D SPL	IT SPOON	DIAMETER OF TYPE OF DRIL	F ROTARY BIT, IN	·		
AMPLER				THEOFORIL	THING MOD			
RE BARRE				AUGER USED		YES	NO	
RE BIT				TYPE AND DI	A SECTION OF THE SECT	سات	٠	
ILL RODS	-		in a second					
				CASING HAM	IMER, LBS.	AVERA	AGE FALL, IN.	41 - 42
				SAMPLER HA	ra it far	AND DESCRIPTION OF THE PARTY OF	AGE FALL, IN.	30
					MMER, LBS.	AND CONTRACTOR OF THE PERSON O	AGE FALL, IN.	30_
ATER LEV	VEL OBSERV	ATIONS IN BORE	HOLE 	SAMPLER HA	MMER, LBS.	AND CONTRACTOR OF THE PERSON O	AGE FALL, IN.	30
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER	MMER, LBS.	140 AVERA	AGE FALL, IN,	30
DATE			1	SAMPLER HA	MMER, LBS. AMER WATE	CONDITIONS OF	F OBSERVATION	30 RUMENTAT
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER	MMER, LBS. AMER WATE	CONDITIONS OF LEVEL	F OBSERVATION	
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER	MMER, LBS. AMER WATE	CONDITIONS OF	FOBSERVATION PRE · I H.ST	
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER	MMER, LBS. AMER WATE	CONDITIONS OF	FOBSERVATION PRE · I H.ST	
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER	MMER, LBS. AMER WATE	CONDITIONS OF	FOBSERVATION PRE · I H.ST	
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER	MMER, LBS. AMER WATE	CONDITIONS OF	FOBSERVATION PRE · I H.ST	
DATE	TIME	DEPTH OF HOLE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER	MMER, LBS. AMER WATE	CONDITIONS OF	FOBSERVATION PRE · I H.ST	
DATE /3D	TIME	DEPTH OF HOLE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER 9	MMER, LBS. AMER WATE	CONDITIONS OF LEVEL	FOBSERVATION PRE INST	
DATE 0/30	TIME 1130	DEPTH OF HOLE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER 9	MMER, LBS. MATE IN	CONDITIONS OF REVEL	FOBSERVATION PRE INST	
DATE 0/30 EZOMETI	TIME 1130 ER INSTALLE	DEPTH OF HOLE IU TYPE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER 9 NO SKETO	MMER, LBS. AMER WATE IN CH SHOWN ON	CONDITIONS OF LEVEL	FOBSERVATION PRE INST	
DATE 30 30 20MET(TIME 1130	DEPTH OF HOLE IU TYPE TYPE	DEPTH OF CASING	SAMPLER HA TYPE OF HAN DEPTH TO WATER 9 NO SKETO ID, IN. OD, IN.	MMER, LBS. AMER WATE IN CH SHOWN ON LEF LEF	CONDITIONS OF RELEVEL STALLAND	FOBSERVATION PRE INST	
DATE 130 ZOMETI ANDPIPE: AKE ELEN	TIME 1130	DEPTH OF HOLE IU TYPE	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER 9 NO SKETO	MMER, LBS. AMER WATE IN CH SHOWN ON LEF LEF	CONDITIONS OF LEVEL	FOBSERVATION PRE INST	
DATE 130 20METI ANDPIPE: AKE ELEN	TIME 1130 ER INSTALLE	DEPTH OF HOLE IU TYPE TYPE	DEPTH OF CASING	SAMPLER HA TYPE OF HAN DEPTH TO WATER 9 NO SKETO ID, IN. OD, IN.	MMER, LBS. AMER WATE IN CH SHOWN ON LEF LEF	CONDITIONS OF RELEVEL STALLAND	FOBSERVATION PRE INST	
DATE 1/30 EZOMETI ANDPIPE: AKE ELEN FER:	TIME 1130 ER INSTALLE MENT:	DEPTH OF HOLE I U TYPE TYPE MATERIAL	DEPTH OF CASING	SAMPLER HA TYPE OF HAM DEPTH TO WATER 9 ID, IN. OD, IN. OD, IN.	MMER, LBS. IMER IN CH SHOWN ON LEF LEF	CONDITIONS OF REVEL STALLATION PO 5 NGTH, FT. NGTH, FT.	FOBSERVATION PRE INST	
DATE 130 20METI ANDPIPE: AKE ELEN FER; Y QUANT	TIME 1130 ER INSTALLE MENT: TITIES Y SAMPLE BOIL	DEPTH OF HOLE I U TYPE TYPE MATERIAL RING	DEPTH OF CASING 10	SAMPLER HA TYPE OF HAM DEPTH TO WATER Q ID, IN. OD, IN. OD, IN.	MMER, LBS. AMER NATE IN LET LET NO. OF 3" SHELE	CONDITIONS OF REVEL STALLAND PO 5 NGTH, FT. NGTH, FT.	FOBSERVATION PRE INST	
DATE 230 20METI ANDPIPE: AKE ELEN TER; Y QUAN " DIA. DR' " DIA. U-S	TIME 1130 ER INSTALLE MENT: TITIES Y SAMPLE BORN SAMPLE BORN	DEPTH OF HOLE I U TYPE TYPE MATERIAL RING	DEPTH OF CASING 10	SAMPLER HA TYPE OF HAM DEPTH TO WATER 9 ID, IN. OD, IN. OD, IN.	CH SHOWN ON LEF NO. OF 3" SHELE NO. OF 3" UNDIS	CONDITIONS OF REVEL STALLATION PO 5 NGTH, FT. NGTH, FT.	FOBSERVATION PRE INST	
DATE 2/30 ANDPIPE: TAKE ELEN TER; Y QUAN " DIA. DR."	TIME 1130 ER INSTALLE MENT: TITIES Y SAMPLE BOIL	DEPTH OF HOLE I U TYPE TYPE MATERIAL RING	DEPTH OF CASING 10	SAMPLER HA TYPE OF HAM DEPTH TO WATER 9 ID, IN. OD, IN. OD, IN.	MMER, LBS. AMER NATE IN LET LET NO. OF 3" SHELE	CONDITIONS OF REVEL STALLAND PO 5 NGTH, FT. NGTH, FT.	FOBSERVATION PRE INST	
EZOMETI ANDPIPE: FAKE ELEN TER: Y QUAN' " DIA. DR' " DIA. U-S RE DRILLII	TIME 1130 ER INSTALLE MENT: TITIES Y SAMPLE BORN SAMPLE BORN	DEPTH OF HOLE IU TYPE TYPE MATERIAL RING	VES LIN. FT. LIN. FT. LIN. FT.	SAMPLER HA TYPE OF HAM DEPTH TO WATER Q ID, IN. OD, IN. OD, IN.	CH SHOWN ON LEF NO. OF 3" SHELE NO. OF 3" UND!! OTHER:	CONDITIONS OF REVEL STALLATIO NGTH, FT. NGTH, FT. NGTH, FT. NGTH, FT. STURBED SAMPLES STURBED SAMPLES	FOBSERVATION PRE INST	
DATE 2/30 EZOMETI ANDPIPE: IAKE ELEN TER: Y QUAN " DIA. DI " DIA. U-S RE DRILLII	TIME 1130 ER INSTALLE MENT: TITIES Y SAMPLE BORI NG IN ROCK ONTRACTOR	DEPTH OF HOLE IU TYPE TYPE MATERIAL RING	DEPTH OF CASING 10 10 VES UN. FT. UN. FT. UN. FT.	SAMPLER HAN TYPE OF HAN DEPTH TO WATER 9 ID, IN. OD, IN. OD, IN.	CH SHOWN ON LEF LEF NO. OF 3" SHELE NO. OF 3" UND!! OTHER:	CONDITIONS OF RELEVEL STALLAND PO 5 NGTH, FT. NGTH, FT. NGTH, FT. NY TUBE SAMPLES STURBED SAMPLES	TOP ELEV. BOT. ELEV.	
DATE 2/30 EZOMETI ANDPIPE: TAKE ELEM TER: Y QUAN' " DIA. DR' " DIA. U-S RE DRILLII	TIME 1130 ER INSTALLE MENT: TITIES Y SAMPLE BORI NG IN ROCK ONTRACTOR	DEPTH OF HOLE IU TYPE TYPE MATERIAL RING	VES LIN. FT. LIN. FT. LIN. FT.	SAMPLER HAN TYPE OF HAN DEPTH TO WATER 9 ID, IN. OD, IN. OD, IN.	CH SHOWN ON LEF NO. OF 3" SHELE NO. OF 3" UND!! OTHER:	CONDITIONS OF RELEVEL STALLAND PO 5 NGTH, FT. NGTH, FT. NGTH, FT. NY TUBE SAMPLES STURBED SAMPLES	FOBSERVATION PRE INST	

BORING NO. GP-8C



BORING NUMBER: GF- IAA

Sheet:

SOIL BORING LOG

PROJECT: Honeywell DMT SIL

LOCATION:

ELEVATION: DRILLING CONTRACTOR: Aquater Drilling and tisking tree Drilling METHOD AND EQUIPMENT: Some samp Drill max 140 : spr sampling

WATER LEVELS:

START: 11915 FINISH: 11915 LOGGER: K. Chatured La

3	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
	çì	SOMIT MARRIED LOSE	301		3" Asphalt. Sity sand with gravel (SM), gray, light brown, (7.5 4R S/U), moist, fire to me diver grave adian	Advanced to 10-0 bys Using Rotosonic moconcom 2" 10 B-01- sample lained cook wire - ve-
	¿V	SOMIC MACED THE	70°		2.0'- some as above, with every and grown (sv-ce). Red-relians, Brown (st-still lift), dry, medium grained saw	



BORING NUMBER: GP-167

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£	S	AMPLE	E	STANDARD PENETRATION	SOIL DESCRIPTION	COMMENTS
ELOW (RY (ft)	TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL,	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS,
DEPTH BELOW (T)	NUMBER	TYPE	RECOVERY (ft)	6" - 6" - 6" (N)	COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	TESTS AND INTRUMENTATION
-					Gol-751- Part Fast Cl AT CONT Red (1074) moist, highly plastic.	ta),
7	57	MACRO CORE	36"		7-5-8-75-51 Hy SAND (SM), Brown. Red-green (7-5)R 4/3), day, Particula. (415 COPR)	ope test the
Q -		2 1995			- 8 75' - Poorly granted share with sill (so done gray (754R 5/2), moist, farticul (GB COPE)	
	SM	SONIT MACED	1211		Silty shows (SM), Raddich - Brown (45'dry, Particulary to moderately today, Lamino Led (HE COPE)	lad. Irstalled the 6" OD outer rasing to adupte of
10	55-5	کړ .	12"	13	dark brown, gray Black (7 518 4/2), moderately by a sad indurated 10-25-10-3' - some at stove, - Reddist - Grown (21-18 5/6), dry - 10-31 - 100 - Poorly grooted snows with 511+ (sp. sh) dark gray - Bbretown, bright Dren Porticing, mount, Particulate 100	-10-5 - I without to se sample below 18' 142 to harmon I was sompler .
117	355	6 52		35	11.8-11.21- same as above redder - sind 11.21-11.31- silty same len, redder - sind (12.5-11.31- silty same len), redder - sind (13.5-11.31) idry, moderately into highly indurated (412.600) In 81.12-1- Robelly (Lis copt), dark Arren (54 414), model	y areard the con-



BORING NUMBER: GP-10 A

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

£	S	AMPL	E	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS 6" - 6" - 6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
12-	55-71	ss	12"	49 .	Poorly graded smo with sill (SP SM), dark green (SV 4 lu), well, Parkentale (als cope) 125'-127'- Same as a kove, Red-Emun (754R S16)	12-01-snowndwater encountered at 12-01 tigs during drilling
13-	52.8	S¢	Ý.	96	13:0'. Sitty santo (EM). Brown in Red (757RS, moist, moderately industrial, laminated English tellows porticles (HBCOPE)	ω.
	53.9	22	611	46	Some as allow mast	
4 -	22-10	دي	211	41	Same as above, wet, \$80 city	,
	2541	22	Ly.	28	Proving graded spring with sill (st-ser), gray and door grown (2544/3), most, Portionate (GB COPE). Bright green	
15 -	55-12	53	Pa	10	Same as above, wet	is o'- Installing the 6" or out
	(3-13	12	en.	7	wet, highly plastic	D,
16-					Bottom of boring at 16.0° bgs	



BORING NUMBER: 16C

Sheet: /

SOIL BORING LOG

PROJECT: HOREYWELL DAT SEE

LOCATION:

ELEVATION:

DRILLING CONTRACTOR: Aque for drilling and fishing Inc

WATER LEVELS: START: 11 10 15 FINISH: 11 10 15 LOGO

FINISH: Illiolis LOGGER: k-chalury dula

£		SAMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND
0-	N S	SOME MACKS COLE	14,	= 12	Sity same with gravel (SM), gray light brown (2-5 42 = 15), most, time to medium grained rand	Trum a' 15' Advanced the ho's asing sonic moins can strate. DPC -
3 4 5	ÇA	Egnile MATHOLOGE	7.0		30 - Some as above. 301 - Topolog graveled struck with they and gravel reprect Resolute Process and total value of sand.	DPC-



BORING NUMBER: GP-LOC

Sheet: 2

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

Œ	1 3	SAMP	LE	STANDARD	SOIL DESCRIPTION	COMMENTS
DEPTH BELOW (ft)	NUMBER	TYPE	RECOVERY (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS TESTS AND INTRUMENTATION
6-1-1-1	53	WARLE CORP.	36,1		G. D-Fo' - FET CLAY (CIV), Red (104 416) moist, highly plashe Fo' - Sithy SAND (SM), Red - BI Own (FS)	\(\rho \s\ 6\)
P -		SAME			dry, fine grained send, barticular to	Dyc.+
	544	Styles Mileso Coffee	12"	,	90. Illysomo (sm), light grow (love wet, bright yellow particles, Partico to slightly indurated (second)	
25	_ક ક	SI	131	15	Rea-Erown (I Sypsig), dry, borged green Particles - moderately to highly indurated, law retted (His corres Dis - increase in gravel content White quarty live grave)	to d Installed the BG 00 out of contract of to depth of 10 logs. 101 Below to switched to se sampling. Tho-la hammer; 30 in drop. 5 de sampler
	gol	SS	II	60	tal Poorly graded smore with site (cross), black gries black (NZ), with bright green for holes Parkinstate 1.61 - City (note (SN), Brown green lay, moderately indurated, laminate	of Good in corn



BORING NUMBER: 97-100

Sheet: 3

SOIL BORING LOG

PROJECT:

LOCATION:

ELEVATION:

DRILLING CONTRACTOR:

DRILLING METHOD AND EQUIPMENT:

WATER LEVELS:

START:

FINISH:

€ SAMPLE	STANDARD	SOIL DESCRIPTION	COMMENTS
NUMBER SYPPE TYPE MAN (ft)	PENETRATION TEST RESULTS	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS AND INTRUMENTATION
SS-2 SS 12"	34	Same as close, wet	12-11- groundwater encountered at 12-10 legs during dalling
3-53-8 311	9	same as above, wet	
-559 85 411	1)	Sandy hor clarified, Red-white Coya	ylu) 150. Duke cowing involved to a depth of live logs.
		Bottom of boringal lund bys	

BORING LOG Page 1 of 7 Honeywell

Site: Dundalk Marine Terminal

Boring No: DMT-11S

Diameter: 8 in Date: 01/06/2006

Northing: 574430.5 Easting: 1448175.9 Elevation: 22.10

Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Consultant: L. Hunt (CH2M Hill, Inc.)

Project No: 327494 Field Book No:

Total Depth: 48.0 Ft GW Depth: 24.0 Ft

Datum: Baltimore City Depth Soil DMT-11S Pattern Soil Description Ft Code Diagram 0 Asphall SANDY CLAY (CL): moist, stark reddish brown, stiff, trace gravel and silt, trace organics, homogeneous, CL 5 GRAVEL (GP); moist to dry, white and black, dense, poorly graded gravel, trace silt and sand,

Page 2 of 7 BORING LOG Honeywell

Site: Dundalk Marine Terminal

Boring No: DMT-11S

Diameter: 8 in Date: 01/06/2006

Northing: 574430.5 Easting: 1448175.9 Elevation: 22.10

Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Consultant: L. Hunt (CH2M Hill, Inc.)

Project No: 327494 Field Book No: Total Depth: 48.0 Ft GW Depth: 24.0 Ft

Datum: Baltimore City Depth g Soil DMT-11S Pattern Soil Description Ft Code Diagram GRAVEL [GP]; moist to dry, white and black, dense, poorly graded gravel, trace sit and sand, GP SANO (SP), most, light two deman fine to coarse grained quartz sand, trace sit, homogeneous, SP SILTY CLAY (CL); moist, red brown, very stiff, irace gravel, sand and organics, homogeneous, rece sand pockets, fill CL 10 SANDY GRAVEL (GP); most gray, dense homogeneous, fill. GP SAND (SP), most, red brown, dense, fine to coarse grained quartz sand, trace sit, well compacted, SP SAND (SIP), moist, brown, mottled green and black, fine to coarse grained quartz sand, trace gravel, silt and clay well compacted, lensed, gravel lense at 12.5°, COPR. SP CLAYEY SILT (ML): moist, gray brown, very stiff, trace gravel, homogeneous, fill ML.

Honeywell

Site: Dundalk Marine Terminal

Boring No: DMT-11S

Diameter: 8 in Date: 01/06/2006

Northing: 574430.5 Easting: 1448175.9 Elevation: 22.10 Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Consultant: L. Hunt (CH2M Hill, Inc.)

Total Depth: 48.0 Ft GW Depth: 24.0 Ft

Datum: Baltimore City Project No: 327494 Field Book No: Depth g Soil DMT-11S Pattern Soil Description Ft Code Diagram CLAY (CL), moist, gray, red and white, sense, fine to coarse grained quartz sand, trace silt, 14 CL SAND (SP), indist, gray, brown, light brown, black and green, dense, fine to coarse grained quartz sand, trace grevel concrete and six well compacted, COPR and fill intermixed. 15 SP No recovery from 17 to 20 \bigvee 20 CLAYEY SAND (SC), most red brown and derk gray, mottled white, stiff, trace gravel and silt, me to coarse grained quartz sand lensess. All, confining COPR above from material below SC

BORING LOG

Page 4 of 7

Driller: A. Chapel (Parratt Wolff, Inc.)

Honeywell

Site: Dundalk Marine Terminal

Boring No: DMT-11S

Diameter: 8 in Date: 01/06/2006

Soil Description

CLAYEY SAND (SC), most not brown and dark gray, mitowid-while, will trace gravel and all fine to coarse grained quartz sand lanses, fill, confining COPH above from millional barook.

Northing: 574430.5 Easting: 1448175.9

Soil

Code

Depth g

Ft

21

Method: Hollow Stem Auger

Elevation: 22.10 Consultant: L. Hunt (CH2M Hill, Inc.)

Pattern

Datum: Baltimore City Project No: 327494 Field Book No: Total Depth: 48.0 Ft

GW Depth: 24.0 Ft DMT-11S Diagram

SC SAND (SP), wat, gray, local, fine to coerse grained quart; sand, little gravet, itomogeneous, reliver materials. 25 SP 28

BORING LOG Page 5 of 7 Honeywell

Site: Dundalk Marine Terminal

Boring No: DMT-11S

Diameter: 8 in Date: 01/06/2006

Northing: 574430.5 Easting: 1448175.9 Elevation: 22.10

Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Consultant: L. Hunt (CH2M Hill, Inc.)

Project No: 327494 Field Book No: Total Depth: 48.0 Ft GW Depth: 24.0 Ft

Datum: Baltimore City Depth g Soil DMT-11S Pattern Soil Description Ft Code Diagram CLAY (CL), moist, light gray, mottled light brown (FeO2), stiff to very stiff, some silt, trace 28 CL 30 SILTY SAND (SM); wet, brown, loose, fine to coarse grained quartz sand, homogeneous. SM

Honeywell

Site: Dundalk Marine Terminal

Boring No: DMT-11S

Diameter: 8 in Date: 01/06/2006

Northing: 574430.5 Easting: 1448175.9 Elevation: 22.10 Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Consultant: L. Hunt (CH2M Hill, Inc.)

Total Depth: 48.0 Ft GW Depth: 24.0 Ft

Datum: Baltimore City Project No: 327494 Field Book No: Depth g Soil DMT-11S Pattern Soil Description Ft Code Diagram SILTY SAND (SM), wet, brown loose, fire to come grained quartz sand, homogeneous. 35 SM SAND (SP), wat, light gray, loose. fine to coarse grained quartz sand, trace sit and organics. homogeneous, native materials. SP 40

BORING LOG Honeywell Site: Dundalk Marine Terminal

Boring No: DMT-11S

Diameter: 8 in Date: 01/06/2006

Northing: 574430.5 Easting: 1448175.9 Elevation: 22.10

Datum: Baltimore City

Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Page 7 of 7

Consultant: L. Hunt (CH2M Hill, Inc.)

Project No: 327494 Field Book No: Total Depth: 48.0 Ft GW Depth: 24.0 Ft

Depth g Soil DMT-11S Pattern Soil Description Ft Code Diagram SAND (SP); wet, light gray, loose, fine to coarse grained quartz sand, trace sit and organics, 42 SP CLAYEV SAND (SC); with gray, medium dense, homogeneous, grades to finer sand and silt at bottom, SC SILTY GLAY (CL); moist, gray, soff, trace mipas, organics, and sand, homogeneous, native materials 45 CL 48.0 Twenty-five feet of augers were lost in first attempt at sampling and well installation. The augers were lost in the process of installing the well, so another borehole was drilled nearby and the well was installed here.

)n(2)		Page 1 of 2	Site: Dundalk Marine Terminal Boring No: DMT-58S CPT No:	Northing: 574* Easting: 1447 Elevation: 13.	839.8		Method: H	Chapel (Parratt Wo ollow Stem Auger : J. Rogers (CH2M)	Total De GW Dep Date: 09	oth: 0.0 F	t
Depth Ft	Recov.	Sample ID	DPC	Soil Code		Soil Description		USCS	Con	nments	Well Diagram	W	Gs	PL	LL	P200
5 —				10	Bonsfiole advanced to 1	(G Nime cruar (a cialleisting elitriphine										
10 -		ovir deuts sou mondo				owen red (5 YR 5/6) will bace gray (10 YR 5/ obanse sand, some gravel, medium self)	H1. molet	ML				15				
15			6.				******							11	OCI	us

BORING LOG Page 2 of 2 Site: Dundalk Marine Terminal Northing: 574170.4 Total Depth: 29.0 Ft Driller: A. Chapel (Parratt Wolff, Inc.) Honeywell Boring No: DMT-56S Easting: 1447839.8 Method: Hollow Stem Auger GW Depth: 0.0 Ft CPT No: Date: 09/04/2007 Elevation: 13.1 Consultant: J. Rogers (CH2M Hill, Inc.) Depth Seco Sample Soil DPC Soil Description uscs Comments Well Diagram W Gs PL LL P200 ID Code 15 SANDY SILT (ML) -yerlower red (5 YR 5/8) with trace gray (10 YR 5/1), moint ML to wet. The sand, trace coarse sand, some gravel, middum stift-SILTY CLAY (CL) -dark gray (10 Yrt Art), trout this year yeary fire land left fredum CL WT-89-8-80-179-178 30 PODRLY-GRADED SAND (SP) -black (10 YR 2/1) and pale brown (10 YR 6/5). 20 grading to pale brown (10 YR 6/3) and very pale brown (10 YR 1/3) at 18 feet saturated, SP fine to medium sand, (dense) No recovery 25 POORLY-GRADED SAND (SP) -pale brown (10 YR 5/3), very pale brown (10. WINESO SHEN 29 VR 7/3) and dark gray (10 VR 4/1), saturated, line to medium sand, (dense) Poor recovery due to runny, saturated sands No recovery

Locus

Lithology described from DPT borehole. Well installed

with hollow-stem auger.

EA ENGINEERING, SCIENCE, AND TECHNOLOGY

LOG OF SOIL BORING

15 LOVETON CIRCLE

SPARKS, MARYLAND 21152

TELE: 301-771-4950

BORING NO. EA 7M

Coordinates:

Surface Elevation: 11.76
Casing Above Surface:
Reference Elevation:
Reference Description:

Start Date: 02-17-87

Location: Dundalk Marine Terminal

Job No. DMT61A

Client: Maryland Port Administration

Drilling Method: Mobile B-61 Drill Rig, 11"D.D.,

6"I.D. Hollow Stem Augers

Sampling Method: Split Spoon 2"O.D.,

24" long driven w/ 140 lb. Hammer dropping 30"

7-87 Completion Date: 02-20-87

SAMP TYPE				DPTH:		FILL	:DEPTH	LOG	SURFACE CONDITIONS Flat asphalt road surface
							0_		SOIL DESCRIPTION
							-	-GF	Asphalt to 10" Green gravel, wet, coarse to 3 ft.
55	24	24	1	4.5	3-3-3-6		-	-	Bright orange red clay, dry to moist, slightly sticky, medium stiff
SS	24	24	2	6.5	3-9-9-6		5_	CL .	Bright orange red clay, dry to moist, slightly sticky, medium stiff plastic Water in augers at 6.5 ft.
55	24	14	3	8.5	6-4-8-12		-	SP	Dark gray green silty sand, moist, fine grained, medium dense, w/ yellow to yellow green tailings fill and odor
SS	24	13	4	10.0	4-5-6-11		10_		Bright orange red clay, dry to moist, slightly sticky, medium stiff, plastic w/ some tailings fill 10.8-11.0 ft.
SS !	24	22	. !		3-2-5-6		-	CL	Bright orange red clay, dry to moist, slightly sticky, medium stiff, plastic
SS	24	14	5	14.0	3-7-7-7		15_	-	Bright orange red clay, dry to moist, slightly sticky, medium stiff, plastic w/ some tailings 14.7-15.0 ft.
							20		TD soil sampling 13:30 hrs at 15.0 ft. Shallow well completion; hole backfilled to 9 ft. w/ bentonits pellets, Morie no. 2 sand foot to 8.7 ft. Install 4"I.D. sched 40 PVC no. 10 slot screen 9.0-6.0 ft. Place morie no. 2 sand pack 9.0-5.5 ft. Flace bentonite pellet seal from 5.5 ft. to 10 in. Manhole installation to follow

SAMPLER TYPE
SS-DRIVEN SPLIT SPOON
SH-PRESSED SHELBY TUBE
OST-OSTERBURG PISTON SAMPLER
DEN-DENISON CORE BARREL SAMPLER
SPT-STANDARD PENETRATION TEST (ASTM D 1586-84)

AFTER 24 HRS.

15 LOVETON CIRCLE

SPARKS, MARYLAND 21152

TELE: 301-771-4950

BORING NO. EA 7M

Coordinates:

Surface Elevation: 11.76

Casing Above Surface: Reference Elevation:

Reference Description:

Start Date: 02-17-87

Location: Dundalk Marine Terminal

Job No. DMT61A

Client: Maryland Port Administration

Drilling Method: Mobile B-80 Drill Rig, 7"O.D. roller bit

mud rotary method

Sampling Method: Split Spoon 2"0.D.,

18" long driven w/ 140 lb. Hammer dropping 30"

Completion Date: 02-20-87

	IDRVA			I DPTH:	BLOWS/6"	FILL	I DEPTH I FEET	GRAPH:	SURFACE CONDITIONS Flat asphalt road surface
SPT	18	0			2-2-3		20_	GM .	Varicolored red clay, green silt and clay, organic mud, with fine to medium subrounded gravel and sand, wood debris
SPT	18	6	6	25.0	2-3-3		25_		Various fill material, clay, sand, gravel, mud and wood debris
SPT	18	14	7	31.0	2-3-4		30_	5M	Light gray streaked brown sand, wet, very fine to fine grained, with some silt, loose
SPT	18	12	8	35.0	1-2-2		35_	CL/ML	Medium gray streaked green silt w/ some clay, wet, some fine sand, soft, low to moderate plasticity
							40_	ML	Dark green silt, fine sand, micaceous composition

SAMPLER TYPE
SS-DRIVEN SPLIT SPOON
SH-PRESSED SHELBY TUBE
OST-OSTERBURG PISTON SAMPLER
DEN-DENISON CORE BARREL SAMPLER
SPT-STANDARD PENETRATION TEST (ASTM D 1586-84)

GROUND-WATER DEPTH BELOW GRADE AT COMPLETION AFTER HRS. AFTER 24 HRS.

15 LOVETON CIRCLE

SPARKS, MARYLAND 21152

TELE: 301-771-4950

BORING NO. EA 7M

Coordinates:

Surface Elevation: 11.76
Casing Above Surface:
Reference Elevation:
Reference Description:

Location: Dundalk Marine Terminal

Job No. DMT61A

Client: Maryland Port Administration

Drilling Method: Mobile B-80 Drill Rig, 7"O.D. roller bit

mud rotary method

Sampling Method: Split Spoon 2"0.D.,

18" long driven w/ 140 lb. Hammer dropping 30"

Start Date: 02-17-87

Completion Date: 02-20-87

SAMP	DRVN	-	-	SAMP:	BLOWS/6"	Cr FILL	DEPTH FEET	GRAPH:	SURFACE CONDITIONS Flat asphalt road surface
SPT	18	15	9	41.0	4-5-7		40_	ML	Dark green silt w/ little very fine sand, moist, stiff, micaceous non-plastic
SPT	18	18	10	46.0	2-2-4		45_		Grading to medium stiff, increasing fines content Dark green silt w/ little fine sand and trace clay, medium stiff, low plasticity
SH	24	24	ST 1	48.0			- 50_	ML/CL	Collect Shelby Tube sample from 48.0-50.0 ft. Dark green silt w. some clay and very fine sand, very moist, slightly micaceous, low plasticity, medium stiff
SPT	18	18	11	56.0	1-2-2		55_		Dark green silt w/ little clay, moist
				1			60_		

SAMPLER TYPE
SS-DRIVEN SPLIT SPOON
SH-PRESSED SHELBY TUBE
OST-OSTERBURG PISTON SAMPLER
DEN-DENISON CORE BARREL SAMPLER
SPT-STANDARD PENETRATION TEST (ASTM D 1586-84)

GROUND-WATER DEPTH BELOW GRADE AT COMPLETION AFTER HRS. AFTER 24 HRS.

15 LOVETON CIRCLE

SPARKS, MARYLAND 21152

TELE: 301-771-4950

BORING NO. EA 7M

Coordinates:

Surface Elevation: 11.76
Casing Above Surface:
Reference Elevation:

Location: Dundalk Marine Terminal

Job No. DMT61A

Client: Maryland Port Administration

Drilling Method: Mobile B-80 Drill Rig, 7"O.D. roller bit

mud rotary method

Sampling Method: Split Spoon 2"0.D.,

18" long driven w/ 140 lb. Hammer dropping 30"

Start Date: 02-17-87

Reference Description:

Completion Date: 02-20-87

1	SAMP		RCVD	SAMP NO.	SAMP!	BLOWS/6*	FILL	DEPTH	GRAPH LDG	SURFACE CONDITIONS Flat asphalt road surface
-	SPT	18	15	12	61.0	2-3-3		60_	ML	Dark green silt w/ little clay and very fine sand, moist, medium stiff, low plasticity
-								65_		
								-		Drilling smoothly
	SPT	18	16	13	71.0	2-2-3		70_		Dark green silt w/ trace of clay
-								75_		Drilling smoothly
-										
								80_		

SAMPLER TYPE
SS-DRIVEN SPLIT SPOON
SH-PRESSED SHELBY TUBE
OST-OSTERBURG PISTON SAMPLER
DEN-DENISON CORE BARREL SAMPLER
SPT-STANDARD PENETRATION TEST (ASTM D 1586-84)

SROUND-WATER DEPTH BELOW GRADE AT COMPLETION AFTER HRS. AFTER 24 HRS.

15 LOVETON CIRCLE

SPARKS, MARYLAND 21152

TELE: 301-771-4950

BORING NO. EA 7M

Coordinates:

Surface Elevation: 11.76
Casing Above Surface:
Reference Elevation:
Reference Description:

Location: Dundalk Marine Terminal

Job No. DMT61A

Client: Maryland Fort Administration

Drilling Method: Mobile B-80 Drill Rig, 7"O.D. roller bit

and rotary method

Sampling Method: Split Spoon 2"D.D.,

18" long driven w/ 140 lb. Hammer dropping 30"

Start Date: 02-17-87

Completion Date: 02-20-87

				DPTH		FILL	DEPTH FEET	(GRAPH)	SURFACE CONDITIONS Flat asphalt road surface
CDT	1 10	1 10	10	81.0	2.4.5		80_		SOIL DESCRIPTION
1 201	10	1 10	1 14	181.0	2-4-5		-	ML/CL	Dark green silt w/little clay and very fine sand, moist micaceous, low to medium plasticity
							-		
SPT	18	12	15	85.5	4-4-7		85_		Dark green silt, micaceous
							-		Penetration difficult past 80.5 ft., but no fluid loss
SS	12	8	16	90.5	35-48		90_	SP/SM	Medium gray sand, wet, fine to medium grained w/ trace to a little amount silt, subangular to subrounded grains, very dense predominantly quartzose composition
							-		Grading to more silt content
SPT	18	12	17	95.5	17-21-24		95_	SM CD /CM	Medium gray sand, wet, fine grained w/ some silt micaceous, dense
SPT	18	13	18	99.9	14-15-17		-	SP/SM:	Less silt content past 95.5-98.5 ft.
-		-	-	1			100_		

SAMPLER TYPE
SS-DRIVEN SPLIT SPOON
SH-PRESSED SHELBY TUBE
OST-OSTERBURG PISTON SAMPLER
DEN-DENISON CORE BARREL SAMPLER
SPT-STANDARD PENETRATION TEST (ASTM D 1586-84)

GROUND-WATER DEPTH BELOW GRADE AT COMPLETION AFTER HRS. AFTER 24 HRS.

THE THOUGH TECHNOLOGY

LOG OF SOIL BORING

15 LOVETON CIRCLE

SPARKS, MARYLAND 21152

TELE: 301-771-4950

BORING NO. EA 7M

Coordinates:

Surface Elevation: 11.76

Casing Above Surface: Reference Elevation: Reference Description: Location: Dundalk Marine Terminal

Job No. DMT61A

Client: Maryland Port Administration

Drilling Method: Mobile B-80 Drill Rig, 7"O.D. roller bit

mud rotary method

Sampling Method: Split Spoon 2"0.D.,

18" long driven w/ 140 lb. Hammer dropping 30"

Start Date: 02-17-87

Completion Date: 02-20-87

	SAMP		IRCVD	ISAMF	DPTH	BLOWS/6"	FILL	DEFTH	GRAPH:	SURFACE CONDITIONS Flat asphalt road surface
								100_	SP/SM	Some fluid loss noted past 100 ft.
1 1 1	SPT	12	0			8-44		105_	SF:/6P	Varicolored to tan gravel and coarse sand, poorly sorted/well graded trace silt content
	SPT	9 1		10	110	70_50/7		110_		Penetration difficult past 107 ft. Lost large amount of fluid past 108 ft.
		,		17		38-50/3				TD soil sampling 15:00 hrs. at 111 ft.
1								115_		
				1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1						
								120_		

SAMPLER TYPE SS-DRIVEN SPLIT SPOON SH-PRESSED SHELBY TUBE OST-OSTERBURG PISTON SAMPLER DEN-DENISON CORE BARREL SAMPLER SPT-STANDARD PENETRATION TEST (ASTM D 1586-84)

GROUND-WATER DEPTH BELOW GRADE AT COMPLETION AFTER HRS. AFTER 24 HFS.

FT.

PROJECT NUMBER 706612.DM.03.30.36.ES

BORING NUMBER

S-1

SHEET 2 OF 2

SOIL BORING LOG

PROJECT : Geotech and GW Invest Area 1501/1602

ELEVATION : TED DRILLING CONTRACTOR : Haynes and Associates

INTERVA		-1, 1/	STANDARD	CORE DESCRIPTION	LOGGER: Lise Conte-
		#/TYPE	PENETRATION TEST RESULTS 6"-6"-6"-6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	DEPTH OF CASING, DRILLING DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION
0,-5,		1-22.	7-11-12-16 (23) Previous autempt on 1-3-19	₹2'-3.6'	Note = Sedinat S. 8 2 helow top rip rap. All are below sodin level in feet, pre
a'-4	0.6 2.0	<i>a-</i> 5	4-5-6-4	Sandy Silt (mu), dark Yellowish brown 104R 4/4. Let, Stiff, No plasticity, fine - to med grained sond, trace Subrounded gravel = 1"	
	1.3'	4 -3	2-3-3-4	4'-5,3' foorly Graded Sad = (SP), very dork gravish brown = 10 yr 3/2, wet, loose, medium = grained Sand, trace silt, trace well rounded gravel 0-2 indes, = at 4-7' bgs	
6-8	3.0'	\$5-4	tioa/di	2'-8' Some as above. Clay lense at 7.3' and 7.6'	
8'-10' 2	.01	V-5	2-2-2-3 (4)	8-10' Some as above -	

PROJECT NUMBER 706612.DM.03.30.36.ES

BORING NUMBER S-1

SHEET 1 OF &

SOIL BORING LOG

PROJECT : Geotech and GW Invest Area 1501/1602

LOCATION: DMT, Baltimore, MD

ELEVATION : TOD

DRILLING CONTRACTOR: Haynes and Associates

		S: N			T: 1-18-19 0245 END:1-18-19 1110	LOGGER: Lisa Carter
	NTERVA	URFACE	(FT)	STANDARD	CORE DESCRIPTION	COMMENTS
	VIERVA		/ERY (IN) #/TYPE	PENETRATION TEST RESULTS 6"-6"-6"-6" (N)	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	DEPTH OF CASING, DRILLING RAT DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION.
111111111111111111111111111111111111111	0'-12'	2.0'	ù-2 ≿	(27) 5-13-14-16	10'-11.5' Poorly Graded Soud - (SP) Very dork grayish brown 10'r 3/2, wet, medium dense, medium grained Soud, + race - Silt.	_
13	પ્રે-14'	1.3' 2.0'	\$5-7		12'- 12.61 Poor braded Sond - (50), Brown 10'R 4/3, dense, wet, medium grained sond, trait S.14 12-6'- 12.8' Poorly braded brown 10'M of Snd (GP), dark brown 10'M 3/3, wet, dense, fine rounded -	
111111111111111111111111111111111111111	1-16	۵.0°	21-8		gravel. 13.8'-13.3' Poonly braded - Soid (SP), brown 5/3 10, R. = wet, dense, fine to medium - grained and; little silt - i4'-14.0' Poorly braded Soil- (SP)+ yellowyh bran 100 R	
	-18'	2.01	y-9	4-8-9-11 (17)	Sto, wet, redismdense, fire - graned Sad, trace silt - 10'-16.3' Some as above -	
18:	~~ .	2.0	S7-10	McH-3-5-	very dork gray love 3/2 of black mattling to 18.9'. From 18.9'. 20' - dork grayish brown 104R 4/2, mo soft to medium consistency 1 -	Set 1" well 10's cover to 18' bas

PROJECT NUMBER 706612

MBER BORING NUMBER

5-2

Page \ of ~

S. A.A.L. F. P. P.	LEVE			15	START: 8 21 1 8 1020 END:		LOGGER	1:1. Raterine
TH BELOW	SURFAC	E (FT)			SOIL DESCRIPTION	USCS		COMMENTS
INTERV		ERY (IN) SAMPLE		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		BLOW COUNTS	DEPTH OF CASING, DRILLIN RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLE
-			(COPR)		0-3.4 No recovery			COMMENTS
- 0	4.0				36-4.0 Clay (CL) 2.57R 218 moderate orange moist, M-F, trace sand at 4.064			
1 1 1 1 1 1								
-				1	IV 4 To 1	CL		
- 4/8	lε				4-65 No recovery (05-6.8 Clay (CL) 2.54R 7/8 moist, firm (0.8-26: Clay (CL) 2.54R 7/8 moist, M-F, trace sand 7.6-8.0 Clay (CL) 2.548 7/8	رد		
			37		8-11.6 No recovery 11.6-12.0 Clay (CL) 258/R 7/8			

PROJECT NUMBER 706612

MBER BORING NUMBER

5-2

Page 2 of 2

			ywell D	MT		LOCATION	: Baltimo	re, MD	
RII I	ATION ING M	ETHO	AND	OURDAN	ENTTIE	DRILLING CONTRACTOF Parratt Wolff			
TD V	VATER	LEVE	ANUE	QUIPME	INT USE	START: 8 21/18 1020 END:		LOGGER	R: L. Raterink
		SURFAC				SOIL DESCRIPTION	USCS	LOGGER	COMMENTS
	INTERV		-6.1	-	COPR		0303	BLOW	
			ERY (IN)		Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,		COUNTS	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION
			#/TYPE	DPC (COPR)		MINERALOGY.			DRILLING ACTIONS/DRILLER COMMENTS
Linn									
F 1 1 T				_			CL		
-	-	-		1,550	_		-		
1111	12-					12-15,5 No recovery 15.5-16 Silty Clay 2.5YR 218 wet, Soft to Hedium. Whe Med sand			
-	16	05				2.5YR 718			0 0 0
-						While Med sand			
_						1777			
-							1 1		
-									
-									
-							-		
-				-					
-									
7									
-									
-		6							
-									
-									
-									
-		- V							
-									

PROJECT NUMBER 706612

BORING NUMBER
5-2 (offset) Page 1

EV	ATION	:	A A A I I I	OLUM		DRILLING CONTRACTOF Parratt Wolff		
RILL TD V	ING M	ETHOL	AND E	QUIPMI	ENT USE	DRILLING CONTRACTOF Parratt Wolff D: +11 000 START: 6 23(19 830END: 6 123(18 1630 SOIL DESCRIPTION	LOGO	GER: L. Rateriuk
PTH	BELOW	SURFAC	E (FT)			SOIL DESCRIPTION US	SCS	COMMENTS
	INTERV	AL (FT)	SAMPLE		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY,	BLOV	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER
_				(COPR)		(3) 49 10 text		COMMENTS
TITLITY	02	0.9	35			0-1.1 No recovery 1.1-1.6 clay (ci) S-M, moist 2.5 YR 718 1.6-2.0 Sitty clay M-F Moist 10 YR 718		
-				-				
	2-4	0.5	55-2			2.0-3.5 100 recovery 3.5-4.0 SAA		
1				-				
111111	40	0.j	553			40-50 NO recovery 5.0-6.0 SAA		
111				1				
Tittliii.	600	1.3	555	_		6.0-6.7 No recovery 67-7.7 SAA 7.7-8.0 Silty clay (CL) Little sand, moist, (transition 2014)		
11-11	8-	1.5	等			8.0-8.5 to recovery 85-9.3 silty clay (CL) moist, S-M 9.3-96 SAA, wet, V.SOA 96-10.0 silty clay, M-F dry-Moist		

PROJECT NUMBER

BORING NUMBER

S-2 (offset)

Page 2 of 2

RILL	ATION ING M	ETHOR	ANDE	QUIPME	NT USF	D: LY 1000			
UV	VALER	LEVE			18	STAKE 18 1331 A OR 30 END : 10 12 3112 10 3		LOGGER	
	INTERV	SURFAC	E(FT)	-	COPR	SOIL DESCRIPTION	USCS	P1 014	COMMENTS
	IIV) ERV		SAMPLE		Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,		BLOW	DEPTH OF CASING, DRILLIN RATE, DRILLING FLUID LOS TESTS, AND STRUMENTATIO DRILLING ACTIONS/DRILLE
			wire	(COPR)		MINERALOGY.		4.	COMMENTS
1111	10-	8.11	55 6	1		10.0-10.2 NO recovery 10.2-10.8 Silty Clay, M-F 254R718, maist			
A CLAP			v	-		10,8-11.3 clayer silt, soft maist # 2.54x718 11.3-11.8 clayer silt, soft, maist 2.54x718 w/	_		
-					1	118-120 clayer silt MF Mast	/		
11111	12-14	1.3	557	1. 1		120-127 No recovery 127-133 same as H.3-11.8 13.3-14.0 clayey 514, moist Medium, little fine sans 2548 218 615 48 516			
TOTAL TATE	jui- le	o.5	53	17		14-15.5 100 recovery			
	18	1.4	55°q	- ,		16.0-16.6 100 recovery 16.6-16.8 ned Sand, wet, 16.6-16.8 ned Sand, wet, 1048 212 168-17.5 ned Sand, wet, 1008e [med, poorly graded 1048612 17.5-18.0 fine to med Sand			
1111111	20	0.9	55.	1		18-19.1 No necovery 19.1-19.3 SAA 19.3-20.0 Sance as 16.8-17.5			
=		1		1			C		

PROJECT NUMBER 706612 BORING NUMBER

5-3

Page | of 2

TH BELOW	LEVE	Li			ED: +17POZ START 8/24/18 1450 END: 3/24/18 1715	urce	LOGGER	COMMENTS
INTERV	AL (FT)	ERY (IN)		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR,	USCS	BLOW COUNTS	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS.
		SAMPLE WTYPE	DPC (COPR)		MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	0.4		TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS
		MC-			0-2.1 No recovery 2.1-3.2 Clay (CL) - 2.5 YR 7/8 moderate orange 5-M, moist, med plasticity			
- 4	19	1			3.2-3.4 saindy gravel, moist			
			- 1	+ +	3.4-40 clayey selt, moist sew f-M sand			
			1 3	+ 1				
4-8	2.5	MC-2	1 1 1 1	1 7 1 1 1	4.0.5,5 no recovery 55-63 SAA 63-6.5 Sandy grave, wet well graded 65-67 clayey SUL, moist 2.5 YR 718, tim 67-76 clayey 511, moist Whe Sand, 54R 516 inedium 7.6-8.0 Sance as 65-6.7			
135	2.1	MC 3			8-9,9 NO recovery 9.9-10,2 Same as 67-76 10.2-10.5 Silky clay, first			

PROJECT NUMBER

BORING NUMBER

5-3

Page 2 of 2

SOIL BORING LOG

PROJECT: Honeywell DMT LOCATION: Baltimore, MD DRILLING CONTRACTOF Parratt Wolff ELEVATION: DRILLING METHOD AND EQUIPMENT USED: +VLPOOD
ATD WATER LEVEL: | START SOUNTS 1480 END: 8114/18 1715 LOGGER: L. Raterine DEPTH BELOW SURFACE (FT) INTERVAL (FT) BLOW DEPTH OF CASING, DRILLING SOIL NAME, USCS GROUP SYMBOL, COLOR, Strata COUNTS RECOVERY (IN) RATE, DRILLING FLUID LOSS. MOISTURE CONTENT, RELATIVE DENSITY. SAMPLE TESTS, AND STRUMENTATION. OR CONSISTENCY, SOIL STRUCTURE, WTYPE DRILLING ACTIONS/DRILLER DPC MINERALOGY. COMMENTS (COPR) 10.5- 10.8 Same as 9.9-10.2 10.8-12.0 clayer sut 25 YR 718, moist few v fine sand 12.0-12.3 no recovery 12.3-12.6 SAA 12.6-12.8 sandy gravel MG Little Sury clay, wet 12-3.7 104R312 16 128-13.9 clayey sitt 13.9-14.6 Silty clay, moist V. SOFT 1042 312 14.6-15.2 medium sand wet 100x to medium, few silt 152-16 clayer silt, mast Firm love 312 16 16-16-2 no recovery 162-17-3 SAA 123-185 clayer silt, M-F, moist, 10487/2 20 MU 5 18.5-19.5 fine sand, Sum sitt, maist to wet 13 1048712 19.5-20 f-4 sand, wet, 104R712

PROJECT NUMBER

706612

BORING NUMBER

=-4

Page of 2

SOIL BORING LOG

PROJECT: Honeywell DMT LOCATION : Baltimore, MD ELEVATION DRILLING CONTRACTOF Parratt Wolff DRILLING METHOD AND EQUIPMENT USED : ED: tripod START S/20/19 1520/END: 9125/18 ATD WATER LEVEL : LOGGER: Raterink DEPTH BELOW SURFACE (FT) SOIL DESCRIPTION COMMENTS USCS INTERVAL (FT) BLOW DEPTH OF CASING, DRILLING SOIL NAME, USCS GROUP SYMBOL, COLOR, RECOVERY (IN) COUNTS RATE, DRILLING FLUID LOSS. MOISTURE CONTENT, RELATIVE DENSITY, SAMPLE TESTS, AND STRUMENTATION. OR CONSISTENCY, SOIL STRUCTURE, W/TYPE DRILLING ACTIONS/DRILLER MINERALDGY COMMENTS COPR 0-25 no recovery 25-30 Silty clay want MU 4 1.5 3.0-36 Silly clay ned moist, red & brown fittle sand (Fine) 1542716 4-5.5 no recovery 1048 76, moist, Firm MC-75 598° clayery sut (a) noist, U-F 257R5110 8-9.4 no recount 94-10.1 SAA W3 10.1-10 concraying sitt 12/26 10-6-11 SA 94-10-1 11-11-2 SA 10-1-10.6

PROJECT NUMBER

706612

BORING NUMBER

5.4

Page of L

SOIL BORING LOG

PROJECT Honeywell DMT LOCATION: Baltimore, MD ELEVATION DRILLING CONTRACTOF Parratt Wolff DRILLING METHOD AND EQUIPMENT USED: + 1000 START: 8123/18 LOGGER: (Rajente ATD WATER LEVEL : END: 8125/12 DEPTH BELOW SURFACE (FT) SOIL DESCRIPTION USCS INTERVAL (FT) DEPTH OF CASING, DRILLING BLOW SOIL NAME, USCS GROUP SYMBOL, COLOR, COUNTS RECOVERY (IN) RATE, DRILLING FLUID LOSS. MOISTURE CONTENT, RELATIVE DENSITY, SAMPLE TESTS, AND STRUMENTATION. OR CONSISTENCY, SOIL STRUCTURE, DRILLING ACTIONS/DRILLER WTYPE DPC MINERALOGY. COMMENTS 112-11 le fine-Med seud 154R312 dare gran, wet, med 184RS14 uet med, gray
118-12 sand (med), wet
med, bown 104R5/6 12-148 SAA 148-16 med sand moist to west; med 1548914 12 MC-4 15.8-16 M10486/2 16-175 no recoury 16 25 MG 175-90 SAA Screens 11-12 15-20

PROJECT NUMBER

BORING NUMBER

5-5

Page / of 2

ELEVAT					DRILLING CONTRACTOF Parratt Wolff	Baltimo	re, MD						
TD WA	G METHOD TER LEVE		QUIPME			USCS	LOGGER	2: L Rectanile					
-	TERVAL (FT)	SAMPLE DPC (COPR)		SAMPLE NTYPE DPC		AL (FT) CC RECOVERY (IN) Sta SAMPLE WTYPE DPC		COPR Strata	OPR COULTURE USOS CROUPS CALCO		BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS. TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS	
11. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	3 27 K		0-1.3 no recevery 1.3-2.0 clayer site med, ned moist 2.0-2.6 setty clay M-F, ned moist 2.6-3.3 SA 1.3-2.0 3.3-3.4 sety clay firm, moist, pine 3.4-4.0 SA 1.3-2.0										
Darrie Land Land	1.1				4-6.9 NO NEWLY 6.9-7.1 SA 3.3-34 3.1-80 SA 1.3-3.0								
	34				8-8.6 ne recours PL-8.8 slassy siet red, soft 8.8-8.9 Angulor granel File material File material That pooling water								

PROJECT NUMBER

BORING NUMBER

5-5

Page 2 of 1

_	_	SURFAC			18	START : BIJ F END : START : BIS SOIL DESCRIPTION LIS	LOGGER	COMMENTS
	INTERV	_	ERY (IN)		COPR	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,	COUNTS	DEPTH OF CASING, DRILLIN RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATIO
			#/TYPE	DPC (COPR)		MINERALOGY.		DRILLING ACTIONS/DRILLE COMMENTS
TILLIA						1.1- F-M Sand Most-MAT		1048112 104812
1						15.8-11.7 SAA W/ MIXIM 251R866		2.5 YR 8/16
1111	12-					117-12 SA 9.1-108 weept 75428/16	1	1042918
and dance Line	16	38	4			12.2-16 med sand, to moist to wet, medium dense		
1 1 1 1 1 1 1	.6		m²,			145AA		
1 1011	30	4,0	3					
111	1		-					

PROJECT NUMBER BORING NUMBER

-

5-6

Al of 3

INC	N: T	D A	ND FO	HOMEN	THE	DRILLING CONTRACTOR: Parratt Wolff Mile D: HSA/SPILT Special - Mobile Drill	HEDES	31400	Hynes and Associates D
VATE	R LEVE	L:	TED	UIPMEN	STAR	1:12-3-18 1020 END: 12-3-18 12	10	3/4 (00	LOGGER: Lisa Curte
	W SURFA TERVAL (I	FT)	RY (FT) SAMPLE #TYPE	DPC (COPR)		SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	USCS	BLOW	COMMENTS DEPTH OF CASING, DRILLING RATE DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION. DRILLING ACTIONS/DRILLER COMMENTS. DRILLING ACTIONS/DRILLER COMMENTS.
-	-3 s	- 10	55-)	()		0.0.2' Debris/Trish, 0.2'-0.5' Poorly Greded, Soud (SO), wet, derived, medium grained Soud. Strong books 7.5 = 4/6, Trace 5.1t,	SP	(3) (-1-0-0	81262 COUNTS 1-1-2-2
- 2	4 10	á	32	- +		2'-21' Some as above _ me Posture DR D1 at 2.7' bs minor reactive. wood fragmosts at 3.1' bys 3.1' ·3.2' - limertone gravel	24	6-8 9-10 (17)	6-7-9-10
- 4,	61.2		ss-3	1 1 1		4.5.3' well Greded gravel with Send (bus) evet, densel, valsaish brown layer 5/4, fine to coarse gravel, engite to sib rainded, median to coarse graved Send.	(دري	8-21 59/3	Y
2 1 1 1 1	80		પ્રત	1-1		6-6.8' Poorly Greded Soud (SP), met, Dork medin dense, derk yellowish brace 10:12 3/6, medium grand Soud, some Silt, trace rounded fine gravel		6-6 6-6 (14)	
- 4	101	- C- C-	\$\$·\$			2'-9.1' S. 14 Sord (sm net, loose to very dense, selloush brown look 5/6, fine to median graced	Sen	2-3	

BORING NUMBER

5-6

Pg 2003

SOIL BORING LOG

			II DMT			LOCATION: Baltimore, MD		Diene	ctest Deilling										
LING N	METH	ODA	ND EQI	JIPMEN	IT USE	D: HSA /Split (2001 - mobile Dill	100	3/4"	A-seers										
WATE	R LE	/EL:	TBI)	STAR	1:12-3-18 1020 END: 12-3-17	171		LOGGER: LIKE CUTE										
H BELOW	V SUR	FACE (F	-T)			SOIL DESCRIPTION	USCS	1	COMMENTS										
	ERVAL	ECOVE	RY (FT) SAMPLE STYPE	SAMPLE STYPE DPC		SAMPLE STYPE DPC		SAMPLE STYPE DPC		MAMPLE DPC		MAMPLE STYPE DPC		TYPE DPC		SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		BLOW	DEPTH OF CASING, DRILLING RATE DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION. DRILLING ACTIONS/DRILLER COMMENTS. DRILLING ACTIONS/DRILLER COMMEN
-	-	_		(COPR)			-	_	DRICEING ACTIONSISTICLER COMMEN										
- 13	1,	100	م) کاک	1 1 1 1 1		10050. 10050. 11.3'-11.8' Fot Clay (CH) wet, stiff, very dork growith brown '3/2257,	ST	10-5- 4-5 (9)											
	- 1	2, 2,	ss-7	1 7 1		high plasticity 12'-13' Some as above Soft to median consistency		(2) 3-3 1-5-											
		3,	ક્ષ્ય	1 4 1		14'-15.2 Jone as Previous		3-3- 3-3-											
		2.0	55°E	1 1 1 1		14-18 some as previous		2-2- 3-1 (s)											
- - - - 2:	1	.2'	O1-12	1 (13'-19' Save asprevious 19'-19'3' Clayer Sord (SC) neet, 10050, cont	Æ	2-3-											

fine to median graved Sc (9)

20-

PROJECT NUMBER

BORING NUMBER

DJECT: I					LOCATION : Baltimore, MI			ta:
VATION:	THOOL	RD	IIPMEN	TUSE	DEILLING CONTRACTOR: Parent Wolff Howe	40	2 40	te,
WATER				STAR		210	.2.14	LOGGER: USE Cote
TH BELOW SI					SOIL DESCRIPTION	USCS		COMMENTS
INTERV	AL (FT)	ERY (FT) SAMPLE STYPE	DPC		SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION. DRILLING ACTIONS/DRILLER COMMENTS.
2	-		(COPR)			-		DRILLING ACTIONS/DRILLER COMMEN
- 32, - 32,	1.2 20	St-1	1.1345		20'-20.8' foorly traded South (SP), wet, look, very derligrapish brown 2.59 3/2, nedway grained South, trave rounded fine growel 20.2'-21,2' Fot Clay(CH)		(3)	Set well from 19'-21' target Sad interval
- 23'- - 24'	10.04	XS-12	1 1 1 1 1 1		very dock size ish brown 25 - 12. 3/2. wet, modern hish plastict. 20-24 Fet clay some as above	CH	3-3- 4-4 (7)	
- W'-	20 20	\$\$ - 12	11111		24'-26 Somers change		2-3- 3-4 (6)	
Tarred Nove					End boring at 26 bys			
1 1 1								

PROJECT NUMBER BORING NUMBER

19/20

refusal

SOIL BORING LOG

PROJECT: Honeywell DMT ELEVATION: LOCATION: Baltimore, MD DRILLING CONTRACTOF Parratt Wolff DRILLING METHOD AND EQUIPMENT USED: START: 9/6/18 END: ATD WATER LEVEL : DEPTH BELOW SURFACE (FT) Costus LOGGER: INTERVAL (FT) COPR COMMENTS SOIL NAME, USCS GROUP SYMBOL, COLOR, RECOVERY (IN) BLOW DEPTH OF CASING, DRILLING MOISTURE CONTENT, RELATIVE DENSITY, SAMPLE COUNTS RATE, DRILLING FLUID LOSS, OR CONSISTENCY, SOIL STRUCTURE, #/TYPE DPC TESTS, AND STRUMENTATION. MINERALOGY. DRILLING ACTIONS/DRILLER COMMENTS SIHT SAND ON GRAVEL 60 Varregated browns > 150 10 ye 6/2 , 10 / 4/3, 10 ye 3/2 MC 85 grove 1 - gt were rounded, goler. 35 dry-most, med to dense 35 12 14 15 SAA 8" drg 4-5.5 SPISM >50 WC-35 7 8" NI (place) west-co SAND 7100/ borney / rofusas SP &. Wet past refusi 15 [4 13" SAA - No bluck med - CS SAND SPISM MCtrace 1-med grower, trace silk 9 - Shord of plaste 9" NI (blue a) Strand word potrol oder 14 wood in Shoe 5 yr DI BOWN 3 h Sheet 150 med - CS SAND, trace grant 8-10 0 MCH 46 + silt, wet , very dens 150

706612

DL-1- SAA

Page 1 of 8

JACOBS

ROJE	CT:	Honey	well DA	AT		LOCATION :		e, MiD	
RILL	ING ME	THOO	AND E		NT USED	ART 1973 Treelin End. 1700	8	LOGGER	T. Limiter Mars
PIH	BELOW	URFACE	(FT)			SOIL DESCRIPTION .	USCS	BLOW	DEPTH OF CASING, DRILLIN
	INTERV	RECOVE			Senie.	SOIL NAME, USCS GROUP SYMBOL, COLOR. MOISTURE CONTENT, RELATIVE DENSITY. OR CONSISTENCY, SOIL STRUCTURE.		COUNTS	RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATIO DRILLING ACTIONS/DRILLE
		71	a/TYPE	DPC (COPR)		MINERALOGY.			COMMENTS
	04	1	1	/	/	Applif emillies	>	Y	10°/0" 10 HISA used
11111	0.5 la	9	221	1 1		13-18"- NO RECEIVED GRAND GLAND, 12-24" F.H.C. S. / Mary Sorry Gland, 13712 5/2 (12120-11) Charry Wart	Gr. Gr	19	sample of ANN rods
1.1.1	2			-		1971 5/2 (Tellerum Charry) Water Lover, With gradul		5	
	2	12	Ssz	1 1		36-12 - 25 PARTY STEE , 1572 SE (V. Dunk 42-46 - SINY STEE, 1572 SE (V. Dunk Grand Brown) Day, course flowery	mu	3 6	
10000	4					Grand parts, water, it fool, Graded	St.	4	Advance HSA
San Lines	4 +4 6	18	333	111		48-51/"- No herenows 54-56"- Security (OSTAINE) 56-68"- Sopr (35 5Hz Cly) 56-68"- Sopr (35 5Hz Cly) 50-11 Formal hours, Size formed 63-72"- STAIN HET, 10 7H 3/2 (V. Danh 63-72"- STAIN HET, 10 7H 3/2 (V. Danh 64-7124 ELWAND) Day, 60000, Howey Gubes	CL	2226	torica (suggest frame) friction and as to be
1	N T D	10	554	() 1		76-74" SLOVEN 74-75" - Carentere 75-32" - ANTHOUT MAD BAR		4 73 50/1	
						96-100-no havevery		_	Alenice FISA 4'- B'. Slaver
77.7	8 4	200	335	-	105-105	GREY 1 9/4 (touch there) WET, LOOKE WELL GRAMM	s.		24.17
1				-	110-120	Granille Brown Amer, JOST, Poonly Greens	a	2	FP. Z H.F

PROJECT NUMBER

DL-1-5/M

Page 2 of 8

JACOBS

PTH	VATER	SURFAC	E (FT)		18	D: CING 35 450 11 IN UD POTTON 1/9/15/15/15/15/15/15/15/15/15/15/15/15/15/	USCS	20002	COMMETTS
	INTERV	AL (FT)	ERV (IN)		COPR	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY.		DLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS
			ATYPE	DPC (COPR)		OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.			
Transfer to	10 10 12	18	37.6	1 7 1 7	HE	126. 126. AV MIRAGENS 126-137" - JAD HEL COME FROM 133 175" 137-140" - SILT (HE CLAR), 7.5 HE 4/6 (STRANTE BROWN) TON, STIFF, PROSEL (HERRY) MINISTER HERRISON 140-144" - CLAYER SILT, 2.57 4/7 (UNIT MINISTER) HOSEL (HERRE)	h.	5 4 14 27	pp=1.5kp
Control to Control	12 14	18	557	1+ 1+	HO GA HO	144 - 147 "-110 MICENSTRY 149 -158" - SILT (HO CURE) SEN (137-140) 158-159" - SEN (140-144)(50) 158-162" - SEN (147-1334)(HO)	KL	9 42 53 60/3	
Trail Greek	14 16	10	738	++	相	168-175"- SLINGH 175-178"- SAA (HB) CATHELITHIN GAR 178-192"-N/A HOULE	hi	100/5	Advance Jisn 14-16
1000	16.5	6	We	++	HB GB	196-196" - SAN (HB) 196-198" - SAN (153-157)(GB) F. SAND, JOHNE JIH 2-374/2 (MINTER ERMAN) MOSIF, LANGE PROMISE (MINER)	FA	6	
11111	16.5 1-	3	2/0	+	CB	(2.5 4/2 4/2) MOD IN CLY (2.5 4/2 4/2) MOD IN CLY (2.5 4/2 4/2) MOD IN LIP IT Show	Sm	1	
11111	17 473		2211	+	GB	Sevel, Z. 5 ya 4/8 (OLIVO Bank) must, LOOSS, USE GADES. Will Markey INDUMED	52.	3/3	
1111	173	3	3312	1		207-210" - CLOY , 2.5 YK 4/6 (AGA)		1	P.P. = 0.75 1st

PROJECT NUMBER
706612

BORING NUMBER

Page 3 of 8

OJE	CT:	Honey	well Di	MT		LOCATION:	Baltimo	re, MD		
	TION:		AND E	OUIPME	NT USE	D: CMB 55 HSD 3 MVD NOTED D	10		- I	
DW	ATER	LEVEL	4	4 5 H 3 H 4	15	TART:/230 7/20/19 END: /400 7/17		LOGGER	: S. ZmJ in The BS	
	BELOW S		E (FT)			SOIL DESCRIPTION	USCS		COMMENTS	
1	INTERVA				COPR	SOIL NAME, USCS GROUP SYMBOL, COLOR.		COUNTS	DEPTH OF CASING, DRILLIN	
- 1		RECOV	SAMPLE	_	State	MOISTURE CONTENT, RELATIVE DENSITY.		247.77	RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION	
		1.0	M/TYPE			OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.			DRILLING ACTIONS/DRILLE COMMENTS	
	-	-		(COPR)	-	St. I AL DOLLE	0.5	Part .		
-	17.5	(9)	X	-		Ship to 16" - Olay a CLOT, two F. Sand, Z. SYRY/L(LED)	CC	101	P.7 = 0.75 +sf	
	7. 6		1			those F. Shard, Z. SYRY/6(RED)		8/00	Isolahor corty to	
	18	N 17	SI		1	WOT,		6.3	he not to 18 bys	
_			7		-			4		
-	100					0-7" SAME AS ABOVE		6	~ ~ ~	
-	18,5		- 1					7	PP: 0.5 TS	
-	-	771	5513					/		
	70		- 33	-			CL	5		
4	20.5						100	60		
d-	100							4		
-				100			2.20	-		
_	-	-	-							
									7	
-							1			
-						1				
						1				
_	ma							1		
-								1		
-						1 Y				
7								1		
					1			1		
-										
-										
					-		-	-		
-						0-6" SANDY ISAN CLAY,			1	
-				14		LITTLE F-M SAND, MOIST, FIRM				
								15		
-	1					2.5 YZ 5/8 PED		6	PP = 0.57	
-	13					A CONTRACTOR OF THE CONTRACTOR		3	66 2 2 2 6	
	1100	6	3514					-2		
-	TO	10	1	-			n.	3		
	25	1		1	_		CL	2		
7										
	1							1		
-								1		
-										
	-							1		
-										
-	1		100		1	and the same of th				

9"

TO

35

38

40'

24" 5517

15516

PROJECT NUMBER 706612

BORING NUMBER

D1-1-SAA

Page 4 of 8

SOIL BORING LOG

5

5

7

6

WOH/

18-2

HH

PP = 0.25 TH

SP-

SM

PROJECT: Honeywell DMT LOCATION: Baltimore, MD ELEVATION: DRILLING CONTRACTOF Parratt Wolff DRILLING METHOD AND EQUIPMENT USED: CHE 55 HSA / MUD ROTARY
ATD WATER LEVEL: START: 1030 7/30/18 END: 1400 9/19/18 LOGGER: C. REED / JAKOBS DEPTH BELOW SURFACE (FT) SOIL DESCRIPTION COMMENTS INTERVAL (FT) COPR BLOW SOIL NAME, USCS GROUP SYMBOL, COLOR, DEPTH OF CASING, DRILLING RECOVERY (IN) COUNTS MOISTURE CONTENT, RELATIVE DENSITY, RATE, DRILLING FLUID LOSS. SAMPLE OR CONSISTENCY, SOIL STRUCTURE, TESTS, AND STRUMENTATION. #/TYPE MINERALOGY. DRILLING ACTIONS/DRILLER (COPR) COMMENTS 0-9" PG SAND (F-M) TRACE SILT WET LOOSE 2.54R 5/1 185 6 9" 5515 RED-GRAY SP (AND COACSE END, CENEL) 12 -54 12 30 13

0.9" SAME AS ABOVE, EXCEPT

0-24" CLAYEY SALT, SOME CLAY, MOIST, V. SOFT, \$5 YR 4/1 DK CRAY

NO BENVEL

PROJECT NUMBER 706612

BORING NUMBER

DL-1-SAA

Page 5 of 8

SOIL BORING LOG

PROJECT: Honeywell DMT ELEVATION!

LOCATION: Baltimore, MD

WATER H BELOW!	LEVEL SURFACE L (FT) RECOVE	(FT)	ZOIFINE	S COPR Strata	DRILLING CONTRACTOR Parratt Wolff D: CME 55 HSA / MUD LOTHRY TART: 1030 7/30/18 END: 1400 9/19 SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,	USCS	BLOW COUNTS	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS
- - - - - - - - - - - - - - - - - - -		#/TYPE	DPC (COPR)		0-21" SAME AS AGME	МН	WOR WOH 2 4	PP = 0.25 T
180		Sz	ه ایر		0-3" PC SAND (F-M), WET, V. 100 SE, SYR 5/1 BRMY — COLLECT SHELBY TUBE	SM		SHELBY TUBE DIMINGED DULNO REMOVAL - CNLY 3" RELOVE
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	3 24	5,1	1 -		0-24" PLAYEY SILT, SOME PLAN MOIST, V. SOFT, SYR 4/1 DK. GUM (TRACE SHELLS)	M	WoH, 12	100 - 05 73

PROJECT NUMBER 706612

DL-1- SAA

Page 6 of 8

SOIL BORING LOG

PROJECT: Honeywell DMT LOCATION: Baltimore, MD DRILLING CONTRACTOR AND ROTTER LOGGER: C. RECD / MCODS COMMENTS NTERVAL (FT) COPR BLOW SOIL NAME, USCS GROUP SYMBOL, COLOR, DEPTH OF CASING, DRILLING Strata COUNTS RATE, DRILLING FLUID LOSS. MOISTURE CONTENT, RELATIVE DENSITY. TESTS, AND STRUMENTATION. OR CONSISTENCY, SOIL STRUCTURE, #/TYPE DPC MINERALOGY. DRILLING ACTIONS/DRILLER (COPR) COMMENTS 0-24" CLAYEY SILT, SOME CLAYFAKELS MOIST, V SOFT, 5YR 4/1 DK. GRAY 58 PP = 0.75 73F TV = 0.25 TSF TO - COLLECT SHELBY TUBE Aly 24" 53 60' MH VANE-SHEAR 61.7-62' 0-24" SAME AS ABOVE, TRACE SHELLS, ORGANIC ODOR 63 PP = 0.5 TSF WOH/ 570 12" 24" MH 65 2 0-24" SAME AS ABOVE PP = 0.5 TSF 68 - COURCE SHELBY TUBE TV = 0.4 TSF NA MH 24" 54 70

> VANE-SHEAR TEST 71.7-72

PROJECT NUMBER 706612

BORING NUMBER DL -1 - SAA

Page 7 of 8

SOIL BORING LOG

PROJECT: Honeywell DMT ELEVATION:

DRILLING CONTRACTOF Parratt Wolff

DRILLING METHOD AND EQUIPMENT USED:

ONLY

ATD WATER LEVEL:

START:/020 7/36/18 END: /400 9/19/18 LOGGER: C. REED / SHOPS

COMMENTS ELEVATION DEPTH BELOW SURFACE (FT) INTERVAL (FT) COPR RECOVERY (IN) SOIL NAME, USCS GROUP SYMBOL, COLOR, DEPTH OF CASING, DRILLING MOISTURE CONTENT, RELATIVE DENSITY, COUNTS RATE, DRILLING FLUID LOSS, OR CONSISTENCY, SOIL STRUCTURE, TESTS, AND STRUMENTATION. #/TYPE DPC MINERALOGY. DRILLING ACTIONS/DRILLER (COPR) COMMENTS 0-24" SAME AS ABOVE, TRACE 73 OLGANIC MATTER wol PP = 0.25 TSF TO MH 5521 24" 75 WOH WOH 3 0-24" SAME AS ABOVE 78 WOZ PP = 0.5 TSF 5324 TO 24" MH WOH 80 WOH 0-24" Samie AS ABOVE - COLLECT SHELBY TUBE 83 PP = 1.0 TSF 55 HH TV = 0.35 TSF 24

> VANE-SHEAR TEST 86.7'-87'

PROJECT NUMBER 706612 BORING NUMBER
DL - 1 - SAA

PAGE 8 OF 8

PTH BE	OW SUF	VEL:	FT)		SIAK	0: CME 55 HSA / MUD TRO-AL 1/030 7/30/18 END: 1400 9/ SOIL DESCRIPTION	USCS	0	LOGGER: C. REED / SACOBS
	INTERVA	L(FT)	ERV (FT) SAMPLE	DPG	Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND INSTRUMENTATION. DRILLING ACTIONS/DRILLER COMMENTS. DRILLING ACTIONS/DRILLER COMMENTS
1. 1. 1. 1. 1. 1. 1.	88 To 96	24"	5523	(COPR)		0-24" SAME AS ABOVE	ин	WOR/ 12" WOH 5	PP = 0.75 TSF
The Carl									
1. 1. 1. 1. 1. 1.	93 70 95'	22"	-04-			0-10" SAME AS ABOVE, EXCEPT TRACE F. 34ND	мн	WOR 29	PP = 0.5 TSF
1115	75		5524			10"-22" PE SAND (F-M) TRACE SILT, MOIST TO WET DENSE, SYZ 6/2 PINIKISH-GRAY	SP. SM	41 27	PATTAPSCO SAND
Para Caral									CONTINUE STEEL (ASING ADVANCEMENTO 103' BGS, THEN OPEN-HOLE DEILLING TO 107' BGS TO ALLOW FOR SSA INSTALLATION

PROJECT NUMBER 706612 DL-Z-I

Page /

of ?

D۷	VATER	LEVE	Li		8	D: CMB 12 HIP & MUD RATEM START: 0945 7/31/18 END: 1030 3/19/18 SOIL DESCRIPTION	USCS	LOGGER	COMMENTS	
	BELOW SURFACE (FT) INTERVAL (FT) RECOVERY (IN) SAMPLE		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY,	0303	BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION			
			#/TYPE	DPC (COPR)		OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.			DRILLING ACTIONS/DRILLER COMMENTS	
1 1	0.5	1	1	1	1	0-6"-Aspholt	1	1		
1 1 1 1 1 1 1	05 62	15	SS1	1 1		6"-9": No recovery 9-12": Asphalt road base 12"-24": F-M silty SAND, SM GU (SM) 104 K4 12 (Dark Grayish BM) DRY, Well Graded Medium Dense	SM	21 17 10	Sample will AW reds Black woven mest	
all facial.	2' 40 4'	22	SS2			24"-26": No recovery 26"-48": SAA (12"-24") Some clay/present from 38"-42".	sM	3255	Transmitted at 22",	
0.10									Advance HSA	
A total total	4 40 6	14	ss3	-		48"-58": No vecovery 58"-70": SAA (12"-24") (SM) bry, well Graded GERNEUN 70"-72": F-CASA, BR SIUT (SP.SM) 1048 416 (DK. Yell Brn) DRY, LOOSE WELL Graded	SM	2469		
1111111111	6 to 8	10	554	1 1		12"-36": No recovery 86": 96": FM Sandy Silt (M) 10YR 4/6 (DK. Yellowish Byn) Mist, V. Loose Poorly Evaded		3 6	Advance HSA	
	8 40 10	1	SSS	4 1		96"-99": 8F-M sandy sitt, trace clay cml) love 4/6 (De ven Brn) Moist, very pense 99"-103": Perta weathered asphale V. Dense	ML	12 50/1°	4'-8'. Slight auger chatter at 8'.	

PROJECT NUMBER

BORING NUMBER

Page Z_of 7

WATER	LEVE	Lt		1	D: (INE 55 HSA) START: 0945 1/31/18 END:		LOGGER	RIT ZMUOZIN (JAKOIS
INTERV	AL (FT)	E (FT) ERY (IN) SAMPLE #/TYPE	DPC (COPR)	COPR Strata	SOIL DESCRIPTION SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	USCS	BLOW	COMMENTS DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS
10 40 12'	18	SSV			120"-126": NO VECOVERY 124"-129" SAM (99"-103") 129"-138" : F-C SAND AND GUL, TESI 104R 413 (BEN) W 2.5 YESTG (RED), MOIST, LOSSE, WEI GARDED 2.5 YR 516 (RED) MED DIVY, POOTY Graded		99918	Advance HSA 8'-10'.
12'	12	722	1 1 1		146"-146": NO TECOVERY 146"-168": SILTY F-MS AND (SM) 2.5 YR 5/6 (LEGD) Dry, Poorry Graded Medium Dense	SM	59319	
14 10	16	SS8	+ 11		168"-176": No veccuery 176"-188": SAA (146"-168") 188"-185": wathered Asphalt 188"-192": F.M. Sand, Some silt (SM) Syr Z. S/Z (Ok. raddish bra) Dry, Poorly Graded	SM	1 18 26 22	Woven fabric encountered at 1710".
10'	12	\$59	+ × +	HB X GB	192"- Introduce recovering 2000000000000000000000000000000000000	SM SM	29 45 ×	* * *
17 to		ss 10	44	HB	204-210" SAA (206"-211")	SM	22	
175		55	+	HB	2025/2008": F- Silty sound (SM) 210"-216": 54R 3/4 (BK Roldish BVN) Wet, POOTIN Graded	SM	27	

PROJECT NUMBER

BORING NUMBER
DL-2-I

Page 3 of 7

HBELOW	LEVE			8	START: 0945 7/31/8 END:	USCS	LOGGER	COMMENTS
INTERV	AL (FT)	SAMPLE		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.	0000	BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER COMMENTS
19.0	9	12	++		216"-219"; No recovery 219"-229"; SAA (210"-216")	SPI	48	N. I
M' +0 20'	12	55	++	CB.	228-232": SIAA (210"-216") 232"-234": F-M SAND, SINSTIT (SIM) 574/1 (DK. Gry) Wet	SM	98	282°-284": MUSED W/25877/6 Lyellow)
20 40 5	6	54	+		200"-246": WOOD wet		8	
205'	6	3S 15	1		246"-248": SAA (Wood) 246"-252": F.M. SAND, TRESILT (Sain) 1048 312 (V. DK Grayish Brn) Wet, Poonly Graded	SP	36	SETS" ISOLATION CASING TO 21' !
3.5	16	5316			10 y R 6/2 (Pale yellowish bown) - f-CS SAND, tr silt, tr f-med grave 1 (SP/SM) - grovel gte, well rounded, som Sphereret, maist, med danse, well graded	58-55	14 16 11 9	376" - 300" Significat Mud loss N75-100 gal

PROJECT NUMBER 706612

BORING NUMBER

Page 4 of 7

	LING M WATER			QUIPM	ENT USE	the state of the s			
_	HBELOW			-	L S	START: END:		LOGGER	R. E. Costas
DEFI	INTERV		E (F1)		0000	SOIL DESCRIPTION	USCS	700	COMMENTS
1	INTERV		ERY (IN)		Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,		COUNTS	DEPTH OF CASING, DRILLI RATE, DRILLING FLUID LOS TESTS, AND STRUMENTATI
			#/TYPE	DPC (COPR)		MINERALOGY.	-	4	DRILLING ACTIONS/DRILL COMMENTS
	10		Č.		*	N6. (med light gray), nother will syr 5/6 (light brown) SILT, tr. fisonc, Moist,	ML	3	PP 1.75
1000	38	20	17	_	****	5+1PC 336"-360".		4	voper Cit?
-			-	1			-	,	3-3-3-
	100	1			5		1		*
		5 5							In-
33	-		200	7.1		396"-420"		0	
1	33.35	21"	22			SAA N5-N6 (med gory to med Ingh STAY) - brown to trove f. sand		3	PD 1.0
0	35		18		-	moist, Stiff	ML	5	trace clay in she
5				9				5	Upper 5:17?
			1			13, 27		-	
- Tar						A .			-
		+.			_===		1		
38		0.0	5515			Files SAND, WET, V. loose	SP	W+ hammy	
11.1	40	∂41 .	,			+ "Fine sand, most, firm	CH.		35 0.2-1.9"
m) -			-		1.7	458-480" Microsos		,	Lower Sitt?

PROJECT NUMBER

BORING NUMBER

Page 5 of 7

_	WATER	12576			- 1	START: END:		LOGGE	R: Ostac
EPT	INTERV	AL (FT)	E (FT)		COPR	SOIL DESCRIPTION	USCS	DIOIN	COMMENTS
			SAMPLE		Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION DRILLING ACTIONS/DRILLER
A STATE OF THE STA	43.45	Эі,	55 20	(COPR)		H' SAN (St-40') CLAY ST 5110" ST 5110"	3M CH	1 2 4	PP :5-,75
)			(
Transfer of the state of	48.50	<i>39</i> "	SS a1			134 SAA SILTY CLAY 574 -591" [MC154] STEPF (by hord 17P) 591 -600" 97" N3 (chira gray) = 117 SAND (fraud morst, and down	CK CK	1 4 13 17	PP 1.25
Trenderick	52	16"	32			608-634": SAA , maist to wet	SM	1 6 7 7	
1111111	53-	24	Sharsi	_		Jul" rocurry of Shelly Lube 5 × 11 Silly CLAY at botton of tube => 55 ft	Сн		PP@50+

PROJECT NUMBER

BORING NUMBER
DL-J-J

Page 6 of

	_	LEVE			. 5	START : END :		LOGGER	1: Costas
EPTH	BELOW		CE (FT)			SOIL DESCRIPTION	USCS		COMMENTS
	INTERV		SAMPLE #/TYPE	DPC (COPR)	COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE, MINERALOGY.		BLOW COUNTS	DEPTH OF CASING, DRILLIN RATE, DRILLING FLUID LOSS TESTS, AND STRUMENTATIO DRILLING ACTIONS/DRILLEI COMMENTS
Legentration	S8/10"	324.	Yully 2			N3/NU CLAY @ 60' - botton of tube to shells	e H	114	PPIT
and breeze		. /) -				-
ALL LILIANS	63-6	29	SS 23	,		NS CLAY, Micoceum, Moist,	0.14	Mon Y	神 15-175
1131611		/							
Trend.	11.13	24	3	_		SAR - notably stiller De 73'	OH		PP 155-1.5

PROJECT NUMBER 706612

BORING NUMBER
DL-2-I

Page 7 of 7

ЕРТН В	ATER ELOW 8	LEVE	La			START: END : 4 14 14	USCS	LOGGER	Costac COMMENTS
n	NTERVA		ERY (IN) SAMPLE		COPR Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,		BLOW	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION
			#/TYPE	DPC (COPR)		MINERALOGY.			DRILLING ACTIONS/DRILLER COMMENTS
-						SAA -SINYCLAY		Wou	35 112-1
=	18.40	24	55 J-1	_		3,11,230	04	Mott	
3		Ó.	3					6	
-								ь	
-									
-	0	^			/				
-	9		0.16			SAA - Am 85' - but sishelby	014	0	PP 1-15
-4	345	34	Subj 4	1			D.		17.1-12
7				-				- 1	
-	$\overline{}$								
=									
=						in the dela (Pale Yellown Som)		23	DI. S.
- 4	8-90	7"	55 25			for the SAND, to Silt, moist.	SP	41	Paterpses Sand
=		1.,	0.	-		transfer poorly graves very deix		39	
3								48	EOB
-									100
-									
-								8 3	
-									
-									
-								1 8	

SOIL BORING LOG

PROJECT: Honeywell DMT

JACOBS

LOCATION Baltimore, MD

DRILLING CONTRACTO! Parratt Wolff

ELEVATION

DRILLING METHOD AND EQUIPMENT USED (MESS HSA

WATER				5	TART 914118 END 914118	uscs	LOGGE	L. Raterink			
INTERV		E.D-41		COPA	SAL DESIGNATION	United	BLOW	DESTRUCT OF STATE AND ADDRESS OF			
1,000		EAT (N)		Strata	SOIL NAME, USCS GROUP SYMBOL, COLOR, MOISTURE CONTENT, RELATIVE DENSITY, OR CONSISTENCY, SOIL STRUCTURE,		COUNTS	DEPTH OF CASING, DRILLING RATE, DRILLING FLUID LOSS, TESTS, AND STRUMENTATION			
		MTYPE	COP4		MINERALOGY			DRILLING ACTIONS/DRILLER COMMENTS			
					See boring log DL-1-SAA for 0-15.5 H bgs						
				4		Harris	Ole				
			++	нв	155-159 Silty sand F-M Weakly to moderately		4				
-155	12	5-				++	GB	15.9- lle 2 silty sand F.M Little Elay, particulate		6	
115									++	4B	hand a day and
					TO P-17.5 NO TELECTORY		19 T				
-125	10	35	++	HB	175-177 SA 162-164 127-185 Clay, 2548 41610) moist, soft, Liner clay		(t				
- 19			-		185-190 no receiving		12				
-							8				
					EDBE 19 14 bas			Set screen W-18.			

Site: Dundalk Marine Terminal

Boring No: INC-48



Driller: Boart Longyear Consultant: CH2M Hill

Method: Rotosonic Geologist: E. Curbo

Start Date: 07/16/2012 End Date: 07/16/2012

Total Depth: 40.0 Ft

Depth				USCS	
Ft	Rec	DPC	Stratum	Code	Stratum Description
0					FAT CLAY with GRAVEL (CH), reddish brown (2.5YR 4/4), with subangular gravel, low plasticity, stiff.
				СН	
				GW-GM	WELL GRADED GRAVEL with SILT and SAND (GW-GM), brown (10YR 4/3), dry, subangular gravel with medium to fine grained sand, very loose.
+		-			FAT CLAY with GRAVEL (CH), reddish brown (2.5YR 4/4), with subangular gravel, low plasticity, stiff.
+				СН	
+					
			НВ	SP	POORLY GRADED SAND with GRAVEL (SP), red (2.5YR 4/6), dry, fine to medium grained sand with subrounded gravel and trace silt, weakly indurated.
i +					POORLY GRADED SAND with GRAVEL (SP), very dark brown (10YR 2/2), dry, fine to medium grained sand with subrounded gravel, trace silt, and yellow particulate, strongly indurated, very loose.
+			НВ	SP	
		++			
			GB	SP	POORLY GRADED SAND (SP), very dark gray (10YR 3/1), dry, fine to medium grained sand with trace rounded gravel, very loose.
+			НВ	SP	POORLY GRADED SAND with GRAVEL (SP), very dark brown (10YR 2/2), dry, fine to medium grained sand with subrounded gravel, trace silt, and yellow particulate, strongly indurated, very loose.
			GB	SP	POORLY GRADED SAND (SP), very dark gray (10YR 3/1), moist, fine to medium grained sand with trace rounded gravel, very loose.
		-		<u> </u>	
			НВ	SP	POORLY GRADED SAND with GRAVEL (SP), very dark brown (10YR 2/2), dry, fine to medium grained sand with subrounded gravel, trace silt, and yellow particulate, strongly indurated, very loose.

30

BORING LOG Page 4 of 4 Site: Dundalk Marine Terminal Boring No: INC-48 CH2MHILL Total Depth: 40.0 Ft **Driller: Boart Longyear** Consultant: CH2M Hill Method: Rotosonic Geologist: E. Curbo Start Date: 07/16/2012 End Date: 07/16/2012 USCS Depth Rec **DPC** Stratum **Stratum Description** Ft Code WELL GRADED SAND with SILT and GRAVEL (SW-SM), very dark grayish brown (10YR 3/2), wet, fine to medium grained 30 sand with subgrounded gravel, very loose. SW-SM 35 -FAT CLAY with SAND (CH), dark grayish brown (10YR 4/2), moist, with fine grained sand, high plasticity, stiff EOB at 40'. CH 40.0

Site: Dundalk Marine Terminal

Boring No: INC-50



Driller: Boart Longyear Method: Rotosonic Consultant: CH2M Hill

Geologist: E. Curbo

Total Depth: 35.0 Ft

Start Date: 07/16/2012 End Date: 07/16/2012

	End Date: 07/16/2012
Depth USCS DPC Stratum Code Stratum Descrip	otion
6 POORLY GRADED SAND with SILT (SP-SM), yellow (10YR 7/6), moist, m	
very loose.	
SP-SM	
POORLY GRADED SAND with SILT (SP-SM), black (10YR 2/1), moist, me very loose.	edium to fine sand with trace gravel,
SD SM	
SP-SM	
9	

Method: Rotosonic

Site: Dundalk Marine Terminal

Boring No: INC-50



Driller: Boart Longyear Consultant: CH2M Hill

Geologist: E. Curbo

Total Depth: 35.0 Ft

Start Date: 07/16/2012 End Date: 07/16/2012

USCS Depth Rec **DPC** Stratum **Stratum Description** Ft Code FAT CLAY with SAND (CH), brown (10YR 4/3), moist, with fine to medium grained sand, medium plasticity, firm. CH POORLY GRADED SAND with GRAVEL (SP), very dark brown (10YR 2/2), dry, fine to coarse sand with gravel and trace HB SP silt, weakly indurated, very loose. POORLY GRADED SAND with GRAVEL (SP), black (10YR 2/1), moist, fine to medium grained sand with subangular gravel and trace yellow particulate, loose. GB SP POORLY GRADED SAND with GRAVEL (SP), very dark brown (10YR 2/2), dry, fine to coarse sand with gravel, some clay, and trace silt, weakly indurated, very loose. HB SP

Method: Rotosonic

Page 5 of 12 Site: Dundalk Marine Terminal

Boring No: INC-50



Driller: Boart Longyear Consultant: CH2M Hill

Geologist: E. Curbo

Total Depth: 35.0 Ft

Start Date: 07/16/2012

End Date: 07/16/2012

Depth Ft	Rec	DPC	Stratum	USCS Code	Stratum Description	
12		++	НВ	SP	POORLY GRADED SAND with GRAVEL (SP), very dark brown (10YR 2/2), dry, fine clay, and trace silt, weakly indurated, very loose. FAT CLAY with SAND (CH), brown (10YR 4/3), moist, with fine to medium grained s	
15 📗				СН		

Page 6 of 12

Site: Dundalk Marine Terminal

Boring No: INC-50



Driller: Boart Longyear

Method: Rotosonic

Consultant: CH2M Hill

Geologist: E. Curbo

Total Depth: 35.0 Ft

Start Date: 07/16/2012

End Date: 07/16/2012

						End Date: 07/16/2012
Depth Ft	Rec	DPC	Stratum	USCS Code	Stratum Description	
15					POORLY GRADED SAND with GRAVEL and SILT (SP-SM), brown (10YR 4/3), wet, must rounded gravel, very loose.	edium to coarse grained sand
+						
		-		SP-SM		
<u> </u>						
8 T I		L	L l	L	L	

BORING LOG Page 7 of 12 Site: Dundalk Marine Terminal Boring No: INC-50 CH2MHILL Total Depth: 35.0 Ft **Driller: Boart Longyear** Consultant: CH2M Hill Method: Rotosonic Geologist: E. Curbo Start Date: 07/16/2012 End Date: 07/16/2012 **USCS** Depth Rec DPC Stratum **Stratum Description** Ft Code POORLY GRADED SAND with GRAVEL and SILT (SP-SM), brown (10YR 4/3), wet, medium to coarse grained sand 18 with rounded gravel, very loose. SP-SM 20 WELL GRADED SAND with GRAVEL (SW-SM), brown (10YR 4/3), moist, coarse sand with rounded gravel, very loose. SW-SM

Page 8 of 12

Site: Dundalk Marine Terminal

Boring No: INC-50



Driller: Boart Longyear

Method: Rotosonic

Consultant: CH2M Hill

Geologist: E. Curbo

Total Depth: 35.0 Ft

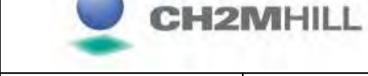
Start Date: 07/16/2012

End Date: 07/16/2012

						End Date: 07/16/2012
Depth Ft	Rec	DPC	Stratum	USCS Code	Stratum Description	
21					POORLY GRADED SAND with SILT (SP-SM), strong brown (7.5YR 5/8), wet, medium to	o fine sand, very loose.
		-		SP-SM		
+						
4 — '		L – – –	L	L – – –	L	

Method: Rotosonic

Boring No: INC-50



Driller: Boart Longyear Consultant: CH2M Hill

Geologist: E. Curbo

Total Depth: 35.0 Ft

End Date: 07/16/2012

Start Date: 07/16/2012

						End Date: 07/16/2012
Depth	Rec	DPC	Stratum	USCS	Stratum Description	
Ft	~ ~	DPC	Stratum	Code		to England was located
24				SP-SM	POORLY GRADED SAND with SILT (SP-SM), strong brown (7.5YR 5/8), wet, medium	i to fine sand, very loose.
				SP-SC	POORLY GRADED SAND with CLAY (SP-SC), dark gray (7.5YR 4/1), wet, with fine g plasticity, very soft.	rained sand, low to medium
5 —					FAT CLAY with SAND (CH), very dark gray (10YR 3/1), moist, with fine to medium gra soft. EOB at 35'.	ined sand, medium plasticity,
		-				
-				СН		
			1	l		

Page 10 of 12

CH2MHILL

Site: Dundalk Marine Terminal

Boring No: INC-50

Driller: Boart Longyear

Method: Rotosonic

Consultant: CH2M Hill

Geologist: E. Curbo

Total Depth: 35.0 Ft

Start Date: 07/16/2012

End Date: 07/16/2012

						End Date: 07/16/2012
Depth Ft	Rec	DPC	Stratum	USCS Code	Stratum Description	
	Rec	-	Stratum	Code	Stratum Description FAT CLAY with SAND (CH), very dark gray (10YR 3/1), moist, with fine to medium grain soft. EOB at 35'.	ed sand, medium plasticity,
30 —						

CH2MHILL

Page 11 of 12

Site: Dundalk Marine Terminal

Boring No: INC-50

Driller: Boart Longyear

Method: Rotosonic

Consultant: CH2M Hill

Geologist: E. Curbo

Total Depth: 35.0 Ft

Start Date: 07/16/2012

						End Date: 07/16/2012
Depth	Rec	DPC	Stratum	USCS	Stratum Description	
Ft 30					I FAT CLAY with SAND (CH), very dark gray (10YR 3/1), moist, with fine to medium grain	ed sand, medium plasticity,
					soft. EOB at 35'.	
		-		СН		
+						
33 —		L	L		L	

CH2MHILL

Page 12 of 12

Site: Dundalk Marine Terminal

Boring No: INC-50

Driller: Boart Longyear

Method: Rotosonic

Consultant: CH2M Hill Geologist: E. Curbo

Total Depth: 35.0 Ft Start Date: 07/16/2012

End Date: 07/16/2012

Depth	0			USCS	
Ft	Rec	DPC	Stratum	Code	Stratum Description
35.0				СН	FAT CLAY with SAND (CH), very dark gray (10YR 3/1), moist, with fine to medium grained sand, medium plasticity, soft. EOB at 35'.

BORING LOG Page 1 of 6 Honeywell				/V		City Devices Marine Farming Ordered		Driller: A Method: Consulta)	Total Depth: 27.0 Ft GW Depth: 0.0 Ft Date: 06/14/2007				
Depth Ft	Recov.	Sample ID	DPC	Soil Code		Soil Description	n	uscs	Comments	w	Gs	PL	LL	P200
0					Precut Asphalt an	d Concrete								
					WELL-GRADED 5 4/1), damp. (loose	SAND (SW) with GRAVEL-brown (1 i), coarse sand, some medium sand	0YR 4/3) and dark gray (10YR brace organics	sw						
	l		in the second		SANDY SILT (ML) red (2.5YR 4/8), d (wood pieces)	i -trace gravel, strong brown (7.5YR amp, medium stiff to hard, debris, fra	5/6), brown (7.5YR 5/2), some agments (brick), trace organics	ML						
					POORLY-GRADE	D SAND (SP) -light brown		SP						
			-		SANDY SILT with red (2.5YR 4/8), d trace organics (wo	GRAVEL (ML) -strong brown (7.5YR amp, medium stiff to hard, debris, fra od pieces)	R 5/6), brown (7.5YR 5/2), some agments (brick), trace gravel,	ML						
5 Ј			Ι	L	L			.11			1	21,	OCI	us

BORING LOG Page 2 of 6 Site: Dundalk Marine Terminal Northing: 573973.3 Driller: A. Chapel (Parratt Wolff, Inc.) Total Depth: 27.0 Ft Honeywell Boring No: INC-1501-J Easting: 1448062.6 Method: Hollow Stem Auger GW Depth: 0.0 Ft CPT No: Elevation: 19.9 Consultant: J. Rogers (CH2M Hill, Inc.) Date: 06/14/2007 Depth 20 Sample Soil DPC Soil Description uscs Comments W Gs PL LL P200 ID Code SANDY SILT with GRAVEL (ML) -strong brown (7.5YR 5/5), brown (7.5YR 5/2), some red (2.5YR 4/8), damp, medium stiff to hard, debris, fragments (brick), trace gravel, ML trace organics (wood pieces) POORLY-GRADED SAND with GRAVEL (SP) -gray (10YR 5/1 and 10YR 5/1), damp, (loose), coarse sand, medium sand, some fine to coarse gravel SP POORLY-GRADED SAND (SP) -light yellowish brown (10YR 6/4) and gray SP (10YR 6/1), moist, (loose), medium sand Locus

BORING LOG Page 3 of 6 Honeywell

Site: Dundalk Marine Terminal Boring No: INC-1501-J

CPT No:

Northing: 573973.3

Elevation: 19.9

Easting: 1448062.6

Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

GW Depth: 0.0 Ft

Consultant: J. Rogers (CH2M Hill, Inc.)

Date: 06/14/2007

Total Depth: 27.0 Ft

epth	Recov.	Sample	DPC	Soil Code	Soil Description	uscs	Comments	w	Gs	PL	LL	P20
0				-	POORLY-GRADED SAND (SP) -light yellowish brown (10YR 6/4) and gray (10YR 6/1), moist, (loose), medium sand	SP						
	l				SANDY SILT with GRAVEL (ML) -some gravel, strong brown (7.5YR 5/6), brown (7.5YR 5/2), some red (2.5YR 4/8), damp, medium stiff to hard, debris, fragments (brick), trace organics (wood piecas)	ML						
		e vel , outures		ю	WELL-GRADED SAND (SW) -gray (10YR 6/1); olive yellow (2.5YR 6/6), damp, (loose), coarse sand, medium sand, trace line grave)	sw		9.9				
1	ŀ		33		SILTY CLAY (CL) -red (2.5YR 5/9), medium stiff, damp, medium plasticity	CL	refusal at 12.3' bgs, material hard, difficult					
	ı			HB	WELL-GRADED SAND with GRAVEL (SW), strong brown (7.5YR 5/8), yellowish red (5YR 5/8), dry, (very dense), moderately to strongly indurated, coarse sand, fine gravel, trace bright yellow nodules	sw	to push hammer					
			**		No Recovery							
				HB	WELL-GRADED SAND with GRAVEL (SW) -strong brown (7.5YR 5/8), yellowish red (5YR 5/8), dry, (very dense), moderately to strongly indurated, coarse sand, fine gravel, trace bright yellow nodules (in shoe), minimal recovery	sw	little recovery, bent macro tube, small porton returned					
			**		No Recovery							
, Ш	ш					-			_	*1	OCI	15

BORING LOG Page 4 of 6 Site: Dundalk Marine Terminal Northing: 573973.3 Driller: A. Chapel (Parratt Wolff, Inc.) Total Depth: 27.0 Ft Honeywell Boring No: INC-1501-J Easting: 1448062.6 Method: Hollow Stem Auger GW Depth: 0.0 Ft CPT No: Elevation: 19.9 Consultant: J. Rogers (CH2M Hill, Inc.) Date: 06/14/2007 Depth Seco Sample Soil DPC Soil Description USCS Comments W Gs PL LL P200 ID Code 15 difficulty SILTY SAND (SM) -strong brown (7.5YR 5/8), damp to moist, moderately indurated, retrieving macro HB 15 2.78 26 NC ISSI ISC 19110 coarse sand, some gravel, bright yellow nodules samplers ++ POORLY-GRADED SAND (SP) -very dark gray (7.5YR 3/1), moist, lightly indurated GB SP 3.10 NE WHILEO TRIVI 21 to particulate, fine sand, trace yellow nodules WELL-GRADED SAND with GRAVEL (SW) -contains strong brown (7.5YR 5/8) and very dark gray (7.5YR 3/1) GB COPR, moist, moderately to lightly indurated, medium sand. 5W some coarse sand, some fine sand, trace gravel, some yellow nodules FAT CLAY (CH) -red (7.5YR 5/8), damp, hard, medium plasticity CH No Recovery

Locus

BORING LOG Page 5 of 6 Site: Dundalk Marine Terminal Northing: 573973.3 Driller: A. Chapel (Parratt Wolff, Inc.) Total Depth: 27.0 Ft Honeywell Boring No: INC-1501-J Easting: 1448062.6 Method: Hollow Stem Auger GW Depth: 0.0 Ft CPT No: Elevation: 19.9 Consultant: J. Rogers (CH2M Hill, Inc.) Date: 06/14/2007 Depth 3 Sample Soil DPC Soil Description uscs Comments W Gs PL LL P200 ID Code 20 SILTY SAND (SM) -gray (7.5YR 5/1), damp, (loose), medium sand, some find sand, some silt, very low plasticity SM NC ISST J SO JOSES POORLY-GRADED GRAVEL (GP) -light gray (7.5YR 7/1), dry, hard, GP (concrete) FAT CLAY (CH) -yellowish red (5YR 5/8), red (2.5YR 5/6), white (2.5YR 8/1), damp; medium stiff, medium to high plasticity, trace organics CH Locus

BORING LOG Page 6 of 6 Site: Dundalk Marine Terminal Northing: 573973.3 Driller: A. Chapel (Parratt Wolff, Inc.) Total Depth: 27.0 Ft Honeywell Boring No: INC-1501-J Easting: 1448062.6 Method: Hollow Stem Auger GW Depth: 0.0 Ft CPT No: Date: 06/14/2007 Elevation: 19.9 Consultant: J. Rogers (CH2M Hill, Inc.) Depth 20 Sample Soil DPC Soil Description uscs Comments W Gs PL LL P200 Code ID 25 FAT CLAY (CH) -yellowish red (5YR 5/8), red (2.5YR 5/6), while (2.5YR 8/1), damp; medium stiff: medium to high plasticity, trace organics CH SILTY CLAY (CL) -rind (2.5YR 5/6), yellowish red (5YR 5/8), moist, very soft, medium CL plasticity, frace organics indinometer installed at this location



Honeywell

Site: Dundalk Marine Terminal Boring No: INC-1501-L CPT No: Northing: 574006.1 Easting: 1448106.9 Elevation: 20.3 Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Consultant: J. Rogers (CH2M Hill, Inc.)

Total Depth: 24.0 Ft GW Depth: 0.0 Ft Date: 06/29/2007

Pepth Ft	Recov.	Sample ID	Blow	DPC	Soil Code	Soil Description	uscs	Comments	w	Gs	PL	LL	P200
0			- 1			Precid author gravel pack							
		(NC+50+), sick dislate	5-6-8-8			WELL-GRADED SAND (SW) very stark gray (10YR 3/1), cark brown (10YR 3/3), gray (10YR 5/1), most medium dense coense sand, medium sand, fine gravel, debris throughout (brisk)	sw		12				
			5-9-10-11			SILTY SAND (SM) -grayab brown (10VIII 4(2), clark-gony i 11VIII 4(1), damp, medium dense, very stiff fine sand, sill, coarse sand, fine gravel	SM						
			12.0			No Recovery							
5 -		HIS 1501 C BOLONOSU	4-5-6-6			SILTY SAND (SM) -grayish brown (10°FR 4/2), dark gray (10°FR 4/1), darkg, medium domain very wiff, him sand, sill, coarse sand, the gravel	SM		11"				
					13	No Recovery							
						SILTY SAND (SAI) -graysh brown (10YR 4/2), dark gray (10YR 4/1), dame, very dense, very stiff, ting sand, sit, coarse sand, fins gravel, asphalt and concrete or shoot	SM	esphalt and concrete in shoe of spoon					
			B 11-50-2	ñ	B	No Recovery							
1	Ц	******						************		_	-61	OCI	

Locus

Honeywell

Site: Dundalk Marine Terminal Boring No: INC-1501-L CPT No: Northing: 574006.1 Easting: 1448106.9 Elevation: 20.3 Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Consultant: J. Rogers (CH2M Hill, Inc.)

Total Depth: 24.0 Ft GW Depth: 0.0 Ft Date: 06/29/2007

Recov.	Sample	Blow	DPC	Soil	Soil Description	uscs	Comments	w	Gs	PL	LL	P200
		25-27-19-17										
	NC AMI Legarania				WELL-ORADED SAND (SW), gray (10YR Srt), dump, dame, midlum sand, coarse sand fine gravel free sand.	sw		7.3				
		26-21/15-15	3.6		7.000							
		50/.2				sw						
	NO 1401 L 401 (+014)	1000				sw		19	2.68			
		.,,,,,,,			No Recovery							
		NCAMI LEQUISITIO	25-27/14-17 MCARM CACAGINID 36-21-15-19	25-27/18-17 BIC 1801 L 60-140140 36-21-15-15	26-21-75-13 - HB 507-2 + HB	No Recovery WELL-CRADED SAND (SW), gray (10YR S1), damp, dense, medium sand, coarse and time gravet free aand. No Recovery. WELL-CRADED SAND (SW), gray (10YR S1), damp, dense, medium sand, coarse sand free gravet free prival manual dense, very dense, medium sand, damp, very dense, sand, free gravet fre	No Recovery WELL-GRADED SAND (SW), gray (10YR S1), damp, genser medium sand, coarse sand this gravel fire sand No Recovery. WELL-GRADED SAND (SW), gray (10YR S1), damp, genser medium sand, coarse sand this gravel fire gravel fire sand No Recovery. WELL-GRADED SAND (SW), gray (10YR S1), damp genser medium sand, coarse sand. fire gravel fire sand. SW No Recovery. HB WELL-GRADED SAND (SW), gray (10YR S1), damp genser medium sand, fire gravel fire sand. SW No Recovery. WELL-GRADED SAND (SW), gray (10YR S1), damp genser medium sand, fire gravel fire sand. SW SW 150/.2 + HB WELL-GRADED SAND (SW), strong troven (7.5YR Sn), sams may dense, particulate coarse sand, medium sand, fire gravel.	No Recovery WELL-CRADED SAND (SW-grey (10YH 5/1), damp, dense, mindium sand, coarse sand the grival first sand No Recovery WELL-CRADED SAND (SW-grey (10YH 5/1), damp, dense, mindium sand, coarse sand, the grival first sand No Recovery WELL-CRADED SAND (SW-grey (10YH 5/1), damp, dense and story sand,	No Recovery WELL-ORADIED SAND (SW), grey (10YR 51), damp, dense, mindrum sand, coarse SW 7.3 No Recovery No Recovery WELL CRADED SAND (SW), grey (10YR 51), damp, dense, mindrum sand, coarse SW 50/2 + No Recovery No Recovery WELL CRADED SAND (SW), grey (10YR 51), damp yearse mindrum sand, coarse sand, fine grevel mas sand. SW MC No No Recovery WELL CRADED SAND (SW), grey (10YR 51), damp yearse mindrum sand, coarse sand, fine grevel mas sand. SW MC No No Recovery WELL CRADED SAND (SW), data yearse mindrum sand, fine grevel mas sand. SW 19	No Recovery MELL-CIRALED SAND (SW) - grey (10YR 2/1) - damp, denies medium sand, coarse SW 7.3	No Recovery WELL-DRADED SAND (SW), gray (10YR 3/1), damp, dense, middum sand, coarse sand fine gravel fine sand No Recovery No Recovery HB Supply instrumed damp, very dame, middum sand fine gravel fine sand. SW 10/2 + No Recovery WELL-DRADED SAND (SW), gray (10YR 3/1), damp, dense, middum sand, coarse sand, fine gravel fine sand. SW 10/4 HB Supply instrumed damp, very damp, middum sand, fine gravel fine sand. SW 10/4 HB WELL-DRADED SAND (SW) -strong travel (7 SYR 3rd) damp, mony damp, portiouste. SW 19 2.68	No Recovery

Honeywell

of 3 Site: Dundalk Marine Terminal Boring No: INC-1501-L

CPT No: Elevation: 20.3

Driller: A. Chapel (Parratt Wolff, Inc.)

Method: Hollow Stem Auger

Consultant: J. Rogers (CH2M Hill, Inc.)

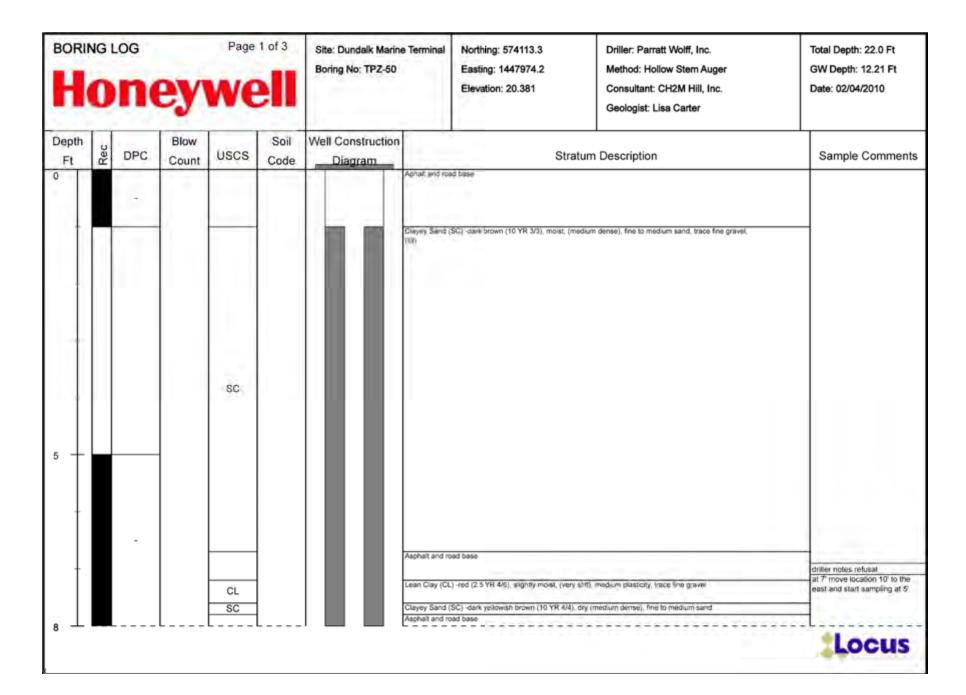
Total Depth: 24.0 Ft GW Depth: 0.0 Ft

Date: 06/29/2007

Depth Ft	Recov.	Sample	Blow	DPC	Soil Code	Soil Description	uscs	Comments	w	Gs	PL	LL	P200
16			60/.2			No Récovery		no penetration, material very hard					
		lyc (801 L-80) (88190	49-13-9-16	#	нв	WELL-GRADED SAND (SW) whong brown (7.5YR 5/8) dame; medium dense, particulate, power sand, markum aand, fine gravel WELL-GRADED SAND (SW) whong tream (7.5YR 5/8), damp, very dense, particulate, coarse sand, medium sand, fine gravel, moved with very dark gray (7.5YR 3/1) OB COPR	sw		16				
20 -						SILTY CLAY (CL) -yellow-lin red (5YR MII), reddish yellow (5YR MII), damp, very elfic slay sill trace fee sund, low pleaticity, DPC reading all accrossmently 16" * DPC reading all approximation 18 ".	ŒĹ						
		horein Leorgiages		×					17				
24.0											*1	OCI	10

Northing: 574006.1

Easting: 1448106.9



Page 2 of 3 **BORING LOG** Site: Dundalk Marine Terminal Northing: 574113.3 Driller: Parratt Wolff, Inc. Total Depth: 22.0 Ft Boring No: TPZ-50 Easting: 1447974.2 Method: Hollow Stem Auger GW Depth: 12.21 Ft Honeywell Elevation: 20.381 Consultant: CH2M Hill, Inc. Date: 02/04/2010 Geologist: Lisa Carter Well Construction Depth Blow Soil Rec DPC USCS Stratum Description Sample Comments Ft Diagram Count Code Asphalt and road base 8 Clayery Sand wen Gravel (SC) -dark yellowish brown (10 YR 4/4), dry. (medium dense), fine to medium sand, some line angular gravel, (fill) SC 10 Lean City (CL) -rod (≥ 5 YR 4/6), slightly moist, (very stiff), medium plasticity, little fine sand CL Sifty Sand (SM) -md5sh yellow (5 YR 6/6), dry, lightly indurated augered through HB to synoptic water levels collected 3-11-10 HB SM 15 Boring GW Depth Locus

BORING LOG Page 3 of 3

Honeywell

Site: Dundalk Marine Terminal

Boring No: TPZ-50

Northing: 574113.3

Easting: 1447974.2

Elevation: 20.381

Driller: Parratt Wolff, Inc.

Method: Hollow Stem Auger

Consultant: CH2M Hill, Inc.

Geologist: Lisa Carter

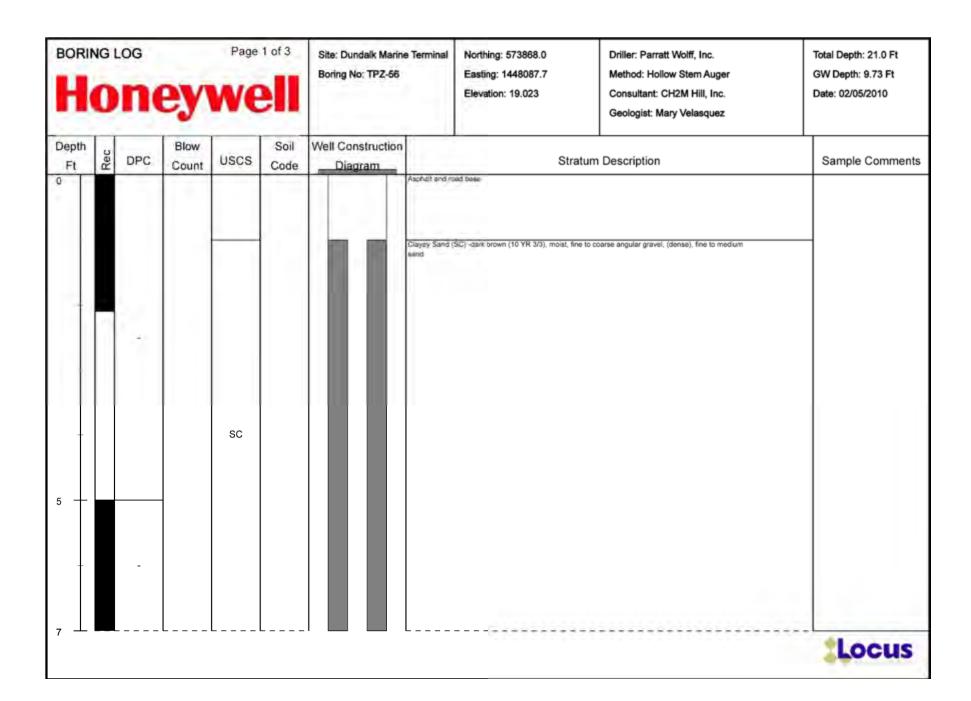
Total Depth: 22.0 Ft

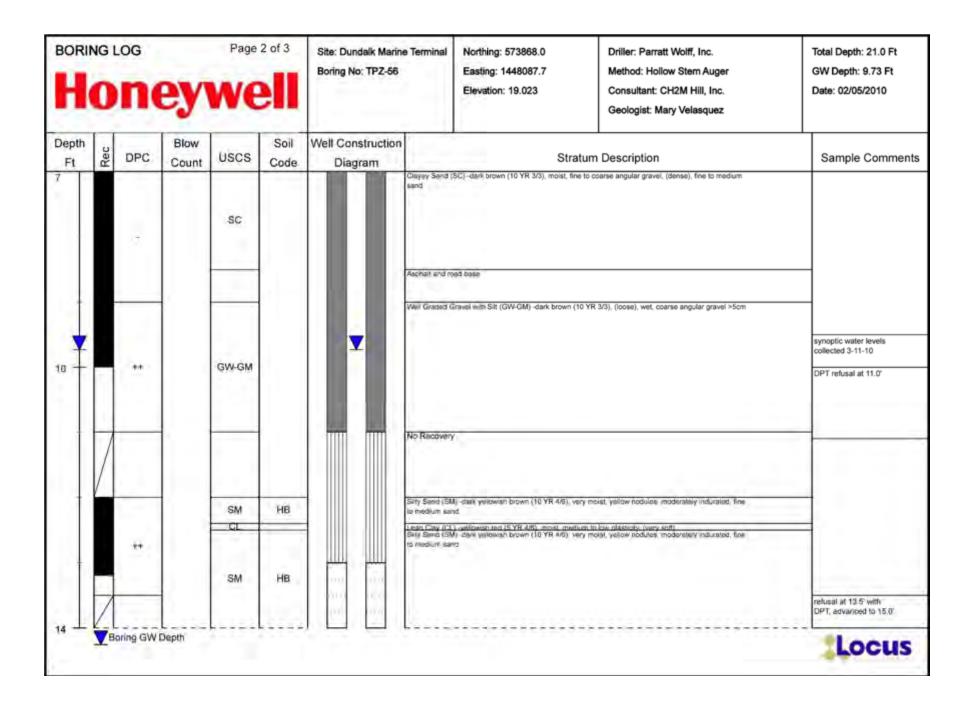
GW Depth: 12.21 Ft

Date: 02/04/2010

Depth Ft	Rec	DPC	Blow	uscs	Soil Code	Well Construction Diagram	Stratum Description	Sample Comments
16			-	SM	HB		Sity Sand (SM) -reddish yellow (5 YR 6/6), dry, lightly indurated	
		++		SM	GB		Sirty Sand (SM) -stack (f0 YR 2/1), wet, lightly indurated, medium sand, some yellow nodules	
1		++	1	SM	HB	100 = 100	Sity Sand (SM) -reddish yellow (5 YR 6/6), dry, lightly indurated	+
20 —		•		SM	GB		Sirty Saint (SM) -black (10 YR 2/1), wet, (dense), medium sand with some silt and clay, trace yellow nodules, (III) Lean Clay (CL) -red (2.5 YR 4(5), most, (etcl), medium plasticity, nome tine sand	







BORING LOG Page 3 of 3 Site: Dundalk Marine Terminal Northing: 573868.0 Driller: Parratt Wolff, Inc. Total Depth: 21.0 Ft Honeywell Boring No: TPZ-56 Easting: 1448087.7 Method: Hollow Stem Auger GW Depth: 9.73 Ft Elevation: 19.023 Consultant: CH2M Hill, Inc. Date: 02/05/2010 Geologist: Mary Velasquez Well Construction Depth Soil Blow Rec DPC USCS Stratum Description Sample Comments Ft Count Code Diagram Sity Sand (SM) -dark yellowish brown (10 YR 4/6), very moist, yellow nodules, moderately indurated, fine 14. 15 refusal at 15.6' with SM HB DPT, advanced to 18.0' Lean Clay (CL) -dark reddish brown (5 YR 5/2), moist. (stiff), some fine sand, trace fine engular gravel. ow plasticity CL 20 + Lean Clay (CL) -mottled strong prown (7.5 YR Mil) and red (7.5 YR 4R), moist, medium to low planticity, (very CL 21.0 Locus

BORING LOG Page 1 of 3 Site: Dundalk Marine Terminal Driller: Parratt Wolff, Inc. Northing: 574396.7 Total Depth: 27.0 Ft Boring No: TPZ-61 Easting: 1449068.1 Method: Hollow Stem Auger GW Depth: 14.36 Ft Honeywell Elevation: 18.614 Consultant: CH2M Hill, Inc. Date: 02/07/2010 - 02/08/201 Geologist: Mary Velasquez Well Construction Soil Depth Blow Rec DPC USCS Sample Comments Stratum Description Ft Diagram Count Code Aschalt and road base 0 Siny Santi (SM) -yellowith brown (10 YR 5/6) and dark grayish brown (10 YR 4/3), medium to coarse sand, subangular gravel, dry. (dense), slag material at bottom, (fill) SM Clayery Sitt with Sand (ML) -mottled yellowish brown (2.5 Y 6/3) and olive yellow (2.5 Y 6/6), no plasticity. dry, some fine sand, (stiff) Sity Sand (SM) -black (2.5 YR 2/1), fine sand, dry. (dense), yellow nodules, particulate ** SM GB 5 Clayey Sit (ML) -mottled light yellowish brown (2.5 Y 5/3) and clive yellow (2.5 Y 6/6). (very stiff). no plasticity, fine sand ML. Fat Clay (CH) -light olive brown (2.5 Y 5/4), very most, high plasticity trace fine sand, (soft) CH Sity Sand (SM) -olive brown (2.5 Y 4/5), yery moist, fine to medium sund, micricagous, (dense) SM Clayey Sit (ML) -mottled light yellowish brown (2.5 Y 5/3) and clive yellow (2.5 Y 6/6). (very stiff). ML. Locus

Page 2 of 3 **BORING LOG** Site: Dundalk Marine Terminal Northing: 574396.7 Driller: Parratt Wolff, Inc. Total Depth: 27.0 Ft Boring No: TPZ-61 Easting: 1449068.1 Method: Hollow Stem Auger GW Depth: 14.36 Ft Honeywell Elevation: 18.614 Consultant: CH2M Hill, Inc. Date: 02/07/2010 - 02/08/201 Geologist: Mary Velasquez Well Construction Depth Soil Blow Rec DPC USCS Stratum Description Sample Comments Ft Code Diagram Count Clayey Set (ML) -motiled light yellowish brown (2.5 Y 6/3) and clive yellow (2.5 Y 6/6), (very stiff), 9 ML 10 Sandy Silt (ML) -grammish gray, moist, very fine sand, (soft), micaceous, color changes to light brownish gray (2.6 Y 5/2) at 11.5 ML Sity Sand (SM) -art yellowish brown (2.5 Y 5/3), wet, medium to coarse sand, (medium dense) SM Clayey Sitt (ML) -light yeilowish brown (2.6 Y 6/2), mottled with strong brown (7.5 YR 5/8), (6/87), medium synoptic water levels plesticity collected 3-11-10 15 ML Boring GW Depth Locus

Page 3 of 3 **BORING LOG** Site: Dundalk Marine Terminal Northing: 574396.7 Driller: Parratt Wolff, Inc. Total Depth: 27.0 Ft Boring No: TPZ-61 Easting: 1449068.1 Method: Hollow Stem Auger GW Depth: 14.36 Ft Honeywell Elevation: 18.614 Consultant: CH2M Hill, Inc. Date: 02/07/2010 - 02/08/201 Geologist: Mary Velasquez Well Construction Depth Soil Blow Rec DPC USCS Stratum Description Sample Comments Ft Count Code Diagram Clayer Set (ML) -light yellowish brown (2.5 Y 6/3), mottled with strong brown (7.5 YR 5/8), (stiff), medium 18 ML Poorty Graded Sand (SP) -brownish yellow (10 YR 6/6), wet, (loose), fine to medium sand wet at 19.5' 20 SP Clayey Sand (SC) -light yellowish brown (15 YR 6/4) and gray (10 YR 6/1), line sand 25 . SC Clayey Sit (ML) -dark gray (Z.5 Y 4/1), very moist. (soft), very fine sand. ML Locus

BORING LOG

Page 1 of 1

Site: Dundalk Marine Terminal

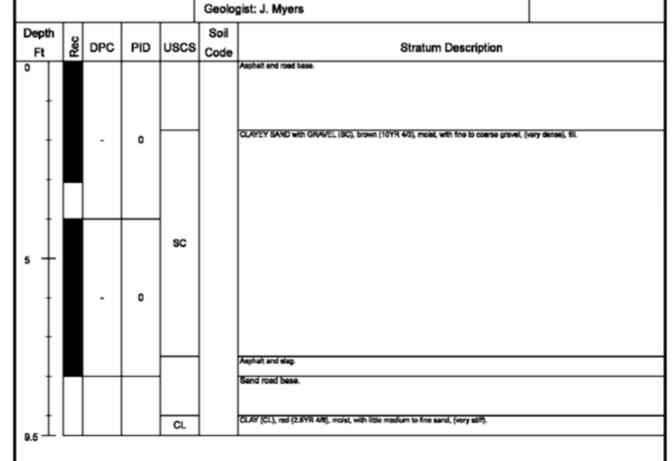
Boring No: TPZ-63



Northing: 573885.724 Driller: Parratt Wolff, Inc. Easting: 1448089.232 Method: Direct Push Elevation: 17.56

Consultant: CH2M Hill

Total Depth: 9.95 Ft Start Date: 07/13/2011



BORING LOG

Elevation: 21.221

Page 1 of 1

Site: Dundalk Marine Terminal

Boring No: TPZ-65



CH2MHILL

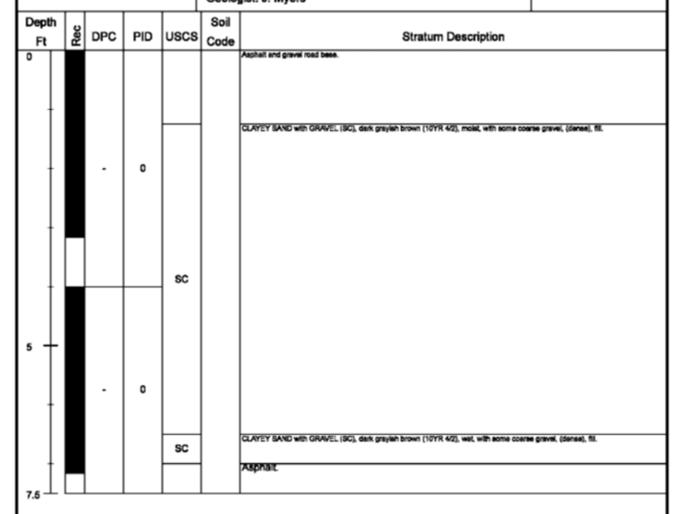
Northing: 574223.837 Driller: Parratt Wolff, Inc. Easting: 1448330.152 Method: Direct Push

Consultant: CH2M Hill

Geologist: J. Myers

Total Depth: 7.5 Ft

Start Date: 07/13/2011

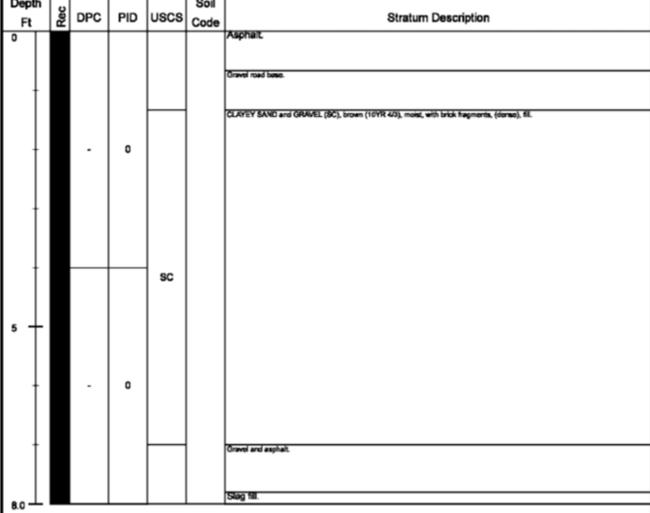


BORING LOG Page 1 of 1 CH2MHILL Northing: 573996.017 Easting: 1448449.78 Elevation: 19.644 Consultant: CH2M Hill Geologist: J. Myers Depth Soil DPC PID USCS Ft Code Asphalt Gravel road base. 0

Site: Dundalk Marine Terminal

Boring No: TPZ-66

Driller: Parratt Wolff, Inc. Total Depth: 8.0 Ft Method: Direct Push Start Date: 07/12/2011



BORING LOG

Page 1 of 1

Site: Dundalk Marine Terminal

Boring No: TPZ-68



CH2MHILL

Northing: 573943.39 Easting: 1448293.727 Driller: Parratt Wolff, Inc. Method: Hollow Stem Auger Consultant: CH2M Hill

Total Depth: 8.0 Ft Start Date: 07/22/2011

Elevation: 18.863

Geologist: C. Reed / C. Houck

Depth	Τ,	,			Soil	
Ft	Rec	DPC	PID	USCS		Stratum Description
-						Not logged.
5 -				SP-SC		POORLY GRADED SAND with CLAY and GRAVEL (SP-SC), dark yellow brown (10YR 3/4), dry, fine to medium send with little angular fine gravel, (canse).
B.0 _	L			ÇL		LEAN CLAY (CL), red (2.5YR 4/6), dry to moiet, trace fine eand, (stiff).
0.0						

Appendix B Sediment and Pore Water Monitoring Work Plan

Dundalk Marine Terminal, Baltimore, Maryland

Sediment and Pore Water Monitoring Work Plan

September 2022

Honeywell Maryland Port Administration

Prepared by

Jacobs Ramboll

Contents

Acron	yms and	l Abbrevi	iations	ii				
1.	Monite	oring of	Sediment Reducing Conditions	1				
	1.1	Introduction						
	1.2	Sampl	Sample Locations and Proposed Winter Sampling Season					
	1.3	Geochemical Parameters and Analytical Methods						
	1.4	ing Procedures	5					
		1.4.1	Sample Collection and Shipping Procedures	5				
		1.4.2	Surface Sediment Samples	5				
		1.4.3	Pore Water Collection	7				
		1.4.4	Sample Nomenclature	8				
		1.4.5	Quality Assurance and Quality Control	8				
		1.4.6	Laboratory Handling	9				
	1.5 Data Evaluation Approach							
Refere	ences			11				
Tables	6							
1-1	Total I	Number (of Proposed Sample Location by Transect	3				
1-2	Geoch	emical P	Parameters to be Measured and Analytical Methods	4				
1-3		Pore Water Containers/Volumes for Direct Push Samples (If Any)Approach To Evaluate Sediment Reducing Conditions from Measured Geochemical Parameters						
1-4								
	Param	ieters		10				
Figure	76							
•				_				
1-1	Proposed Sediment and Pore Water Locations							
1-2 1-3			Proposed Sediment and Pore Water Locationsection Methods					
1-5	Jeuilli	CITE COILE	COUCH INICITIONS					

Acronyms and Abbreviations

AVS acid volatile sulfides

COC chain of custody

Cr(III) trivalent chromium

Cr(VI) hexavalent chromium

DMT Dundalk Marine Terminal

DO dissolved oxygen

DOC dissolved organic content

Eh reduction-oxidation potential

EqP Equilibrium partitioning

ESB equilibrium partitioning sediment benchmark

Fe(II) Ferrous iron

GPS global positioning system

MDE Maryland Department of the Environment

MS/MSD matrix spike/matrix spike duplicate

ORP oxidation-reduction potential

QA/QC Quality assurance/quality control

QAPP quality assurance project plan

TOC organic content

USEPA U.S. Environmental Protection Agency

1. Monitoring of Sediment Reducing Conditions

1.1 Introduction

The objective of the monitoring described in this section is to "confirm sediment reducing conditions" in the Patapsco River adjacent to DMT, in accordance with the components of CMAA Alternative 3 (CH2M Hill 2011). The scope and methods for this monitoring program are based on the results of work completed at DMT in 2007 and 2008 (CH2M Hill and ENVIRON 2007abc; 2008ab, 2009). The confirmation of reducing conditions will be determined based on the assessment of sediment and sediment pore water geochemical conditions in areas previously identified groundwater upwelling areas adjacent to the DMT site. As specified in the CMAA, this monitoring will be conducted at 5-year intervals following MDE approval of this plan.

Confirmation of reducing sediment conditions at DMT in areas where groundwater upwelling is known to occur will indicate that chromium is present as Cr(III) because Cr(VI) cannot persist in reducing conditions. In addition, confirmation of reducing conditions will indicate that if Cr(VI) migration toward the river does occur, the Cr(VI) would be reduced to Cr(III) within the sediment column prior to discharge to the river, ensuring that the reduction of Cr(VI) reduction to Cr(III) does not occur in the river. Confirmation of sediment reducing conditions will be based on the geochemical parameters described in this section.

The reminder of this section provides the following:

- Section 1.2 identifies sampling locations and proposed sampling season based on previous geochemical results for DMT.
- Section 1.3 provides an overview of the geochemical parameters that will be monitored for DMT.
- Section 1.4 summarizes the sediment/pore water sample collection approaches, sample nomenclature, quality assurance/quality controls, and laboratory handling procedures.
- Section 1.5 provides the data evaluation approach.

1.2 Sample Locations and Proposed Winter Sampling Season

Twenty-four sediment and twenty-four pore water sample locations will be sampled for the CMAA monitoring to confirm reducing conditions in sediment, as indicated on Figure 1-1, Figure 1-2, and Table 1-1. These sample locations are a subset of those previously sampled (CH2M Hill and ENVIRON, 2007a; 2009).

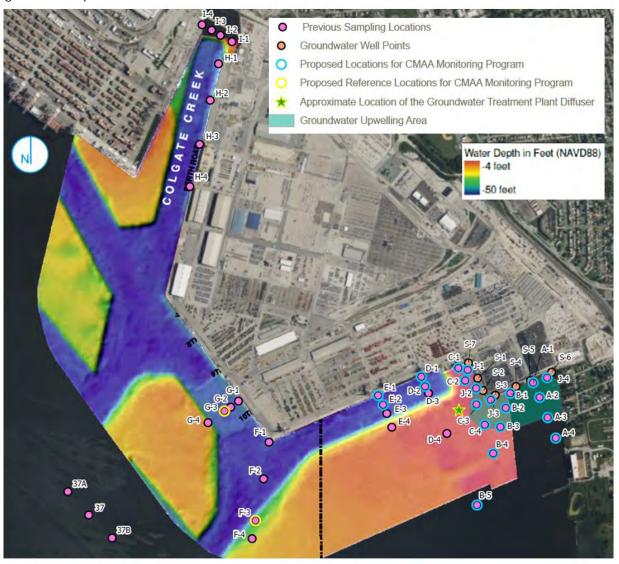


Figure 1-1. Proposed Sediment and Pore Water Locations

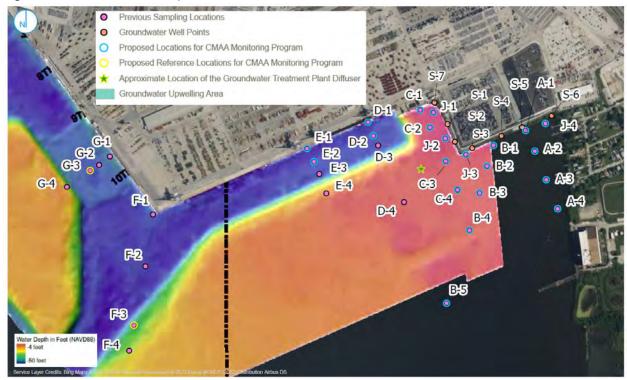


Figure 1-2. Zoom View of Proposed Sediment and Pore Water Locations

Table 1-1. Total Number of Proposed Sample Location by Transect

Transect	Surface Sediment	Pore Water			
Within or Adjacent to Groundwater Upwelling					
А	4	4			
В	4	4			
С	4	4			
J	4	4			
Adjacent to the Bulkhead					
D	2	2			
E	2	2			
References Samples					
F	1	1			
G	1	1			
Total	24	24			

The sample locations focus on sampling locations within and adjacent to the groundwater upwelling area and adjacent to the bulkhead, as follows:

• Transects A, B, C, and J are clustered near the southeast end of DMT within and adjacent to the area where prior investigations identified groundwater upwelling and in the area of the 14th and 15th

Street outfalls.. Four locations will be sampled on each transect. Note that the groundwater treatment plant discharge via submerged diffuser T is adjacent to Transect C, location C-3 (Figure 1-2).

- Transects D and E are adjacent to the bulkhead, placed at smaller stormwater outfalls. The two locations closet to the terminal will be sampled.
- Transects F and G will serve as reference area conditions for geochemical parameters. The previous sampling results showed low chromium concentrations and there are no outfalls in the vicinity of these transects. In addition, the results from Transects F and G were similar to the geochemical parameter results from the previous reference areas 37A, 37B, and 37C from the deeper river channel (illustrated on Figure 1-1). This sampling will include reference location information to provide insight into the reducing conditions that would be expected to occur in areas unimpacted by groundwater upwelling to aid in the interpretation of results within areas of potential groundwater upwelling.

The CMAA monitoring of geochemical parameters will be conducted in winter timeframe because the previous investigation of sediment and pore water demonstrated that winter conditions tend to be less favorable for geochemical conditions compared to warmer seasons (CH2M HiII and ENVIRON, 2009).

1.3 Geochemical Parameters and Analytical Methods

The geochemical parameters that will be monitored and the rationale for monitoring these parameters are listed in Table 1-2 and are consistent with the previous study of sediment conditions at DMT (CH2M Hill and ENVIRON, 2009). The basis of these parameters is discussed further in the data evaluation approach, Section 1.5.

Table 1-2. Geochemical Parameters to be Measured and Analytical Methods

Parameter	Surface Sediment	Pore Water
Acid volatilized sulfides (AVS)	EPA Method 821-R-91-100	NA
Total Sulfides	Prep: EPA 9030B (acid soluble fraction only); Analyses: EPA 9034	NA
Ferrous iron (Fe(II))	Prep: Extraction 1 g:100 mL with pH 2 (HCI) DI water—5 minutes—lab to take initial and final pH; Analyses: Standard Method 3500-D	Analyses: Standard Method SM-3500-D
Total organic carbon (TOC)	Prep and analyses: EPA 9060M	NA
DOC	NA	Prep and analyses: EPA 9060A
Dissolved oxygen (DO)	NA	Laboratory confirmation or field monitoring, as appropriate (a)
Oxidation-reduction potential (ORP)	Field monitoring	Laboratory confirmation or field monitoring, as appropriate (a)
Eh/pH, salinity	Field monitoring	Field monitoring
рН	Field monitoring	Field monitoring

Table 1-2. Geochemical Parameters to be Measured and Analytical Methods

Parameter	Surface Sediment	Pore Water	
Salinity		Field monitoring	
Hardness	NA	SM 2340-C	

NA: Not applicable

(a) field monitoring only if pore water is collected by direct push sampling instead of centrifugation

1.4 Sampling Procedures

This section provides the procedures for assigning sample collection (sediment and sediment pore water), sample nomenclature, quality assurance/quality control (QA/QC), and laboratory handling.

1.4.1 Sample Collection and Shipping Procedures

All samples will be shipped to a designated laboratory for analysis. Sample shipment shall be controlled by sample chain of custody (COC) forms. Each transect will be surveyed using global positioning system (GPS) and target coordinates established from past sampling efforts at DMT.

1.4.2 Surface Sediment Samples

Sample Collection Method

The surface sediment interval that will be sampled is the upper 6 inches of the sediment column at each location. A sampling vessel will be used to collect surface sediment samples. Surficial sediment samples will be collected from a grab sampler or box core concurrently with pore water sampling, as illustrated in Figure 1-3.

Figure 1-3. Sediment Collection Methods



Sediment sampling collection, as illustrated in Figure 1-3:

- a) A sediment box core or grab sample will be collected by field staff wearing clean nitrile gloves by lowering the collection unit into the sediment on one or more occasions to get intact samples.
 - Date, time, and weather conditions will be recorded by the sampling crew in the field book.
 - The choice of box core or grab sampler will be based on the most efficient method available for collecting an intact sample based on conditions observed at the time of sampling.
 - A sediment box corer or grab sampler is a sediment sampling device that is lowered vertically
 to the seabed. Weights will be used to force the stainless steel sampling device (box or grab
 sampler) into the sediment. The core sample size is controlled by the speed at which the corer
 is lowered into the substrate. Firmer substrates require a higher speed to obtain a complete
 sample.
 - Once the core box or grab sampler has penetrated and is filled with sediment, the sample is secured by a spade that cuts across the sediment (box core) or the closure of the sediment grab sampler to secure the sample.
 - The box spade (or grab sampler closure flap) prevents sample loss during retrieval and limits disturbance of the sediments collected.
 - Sediments will be placed on a wash down table.
 - Field measurements of reduction oxidation potential (Eh), pH, and oxidation-reduction potential (ORP) will be performed on each box core or grab sample using field sampling equipment such as an Oakton Acorn pH/mV/ORP meters; one with a double juncture pH probe, and the other with an ORP platinum band electrode for Eh readings.
- b) Pre-cut and decontaminated Lexan tubes will be pushed into the sample by the field team.
 - Samples will not be homogenized to maintain the natural geochemical conditions to the extent possible.
 - This method will be used to collect samples for bulk sediment analyses and for pore water analyses.
- c) Sediment and tubes will be carefully deposited on the boat wash down table, with the Lexan tubes in place for the remainder of handling.
 - Water within the box core/grab sampler will be collected using turkey baster and will be added back to the sample to eliminate head space, as needed.
- d) Each end of each tube will be covered with a Teflon sheet to minimize sediment contact with air in the head space within the sample container.
 - Sediment/water from the turkey baster collected from the sediment sample will be added to the Lexan tubes to top off the sediment in any tube to limit headspace.
- e) Each end of each tube will be capped after the Teflon sheet is in place.
- f) Each tube will be cleaned, labeled (if not pre-labeled), and sealed with tape for shipment to the laboratory.

- Sealing of the tubes will be done using duct tape or other similar tape that will prevent leakage of water out of the sediment and prevent air intrusion into the samples (i.e., to maintain the geochemical conditions).
- The top of each tube will be identified, and samples will be maintained in an upright position for all handling and shipping activities.
- Sufficient sediment volume will be collected for both sediment (approximately 16 ounces of sediment for bulk sediment analyses) and approximately 3 liters of sediment for sediment pore water samples (pore water handling is discussed further in Section 1.4.3). Prior to sampling, the laboratory will be consulted to confirm sample volume preferred by the laboratory for centrifugation.
- Collected samples will be processed and shipped to offsite laboratories. When transported to shore for processing, they will be kept vertical to prevent mixing the unconsolidated surficial sediments.
- All sediment tubes will be transferred to pre-cleaned plastic bags and transported on ice to the laboratory using a COC protocol.

The following equipment or equivalent will also be used for collecting sediment samples:

- Field book/sampling log/ GPS Unit
- Polycarbonate (Lexan®) tubes and caps
- Pipe wrap, Teflon tape, and duct tape
- Box corer or grab sampler
- Ziploc bags and Indelible markers
- Nitrile or latex gloves
- Cooler, ice, tape, COC forms and shipping labels
- Plastic spoons and scoops (if needed to ensure no headspace in Lexan tube liners)
- Turkey baster (to add water from sediment sample to eliminate headspace in Lexan tub liners)

1.4.3 Pore Water Collection

At least 3.0 L of sample volume of sediment will be collected at each station using polycarbonate (Lexan®) tubes as sample containers, as was described in Section 1.4.2 for surface sediment. Pore water will be extracted from sediment at the analytical laboratory using a centrifugation approach, which was used in the 2007/2008 sampling program and by MDE in the collection of pore water as part of the Water Quality Analyses of Chromium in the Inner Harbor/Northwest Branch and Bear Creek (MDE, 2004).

Efforts will be made to collect pore water samples using the box core or grab sampling approach. However, during past sampling at DMT, some of the locations in Transects A, B, C, and J were sandy and obtaining sufficient pore water volume using the box or grab core approach was challenging. Therefore, if sediments drain rapidly and it is suspected that sufficient pore water volume cannot be obtained using the box or grab sampling approach, pore water sample will be collected using a direct push method from a depth of approximately 0.6 inches to 1 foot below sediment surface using a probe flange to avoid the collection of surface water instead of sediment pore water. If a direct push probe is used, the following collection techniques will be considered:

- Pore water collected by direct push probe will be collected within an anaerobic handling bag to ensure maintenance of the geochemical conditions.
 - Clean samples containers from the laboratory will be filled and sealed within an anaerobic bag.

- Prior to sealing samples, field measurements of Eh, pH, and in pore water sample will be measured using field sampling equipment such as an Oakton Acorn pH/mV/ORP meters; one with a double juncture pH probe, and the other with an ORP platinum band electrode for Eh readings. This will be also done within the anaerobic handling bag.
- Once the containers are sealed, they will be removed from the bag and final labels will be added and samples will be secured for shipment to the laboratory under COC protocols similar to those used for bulk sediment samples identified for centrifugation.
- Sample collection technique (i.e., direct push probe) will be recorded in the field notes and in the sample nomenclature. Sample containers/volumes will be those identified for each analysis and preservation as indicated in Table 1-3.

Table 1-3. Pore Water Containers/Volumes for Direct Push Samples (If Any)

Method	Containers / Volumes
Fe(II): Standard Method SM-3500-D	0.5 Liter Glass
DOC: EPA 9060A and Hardness: SM20-2340-C	1 Liter Glass

1.4.4 Sample Nomenclature

A systematic nomenclature will be used to identify samples collected during the sampling program. Samples will be identified and labeled with a code identifying media, sample station number, and sample depth, in the following order: medium type, station, sample type, and sample depth.

- Medium will be represented by a two-letter code:
 - BS—bulk sediment
 - PW—pore water
- Station locations will be identified with a two-number code following medium type. Stations will run sequentially, beginning with 01 (e.g., BS-01).
- Pore water samples will be further differentiated by a sample type code after the location code:
- PW-xx-CEN—pore water collected from sediment via centrifuge
- PW-xx-DP—pore water collected with direct push probe, such as a Trident probe
- Sample depth will be identified with a four-number code indicating depth in feet, with all sediment samples labeled as 0005 reflecting the surface sediment interval (i.e., 0.0 to 0.5 feet below sediment surface).

1.4.5 Quality Assurance and Quality Control

The quality assurance/quality control samples will be collected in accordance with the DMT Quality Assurance Project Plan (QAPP). Specifically:

- All non-dedicated sampling equipment will be decontaminated before and between uses, as described in the QAPP.
- Dedicated sampling equipment designed for a single use and delivered from the supplier pre-cleaned, will not be decontaminated in the field before use.

- Sample duplicates will be collected at a frequency of 10% and matrix spike/matrix spike duplicates will be collected at a frequency of 20%.
- Samples will be validated in accordance with the QAPP.

1.4.6 Laboratory Handling

Sediment Handling for Sediment Analyses

The purpose of this sampling is to confirm sediment reducing conditions. Therefore, the handling of sediments in the laboratory must also maintain geochemical conditions to the extent possible. For sediment handling, arrangements will be made with the laboratory in advance to ensure that geochemical conditions are maintained to the extent possible, such as:

- The top of each tube will be identified, and samples will be maintained in an upright position for all handling and shipping to the laboratory.
- The COC(s) will indicate that samples are to be maintained in upright position in the laboratory and that sediments are to be extracted from the top of the container for obtaining samples for sediment analyses.
- The laboratory will be requested to obtain sediment aliquots planned for sediment analyses via handling of tubes in anaerobic conditions until immediately before preparation/analyses.

Sediment Handling for Pore Water Centrifugation and Pore Water Analyses

Pore water will be collected by sediment centrifugation unless collected by direct push, as discussed above. Sediment handling for pore water analysis will be arranged with the laboratory, such as:

- Sediment will be transferred from the Lexan tubes to centrifugation containers in anaerobic handling conditions (bag or box, as available from the laboratory).
- Pore water will be extracted from the sample containers using a centrifuge operated at a maximum of 10,000 g for up to 30 minutes.
- Pore water will be extracted from the centrifugation tubes for analysis under anaerobic conditions and will be maintained in containers with no head space until immediately before preparation/analyses.
- Eh, pH, and ORP in each pore water sample will be measured using equipment such as an Oakton Acorn pH/mV/ORP meters or other similar equipment used by the analytical laboratory.
- Samples will not be filtered prior to analysis.

1.5 Data Evaluation Approach

The geochemical parameters, the rationale for each parameter, and the evaluation criteria are identified in Table 1-4. These parameters and evaluation criteria are consistent with regulatory guidance and scientific literature, cited by MDE in the Water Quality Analysis of Chromium in Northwest Branch and Bear Creek Portions of the Patapsco River (2013). MDE stated:

"Chromium present within the aquatic environment (water column or sediment) exists in two oxidation states, trivalent (Cr(III)) or hexavalent (Cr(VI)). The distinction between these two oxidation states is significant due to the toxicity associated with each species; Cr(III) is relatively non-toxic at levels typically found within the environment and Cr(VI) is highly toxic. Reduction/oxidation (Redox) conditions within the water column or sediment govern the speciation of chromium. Within Northwest Branch and Bear Creek, low levels of dissolved oxygen (DO) in the water column and elevated levels of biologically oxygen demanding (BOD) substances, produce anoxic conditions within the sediment supporting a

reducing environment. Reductants present within the sediment (total organic carbon (TOC), acid volatile sulfides (AVS), and divalent iron (Fe (II))) facilitate the conversion of Cr(VI) to Cr(III). Cr(III) exhibits low solubility and will undergo reactions to form stable oxides/hydroxides resulting in partitioning from pore water to sediment. As Cr(III) is not present in the pore water at elevated levels under these conditions, it is no longer bioavailable to sediment dwelling organisms through the mechanisms of respiration and dermal absorption. Therefore, chromium remains bound in the sediment in its trivalent state and has no toxicological impact on benthic life."

Consideration of geochemical parameters is consistent with USEPA's efforts related to understanding chromium in aquatic environments are summarized in the 2005 Equilibrium Partitioning (EqP) Guidance for Metals Mixtures, Appendix D Chromium (USEPA, 2005; Berry et al., 2002, Boothman et al., 2000;). The sediment and pore water sampling design presented herein is consistent with the USEPA EqP approach. USEPA states that geochemical processes govern the reduction of Cr(VI) to Cr(III) in aquatic environments, and geochemical processes are critical to the attenuation of chromium in sediments, including AVS, ferrous iron, and organic carbon.

Scientific literature support the use of geochemical parameters as well. Specifically, studies show that Cr(III) is relatively insoluble at environmentally relevant pH, due to the formation of insoluble hydroxide and oxide compounds by strong complexation with sediment minerals and organic ligands (Sass and Rai, 1987; Fendorf and Zasoski, 1992; James, 2002). Studies also show that anaerobic sediments with sulfides, ferrous iron, and organic matter facilitate rapid reduction of Cr(VI) to Cr(III) (Hansel et al., 2003; James, 2002).

The evaluation approach identified in Table 1-4 is considered appropriate for the confirmation of reducing conditions in sediment. Past conditions at DMT have confirmed the presence of reducing conditions in sediment and sediment pore water (CH2M Hill and ENVIRON, 2009).

Table 1-4. Approach To Evaluate Sediment Reducing Conditions from Measured Geochemical Parameters

Physical Parameter	Rationale for Monitoring	Evaluation Criteria
AVS and sulfide	AVS and sulfide will be measured in sediment because thse conditions demonstrate reducing conditions (USEPA, 2005; MDE, 2013; DiToro et al. 2005; Berry et al. 2002; Boothman et al. 2000).	The presence of AVS or presence of sulfide will confirm reducing conditions.
Fe(II)	Fe(II) will be measured in sediment and pore water. Under reducing conditions iron is present in the form of Fe(II); under oxidizing conditions Fe(III) will dominate (USEPA, 2005; MDE, 2013; DiToro et al. 2005).	Presence of Fe(II) will confirm reducing conditions.
DO	DO will be measured in pore water. Reducing conditions are indicated by anaerobic sediments (i.e., in the absence of DO). DO varies with temperature and season (Stanin, 2005; Eary and Rai, 1987; USGS, 2018).	DO less than 3 mg/L will reflect reducing conditions because sampling is planned during winter conditions.
ORP	ORP is a direct measure of oxidation reduction potential. Redox zones of sediments can fall into three classes: the aerobic (oxygen reduction), the suboxic (nitrate and iron reduction), and the anoxic (sulfate reduction and methanogenesis) (Zhang et al., 2014; Di Toro et al., 2005).	Aerobic conditions: ORP>1 Reducing conditions: ORP<1

Table 1-4. Approach To Evaluate Sediment Reducing Conditions from Measured Geochemical Parameters

Physical Parameter	Rationale for Monitoring	Evaluation Criteria
Eh, pH, hardness, and salinity	Eh/pH, and salinity provide insight about the water quality relative to groundwater conditions and reference area conditions.	These parameters will be used in conjunction with other parameters to evaluate water quality compared to reference locations.
TOC and DOC	TOC and DOC will be measured in sediment and pore water, respectively. Organic ligands can also serve as reducing agents, although the reduction kinetics are slower than for AVS or Fe(II) (USEPA, 2005; Eisler, 1986).	Detected TOC and DOC will be used in addition to AVS, Sulfide, and Fe(II) to indicate reducing conditions.

References

Berry, W.J. et al. 2002. Effects of Chromium in Sediment: 1 Toxicity Tests With Saltwater Field Sediments, USEPA, National Health and Environmental Effects Research Laboratory, Atlantic Ecology Division. Narragansett, RI.

Boothman W.S., et al. 2000. Predicting Toxicity of Chromium Spiked Sediments by using Acid Volatile Sulfide and Interstitial Water Measurements. USEPA, National Health and Environmental Effects Research Laboratory, Atlantic Ecology Division. Narragansett, RI.

CH2M HILL and ENVIRON. 2007a. Final Sediment and Surface Water Study Work Plan, Dundalk Maine Terminal, Baltimore Maryland. April.

CH2M HILL and ENVIRON. 2007b. Sediment and Surface Water Investigation Technical Memorandum: May 2007 Sampling Event, Dundalk Marine Terminal, Baltimore, Maryland.

CH2M HILL and ENVIRON. 2007c. Sediment and Surface Water Investigation Technical Memorandum: August 2007 Sampling Event, Dundalk Marine Terminal, Baltimore Maryland.

CH2M HILL and ENVIRON. 2008a. Sediment and Surface Water Investigation Technical Memorandum: December 2007 Sampling Event, Dundalk Marine Terminal, Baltimore Maryland.

CH2M HILL and ENVIRON. 2008b. Sediment and Surface Water Investigation Technical Memorandum: February 2008 Sampling Event, Dundalk Marine Terminal, Baltimore Maryland.

CH2M HILL and ENVIRON. 2009. Sediment and Surface Water Study Report., Dundalk Marine Terminal, Baltimore Maryland. May.

Chang Zhang, Zhi-gang Yu, Guang-ming Zeng, Min Jiang, Zhong-zhu Yang, Fang Cui, Meng-ying Zhu, Liuqing Shen, Liang Hu. 2014. Effects of sediment geochemical properties on heavy metal bioavailability, Environment International, 73: 270-281.

Di Toro, D.M., C.S. Zarba, D.J. Hansen, W.J. Berry, R.C. Swartz, C.E. Cowan, S.P. Pavlou, H.E. Allen, N.A. Thomas, and P.A. Paquin. 1990. Technical Basis for Establishing Sediment Quality Criteria for Nonionic Organic Chemicals Using Equilibrium Partitioning. *Environ. Toxicol. Chem.* 10, 1541–1583.

Di Toro, D. M., J.A. McGrath, D.J. Hansen, W.J. Berry, P.R. Paquin, R. Matthews, et al. 2005. Predicting sediment metal toxicity using a sediment biotic ligand model: methodology and initial application.

ENVIRON. 2008. Relationship Between the Presence of Chromium and Toxicity in Baltimore Harbor: New Scientific Evidence. Summary of Johns Hopkins University Studies.

Eary LE, Rai D. 1987. Kinetics of chromium(VI) oxidation to chromium(VI) by reaction with manganese dioxide. Environ Sci Technol 21:1187-1193.

Eisler, R. 1986. Chromium Hazards to Fish, Wildlife, and Invertebrates: A Synoptic Review. *U.S. Fish Wildl. Serv. Biol. Rep. 85*(1.6).

Fendorf, S.E. 1995. Surface Reactions of Chromium in Soils and Waters. *Geoderma.* 57, 65–71.

Fendorf, S.E., and R.J. Zasoski. 1992. Chromium (III) Oxidation by δ -MnO2. 1. Characterization. *Environ. Sci. Technol. 26*, 79–85.

Gambrell, RP (1994) Trace and toxic metals in wetlands--A review. J Environ Qual 23:883-891

Hansel CM, Wielinga BW, Fendorf S. 2003. Fate and stability of Cr following reduction by microbially generated Fe(II). In: <u>Science Highlight</u>. Department of Geological and Environmental Sciences, Stanford University and 2MFG, Inc., Fort Collins, CO. May.

James, B.R. 2002. Chemical Transformations of Chromium in Soils: Relevance to Mobility, Bioavailability and Remediation. In *The Chromium File*, No. 8. The International Chromium Development Association. Available at http://www.chromium-assoc.com/publications/ crfile8feb02.htm. February.

Magar, V., LB Martello, B. Southworth, P. Fuchsman, M. Sorensen, and RJ Wenning. 2008. Geochemical Stability of Chromium in Sediments from the Lower Hackensack River, New Jersey. Sci of the Total Environ. 394: 103 – 111.

MDE. 2004. Water Quality Analyses of Chromium in the Inner Harbor/Northwest Branch and Bear Creek Portions of Baltimore Harbor in Baltimore City and Baltimore County, Maryland. August. //efaidnbmnnnibpcajpcglclefindmkaj/https://mde.maryland.gov/programs/water/TMDL/Baltimore_Harbor_02130903/Harbor_Cr_WQA_FA.pdf

MDE. 2013. Water Quality Analysis of Chromium in Northwest Branch and Bear Creek Portions of the Patapsco River Mesohaline Tidal Chesapeake Bay Segment, Baltimore City and

Baltimore County, Maryland.

Rai D., Sass B.M., Moore DA. 1987. Chromium (III) hydrolysis constants and solubility of chromium (III) hydroxide. Inorganic Chem 26:345-349

Sass, B.M., and D. Rai. 1987. Solubility of Amorphous Chromium (III)-Iron(III) Solid Solution. *Inorg. Chem.* 26, 2228–2232.

Saleh FY, Parkerton TF, Lewis RV, Huang JH, Dickson KL. 1989. Kinetics of chromium transformations in the environment. Sci Total Environ 86(1-2):25-41.

Stanin, F.T. 2005. The Transport and Fate of Chromium (VI) in the Environment. In: Chromium (VI) handbook / written by Independent Environmental Technical Evaluation Group (IETEG); edited by Jacques Guertin, James A. Jacobs, Cynthia P. Avakian. Boca Raton, Fla: CRC Press.

United States Environmental Protection Agency (USEPA). 2005. Procedures for the Derivation of Equilibrium Partitioning Sediment Benchmarks (ESBs) for the Protection of Benthic Organisms: Metal Mixtures (Cadmium, Copper, Lead, Nickel, Silver, and Zinc). Appendix D: Chromium. *EPA-600-R-02-011*. Office of Research and Development.

US Geological Survey. 2018. Dissolved Oxygen and Water. https://www.usgs.gov/special-topics/water-science-school/science/dissolved-oxygen-and-water.

Wadhawan, Amar. 2012. Geochemical Influences in Chromium Speciation and Fate in Estuarine Sediments; Importance of Redox Interactions with Manganese Sulfide Minerals. Johns Hopkins University Dissertation.

Appendix C Groundwater Action Levels

Technical Appendix C PERFORMANCE MANAGEMENT PROGRAM SENTINEL GROUNDWATER MONITORING PLAN

DUNDALK MARINE TERMINAL BALTIMORE, MARYLAND

Prepared for



115 Tabor Road Morris Plains, NJ 07950

and



Maryland Port Administration 401 East Pratt St. Baltimore, MD 21202

Prepared by



Ramboll US Consulting, Inc. Princeton, New Jersey

March 2023

Joint Defense Privileged

D R A F T Sentinel Groundwater Monitoring Plan March 2023



Contents

1.	Intro	1-1	
2.	Back	ground	2-1
3.	Later	ral Migration	3-1
		Zone 1	
	3.2	Zone 2	3-1
4.	Verti	cal Migration	4-1
5.	Refe	rences	1

Figures

Figure 1: Dundalk Marine Terminal Site

Figure 2: Geology of the DMT Site

Figure 3: Groundwater Zones

Figure 4: Groundwater Upwelling Area

Figure 5: Low Tide Water Depths

Figure 6: Upper Sand Unit Monitoring Wells

Tables

Table 1: Modeled Groundwater Outflows



Acronyms and Abbreviations

Cr (VI) hexavalent chromium; the more toxic form of chromium

COPR chromium ore processing residue

DF dilution factor

DMT Dundalk Marine Terminal

gpm gallons per minute

MCL maximum contaminant level

MDE Maryland Department of the Environment

mg/L milligrams per liter

PSU practical salinity units

SWQS surface water quality standard

ug/L micrograms/liter

USEPA United States Environmental Protection Agency



1. Introduction

As part of the Sentinel Groundwater Monitoring Plan for the Dundalk Marine Terminal (DMT) site (Figure 1), risk-based action levels have been developed for assessing the potential significance of measured chromium concentrations in groundwater. As the sentinel monitoring plan defines initial threshold conditions for follow-up assessment (e.g., initiating an assessment when hexavalent chromium is initially detected in a well where it has not been previously or recently detected), the action levels developed in this technical appendix are intended to provide an early warning of off-site migration beyond the property boundaries, potentially significant surface water impacts in the Patapsco River or vertical migration of groundwater impacted by chromium from the shallow fill hydrogeologic unit to the Patapsco Aquifer. This appendix describes the methods used to calculate the groundwater action levels and how the action levels will be applied to the evaluation of groundwater data collected as part of the sentinel monitoring program. The action levels are intentionally conservative and provide a threshold to trigger an investigation of potential impacts beyond the boundaries of the property, to the Patapsco River or Patapsco Aquifer rather than an indication of actual impacts.



Figure 1. Dundalk Marine Terminal Site



2. Background

Shallow groundwater in some portions of the DMT site contains chromium as a result of the use of chromium ore processing residue (COPR) as fill material, both by itself and mixed with other fill materials. The approximate boundary of COPR-containing materials is shown on **Figure 1**. As stated in the Semiannual Groundwater Monitoring Report (Jacobs, 2021), the chromium impacts are limited to the COPR and Non-COPR Fill and Shallow Fill unit and the Upper Saturated Zone (collectively referred to as the "Upper Water Bearing Unit", or "UWB Unit"), which is hydraulically connected to the Patapsco River and is underlain by the Upper Confining unit. Below the Upper Confining unit is the Upper Sand unit, the Lower Confining unit, and then the Patapsco Aquifer (**Figure 2**). Monitoring wells in the UWB Unit will be used to monitor for lateral migration, and monitoring wells in the Upper Sand unit will be used to monitored for vertical migration of chromium.

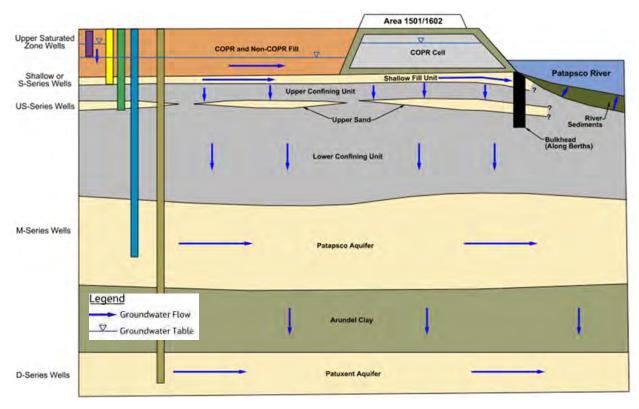


Figure 2. Geology of the DMT Site

For evaluating groundwater monitoring data for lateral migration within the UWB Unit, the site was divided into two zones, as shown by arrows in **Figure 3**. Zone 1 wells are outside the footprint of the COPR fill limit within the portion of DMT that is contained within the perimeter bulkhead, which reduces the potential for groundwater discharges to the river. Zone 2 wells are in areas where groundwater may discharge to the Patapsco River from the Site. Zone 2 is defined as the discharge area between the site bulkhead and the COPR Cell, under the COPR cell, and around the southeast side of the COPR Cell (between COPR Cell at Area 1501/1602 and Broening Highway).



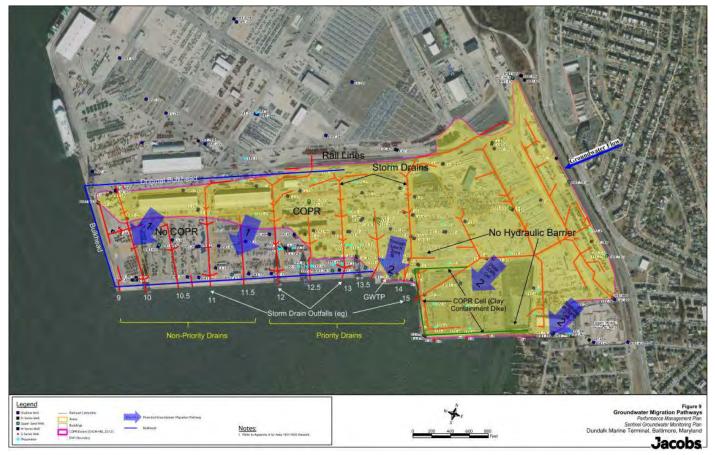


Figure 3. Groundwater Zones

The potential for significant vertical migration of chromium mass from the UWB Unit to the Patapsco Aquifer is to be evaluated based on a site-wide assessment of chromium concentrations using data from wells screened in the Upper Sand Unit.



3. Lateral Migration

3.1 Zone 1

Because the groundwater in Zone 1 is isolated from the Patapsco River by the perimeter bulkhead, the groundwater will be monitored for a statistically significant increasing trend in chromium concentration rather than compared to a concentration-based action level.

For a sentinel monitoring well outside the COPR Fill area in which hexavalent chromium is detected during future monitoring, the data from that well will be evaluated for evidence of a statistically significant increasing trend that would indicate migration of hexavalent chromium in groundwater from the COPR Fill. To assess the temporal trend in a well, a nonparametric Mann-Kendall trend test using all data collected from the impacted well will be conducted. The Mann-Kendall test will be performed as a one-tailed test, where statistically significant increasing concentrations of Cr (VI) are identified at a significance level of 5%. For the testing, non-detect results will be calculated using the sample quantitation limit as the concentration. When collected, field duplicate sample results will be averaged with the results of their respective parent samples. If a well is re-sampled as part of an event, the latter of the results will be used in the test.

3.2 Zone 2

For Zone 2, an action level has been developed based on protection of ecological receptors in the Patapsco River where UWB Unit groundwater upwelling may occur. Salinity measurements in the Patapsco River are generally between 5 and 15 practical salinity units (PSU), indicating an estuarine to saline environment. Therefore, to assess the potential significance of exposure to Cr (VI) by ecological receptors, the surface water quality standard (SWQS) for protection of ecological receptors in saltwater is more appropriate than that for freshwater. The chronic SWQS is conservative because Cr (VI) does not bioaccumulate. The chronic saline SWQS for Cr (VI) of 0.05 mg/L is therefore the basis for computing the groundwater action level for lateral migration in Zone 2.

As groundwater flows from the Site and discharges to surface water, chromium is subject to dilution and attenuation prior to and concurrent with mixing with the nearshore surface water column. The site-related chromium surface water concentrations that may result from shallow groundwater discharge near the shoreline are conservatively estimated considering the following processes:

- Groundwater is assumed to discharge to the surface water from the UWB Unit and potentially from the Upper Sand Units.
- Groundwater flux is calculated using the three-dimensional groundwater flow model developed for the site. The model accounts for variability in hydraulic conductivities across the site in each hydrogeologic layer, including the riverbed material. The model was updated to include recent (2020) groundwater elevation monitoring and to simulate the lining of the 12th through 15th Street storm drains. Groundwater outflow estimates for the UWB Unit are shown on Table 1. The flow is shown in multiple segments but is considered together for the calculation of the action level.
- Groundwater is assumed to discharge into the nearshore area of the Patapsco River (see Figure 4) in Zone 2 as determined during prior investigation of sediment / porewater (CH2M Hill and ENVIRON 2009).
- The groundwater discharge rate is assumed to be constant, with no exchange of surface water occurring outside of the mixing zone, including via tidal mixing.
- The estimated average water depth during low tide is 6 feet (see Figure 5). The groundwater is assumed to mix only with the bottom 6 inches of the surface water column.
- No attenuation of Cr (VI) to Cr (III) is assumed to occur.



Table 1: Modeled Groundwater Outflows		
Outflow Segment	Outflow (ft³/day)	
Between bulkhead and COPR Cell	190.8	
West side of COPR Cell	140.3	
South side of COPR Cell	187	
South of COPR Cell	95.4	
Total groundwater discharge	613.5 ft ³ /day	

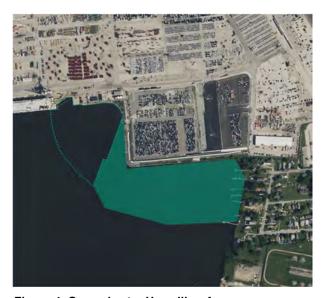


Figure 4. Groundwater Upwelling Area

Figure 5. Low Tide Water Depths

The total mixing zone area is estimated at 1.2 million ft². Assuming an initial mixing layer of 6 inches (0.5 ft), the surface water mixing volume is 602,574 ft³.

Considering the groundwater discharge and surface water mixing volume over a 24-hour period, the estimated mixing of groundwater discharging into surface water (DF = Surface Water Volume/Groundwater Discharge Volume) is approximately 982. This calculated discharge factor is used in combination with the SWQS (0.05 mg/L) to calculate the action level for lateral migration of chromium in groundwater, as follows:

 $Groundwater\ Action\ Level = SWQS\ imes\ DF$

This results in a Cr (VI) action level of 49 mg/L for Zone 2. With an additional factor of safety of 2, the action level is 24.5 mg/L.



4. Vertical Migration

A mixing model based on the groundwater flow model developed for the Site is used to calculate the vertical migration action level that is protective of the Patapsco Aquifer. The action level is based on meeting the federal and Maryland maximum contaminant level (MCL) for total chromium of 0.1 mg/L, or 100 ug/L and the Maryland groundwater cleanup standard for Cr (VI) of 0.000035 mg/L, or 0.035 ug/L (Maryland Department of the Environment 2018) in the Patapsco Aquifer. The groundwater action level for the Upper Sand Unit is computed based on these target concentrations and a dilution factor (DF) that accounts for the vertical groundwater flux relative to the horizontal groundwater flux through the Patapsco Aquifer beneath the Site, as follows:

$$Groundwater\ Action\ Level = Target\ Concentration\ imes\ DF$$

The DF for vertical migration is conservatively estimated without considering attenuation by assuming that groundwater moving vertically from the Upper Sand Unit to the Patapsco Aquifer mixes uniformly with groundwater flowing through the Patapsco Aquifer. The vertical rate for groundwater flow through the Lower Confining unit into the Patapsco Aquifer was estimated by the groundwater model (CH2M Hill, 2015) to be 0.6 gallons per minute (gpm). The estimated lateral flow in the Patapsco Aquifer from the upgradient boundary onto the site is estimated to be 154 gpm. The DF computed based on these flow rates is:

$$DF = \frac{\text{Horizonal Flow (154 gpm)}}{\text{Vertical Flow (0.6 gpm)}} \approx 257$$

With this DF, the action levels are calculated as:

Total Cr Groundwater Action Level =
$$0.1 \frac{mg}{L} \times 257 = 25.7 \frac{mg}{L}$$

and

$$Cr(VI)$$
 Groundwater Action Level = $0.000035 \frac{mg}{L} \times 257 = 0.009 \frac{mg}{L}$

Because the DF is calculated assuming uniform vertical migration with mixing across the entire Site, the groundwater action level is not intended to be compared to the concentration of total chromium or Cr (VI) measured in a single groundwater sample. Rather, it is intended to be compared to the average concentration across the Site. To account for varied spacing of wells included in the groundwater monitoring program (Figure 3), a spatially weighted average concentration will be computed for total chromium and Cr (VI) from Upper Sand Unit wells. This is accomplished by dividing the site into Thiessen polygons, with each polygon associated with the wells that will be sampled (Figure 6). To properly account for the differences in the size of each polygon, the concentration for each well will be weighted by the ratio of the polygon's area to the total Site area. Because most of the Upper Sand Unit wells are not known to be impacted by chromium, samples in which chromium are not detected should be assumed to have a concentration of 0 mg/L.





Figure 6. Upper Sand Unit Monitoring Wells

5. References

CH2M Hill and ENVIRON. 2009. Sediment and Surface Water Study Report, Dundalk Marine Terminal, Baltimore, Maryland. May.

CH2M Hill, 2015. Appendix B, Groundwater Flow Model Development.

Jacobs. 2021. Semiannual Groundwater Monitoring Report, Dundalk Marine Terminal.

Maryland Department of the Environment. 2018. Cleanup Standards for Soil and Groundwater. Interim Final Guidance, Update No. 3. October.

Appendix D Site Drinking Water Plan

Site Drinking Water Plan, Dundalk Marine Terminal, Baltimore, Maryland

Prepared for

Honeywell International Inc.

Maryland Port Administration

Updated February 2023

Updated by: Maryland Environmental Service 259 Najoles Road Millersville, MD 21108

Contents

Sect	ion			Page
Acro	nyms an	d Abbrev	iations	V
	Backo	ground		1-1
	Background. Water Quality Monitoring. 2.1 Baseline Monitoring. 2.2 Routine Monitoring. 2.2.1 Sampling Plan. 2.2.2 Sample Analysis. 2.2.3 Response Plan. 2.2.4 User Notification. 2.2.5 Alternate Water Supply. 2.2.6 Mitigation. 2.2.7 Return to Normal Operation. Pipe Break Contingency Plan. 3.1 Pipe Break Detection. 3.2 Pipe Break Isolation. 3.3 User Notification. 3.4 Alternate Water Supply.			
	2.1 Baseline Monitoring 2.2 Routine Monitoring 2.2.1 Sampling Plan 2.2.2 Sample Analysis 2.2.3 Response Plan 2.2.4 User Notification		2-1	
	2.2	Routin	e Monitoring	2-1
		2.2.1	Sampling Plan	2-2
		2.2.2	Sample Analysis	2-2
		2.2.3	Response Plan	2-2
		2.2.4	User Notification	2-2
		2.2.5	Alternate Water Supply	2-3
		2.2.6	Mitigation	2-3
		2.2.7	Return to Normal Operation	2-4
	Pipe I	Break Coi	ntingency Plan	3-1
	3.2	Pipe B	reak Isolation	3-1
	3.3	User N	lotification	3-1
	3.4	Altern	ate Water Supply	3-1
	3.5		tion	
	3.6		ing and Response	
	3.7	•	to Normal Operation	
			•	

Appendix

A Standard Operating Procedure for Opening and Closing Hydrants during System Flushing

Tables

- 1 DMT Buildings with Water Service
- 2 DMT Quarterly Sample Locations

Figures

- 1 Water System Configuration and Primary System Flushing Locations
- Water System Sampling Locations

Acronyms and Abbreviations

COPR chromium ore processing residue

DMT Dundalk Marine Terminal

EPA U.S. Environmental Protection Agency

gpm gallons per minute

GWTP groundwater treatment plant MCL maximum contaminant level

MDE Maryland Department of the Environment

MES Maryland Environmental Service

MPA Maryland Port Administration

POC point of contact

POV personally operated vehicle

٧

SECTION 1

Background

This Site Drinking Water Plan is an integrated component of the overall Performance Management Program (PMP) for the long-term monitoring and maintenance of the chromium remedy being implemented at the Dundalk Marine Terminal (DMT).

Pursuant to the Consent Decree among the Maryland Department of the Environment (MDE), the Maryland Port Administration (MPA), and Honeywell International, Inc. (Honeywell), and the Corrective Measures Alternatives Analysis required by the Consent Decree, MDE has selected an Enhanced Isolation and Containment remedy for the Site. This Site Drinking Water Monitoring Plan fulfills the PMP requirements with respect to the potable water and outlines the approach for monitoring the water distribution system to verify that chromium is not present in the drinking water supply at the Dundalk Marine Terminal (DMT). This plan supersedes the initial Site Drinking Water Monitoring Plan that was established in response to Section III.B.9.c of the April 2006 Consent Decree entered into by the MDE, MPA, and Honeywell. To date, routine monitoring confirms that the water distribution system remains free of chromium concentrations which exceed the U.S. Environmental Protection Agency (EPA) regulated maximum contaminant level (MCL) of 0.1 mg/L.

The DMT is a port facility, owned and operated by the MPA, located along the southeast edge of the City of Baltimore, Maryland, and Baltimore County, and is bounded by Broening Highway, the Patapsco River, and Colgate Creek. As the operator of DMT, the MPA is responsible for inspection, maintenance, and monitoring of the potable water system at the DMT. Potable water is defined as water that meets the standards for drinking purposes and meets the quality standards prescribed by the EPA's National Primary Drinking Water Regulations, published in 40 CFR Part 141. The groundwater at or near the DMT is not used for potable water at the site. The potable water at the site is supplied by the City of Baltimore municipal water supply. The potable water system at DMT contains approximately 16 miles of pipe ranging in size from 4 to 16 inches in diameter. Portions of the water system pass through areas of the site where chromium ore processing residue (COPR) was historically used as fill material (Figure 1). The water is supplied to the DMT system by the City of Baltimore at two connection points: one located at the Personally Operated Vehicle (POV) Gate on North Service Road at Broening Highway and the second located off Broening Highway near Lot 99 (Figure 1). Both connections are 16-inch-diameter water mains. The POV Gate connection is a Baltimore City-owned and maintained double-check valve, and the Lot 99 (Broening Highway) connection is fitted with a backflow prevention device that is inspected and maintained by MPA. Both connections are designed to prevent the backflow of water from the DMT water conveyance system into the Baltimore City water supply system.

Building 1702 (Figure 2) is supplied via a single 6-inch line that connects to a 12-inch Baltimore City main on Maryland Avenue, to the south. Building 1702 has no connection to the main distribution system at DMT. The configuration of the system and its connections to the Baltimore City Water System are shown in Figure 1.

The remainder of this plan consists of two sections. The first section describes the sampling approach that was used to determine baseline conditions of the water distribution system in relation to the presence or absence of chromium. This section also confirms the routine sampling program currently in place to test for chromium in the water distribution system. The second section provides a contingency plan for operating the water system in the event of a pipe break, including measures to flush and resample the system after breaks have been repaired so that the system can be returned to service.

SECTION 2

Water Quality Monitoring

2.1 Baseline Monitoring

The purpose of the baseline monitoring of the DMT water distribution system was to quantify baseline conditions for comparison to future routine monitoring events.

To monitor the water distribution system for chromium, initial baseline monitoring was conducted for the system in 2007 (CH2M HILL, 2007d) to determine the presence or absence of chromium. The baseline monitoring determined if total chromium was present in the system above the EPA-regulated MCL of 0.1 mg/L or if there was detection of hexavalent chromium that needed to be addressed prior to performing ongoing routine monitoring. Baseline monitoring indicated absence of chromium in the water distribution system.

The initial 2007 baseline sampling of potable water was conducted in existing buildings, and as new buildings were added to the site, this sampling plan was updated to include additional baseline water sampling at those locations (CH2M HILL, 2011 and CH2M HILL, 2013). Table 1 lists the current sampling locations, including existing buildings at the DMT with water service, the two main water supply points, and the water sampling location descriptions for each. Since 2007, Building 201A, Building 1600, and MPA Engineering Trailers have been demolished or removed from site, and water service has been disconnected from Shed 4. Table 1 includes those updates and is reviewed and updated annually in coordination with MPA Maintenance to remove or add sample locations.

2.2 Routine Monitoring

The purpose of routine monitoring is to confirm that the DMT water system remains free of chromium above the MCL and to verify that public health is being protected. This routine sampling will provide assurance that the system continues to meet EPA drinking water regulations and is suitable for potable use.

The water is supplied to the system by the City of Baltimore at two connection points, one located at the POV Gate on North Service Road at Broening Highway and the second located off Broening Highway near Lot 99 (Figure 1). Upon MPA request, Baltimore City will perform routine maintenance inspection of the POV Gate double-check valve connection, which is property of Baltimore City. Routine monitoring of the backflow prevention devices on the DMT site, including at the Lot 99 (Broening Highway) connection, are the responsibility of MPA Maintenance. The backflow devices onsite are monitored annually to ensure they are compliant with all applicable federal, state, and local drinking water codes.

The routine water sampling is conducted quarterly from a rotating selection of the buildings with water service. This sampling plan provides regular monitoring for presence of chromium in the water and a schedule that ensures the annual collection and analysis of water samples at each building at DMT with water service. Table 1 is a comprehensive list of all the current sampling locations. Table 2 provides a list of the sample points for routine monitoring and the sample schedule for appropriate spatial variation within each quarterly sampling event. This includes the DMT buildings with water service and the two main water supply points. The water supply points will be sampled every quarter to verify the quality of water being supplied to the site. Figure 2 shows current sampling locations.

The samples will be collected and analyzed in accordance with the procedures discussed in Sections 2.2.1 and 2.2.2. All sample results must be submitted to MDE using normal reporting procedures within 30 days of receiving the results.

2.2.1 Sampling Plan

The following process will be used in the collection of a water sample at each of the locations described above and listed in Table 1:

- 1. Identify the described sample point
- 2. Complete appropriate sample label
- 3. Put on nitrile or latex gloves
- 4. Turn on faucet and take water temperature reading
- 5. Allow water to flow for a minimum of 5 minutes and until the water temperature drops below the initial temperature reading
- 6. For total chromium, collect the samples in plastic 250-mL, HNO₃-preserved bottle ware provided by the analytical laboratory. For hexavalent chromium, collect the samples in 250-mL non-preserved plastic bottle ware provided by the analytical laboratory.
- 7. Store sample in cooler with ice to reduce temperature to 2°C
- 8. Complete chain-of-custody log and request analysis for total chromium in compliance with EPA analytical method requirements (EPA Method 200.7 or 200.8 and hexavalent chromium by EPA Method 7199); both analyses should have a detection limit of <0.01 mg/L at a minimum.

2.2.2 Sample Analysis

Analysis of collected samples will be conducted at an EPA-certified laboratory for hexavalent chromium by Method 7199 and total chromium by Method 200.7 or 200.8. The analysis for hexavalent chromium is time critical, with samples having a maximum holding time of 24 hours. In addition, the laboratory will be instructed and required to notify MPA and/or its designee, Maryland Environmental Service (MES) immediately upon determining and confirming that a drinking water sample has a total chromium detection over the MCL or a hexavalent chromium detection over the reporting limit. For this Plan, "immediately" means the same day when determined during standard business hours, or next day if the determination is made after normal business hours.

All sample results will be submitted to MDE within 30 days of receiving the results from the laboratory. In the event that an exceedance of the MCL for total chromium or a detection of hexavalent chromium over the reporting limit is identified in any sample, MPA will notify MDE immediately upon receipt of notification from the laboratory. The sample results will also be on file at the MPA Office of Environmental Quality.

2.2.3 Response Plan

The EPA regulates chromium in drinking water having a MCL for total chromium of 0.1 mg/L. If any of the routine samples show a level higher than this MCL, the mitigation actions provided in Section 2.2.6 will be taken to reduce the total chromium concentrations below the MCL. Additionally, if any concentration of hexavalent chromium is detected in water samples, the same mitigation actions will be performed.

If the MCL for total chromium is amended, the text in the plan will be revised within 30 days of enactment of the new standard.

2.2.4 User Notification

In the event of detectable chromium concentrations above actionable limits, DMT personnel will be advised to not use the water system for potable water uses until further notified. This will be conducted by notifying a point of contact (POC) for the buildings by a tenant advisory issued by MPA Operations. Signs will be posted at the buildings that the water supply is temporarily to be for only non-potable uses.

Building 1702 is not connected to the main distribution system at DMT, and therefore the use of potable water at this building will continue to be allowed, unless the detection is specific to this building. Conversely, if chromium

is detected at Building 1702 only, it will not trigger the notification of tenants in other portions of the terminal, who are not affected.

2.2.5 Alternate Water Supply

An alternate supply of potable water service shall be made available to all users affected by the temporary loss of municipal potable water service due to chromium detection.

2.2.6 Mitigation

In conjunction with notifying users, the following actions will be taken to verify the sample result.

- The affected building's system will be flushed using all service points for a period of 15 minutes, and a confirmatory sample will be collected. The samples will be collected and analyzed in accordance with the procedures discussed in Sections 2.2.1 and 2.2.2.
- The affected building's system will then be flushed for an additional 15 minutes, and a second confirmatory sample will be collected. The samples will be collected and analyzed in accordance with the procedures discussed in Sections 2.2.1 and 2.2.2.
- Both samples will be submitted for rush laboratory analysis to confirm the sample result.
- If the analytical results of the confirmation samples do not exceed the MCL for total chromium or result in a detection of hexavalent chromium, the building location will be put back into service.
- If the analytical result of either confirmation sample again shows an exceedance of the MCL for total chromium or a detection of hexavalent chromium, the following procedure will be followed to remove the contaminated water from the potable water conveyance system. (A standard operating procedure for opening hydrants and purging the system is included as Appendix A.)
 - Utilize the three nearby hydrant(s) or adjacent water pits for primary flushing of the system.
 - Open the three hydrants identified for flushing (or water pits nearest them). Collect a water sample for laboratory analysis from each hydrant or water pit and test for hexavalent chromium. Initial screening for hexavalent chromium can be conducted at the onsite DMT Groundwater Treatment Plant (GWTP).
 - If the sample has a hexavalent chromium concentration less than 1,100 ppb (the Ambient Water Quality Criteria for saltwater acute exposure limit), discharge all flushing water to river in accordance with MDE General Permit Number 17HT9473, avoiding the use of the 14th and 15th Street storm drains.
 - If the hexavalent chromium concentration is greater than 1,100 ppb, a plan for discharge of the flush water will be developed so that it can be discharged to the 14th and/or 15th Street storm drains and treated at the onsite GWTP in accordance with MES's 2007 "Interim Operations Plan." As stipulated in the "Interim Operations Plan," the water will be managed as a wet-weather event, and the first flush will be captured for treatment and the remaining flow will be discharged to the river. If the GWTP is not operational, alternate temporary water storage and treatment/disposal, if required, will be coordinated with MPA Office of Environmental Quality.
 - Any discharge that may affect the operational capacity of the GWTP will be coordinated with the personnel managing the GWTP.
 - Identify the nearest appropriate storm drain that will receive the water from the flushing hydrants or water pits. Check that flushing of the hydrant or water pits will not cause localized flooding due to blockage of stormwater inlets.
 - Set up a system for dechlorination of flushed water, such as a diffuser or inlet blankets with dechlorination tablets.

- Notify the buildings with auxiliary fire pumps (Sheds 4, 6, 400 and 500) that flushing activities will be taking place which could cause low-pressure alarms associated with the pumps to trip.
- Flow the designated hydrants for approximately 6 hours at a minimum combined flow rate of 1,000 gallons per minute (gpm). If water pits are used, flow the pits for approximately 12 hours at a minimum flow rate of 500 gpm. Conduct the flushing activity in accordance with MDE General Permit Number 17HT9473 and added special conditions.
- Collect a water sample at each sample location provided in Table 1 using the procedures described in the sampling plan. Rush analysis will be requested so potable service can be restored as quickly as possible.
- If any sample taken at a sample location listed in Table 1 is determined to have total chromium concentrations higher than 0.1 mg/L or hexavalent chromium is detected, flush the identified hydrant(s) for approximately 6 hours or the water pits for approximately 12 hours and resample. Continue sampling and flushing at 6- or 12-hour intervals until the chromium concentration drops below the MCL of 0.1 mg/L and hexavalent chromium is not detected.

2.2.7 Return to Normal Operation

After all sampling has demonstrated full mitigation of chromium contamination in the water system, the system will be returned to service. The POC of each building on the terminal will be notified by a tenant advisory issued by MPA Operations. The signs stating the water supply is temporarily to be for only non-potable uses will be removed from the buildings.

TABLE 1 - DMT BUILDINGS WITH WATER SERVICE (38 BUILDINGS, LOT 90, AND LOT 99)

Building ID	Sample Location Description
90A	Kitchen in lunchroom
91A	Lavatory at maintenance bay
91B	Lavatory at maintenance bay
91C	Lavatory at maintenance bay
94A	Exterior spigot
George's Café	Lavatory at entrance
95B	Lavatory
96E	Kitchen
97C	Lavatory at entrance
97D	Lavatory at entrance
100A	Lavatory
100B	Lavatory
201D	Lavatory
201D-2	Lavatory
201B	Utility sink at north end of building
Ceres Office Trailer	Kitchen Sink
Ceres Lavatory	Lavatory
301A	Lavatory at maintenance bay
301B	Kitchen in lunchroom
401A	Lavatory in office area
401B	Lavatory
401C	Kitchen sink
403A	Break room sink

Building ID	Sample Location Description
500 Office Trailer	Lavatory
502A	Lavatory
502B-1	Trailer 1 bathroom
502B-2	Trailer 2 bathroom
503A	Lavatory
MAT	Lavatory at entrance
Shed 6	Lavatory at south end of building
Shed 8	Lavatory at west end of building
Shed 11	Lavatory at west end of building
Shed 12	Lavatory at west end of building
1200	Lavatory at south end of building
1300	Lavatory at south end of building
MES Trailer	Kitchen sink
MES GWTP Trailer	Lavatory
1702	Lavatory at maintenance bay
Lot 90 (west of POV gate)	Hydrant (NE corner of Lot 90)
Lot 99	Backflow prevention/meter pit

Table 2 - DMT Quarterly Sample Locations

QUARTER 1	QUARTER 2	QUARTER 3	QUARTER 4
Building ID	Building ID	Building ID	Building ID
Shed 8	Shed 6	Shed 12	94A
91B	Shed 11	90A	95B
96E	91A	97D	401B
97C	100B	201B	1702
George's Café	201D	301A	502B-2
100A	401A	401C	1200
201D-2	500 Office Trailer	502B-1	1300
301B	502A	Ceres Lavatory	Ceres Office
403A	MES GWTP Trailer	MAT	MES Trailer
503A		91C	
Lot 90	Lot 90	Lot 90	Lot 90
Lot 99	Lot 99	Lot 99	Lot 99

Pipe Break Contingency Plan

The following plan has been developed to outline the measures to be taken in the event of a pipe break in the COPR fill area (Figure 1) or a pipe break in the non-COPR area where testing indicates potential chromium presence. If a pipe break occurs in the non-COPR area and testing indicates the absence of chromium, the following planned actions are not necessary, and the routine pipe break repair approach will be utilized. The plan details actions to isolate the break and return the system to normal operation.

3.1 Pipe Break Detection

A pipe break can be detected by a number of methods. The most important element of break detection is to identify it quickly and isolate it to reduce potential impacts from vacuum conditions in the pipe or damage to nearby property. The most common methods of break detection at the port are observing above-ground water pooling or a leak or stream of water from an opening in the ground surface along pipe alignments. These or other recommended methods of detection will be achieved through maintenance personnel or any personnel working at the DMT who may notice evidence of a pipe break and contact the DMT MPA Maintenance Department.

Once a pipe break is identified, MPA will determine its location relative to the defined COPR area. If the break is in a non-COPR fill area, the water from the leak and the excavation around the pipe break will be visually inspected for the presence of chromium by MPA Maintenance. If visual observations indicate presence of chromium, samples of the system water and groundwater will be collected and screened for total chromium and hexavalent chromium at the GWTP. If required, a duplicate sample will be sent to an EPA-certified laboratory for confirmation, with requested rush analysis. If (1) there is hexavalent chromium present or (2) the total chromium concentration in either the system water or the groundwater sample exceeds the MCL for drinking water Sections 3.2 through 3.7 of this plan must be implemented. The health and safety measures outlined in MPA's "Site Specific Health and Safety Plan for Utility Repair (Water Main Break)" and the "Standard Operating Procedure for Surface Cover Penetration" may be required. Otherwise, the response to the break can proceed as normal without following the requirements set forth in this plan for pipe break repair in the COPR area.

If the break occurs within the COPR fill area, precautions must be taken as presented in Sections 3.2 through 3.7. The health and safety measures outlined in MPA's "Site Specific Health and Safety Plan for Utility Repair (Water Main Break)" and the "Standard Operating Procedure for Surface Cover Penetration" will be followed. This plan does not replace those measures but simply provides additional clarification and actions that once completed will restore the potable water system.

3.2 Pipe Break Isolation

After a pipe break is identified, the section of broken water main that is leaking will be isolated by closing the two nearest valves. MPA Maintenance Department will maintain a plot plan of all valve locations to assist in the isolation effort.

3.3 User Notification

Simultaneously with isolation of the pipe break, potential users in the affected buildings will be immediately notified not to use the water system until further notification. A POC for every building with affected water service will be notified. The water supply to affected portions of the port will lose all of the water supply during the period that repairs and water testing are performed.

3.4 Alternate Water Supply

During the period of break isolation, repair, and return to service, an alternate supply of potable water service will be provided to the affected buildings.

3.5 Mitigation

Pipe break repair activities will begin in conjunction with users being notified. Pipe break repair activities will be conducted in a manner to limit the disturbance of COPR material and exposure of new and existing pipes to COPR material. All pipe repairs will be performed in accordance with the health and safety measures outlined in MPA's "Site Specific Health and Safety Plan for Utility Repair (Water Main Break)" and the "Standard Operating Procedure for Surface Cover Penetration." Once the section of broken pipe is repaired, the following process will be implemented in coordination with MPA DMT Maintenance:

- Identify a hydrant within the isolated section of pipe that can be used for flushing the repaired pipe.
- If the originally isolated section of pipe does not include a hydrant, additional valves will be closed in order to access a hydrant within the isolated portion of DMT.
- Open the hydrant to be used for flushing using the standard operating procedure included as Appendix A.
 Then open only one of the closed valves to pressurize the pipe, limiting the flow of water during flushing
 activities in one direction and restricting potentially contaminated water from moving to another portion of
 the pipe network. Run water until clear for 5 minutes using a sediment filter bag.
- Samples will be collected and screened for total chromium and hexavalent chromium at the GWTP, and a
 duplicate sample will be sent to an EPA-certified laboratory for confirmation with rush analysis. If the
 hexavalent concentration is greater than 1,100 ppb, the plan for discharging the flushing water requires
 temporary collection of the water in an appropriate collection and storage container. The flushing water will
 be transported for treatment to the GWTP after coordinating with the personnel managing the GWTP. If the
 GWTP is not operational, alternate temporary water storage and treatment/disposal, if required, will be
 coordinated with MPA Office of Environmental Quality.
- Identify the nearest appropriate storm drain that will receive the water from the identified flushing hydrant. Check that flushing of the hydrant will not cause localized flooding related to blockage of stormwater inlets.
- Set up a system for dechlorinating flushed water, such as a diffuser or inlet blankets with dechlorination tablets.
- Flow the hydrant until the system is free of air and sediment.
- Repeat flushing procedure for all of the closed valves needed for the repair. This ensures all lines are flushed and flow is maintained in only one direction, towards the isolated hydrant.

3.6 Sampling and Response

Once the water line break has been repaired and the new section of pipe has been adequately flushed, confirmation sampling will be conducted at the affected building(s) and submitted for laboratory analysis and screened at the GWTP in coordination with MPA Office of Environmental Quality.

3.7 Return to Normal Operation

After all confirmation sampling has demonstrated full mitigation of chromium contamination in the water system, the system can be returned to service. MPA will notify the POC of each affected building that the water system has been returned to service.



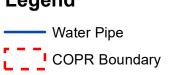
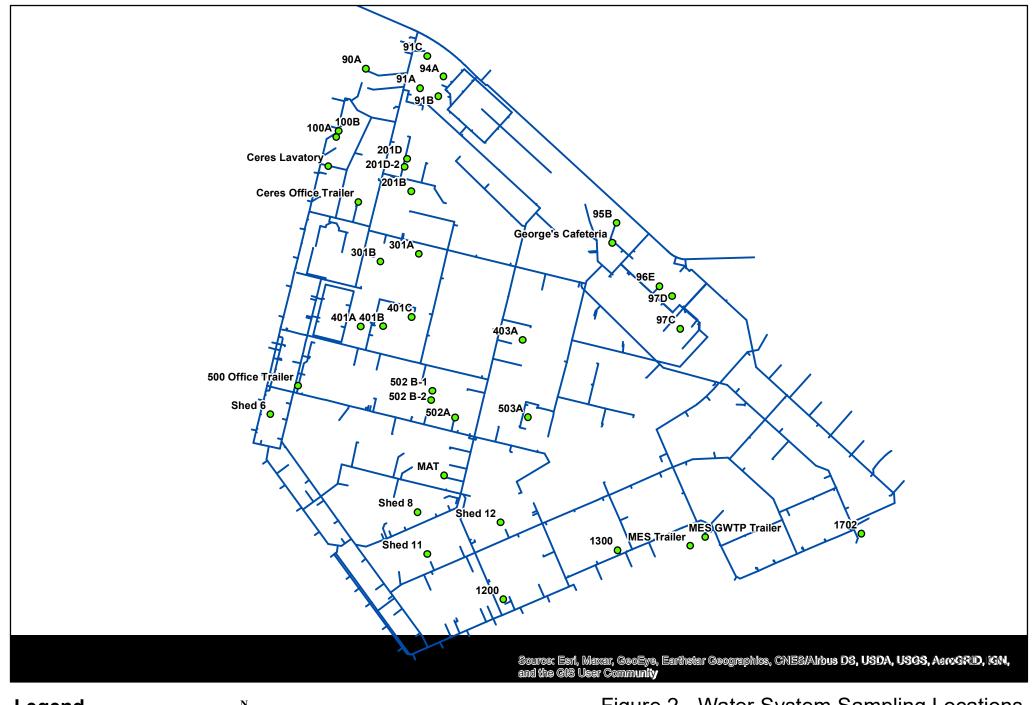




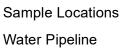


Figure 1 - Water System Configuration Dundalk Marine Terminal Baltimore, Maryland



Legend

Sample Locations



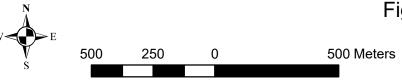
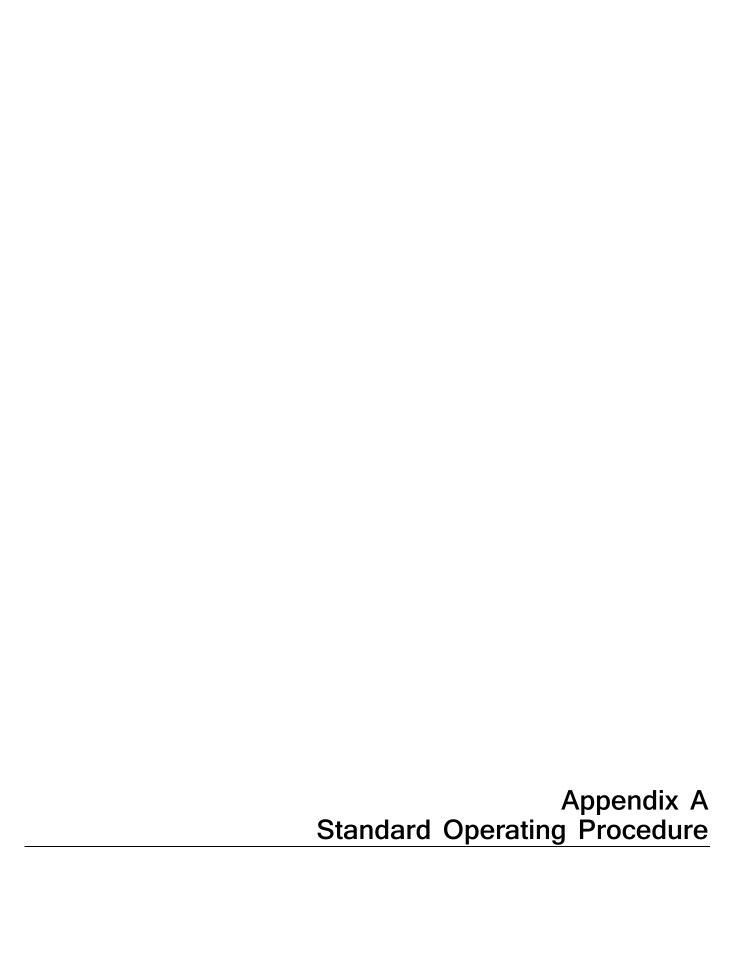


Figure 2 - Water System Sampling Locations **Dundalk Marine Terminal** Baltimore, Maryland



Standard Operating Procedure for Opening and Closing Hydrants during System Flushing

The operation of water distribution system hydrants may be necessary for flushing purposes as defined in the Site Drinking Water Plan. Under the specified conditions that require flushing activities, it is important that proper hydrant operating procedures be used to prevent the formation of hydraulic transients in the pipe network. These powerful hydraulic transients can cause pipe leaks and breaks. The following procedure should be followed to operate hydrants in a manner to prevent the formation of hydraulic transients.

- 1. Ensure that all the pertinent steps outlined in the Site Drinking Water Plan have been taken. These include but are not limited to following all MDE regulations and providing appropriate control of discharge water from the hydrant to prevent flooding.
- 2. After removal of the hydrant cap on the chosen nozzle to be used for flushing, the hydrant wrench should be secured tightly to the operating nut.
- 3. The hydrant should be opened slowly. Specifically, the hydrant nut should be turned at an even rate, not exceeding more than five revolutions per minute until the hydrant is fully opened or has reached the minimum required flow rate defined in the Site Drinking Water Plan.
- 4. After the flushing activity required in the Site Drinking Water Plan is achieved, the hydrants should be closed slowly. Specifically, the hydrant nut should be turned at an even rate, not exceeding more than five revolutions per minute until the hydrant is fully closed. After the hydrant is closed, the hydrant cap should be replaced on the hydrant nozzle.

Appendix E Maryland Port Administration Health and Safety Plan

MARYLAND PORT ADMINISTRATION MASTER HEALTH AND SAFETY PROGRAM FOR CHROMIUM-RELATED WORK PROJECTS

At:

Dundalk Marine Terminal

Maryland Port Administration

December 2022
KCI Technologies, Inc

KCI Job #0107160224 GE-3

TABLE OF CONTENTS

Abbreviations	iii
1.0 Background	1
1.1 Purpose	1
1.2 Site Description	1
1.3 Consent Decree Requirements and Tasks	2
1.4 Definition of Chromium-Related Work Projects	3
1.5 Health & Safety Roles and Responsibilities	3
1.5.1 MPA Management Team (MPA Team)	3
1.5.2 Primary Contractor (PC)	5
1.5.3 Secondary Contractors (SC)	6
1.5.4 MPA Employees Who Perform Work in COPR Area	6
1.5.5 Visitors	6
1.6 Applicable Health and Safety Regulations and Criteria	6
2.0 General Hazard Assessment	6
2.1 Health Effects of Cr (VI)	7
2.2 Exposure Assessment of Cr (VI) at DMT	8
2.2.1 Exposure Classifications	8
2.2.2 Mechanism to Determine Exposure Group Classification	9
3.0 Health and Safety Requirements for Each Cr (VI) Exposure Group	9
4.0 Procedures for Identifying Chromium-Related Work Projects	9
5.0 Air Monitoring Requirements for Chromium-Related Work	10
5.1 Personal Monitoring	10
5.2 Perimeter Monitoring	10
5.3 Method of Sampling and Analysis	10
5.4 Performance-Oriented Exposure Determination	11
5.5 Recordkeeping	11
6.0 Personal Protective Equipment (PPE) and Respiratory Protection	11
6.1 PPE	11
6.2 Respiratory Protection	12
7.0 Health & Safety Training Requirements	12

8.0 Medical Surveillance Requirements	13
9.0 Decontamination and Personal Hygiene	14
10.0 Emergency Response Procedures	14
10.1 Evacuation Procedures	17
Figure 1: Boundary of the COPR-fill area (as defined in April 2022)	2
Appendix A: Minimum Requirements for SSHASPs at Chromium-Related Work Projects	
Appendix B: Minimum Requirements for Potentially Chromium-Related Projects	
Appendix C: Standard Operating Procedures for Surface Cover Penetration	

Abbreviations

CM Construction Management

COPR Chromium Ore Processing Residue

Cr (III) Trivalent Chromium
Cr (VI) Hexavalent Chromium

DMT Dundalk Marine Terminal

EPA United States Environmental Protection Agency

HASP Health and Safety Plan
HAZMAT Hazardous Materials

HAZWOPER OSHA Standard for Hazardous Waste Operations and Emergency Response

MDE Maryland Department of the Environment

MDTA Maryland Transportation Authority
MES Maryland Environmental Service

MOSH Maryland Occupational Safety and Health Administration

MPA Maryland Port Administration

NEG Non-Exposure Group

OSHA US Occupational Safety and Health Administration

PC Primary Contractors, Tenants
PEG Possible Exposure Group
PEL Permissible Exposure Level

PPE Personal Protective Equipment

SC Secondary Contractors, Subcontractors, Consultants

SSHASP Site-Specific Health and Safety Plan

1.0 Background

1.1 Purpose

The purpose of this Maryland Port Administration (MPA) Master Health & Safety Plan (HASP) is to develop health and safety procedures and processes to address all chromium-related or potentially chromium-related work projects at the Dundalk Marine Terminal (DMT) pursuant to Section 9(d) of the DMT Consent Order dated April 4, 2006 (the "Consent Decree") entered into by the Maryland Department of the Environment (MDE), MPA, and Honeywell International, Inc (Honeywell).

MPA has developed the Master HASP to provide both general and specific requirements regarding potential exposure to chromium and chromium ore processing residue (COPR) at DMT. This program must be followed by all personnel performing chromium-related or potentially chromium-related work projects at DMT. This plan outlines the responsibilities of all parties operating on this site and requires coordination of all health and safety plans to ensure compliance with Maryland Occupational Safety and Health (MOSH) regulations and other applicable regulations. The Master HASP is intended to reduce the potential of health and safety hazards to personnel performing chromium-related or potentially chromium-related work projects at DMT.

It is a general requirement at DMT that all contractors, subcontractors, and tenants performing chromium-related work at the site exercise reasonable care to ensure that their employees, representatives, and visitors are following appropriate health and safety procedures. Each contractor, subcontractor, and tenant working at the DMT is responsible for the health and safety of its employees and authorized visitors. Each contractor, subcontractor, and tenant is also responsible for complying with the procedures in this document and for developing appropriate Site-Specific Health and Safety Plans (SSHASP) covering the activities on its site.

This Master HASP, including the minimum requirements for SSHASPs in Appendix A of this document, solely address hazards relating to COPR. Contractors, tenants, and the MPA are responsible for developing their own health and safety programs to address other hazards present or suspected in their workplaces. The MPA, depending on contractual agreement, may require a copy of contractors' and tenants' HASPs for review and acceptance by the MPA.

1.2 Site Description

The DMT, located at 2700 Broening Highway, is an active marine terminal consisting of approximately 600 acres split by the Baltimore City/Baltimore County boundary. The site perimeter of DMT consists of the Patapsco River, Colgate Creek, and Broening Highway. The DMT is an active marine terminal with a wide variety of operations and activities. The majority of DMT is covered by asphalt or other similar materials that act as a physical barrier covering the subsurface soil, which may consist of COPR or potentially COPR-contaminated soil.

The "COPR-fill area" refers to approximately 148 acres where COPR is known to have been used as fill on the DMT and is located beneath an asphalt cap or other similar barrier. The boundaries of the COPR-fill area are identified in Figure 1. COPR was also used as fill in some areas outside of the known COPR-fill area, and soil outside of the COPR-fill area may have been contaminated by groundwater from the COPR-fill area.

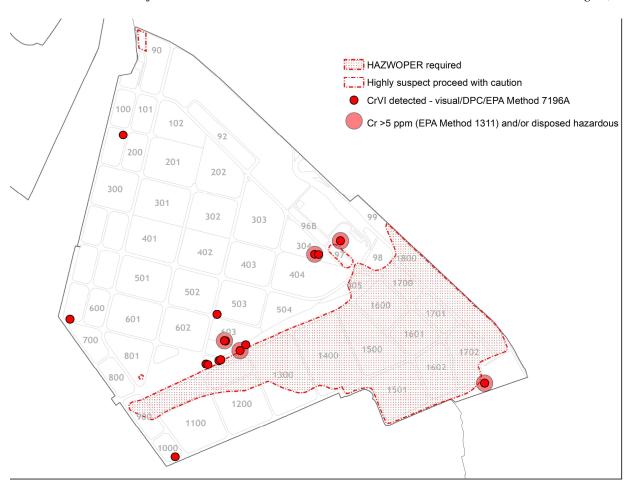


Figure 1: Boundary of the COPR-fill area, as defined in April 2022 and areas outside of COPR area where CrVI has been identified or suspected.

1.3 Consent Decree Requirements and Tasks

The Consent Decree is designed to protect the public health and the environment by providing a final remedy for addressing, treating, controlling, preventing, or mitigating the presence and/or releases or threatened releases of chromium at or from the DMT. Generally, the Consent Decree is a continuation of corrective measures taken pursuant to a 1992 administrative consent order between MPA and MDE. Under the 1992 administrative consent order, MPA undertook a variety of remedial measures, including, but not limited to: (i) the preparation of a Corrective Measures Implementation Project Plan; (ii) construction of outfall structures to serve as backflow preventers; (iii) rehabilitation of the storm water pipes; (iv) monitoring of groundwater and surface water flow; (v) a new plan for management of chromium contaminated soils excavated; and (vi) design and construction of extraction wells and a wastewater treatment plant to treat contaminated groundwater and dry weather flow in the storm drains. Under the updated Consent Decree, MPA and Honeywell will update and continue corrective measures commenced under the 1992 administrative consent order; perform certain interim corrective measures and studies; complete a Corrective Measures Alternatives Analysis to evaluate all permanent and/or long-term remedial alternatives; and implement all permanent or long-term corrective measures chosen by MDE within 17 years.

As part of the Consent Decree, MPA and Honeywell are required to develop and implement a master health and safety plan that addresses all chromium-related work projects at the site. This Master HASP and its attachments are designed to meet this requirement.

1.4 Definition of Chromium-Related Work Projects

<u>Chromium-Related Work Projects</u> – For the purposes of this HASP, chromium-related work projects shall include any work to be performed in the COPR-fill area at DMT that involves, or is reasonably likely to involve, contact with known or potential COPR or chromium-contaminated soil and water. This covers all sub-surface activities in the COPR-fill area. Additionally, work performed in electrical or communication manholes or vaults on the terminal is categorized as chromium-related due to the presence of chromium-contaminated groundwater.

<u>Potentially Chromium-Related Work Projects</u> – Any sub-surface work performed outside the boundaries of the COPR-fill area at DMT will be considered potentially chromium-related, and all contractors, subcontractors, and tenants must be aware of the potential for encountering COPR or chromium-contaminated soil or water and may request screening, sampling or other analytical methods as set forth in this Master HASP.

<u>Non-Chromium-Related Work Projects</u> – Work activities that are determined by screening, sampling, observations, planning, or historical data to have no reasonable probability of contacting chromium-contaminated soil or water will not be considered a chromium-related work project or activity.

1.5 Health & Safety Roles and Responsibilities

At the DMT, there may be multiple chromium-related projects running simultaneously and involving various companies and organizations. MPA personnel, contractors, subcontractors, and consultants routinely conduct maintenance, construction, and remedial activities, which may involve contact with chromium-contaminated materials. Each individual company and organization is responsible for its own health & safety program and procedures and, ultimately, the performance of its employees and subcontractors. There is a need, however, to establish a health & safety management system that ensures that each program meets minimum requirements, is coordinated to work with other health & safety programs, and is technically correct, reasonable, and effective.

The health & safety organization at DMT consists of five major groups:

- MPA Management Team (MPA Team)
- Primary Contractors or Tenants (PC)
- Secondary Contractors, Subcontractors and Consultants (SCs)
- MPA Employees who perform maintenance work in COPR areas
- Visitors

Responsibilities of each group will be defined in sections 1.5.1 - 1.5.5.

1.5.1 MPA Management Team (MPA Team)

The MPA management team (MPA Team) is the group that has project management oversight for all health and safety related matters and the Master HASP. The MPA Team consists of six groups:

- MPA Engineering
- MPA Construction Management (CM)
- MPA Safety and Risk Management Department
- MPA Office of Environmental Quality
- MPA DMT Security
- MPA-approved consultant support, including Maryland Environmental Service (MES)

The overall role of the MPA Team is to ensure that procedures are in place and being implemented to:

- Protect workers, the public, and the environment from possible chromium-related contamination associated with activities being conducted on site.
- Provide security for the DMT.
- Establish health and safety management systems to maintain regulatory compliance, worker safety, and environmental protection.

<u>MPA Engineering</u> - MPA Engineering has overall project authority for MPA construction projects. MPA Engineering has the following health and safety responsibilities:

- Developing clear project specifications identifying work;
- Designating health and safety responsibilities (e.g., PCs and SCs for each project and/or project phase);
- Providing direction in disputes that involve health and safety issues;
- Communicating and meeting regularly with the MPA Team regarding health and safety issues;
- Serving as the liaison with public officials as required.

<u>MPA Construction Management (CM)</u> – CM's primary job function is to provide direction and construction oversight of site activities to determine compliance with the project work plan and other project performance expectations. Specific health and safety responsibilities may include:

- Becoming knowledgeable of the health and safety requirements, procedures, organization and responsibilities of the project and the companies/organizations involved;
- Receiving and maintaining health and safety documentation which includes: health and safety
 plans, personnel training, job and medical qualification documents, incident reports, and other
 health and safety documentation as appropriate;
- Obtaining and distributing SSHASP and other related health and safety documentation to appropriate persons:
- Documenting daily health and safety oversight activities and providing feedback to appropriate parties;
- Terminating work for violation or discrepancy with agreed-to health and safety work plans;
- Communicating and meeting regularly with the MPA Team regarding health and safety issues.

MPA Safety and Risk Management Department/MPA Office of Environmental Quality - Safety has responsibility for health and safety and emergency response for the MPA. Specific health and safety responsibilities related to the Master HASP include:

- Reviewing and accepting or rejecting all chromium-related contractor health and safety plans, including the SSHASP, and providing comments to appropriate persons; major changes must also be reviewed by the MPA Safety Department;
- Providing health and safety advice or consultation on projects or work activities where MPA Team is taking the role of the PC or MPA employees are performing the work;
- Providing health and safety oversight, in conjunction with CM, of contractor health and safety program implementation;
- Conducting Health Hazard Evaluations and oversight as necessary;
- Determining health and safety actions as required;
- Leading Incident Investigations, as needed;
- Providing health and safety technical resources and support as needed;
- Communicating and meeting regularly with the MPA Team regarding health and safety issues;
- Conducting sampling for chromium in soil and water;
- Serving as the liaison with public officials when directed by MPA Engineering.

<u>MPA DMT Security</u> – DMT Security consists of the Maryland Transportation Authority (MDTA) Police, a law enforcement agency, and the MPA Security Department and its subcontractor, a guard force. DMT Security health and safety responsibilities include:

- Managing the overall DMT security program;
- Managing the site emergency response program; all emergencies must be coordinated through the MDTA Police Dispatch (410.537.7911).

<u>MPA-approved Health and Safety Support (MES)</u> – MES is an MPA PC at DMT that provides a variety of site environmental and maintenance services as directed by MPA. For the majority of work onsite, MES must meet the health and safety responsibilities of the PC as detailed in Section 1.5.2. MES also provides a technical support role as directed by the MPA. This support role includes:

- Assisting with sampling for chromium in soil and water;
- Coordinating with sub-contractors for hazardous waste handling.

1.5.2 Primary Contractor (PC)

MPA Engineering will formally designate a PC for each project or project phase prior to the onset of field project activities. The PC is responsible for coordinating overall project site health and safety activities within its area of control, so that all groups associated with the project can successfully integrate their SSHASP. The PC's SSHASP must address all the construction type hazards that may apply to the project task, as well as the potential hazards associated with possible chromium contamination. The SSHASP of SCs must be integrated with the plans and procedures of the PC, when necessary. No project activity may begin until the PC's SSHASP, including integration of the SC's SSHASP, has been reviewed and accepted by the MPA. The PC's SSHASP must meet the requirements of the MOSH/OSHA Hazardous Waste Standard (29 CFR 1910.120 (b)) and other applicable MOSH/OSHA, United States Environmental Protection Agency (EPA), and MDE standards and regulations, as well as this Master HASP. As part of this plan, the PC must ensure that the plans and procedures of all other designated SCs are coordinated and integrated.

1.5.3 Secondary Contractors (SC)

Secondary Contractors (SC) are designated for a specific project or a specific phase of a chromium-related project. The SC can consist of one or more or the following: engineering and environmental consultants, construction contractors or subcontractors, transportation contractors and/or other support organizations for a designated project.

1.5.4 MPA Employees Who Perform Work in COPR Area

MPA maintenance employees are required to perform utility repairs and other sub-surface work at the DMT in accordance with a SSHASP that meets the minimum requirements set forth in this Master HASP.

1.5.5 Visitors

Any personnel onsite who may be involved with chromium-related projects and are not part of the MPA Team, PC, or SC is considered a Visitor as part of this health and safety procedure. Visitors may not enter exclusion zones or hazardous areas. Anybody who is required to enter an exclusion zone must be classified as an SC and must meet the training and other requirements of an SC under this Master HASP. All visitors must meet the requirements set by MPA Security.

Visitors fall into one of two categories:

- Escorted Visitors This category includes people that come to DMT and are authorized to be onsite at a potentially chromium-related project for a specific limited purpose. They must always have an escort and are limited to site areas and operations that are classified as non-hazardous.
- Restricted Visitors This category includes people that come to DMT and are authorized to be at a
 chromium-related project site for a specific purpose that may require visiting for an extended period
 of time. They do not always need an escort if they are limited to site areas and operations that are
 classified as non-hazardous.

1.6 Applicable Health and Safety Regulations and Criteria

- Chapter 01 Hazardous Waste Management System: General Authority: Environment Article, Title 7, Subtitle 2, Annotated Code of Maryland
- Maryland Occupational Safety and Health (MOSH) Occupational Health and Safety Administration (OSHA), Title 29
- 29 CFR 1926 Construction (including but not limited to)
 - o Subpart D, Section 65, Hazardous Waste Operations and Emergency Response
 - o Subpart Z, Section 1126, Occupational Exposure to Chromium (VI)
- 29 CFR 1910 General Industry (including but not limited to)
 - o Subpart H, Section 120, Hazardous Waste Operations and Emergency Response
 - o Subpart Z, Section 1026, Occupational Exposure to Chromium (VI)
- 40 CFR 260-282 Hazardous Waste Management

2.0 General Hazard Assessment

Chromium compounds are present in COPR, which is located primarily in areas of DMT shown on the MPA COPR-fill area site map (Figure 1). Chromium compounds may also be present in groundwater or in soil that has come into contact with chromium-contaminated groundwater. Potential exposure to chromium is possible when the areas containing COPR or chromium-contaminated soil or water are disturbed during

construction activities. To minimize the potential for exposure, engineering controls and work practices have been put into place.

The COPR contains both trivalent (Cr (III)) and hexavalent chromium (Cr (VI)). The primary chemical of concern is Cr (VI) since the OSHA permissible exposure limit (PEL) for Cr (VI) is 5 micrograms/cubic meter of air (μ g/m³). This is 100 times lower than the PEL for Cr (III), which is 500 μ g/m³. Methods implemented to minimize exposure to Cr (VI) are focused on controlling exposure to COPR dust and will provide appropriate protection for exposure to Cr (III) as well as Cr (VI).

2.1 Health Effects of Cr (VI)

The primary means of exposure to Cr (VI) are inhalation, ingestion, and skin contact with Cr (VI)-containing dust. Cr (VI) may be inhaled when dust containing Cr (VI) is present in the air. Particles of Cr (VI)-containing dust may contaminate hands, clothing, hair, food, and beverages. MOSH/OSHA has established a PEL for Cr (VI) of 5 μ g/m³ (8-hour, time-weighted average) with an action level (AL) of 2.5 μ g/m³. The MPA Master HASP and SSHASPs are designed to keep worker exposure well below the recognized OSHA PEL for Cr (VI) and to prevent accidental ingestion or adverse dermal effects.

OSHA considers the primary health effects from workplace overexposure to Cr (VI) as lung cancer, nasal irritation, nasal tissue ulcerations, nasal septum perforations, asthma, eye irritation, and damage to the skin. The information that OSHA used to define these health effects and establish the PEL was based on occupational health studies from many different industries including chromium ore mining and processing; chromium compound manufacturing and application (including pigments and paints); stainless steel production and welding; and chromic acid manufacturing and use. No operations handling COPR were included in the OSHA investigation. The type of and mechanism for exposure to Cr (VI) through COPR may be different from those studied by OSHA. Below is a summary of the health effects from overexposure to Cr (VI).

The scientific literature documents that at very high exposure concentrations, Cr (VI) may increase the risk of developing lung cancer. It is not anticipated that Cr (VI) exposures at DMT will be at these high concentrations or at *overexposure* concentrations exceeding those considered acceptable by MOSH.

Occupational overexposure to Cr (VI) can lead to nasal tissue ulcerations and nasal septum perforations through direct "hand to nose" contact in addition to inhalation of Cr (VI), which can contaminate the nasal passages. The nasal septum separates the nostrils and is composed of a thin strip of cartilage. The nostril tissue consists of an overlying mucous membrane known as the mucosa. The initial lesion after Cr (VI) exposure is characterized by localized inflammation or a reddening of the affected mucosa, which can later lead to atrophy. This may progress to an ulceration of the mucosa layer upon continued overexposure. If exposure is discontinued, the ulcer progression will stop, and a scar may form. If the tissue damage is sufficiently severe, it can result in a perforation of the nasal septum, sometimes referred to as a chrome hole. Individuals with nasal perforations may experience a range of signs and symptoms, such as a whistling sound, bleeding, nasal discharge, and infection. Some individuals may experience no noticeable effects.

Cr (VI) is considered an airway sensitizer. Airway sensitizers cause asthma through an immune response. The sensitizing agent initially causes production of specific antibodies that attach to cells in the airways. Subsequent exposure to the sensitizing agent, such as Cr (VI), can trigger an immune-mediated narrowing

of the airways and onset of bronchial inflammation. Not all workers overexposed become sensitized to Cr (VI), and the asthma only occurs in sensitized individuals.

OSHA has stated that it is not clear what occupational exposure levels of Cr (VI) compounds lead to airway sensitization or the development of occupational asthma. Occupational asthma is considered "a disease characterized by variable airflow limitation and/or airway hyper-responsiveness due to causes and conditions attributable to a particular occupational environment and not to stimuli encountered outside the workplace." Asthma is a serious illness that can damage the lungs and in some cases be life threatening. The common symptoms associated with asthma include heavy coughing while exercising or when resting after exercising, shortness of breath, wheezing sound, and tightness of chest.

Occupational overexposure to Cr (VI) is a well-established cause of adverse health effects of the skin. Direct dermal contact with Cr (VI) is the most relevant factor in the development of dermatitis and ulcers. The mildest skin reactions consist of erythema (redness), edema (swelling), papules (raised spots), vesicles (liquid spots), and scaling. Cr (VI) compounds can also have a corrosive, necrotizing effect (i.e., causing cell death) on living tissue, forming ulcers, or "chrome holes".

Chrome ulcers generally occur on areas of the body exposed to Cr (VI), chiefly on the hands and forearms. The lesions are initially painless and are often ignored until the surface ulcerates with a crust, which, if removed, leaves a crater two to five millimeters in diameter with a thickened, hardened border. The ulcers can penetrate deeply into tissue and become painful. Chrome ulcers may penetrate joints and cartilage. The lesions usually heal in several weeks if exposure to Cr (VI) ceases, leaving a scar. If exposure continues, chrome ulcers may persist for months. Some individuals may develop an allergic sensitization to Cr (VI). In sensitized workers, contact with even small amounts may cause a serious skin rash.

2.2 Exposure Assessment of Cr (VI) at DMT

In compliance with the MOSH/OSHA Cr (VI) Standard, a combination of historical and current exposure monitoring data has been used to define the potential for exposure for all workers on the DMT site. Between 2007 and 2009, 843 air samples were collected and analyzed for Cr (VI) and total dust. The results, methodologies, and discussions are included in "Assessment of Potential Human Exposure to Hexavalent Chromium at Dundalk Marine Terminal," dated August 7, 2009, which is available from the MPA Safety Department. Since 2009, an additional 543 samples have been collected and analyzed for Cr (VI).

Based on the results of this study and confirmed by subsequent air samples, it has been determined that individuals working at sub-surface sites in the COPR-fill area are at risk for some exposure to Cr (VI). Most, if not all, of these individuals are expected to be exposed at concentrations below the OSHA PEL. All other individuals working at the DMT, including contractors, tenants, MPA employees, and visitors are expected to have very low to no exposure to Cr (VI).

2.2.1 Exposure Classifications

All individuals who may work or visit DMT will be categorized into one of two exposure categories:

<u>Possible Exposure Group (PEG)</u> - Any individual who is working specifically at a site in the COPR-fill area (Figure 1) where the asphalt cap has been penetrated will be included in the PEG. Additionally, any individual working at DMT outside of the COPR-fill area where the asphalt cap has been penetrated will be included in the PEG if, in the opinion of the PC or MPA Safety Department, there is reason to believe

that soil or groundwater may be contaminated with Cr (VI). When deciding whether workers at a site outside of the COPR-fill area should be classified as PEG, the PC or MPA Safety Department will use historical data, results of soil testing or field screening, proximity to the COPR-fill area, and/or the visual appearance of the soil.

<u>Non-Exposure Group (NEG)</u> - All individuals whose work at DMT does not involve direct contact with chromium-contaminated soil or water are considered to have an extremely low probability of exposure to Cr (VI).

2.2.2 Mechanism to Determine Exposure Group Classification

The PC is responsible for the classification of workers and others on its work site. The workers will be categorized as described in Section 2.2. 1. A strategy for defining these exposure groups may be found in A Strategy for Assessing and Managing Occupational Exposures, 4th edition (AIHA, 2015).

3.0 Health and Safety Requirements for Each Cr (VI) Exposure Group

<u>Possible Exposure Group (PEG)</u> - This group must meet specific requirements for personal protective equipment (PPE) (Section 6.1), training (Section 7.0), medical surveillance (Section 8.0) and decontamination to mitigate possible exposure to Cr (VI).

<u>Non-Exposure Group (NEG)</u> - The requirements for this group are limited to site orientation and exclusion from hazardous areas.

4.0 Procedures for Identifying Chromium-Related Work Projects

All work at DMT can be divided into three classifications:

<u>Chromium-related work</u> - All sub-surface work performed within the delineated COPR-fill area is considered *chromium-related work*. Additionally, sub-surface work conducted at any site where soil has been found to contain greater than 50 mg/kg Cr (VI) will be considered *chromium-related work*, regardless of the site location. This 50 mg/kg threshold is based on sampling data analyzed in "Analysis of Solid Data for COPR Monitoring Sites", dated December 9, 2008, available from MPA Safety Department. This threshold may be updated as additional data is made available. General requirements for *chromium-related work* can be found in Sections 5 – 10 and Appendix A of this Master HASP.

<u>Potentially chromium-related work</u> - Since COPR may have been used as fill in various locations outside of the delineated COPR-fill area, and because soil outside of the COPR-fill area may have been impacted by chromium-contaminated groundwater, the MPA has taken the position that all sub-surface work outside of the COPR-fill area will be considered *potentially chromium-related work*. Requirements for *potentially chromium-related work* can be found in Appendix B of this Master HASP.

<u>Non-chromium-related work</u> - Work that takes place above the asphalt cap and that is not likely to involve direct contact with chromium-contaminated soil or ground water is considered *non-chromium-related work*.

Methods for soil testing and classifying excavation sites as *chromium-related* or *potentially chromium-related* may be found in Attachments B and C of this Master HASP.

5.0 Air Monitoring Requirements for Chromium-Related Work

MPA requires all contractors and subcontractors to perform air monitoring for all chromium-related work for which exposures have not been previously characterized. The contractors' and subcontractors' proposed air-monitoring program must be submitted to MPA for approval prior to initiating work activities. Except for tasks for which previous monitoring has adequately characterized exposures, air monitoring must be performed to determine the zone delineation within the project work area and to ensure that employees are not exposed above the OSHA PEL to Cr (VI).

For sites where exposures have not been adequately characterized, air samples for Cr (VI) will be collected at the perimeter of the exclusion zone and from the breathing zones of employees working within the exclusion zone. In addition, real-time total particulate measurements will be monitored at the exclusion-zone perimeter downwind of the work area.

5.1 Personal Monitoring

Personal air sampling will be conducted to comply with exposure assessment requirements set forth in 29 CFR 1910.120 and 29 CFR 1910.1026. The results of this monitoring will be compared to the OSHA AL and PEL for Cr (VI) (2.5 μ g/m³ and 5 μ g/m³, respectively). Personal air sampling will be conducted in a manner sufficient to characterize employee's exposures for all tasks within a project.

5.2 Perimeter Monitoring

Perimeter monitoring will consist of real-time total dust monitoring along with sampling for Cr (VI) at the perimeter of the exclusion zone. The need for perimeter monitoring during tasks for which exposures have been adequately characterized will be determined by the Manager, Safety and Risk Management.

Real-time total dust monitoring will be conducted at the perimeter of the exclusion zone, downwind of the work area, during all work that takes place inside of the exclusion zone. The real time measurements of total dust will be compared against the MPA site specific total dust AL of 1.0 mg/m³. This AL is a concentration of total dust below which it is considered unlikely that the PEL for Cr (VI) would be exceeded. When this AL is exceeded, additional dust suppression controls must be implemented. If additional suppression is not effective, the work must be stopped.

The AL has been calculated using the 95th percentile concentration of Cr (VI) in airborne dust at sites where the asphalt cap has been penetrated. The supporting documentation for this AL, "Re-Evaluation of Maryland Port Administration's Action Level of 1.0 mg/m³ for Total Dust", dated March 31, 2009, is available from MPA Safety and Risk Management.

Air samples will be collected at the perimeter to assess the effectiveness of the AL and to ensure that people working outside of the exclusion zone are not exposed to Cr (VI) above the OSHA AL of $2.5 \,\mu\text{g/m}^3$.

5.3 Method of Sampling and Analysis

Air samples will be collected and analyzed using OSHA Method ID-215 and NIOSH Method 0500. An air sample will be collected using a pre-weighed, 37-mm diameter polyvinyl chloride (PVC) filter (5µm pore size) contained in a polystyrene cassette. A calibrated sampling pump is used to draw a representative air sample from the breathing zone of an individual through the cassette and collect particulate on the filter. A post-sampling weight is determined for the filter, providing a measure of total particulate. The amount of

Cr (VI) is determined using ion chromatograph with UV-visible detection. The sample analyses will be conducted by a laboratory accredited for these methodologies by the American Industrial Hygiene Association.

5.4 Performance-Oriented Exposure Determination

Requirements for air sampling may be met using any combination of air monitoring data, historical monitoring data or objective data sufficient to accurately characterize employees' exposure to Cr (VI). Any historical data must be from a sufficiently similar work site to ensure that the data are applicable. Because exposure is likely to vary greatly depending on the type of work performed, the concentration of Cr (VI) in soil, shift duration, the equipment and work practices used, care must taken to ensure that any historical exposure data adequately represent conditions likely to be present at the current site. Additional monitoring will be required if work practices or site conditions change.

5.5 Recordkeeping

Information documented during personal sampling will include:

- Pre-sampling flow rate
- Post-sampling flow rate
- Field observations
- Calculations and chain of custody forms
- Analytical results
- Work activities being performed
- Location of the sample relative to the location of intrusive activity
- Wind and other relevant weather conditions

6.0 Personal Protective Equipment (PPE) and Respiratory Protection

6.1 PPE

Employees in the PEG must, at a minimum, use the following PPE when working within the exclusion zone:

Modified Level C

- Disposable protective coveralls when working with liquids, use impervious coveralls
- Half-face or full-face negative pressure respirator
- Inner and outer gloves
- Eye protection
- Rubber boots, when working with liquids

All PPE must meet or exceed the following standards:

Eye and Face: 29 CFR 1910.133 29 CFR 1926.102 ANSI Z87.1-1989

Respiratory: 29 CFR 1910.134 29 CFR 1926.103 ANSI Z88.1-1992

Head: 29 CFR 1910.135 29 CFR 1926.100 ANSI Z89.1-2003

Foot: 29 CFR 1910.136 29 CFR 1926.96 ANSI Z41.1-1991

ANSI = American National Standards Institute

6.2 Respiratory Protection

With the exception of tasks for which exposures have been adequately assessed in accordance with Section 5.4 and found to be below the PEL, workers in the PEG must be supplied with respiratory protection as part of a written Respiratory Protection Program that complies with the OSHA respiratory protection standard (29 CFR 1910.134). Only approved respiratory protective equipment that has been properly selected for the project will be used. Employees will be instructed on the selection and proper fit, maintenance procedures of equipment, and warning signs of respirator failure. The use of all respiratory protective equipment will conform to the manufacturer's operating instructions and training provided to the employee. In all environments where it has been determined that respiratory protection is necessary, additional ambient air monitoring may be warranted. All personnel on-site must be properly fit-tested for each type of respiratory protection they will use.

All individuals required to use respiratory protection must successfully pass a medical evaluation and receive written approval from the physician or licensed health care provider. The written approvals for all site personnel must be maintained by the PC.

Respirators will not be worn when conditions exist which prevent a good face to face piece seal. These conditions include, but are not limited to, the growth of a beard or sideburns, a skull cap which projects under the face piece, or the use of regular corrective glasses because the temple bards prohibit a proper seal. Also, the absence of one or both dentures can seriously affect the fit of any respiratory protection.

All respirators must be cleaned and disinfected at a frequency necessary to ensure that the proper protection is provided to the wearer. Respirators used by more than one worker must be cleaned and disinfected after each use. All respiratory protection must be stored in a convenient, clean, and sanitary location and according to specific manufacturer recommendations. Special attention must be paid to protecting respiratory protection from dusty conditions, temperature extremes, and potential contamination during storage.

All respiratory protection equipment used on a routine basis will be inspected prior to use. Worn or deteriorated parts must be immediately replaced or the respirator must be tagged with a "Do Not Use" sign and taken out of service. The inspection procedure for each type of equipment will follow the manufacturer's recommended procedure.

7.0 Health & Safety Training Requirements

MPA requires that all employees in the PEG must receive a minimum of 40-hour training as detailed in the OSHA HAZWOPER standard (29 CFR 1910.120). Additionally, the PC is responsible for providing a site-specific initial training to all employees, visitors, and sub-contractors prior to allowing onto the work site. This training will include description of the potential hazards at the site and controls in place to protect individuals on the site. Additional training may be required depending on the work performed and potential

hazards. Additional training may include confined space entry and lockout/tagout procedures. The PC will provide, in writing, to the MPA the names of the employees trained, the training date, training certifications, and the specific training. Copies of certifications or training records may be requested by the MPA.

In addition to health and safety training, all employees who handle hazardous waste must receive Resource Conservation and Recovery Act (RCRA) training, in accordance with 40 CFR 214.16 and 215.16. Additionally, all employees who sign hazardous waste manifests, or who label hazardous waste for transportation must receive Department of Transportation (DOT) training in accordance with 49 CFR 172.702.

8.0 Medical Surveillance Requirements

Medical surveillance must be made available to workers in the following categories:

HAZWOPER Medical Surveillance Requirements as indicated for General Industry in 29CFR1910.120 and Construction in 29CFR1926.65:

- Workers who are or may be exposed to hazardous substances or health hazards at or above the established permissible exposure limit, above the published exposure levels for these substances, without regard to the use of respirators, for 30 days or more a year;
- Workers who are injured, become ill or develop signs or symptoms due to possible overexposure
 involving hazardous substances or health hazards from an emergency response or hazardous waste
 operation; or
- Members of HAZMAT teams.

Hexavalent Chromium Standard Medical Surveillance Requirements as indicated for General Industry in 29CFR 1910.1026 and Construction in 29CFR 1926.1126:

- Workers with the potential for exposure to Cr (VI) above the action level for 30 days or more per year;
- Workers who experience signs or symptoms of the adverse health effects associated with Cr (VI) exposure; or
- Workers who are exposed to Cr (VI) in an emergency.

Respiratory Protection Standard Medical Evaluation Requirements as indicated for General Industry and Construction in 29CFR 1910.134:

• Workers that are required to wear a respirator (prior to being fit tested or using the respirator in the workplace)

Employers will make available the appropriate medical examination as required by the OSHA. All consultations or examinations will be performed by or under the supervision of a physician or licensed health care provider. Employers will establish and maintain an accurate record for each employee who was either offered or provided with medical surveillance. These records are considered to be medical records and the employer will maintain them and make them available in accordance with OSHA Access to Medical Records Standard, 29 CFR 1910.1020.

The PC will provide, in writing, the names of the employees required to participate in any of the above medical evaluation programs and certify that the employees named are physically qualified to work in the chromium-contaminated areas and wear respirators.

9.0 Decontamination and Personal Hygiene

SSHASPs for chromium-related projects must include a description of procedures and equipment that will be used to decontaminate personnel and equipment. At a minimum, this description will include who is responsible for establishing and overseeing decontamination equipment and procedures and methods for containing water used during decontamination.

PCs must provide potable water and hand washing facilities at chromium-related project sites. PCs must establish personal hygiene procedures in their SSHASPs, requiring employees to eat, smoke, drink, or chew tobacco or gum only in designated areas and only after washing their face and hands.

10.0 Emergency Response Procedures

All field personnel working in the DMT will receive health and safety training prior to the initiation of any site activities. On a day-to-day basis, individual personnel should be constantly alert for indicators of potentially hazardous situations and for signs and symptoms in themselves and others that warn of hazardous conditions and exposures.

Rapid recognition of dangerous situations can avert an emergency. Before daily work assignments, regular toolbox safety meetings should be held. Discussion should include:

- Tasks to be performed, Health and Safety Hazard Analysis;
- Time constraints (e.g. rest breaks, respirator cartridge changes);
- Hazards that may be encountered, including their effects, how to recognize symptoms or monitor them, concentration limits or other danger signals;
- Wearing, use and fitting requirements of site PPE;
- Emergency procedures, points of contact, muster points;
- Potential residential exposure issues;
- Weather conditions; and
- Recent incidents, near misses and at-risk behaviors.

An Emergency Contingency Plan must be developed to address any potential emergencies that may occur while working at the DMT. A contingency plan is a written document, established before work on the site begins, outlining policies and procedures for responding to, handling, and reporting emergencies. Potential emergencies include:

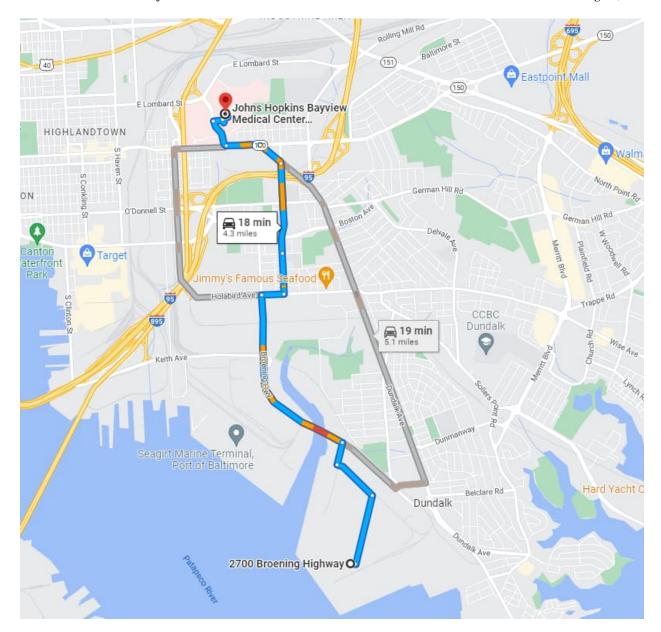
- Medical emergency
- Chemical exposure
- Structural failure
- · Hazardous materials release
- Fire or explosion
- Severe weather

• Civil disturbance

When an emergency occurs, decisive action is required. Delays of minutes can increase life-threatening situations. A response needs to be immediate and accurate. Emergency situations can be characterized as a fire, explosion, serious weather conditions, a hazardous environmental release, or accident or injury to personnel. The MPA has emergency procedures that apply to DMT. These procedures will be reviewed and made available to PC prior to starting work. The PC must develop an emergency contingency plan for its work site; the PC's contingency plan should incorporate the MPA's plans.

In the event of emergency, all communications must be routed through the MDTA police at (410) 537 7911. If 911 is called during an emergency, the caller must notify the MDTA police.

In the event of a medical emergency, the nearest Emergency Room (ER) is at the Johns Hopkins Bayview Medical Center located at 4940 Eastern Ave, Baltimore, MD 21224. Below is a map with turn-by-turn directions to the ER:



- Turn left onto Broening Hwy 1.4 mi
- Turn right onto Holabird Ave 0.2 mi
- Turn left onto Charlotte Ave 0.3 mi
- Continue onto Gusryan St 0.6 mi
- Turn left onto Dundalk Ave 0.2 mi
- Use any lane to turn slightly left onto Eastern Ave 0.2 mi
- Turn right onto Bayview Blvd 0.2 mi
- Continue on Hopkins Bayview Cir to your destination (0.2 mi)
- Turn right onto Hopkins Bayview Cir 0.1 mi
- Turn right: Destination will be on the right

10.1 Evacuation Procedures

PC shall develop an Evacuation Plan that provides an orderly and safe means to evacuate persons working or visiting its worksite. An evacuation may result from a weather event, a HAZMAT incident, a fire, an act of terrorism, or other emergencies. The PC is responsible for determining muster points for its personnel. A copy of the plan should be included in the PC's SSHASP.

The plan must include plans for marine terminal evacuation as well. The current MPA Marine Terminal Evacuation Plan is available from Safety and Risk Management.

Appendix F DMT NPDES Permit



Larry Hogan Governor

Boyd Rutherford Lieutenant Governor

Ben Grumbles Secretary

AUG 2 4 2517

CERTIFIED MAIL

Bill Richardson, Environmental Maryland Port Administration 2700 Broening Highway Baltimore, MD 21224

Re: State Discharge Permit No. 10DP3060, NPDES Permit MD0066818

Dear Mr. Richardson,

Enclosed is the issued discharge permit referenced above with the effective date indicated on the cover page. The permittee is responsible for complying with all permit conditions. You are therefore advised to read the permit carefully and become thoroughly familiar with the requirements.

The U.S. Environmental Protection Agency (EPA) recently promulgated a final rule to modernize Clean Water Act reporting for municipalities, industries, and other facilities by converting to an electronic data reporting system (see 40 CFR 127.16). Under the final rule, any Discharge Monitoring Reports (DMRs) to be submitted on or after <u>December 21, 2016</u> must be electronically reported to the Department.

Thus Maryland Department of the Environment now requires use of NetDMR for filing your required NPDES DMRs. NetDMR is a freely available Web based tool that allows NPDES permittees to electronically sign and submit their DMRs to EPA via a secure internet connection. NetDMR is designed to improve data quality, reduce reporting liabilities, save paper, and provide cost savings. It allows participants to discontinue mailing in hard copy forms under 40 CFR 122.41 and 403.12. For more information go to the EPA website (www.epa.gov/netdmr) or call the MDE Water Management Administration, Compliance Program, at 410-537-3520 and ask to speak to a NetDMR coordinator.

As indicated in Condition II.A.2 of your permit, before you can submit official DMRs using NetDMR you must attend a training Webinar and successfully set-up and submit test monitoring results electronically. If you do not attend the required training in a timely manner, you will be at risk of violating the new U.S. EPA NPDES electronic reporting rule.

Enclosed is also a copy of the Federal Register, Part 136 - "Guidelines Establishing Test Procedures for Analysis of Pollutants". Unless otherwise specified, these guidelines are to be used for the analyses required by this permit. The most current version of 40 C.F.R. Part 136 can be found online at

Bill Richardson Page 2

EPA's website (<u>www.epa.gov/epahome/cfr40.htm</u>). Finally you'll find enclosed a brochure for NetDMRs.

Please direct all future correspondence regarding permit compliance to the following address:

Attention: Discharge Monitoring Reports
Water and Science Administration – Compliance Program
Maryland Department of the Environment
1800 Washington Boulevard, Suite 425
Baltimore, Maryland 21230-1708

If you have any other questions, please do not hesitate to call Paul Hlavinka, Industrial and General Permits Division, at 410-537-3323.

Sincerely,

D. Lee Currey, Director

Water and Science Administration

LB:aw

Enclosures (3)

Cc: William Lee (delivered electronically)
WSA-Compliance, Central Division Chief

Virginia of Keainey/ for



Larry Hogan Governor

Boyd Rutherford Lieutenant Governor

Ben Grumbles Secretary

STATE DISCHARGE PERMIT NUMBER	10-DP-3060	NPDES PERMIT NUMBER	MD0066818
EFFECTIVE DATE	October 1, 2017	EXPIRATION DATE	September 30, 2022
MODIFICATION DATE:	N/A	REAPPLICATION DATE	September 30, 2021

Pursuant to the provisions of Title 9 of the Environment Article, <u>Annotated Code of Maryland</u>, and regulations promulgated thereunder, and the provisions of the Clean Water Act, 33 U.S.C. § 1251 <u>et seq</u>. and implementing regulations 40 CFR Parts 122, 123, 124, and 125, the Department of the Environment, hereinafter referred to as the "Department," hereby authorizes

Maryland Port Administration 401 E. Pratt Street, Suite 1653 Baltimore, Maryland 21202

TO DISCHARGE FROM

a municipal separate storm sewer system (MS4) containing infiltrated groundwater and a wastewater treatment system at a terminal and cargo handling facility

LOCATED AT

2700 Broening Highway, Baltimore, Maryland

VIA OUTFALLS

001, 002, 004, 005, 006, and 013 as identified and described herein

TO

Patapsco River which is protected for (Use II) water contact recreation, fishing, aquatic life, wildlife in accordance with the following special and general conditions and map(s) made a part hereof.

I. SPECIAL CONDITIONS

A.1. EFFLUENT LIMITATIONS AND MONITORING REQUIREMENTS

During the effective period of this permit, the permittee is authorized to discharge treated effluent, including but not limited to dry weather flows from the Outfall 005 and Outfall 006 storm drain systems, via the wastewater treatment system to Outfall 001 (Maryland Coordinates 1446.82E and 574.45N).

As specified below, such discharge shall be limited and monitored by the permittee at the sampling port in the onsite treatment plant.

	QUANT	QUANTITY OR LOADING		QUALITY OR CONCENTRATION				FREQUENCY		
PARAMETER	MONTHLY AVERAGE	DAILY MAXIMUM	UNITS	MIN	MONTHLY AVERAGE	DAILY MAXIMUM	UNITS	OF ANALYSIS	SAMPLE TYPE	NOTES
Flow	Report	Report	MGD					1/Week	Measured	
Total Suspended Solids					26	52	mg/l	1/Month	24-hour Composite	
Oil and Grease						15	mg/l	1/Month	Grab	
Total Chromium					1.71	2.77	mg/l	1/Week	24-hour Composite	(1)
Hexavalent					0.05	0.10	mg/l	1/Week	24-hour Composite	(1)
Chromium										` ′
Total Nickel					Report		mg/l	1/Month	24-hour Composite	(1)
Dissolved Nickel					Report		mg/l	1/Month	24-hour Composite	(1)
Total Copper					Report		mg/l	1/Month	24-hour Composite	(1)
Dissolved Copper					Report		mg/l	1/Month	24-hour Composite	(1)
Hardness					Report		mg/l	1/Month	Grab	(1)
Total Cyanide					Report		mg/l	1/Month	24-hour Composite	<u> </u>
Available Cyanide					0.049	0.049	mg/l	1/Month	24-hour Composite	(2), (3),
										(4)
pН				6.0		9.0		1/Day	Grab	

There shall be no discharge of floating solids or persistent foam in other than trace amounts. Persistent foam is foam that does not dissipate within one half-hour of point of discharge.

- (1) Results for hardness to be taken the same time as the metals.
- (2) The permittee shall report the result from a combination of individual grab samples representative of a 24-hour period consistent with the EPA test methodology and holding times. Cyanide shall be measured as available cyanide (or alternatively cyanide amenable to chlorination) using EPA Method OIA-

Permit Number: 10-DP-3060 (MD0066818)

Page 3 of 24

I. SPECIAL CONDITIONS

A.1. <u>EFFLUENT LIMITATIONS AND MONITORING REQUIREMENTS</u> – Continued from previous page

1677. The minimum level (ML) for cyanide is $2.0 \mu g/l$. Analytical results for cyanide below the minimum level shall be reported as zero. Other NPDES approved test methods may be used if minimum levels/quantitation levels at or below the applicable permit limit can be demonstrated.

- (3) Limitation becomes effective 3 years after the effective date of this permit. See Special Condition O. Monitoring without limits is required until that time.
- (4) Limits are based on discharge through the existing outfall and diffuser. Any discharge event that bypasses the diffuser must meet a limit of 0.001 mg/l.

Permit Number: 10-DP-3060 (MD0066818) Page 4 of 24

1. During the effective period of this permit, the permittee must select, design, install, and implement control measures (including best management practices) to minimize pollutant discharges of infiltrated contaminated ground water from the following MS4 stormdrains: Outfall 002 (12.5 Street Storm Drain), Outfall 004 (13.5 Street Storm Drain), Outfall 005 (14 Street Storm Drain), Outfall 006 (15 Street Storm Drain), and Outfall 013 (12 Street Storm Drain). The permittee must minimize exposure of stormwater to infiltrated contaminated groundwater by using available engineering practices as control measures (such as lining of the storm drains, capping the area, etc.).

- 2. The Dundalk Corrective Measures Alternatives Analysis (CMAA) resulted in an approved approach to reline storm drains leading to Outfalls 002, 004, 005, 006 and 013. The permittee shall submit to the Department (WSA) on a quarterly basis the current "Storm Drain Rehabilitation Schedule" (see Definition B.23) via NetDMR.
- 3. If the selected control measures are not achieving the intended effect of minimizing pollutant discharges as indicated by action level results required below, the permittee must modify these control measures per the corrective action requirements in Special Condition Q.
- 4. Beginning on the dates specified in paragraph A.2.5 below, such discharges shall be monitored by the permittee as specified below at the monitoring points established for the outfall pipes to the Patapsco River or, if inaccessible due to tidal influence, at the nearest manhole or tidal exclusion vault upstream of any tidal influence.

PARAMETER	ACTION LEVEL	UNITS	FREQUENCY OF ANALYSIS	SAMPLE	NOTES
Flow	Report	MGD	1/Month	Estimated	
Total Chromium (Wet Weather)	Report	mg/l	1/Month	Composite	(1) (2) (3)
Hexavalent Chromium (Wet Weather)	0.55	mg/l	1/Month	Composite	(1) (2) (3) (4)
Total Chromium (Dry Weather)	Report	mg/l	1/Month	Composite	(1) (4)
Hexavalent Chromium (Dry	0.55	mg/l	1/Month	Composite	(1) (3) (4)

(1) Sampling and reporting requirements apply to discharges from at least one wet weather event and one dry weather event (refer to Special Condition U) per sampling period for all outfalls except Outfalls 005 and 006. For Outfalls 005 and 006, only wet weather monitoring applies, since all dry weather flows must be pumped via the wastewater treatment system to Outfall 001 (Special Condition A.1), until such time that the treatment plant is closed and Outfall 001 is removed from the permit after a major permit modification. Sampling results from dry weather events shall be reported with the designation "-D" after the Outfall number, such as "Outfall 002-D", and results of wet weather events shall be reported with the designation "-W" after the outfall number, such as "Outfall 002-W". If a dry weather event or a wet weather event discharge does not occur during the reporting period, the permittee shall report "No Discharge" for the applicable parameter(s).

Permit Number: 10-DP-3060 (MD0066818) Page 5 of 24

(2) Beginning on the effective date of the permit, the permittee shall monitor the discharges from Outfalls 005, 006, and 013 at a frequency of twice per year until the effective date of the Action Levels in paragraph A.2.5.

- (3) See "Action Level Implementation Requirements" in Special Condition P. When an action level is exceeded see "Corrective Actions and Deadlines in Special Condition Q."
- (4) Monthly monitoring of an outfall may be reduced to quarterly monitoring after 4 consecutive months of sampling in which all samples have concentrations of Hexavalent Chromium at or below the Action Level, by notifying the Department (WSA). However, if a quarterly sample exceeds the Action Level, monthly monitoring will again be required for a minimum of another 4 months.
- 5. Action Level monitoring shall begin no later than the following dates:

OUTFALL NUMBER	MONITORING START DATE
Outfall 002 (12.5 Street Storm Drain)	Permit effective date
Outfall 004 (13.5 Street Storm Drain)	Permit effective date
Outfall 013 (12 Street Storm Drain)	November 1, 2019
Outfall 005 (14 Street Storm Drain)	November 1, 2022
Outfall 006 (15 Street Storm Drain)	November 1, 2022

SPECIAL CONDITIONS

B. DEFINITIONS

- 1. "Action Level": This permit establishes pollutant action level concentrations applicable to discharges from this facility. The action level concentrations are not effluent limitations; and an action level exceedance therefore, is not a permit violation. Action level monitoring data is primarily for the permittee's use to determine the overall effectiveness of control measures, and to indicate when additional corrective actions may be necessary.
- 2. "Bypass" means the intentional diversion of wastes from any portion of a treatment facility.
- 3. "Clean Water Act: means the Federal Water Pollution Control Act, as amended, 33 U.S.C. Section 1251 et seq.
- 4. "CFR" means the Code of Federal Regulations.
- 5. "Composite sample" means a combination of individual samples obtained at least at hourly intervals over a time period. Either the volume of each individual sample is proportional to discharge flow rates or the sampling interval (for constant volume samples) is proportional to the flow rates over the time period used to produce the composite.

Permit Number: 10-DP-3060 (MD0066818) Page 6 of 24

6. "Corrective Measures Alternatives Analysis" (or "CMAA"): The CMAA is a document that was produced under the supervision of the Department (LMA) to investigate, analyze, and address environmental conditions related to chromium at the Port of Baltimore's Dundalk Marine Terminal (DMT).

- 7. "Daily determination of concentration" means one analysis performed on any given sample representing flow during a calendar day, with one number in mg/l or other appropriate units as an outcome.
- 8. The "daily maximum" effluent concentration means the highest reading of any daily determination of concentration.
- 9. "Department (LMA)" means, for the purposes of this permit, the Land Management Administration of the Maryland Department of the Environment.
- 10. "Department (WSA)" means, for the purposes of this permit, the Water and Science Administration of the Maryland Department of the Environment.
- 11. "Draft Construction Completion Report": This report is produced to document the completion of relining of an outfall. This report is presented to the Department (LMA) upon successful completion of the relining and verification that completion of the required contractor work has been accepted by the Maryland Port Administration.
- 12. "Dry weather" means the sample was taken from flow that results in discharge from an outfall that begins when there is no longer any precipitation runoff flowing into the facility's storm drains. It ends with any precipitation event that produces runoff.
- 13. "Final Completeness Report": Upon completion of all relining projects to address Outfall 002-006 and Outfall 013, this report will document the final completeness to the Department (LMA) of the relining work.
- 14. "Grab sample" means an individual sample collected over a period of time not exceeding 15 minutes. Grab samples collected for pH and total residual chlorine shall be analyzed within 15 minutes of time of sample collection.
- 15. "Infeasible" means not technologically possible or not economically practicable and achievable in light of best industry practices.
- 16. "Measured" flow means any method of liquid volume measurement the accuracy of which has been previously demonstrated in engineering practice, or for which a relationship to absolute volume has been obtained.
- 17. "Minimize" means reduce and/or eliminate to the extent achievable using control measures (including best management practices) that are technologically available and economically practicable and achievable in light of best industry practice.
- 18. The "minimum" value means the lowest value measured during a 24-hour period.
- 19. The "monthly, quarterly, semi-annual, or annual average" effluent concentration means the value calculated by computing the arithmetic mean of all the daily determinations of concentration made during any calendar-month, 3-month, 6-month, or 12-month period respectively.

Permit Number: 10-DP-3060 (MD0066818) Page 7 of 24

20. Municipal Separate Storm Sewer System (MS4) – a conveyance or system of conveyances (including roads with drainage systems, municipal streets, catch basins, curbs, gutters, ditches, man-made channels, or storm drains):

- a) Owned or operated by a State, city, town, borough, county, parish, district, association, or other public body (created by or pursuant to State law) having jurisdiction over disposal of sewage, industrial wastes, stormwater, or other wastes, including special districts under State law such as a sewer district, flood control district or drainage district, or similar entity, or an Indian tribe or an authorized Indian tribal organization, or a designated and approved management agency under section 208 of the CWA that discharges to waters of the United States;
- b) Designed or used for collecting or conveying stormwater;
- c) Which is not a combined sewer; and
- d) Which is not part of a Publicly Owned Treatment Works (POTW) as defined at 40 CFR 122.2. See 40 CFR 122.26(b)(4) and (b)(7).
- 21. "NetDMR" means a nationally-available electronic reporting tool, initially designed by states and later adapted for national use by EPA, which can be used by NPDES-regulated facilities to submit discharge monitoring reports (DMRs) electronically to EPA through a secure Internet application over the National Environmental Information Exchange Network (NEIEN). EPA can then share this information with authorized states, tribes, and territories.
- 22. "Oil and Grease" refers to the use of and results yielded from EPA Method 1664 or any EPA approved revisions of this analytical test method approved for use with Clean Water Act monitoring programs.
- 23. "Stormdrain Inspection and Maintenance Plan": This term refers to a plan the Maryland Port Administration will implement to address both inspection and maintenance of the completed relining projects.
- 24. "Storm Drain Rehabilitation Schedule": This term refers to the schedule for the relining project, which includes Draft Construction Completion Report(s), and Final Completeness Report milestones, and is the primary tool used to communicate to the Department (LMA) the status of the relining control measures.
- 25. "Uncontaminated ground water infiltration" means water below the land surface in the zone of saturation which has been impacted by activities associated with disposal of Chrome Ore Processing Residue (COPR) material and infiltrates into the MS4 system, discharges only as the result of wet weather events, and results in discharge levels below the action level defined in this permit.
- 26. "Upset" means the exceptional incident in which there is unintentional and temporary noncompliance with technology-based permit effluent limitations because of factors beyond the reasonable control of the permittee. An upset does not include noncompliance to the extent caused by operational error, improperly designed treatment facilities, inadequate treatment facilities, lack of preventive maintenance, or careless or improper operation.
- 27. "Wet weather" means the period during which precipitation or melting snow causes visible runoff from the facility that results in discharge from an outfall.

Permit Number: 10-DP-3060 (MD0066818) Page 8 of 24

C. TOXIC POLLUTANT REPORTING

The permittee shall notify the Department (WSA) as soon as it is known or suspected that any toxic pollutants at Outfall 001 which are not specifically limited by this permit have been discharged at levels specified in 40 CFR Part 122.42(a).

D. REMOVED SUBSTANCES

- 1. Within 30 days after notification by the Department (WSA), the permittee shall provide information on the disposal of any removed substances, as defined by General Condition B.7, including the following information:
 - a. A suitable map showing all areas used for disposal of removed substances.
 - b. The physical, chemical, and biological characteristics, as appropriate; quantities of any removed substances; and the method of disposal.
 - c. If disposal is handled by persons other than the permittee, identification of the contractor or subcontractor, their mailing address, and the information specified in a and b above.
- 2. The Department's notification may also require the permittee to provide the above information prior to the use of new or additional disposal areas, contractors, or subcontractors.

E. ANALYTICAL LABORATORY

Within 30 days after the effective date of this permit, the permittee shall submit to the Department (WSA) the name and address of the analytical laboratory (including the permittee's own laboratory) which is used to perform the monitoring required by this permit.

If the laboratory changes during the effective period of this permit, the permittee shall notify the Department (WSA) of the new laboratory within 30 days after the change.

F. WASTEWATER OPERATOR CERTIFICATION

As of the effective date of this permit, the permittee's facility shall be operated by an industrial wastewater operator duly certified by the Maryland Board of Waterworks and Waste Systems Operators. Certification shall be for operation of a Class 6 industrial wastewater works, unless the Board determines that a different classification is appropriate. At no time during the effective period of this permit shall the treatment facilities be operated for more than two months without a certified operator.

G. FLOW MONITORING

In lieu of providing measured flow (defined in the Special Conditions Definitions section) at Outfalls 001, 002, 004, 005, 006 and 013, the permittee may estimate flows and submit the following information at the time of submission of the initial discharge monitoring report and/or upon any change in the methodology:

1. a description of the methodology used to estimate flow at each outfall where flow measurement equipment is not present;

Permit Number: 10-DP-3060 (MD0066818) Page 9 of 24

2. documentation appropriate to the methodology utilized which provides information necessary to support the validity of the reported flow estimate. If actual measurements or observations are made, a description of typical sampling times, locations, and persons performing the measurements/observations should also be provided.

3. a description of the factors (e.g., batch discharges, intermittent operation, etc.) which cause flow at the outfall to fluctuate significantly from the estimate provided.

H. FLOW BASIS FOR ANNUAL DISCHARGE PERMIT FEE – [Reserved]

I. REAPPLICATION FOR A PERMIT

The Department (WSA) is implementing a schedule for issuance of discharge permits grouped by geographical areas (watersheds). To implement the watershed-based schedule, the Department (WSA) may revoke and reissue this permit concurrently with other permits in the watershed.

Unless the Department (WSA) grants permission for a later date, the permittee shall submit a renewal application by no later than 12 months prior to the expiration date on the first page of this permit, or notify the Department (WSA) of the intent to cease discharging by the expiration date.

In the event that a timely and sufficient reapplication has been submitted and the Department (WSA) is unable, through no fault of the permittee, to issue a new permit before the expiration date of this permit, the terms and conditions of this permit are automatically continued and remain fully effective and enforceable.

J. PERMIT REOPENER FOR TOTAL MAXIMUM DAILY LOAD (TMDL) AT OUTFALL 001

This permit may be reopened as a major modification to implement any applicable requirements associated with a Total Maximum Daily Load (TMDL) issued or approved for this watershed <u>Baltimore Harbor</u>, 02.13.09.03, including but not limited to: Total Chromium and Hexavalent chromium.

The terms and conditions of this permit are in conformance with the Chesapeake Bay Total Maximum Daily Load (TMDL) for Sediments, Nitrogen and Phosphorus, approved December 29, 2010.

At this time, the permit limits total suspended solids, but does not introduce limits for total nitrogen and total phosphorus. Such limitations are to prevent water quality degradation of the receiving waters and ultimately the Chesapeake Bay. This determination has been based on facility operations and/or discharge characteristics.

To ensure the Chesapeake Bay and its tributaries are protected from discharges of sediments, nitrogen and phosphorus, this permit may be reopened as a major modification to implement any applicable requirements associated with the Chesapeake Bay TMDL. The permittee may become subject to a Department-issued General Permit regarding the discharge of such pollutants.

K. <u>BIOMONITORING PROGRAM – [Reserved]</u>

L. TOXICITY REDUCTION EVALUATION AT OUTFALL 001

The permittee shall conduct a Toxicity Reduction Evaluation (TRE) when a review of toxicity test data by the Department (WSA) indicates unacceptable acute or chronic effluent toxicity. A TRE is an

Permit Number: 10-DP-3060 (MD0066818) Page 10 of 24

investigation conducted to identify the causative agents of effluent toxicity, isolate the source(s), determine the effectiveness of control options, implement the necessary control measures and then confirm the reduction in toxicity.

- 1. Within 90 days following notification by the Department (WSA) that a TRE is required, the permittee shall submit a plan of study and schedule for conducting a TRE. The permittee shall conduct the TRE study consistent with the submitted plan and schedule.
- 2. This plan should follow the framework presented in Generalized Methods for Conducting Industrial Toxicity Reduction Evaluations (EPA/600/2-88/070).
- 3. Beginning 60 days following the date of the Department's (WSA) acceptance of the TRE study plan and every 60 days thereafter, the permittee shall submit progress reports including all relevant test data to the Department (WSA). This shall continue until completion of the toxicity reduction confirmation.
- 4. Within 60 days following completion of the toxicity identification, or the source identification phase of the TRE, the permittee shall submit to the Department (WSA) a plan and schedule for implementing those measures necessary to eliminate acute toxicity and/or reduce chronic toxicity to acceptable levels. The implementation of these measures shall begin immediately upon submission of this plan.
- 5. Within 60 days after completing implementation of the control measures to eliminate or reduce toxicity, the permittee shall submit to the Department (WSA) for approval a study plan to confirm the elimination or reduction of toxicity by using biomonitoring.
- 6. If, for any reason, the implemented measures do not result in compliance with the Department's toxicity limitations, the permittee shall continue the TRE.

M. MIXING ZONES AND POLLUTION PREVENTION AT OUTFALL 001

"Chesapeake 2000" is a comprehensive Agreement for the restoration of the Chesapeake Bay signed June 28, 2000 by the State of Maryland, Commonwealths of Virginia and Pennsylvania, the District of Columbia, U.S. Environmental Protection Agency and Chesapeake Bay Commission. Among its goals the Agreement includes the following:

"Through continual improvement of pollution prevention measures and other voluntary means, strive for zero release of chemical contaminants from point sources, ... Particular emphasis shall be placed on achieving ... elimination of mixing zones for persistent or bioaccumulative toxics."

To support attainment of this goal the permittee shall strive to meet water quality standards (WQS) for toxic substances at the point of discharge, including WQS for copper, through continual improvement of pollution prevention measures and other means. Beginning within 12 months after the effective date of this permit and continuing annually thereafter, the permittee shall report to the Department (WSA) on progress made toward the elimination of mixing zones for persistent or bioaccumulative toxics.

N. PROTECTION OF WATER QUALITY AT OUTFALL 001

It is a violation of this permit to discharge any substance not otherwise listed under the permit's "Effluent Limitations and Monitoring Requirements" special conditions at a level which would cause or contribute to any exceedance of the numerical water quality standards in COMAR 26.08.02.03

Permit Number: 10-DP-3060 (MD0066818) Page 11 of 24

unless the level and the substance were disclosed in writing in the permit application prior to the issuance of the permit. If a discharge regulated by this permit causes or contributes to an exceedance of the water quality standards in COMAR 26.08.02.03, including but not limited to the general water quality standards, or if the discharge includes a pollutant that was not disclosed or addressed in the public record for the permit determination, the Department (WSA) is authorized to modify, suspend or revoke this permit or take enforcement action to address unlawful discharges of pollutants.

O. COMPLIANCE SCHEDULE FOR CYANIDE LIMITS AT OUTFALL 001

- 1. Every six months, the permittee shall submit to the Department (WSA) a status report detailing current plans for meeting cyanide limits in Special Condition A.1. This report is due six months after the effective date of the permit and every six months thereafter until compliance with the permit limits have been achieved or until a permit modification under condition O.2 has been implemented. The plans may include alternative treatment technologies or other discharge options which will result in compliance with the final discharge limitations.
- 2. No later than 24 months after the effective date of this permit, the permittee may apply for a permit modification to revise or remove the permit limit(s) based on any combination of the following options allowed under COMAR 26.08:

Additional Data: As part of the application for a permit modification, the permittee may submit any new or additional data which demonstrates that there is no reasonable potential for violation of applicable water quality standards.

Alternatives for making such a demonstration include, but are not limited to, the following:

- i. collection of effluent data using analytical methods which have lower detection levels;
- ii. collection of data using techniques designed to minimize the effects of contamination on sample results; and
- iii. collection of data utilizing alternate monitoring locations to show compliance with water quality standards.

P. ACTION LEVEL IMPLEMENTATION REQUIREMENTS

- 1. Lining of the stormdrains leading to Outfalls 002 and 004 has already been completed, and the requirements in Special Condition A.2 for action levels at these outfalls begin upon the effective date of the permit until the outfalls are removed from this permit in accordance with Special Condition T.
- 2. When lining of the storm drain leading to any specific Outfall (005, 006 or 013) is completed, the permittee shall notify the WSA-Compliance Program in writing within 14 days of lining completion for that outfall and shall:
 - a. Include in the notification a copy of the Draft Construction Completion Report:
 - b. Identify and label a discharge monitoring location for the completed outfall and provide a written location description to the Department's WSA-Compliance Program;
 - c. Include updated status details for any remaining Outfalls not yet complete;
 - d. Update the "Stormdrain Inspection and Maintenance Plan" as each outfall is completed: and

Permit Number: 10-DP-3060 (MD0066818) Page 12 of 24

e. Begin action level monitoring in the first month following the notification of lining completion for that outfall, but no later than the dates specified in Special Condition A.2.5.

- 3. Special Condition A.2.4 specifies a pollutant action level concentration that is primarily for the permittee's use to determine the overall effectiveness of control measures and to assist the permittee in determining when additional corrective action(s) may be necessary to comply with the effluent limitations. The action level concentration is not an effluent limitation; an action level exceedance, therefore, is not a permit violation. However, if corrective action is required as a result of an action level exceedance, failure to conduct required corrective action is a permit violation.
- 4. In the event that an action level specified in Special Condition A.2.4 has been exceeded, the permittee shall take corrective actions as specified in Special Condition Q.

Q. CORRECTIVE ACTIONS AND DEADLINES.

The permittee must implement Immediate Actions (Q.1) and, if required, Subsequent Actions (Q.2) if control measures are not stringent enough for the discharge to meet the non-numeric effluent limits in this permit based on any sampling result exceeding an Action Level (see Special Condition P).

1. Immediate Actions. If corrective action is needed, the permittee must immediately take all reasonable steps necessary to minimize or prevent the discharge of pollutants until a permanent solution is installed and made operational. The permittee shall submit the actions taken and results together with a plan and schedule of any remaining corrective actions to the Department (WSA) for approval prior to the next required sampling event. In addition, if the concentration for Hexavalent Chromium is above 1.1 mg/L, the permittee must notify the Department (WSA) by telephone within 24 hours and written confirmation within 5 days.

Note: In this context, the term "immediately" requires the permittee to, on the same day a condition requiring corrective action is found, take all reasonable steps to minimize or prevent the discharge of pollutants until a permanent solution is installed and made operational. However, if a problem is identified at a time in the work day when it is too late to initiate corrective action, the initiation of corrective action must begin no later than the following work day. "All reasonable steps" means that the permittee has undertaken initial actions to assess (such as immediate confirmatory sample) and correct the condition causing the discharge to exceed the action level, including, for example, making arrangements (i.e., scheduling) for a new BMP to be installed at a later date.

2. Subsequent Actions. If the permittee or the Department determines that additional actions are necessary beyond those implemented pursuant to Q.1, the permittee must complete the corrective actions (e.g., install a new or modified control and make it operational, complete the repair) before the next storm event if possible, and within 45 calendar days from the time of discovery of the corrective action condition. If it is infeasible to complete the corrective action within 45 calendar days, the permittee must document why it is infeasible to complete the corrective action within the 45-day timeframe. The permittee must also identify the permittee's schedule for completing the work, which must be done as soon as practicable after the 45-day timeframe. If the completion of corrective action will exceed the 45 day timeframe, the permittee may take the minimum additional time necessary to complete the corrective action, provided that the permittee notify the Department (WSA) of the permittee's intention to exceed 45 days, the permittee's rationale for an extension, and a completion date, which the permittee must also include in the permittee's corrective action documentation.

Permit Number: 10-DP-3060 (MD0066818) Page 13 of 24

However, any schedule for completing the corrective action greater than 18 months after discovery must be submitted to the Department (WSA) in the form of a permit modification application which invokes the public participation process.

Where the permittee's corrective actions result in changes to any of the controls or procedures documented in the Stormdrain Inspection and Maintenance Plan, the permittee must modify the Stormdrain Inspection and Maintenance Plan accordingly within 14 calendar days of completing corrective action work.

R. HAZARDOUS WASTE PROGRAM REPORTING

The results of any new or additional effluent monitoring required by the Department's Land Restoration Program is required to be concurrently submitted via NetDMR as an attachment.

S. <u>ILLICIT CYANIDE DISCHARGE DETECTION AND ELIMINATION (FOR OUTFALLS 002, 004, 005, 006 and 013).</u>

If after the three (3) year compliance schedule for cyanide in Outfall 001 (Special Conditions I.A and O), the limits for cyanide are implemented and the source of cyanide is not from the treatment plant process itself, then the permittee must comply with this special condition. If the source of cyanide is from the treatment system, the permittee must notify the Department, with supporting documentation for review. If the Department concurs that the source of cyanide is the treatment system, this special condition is satisfied.

The permittee shall develop, implement, and maintain a program to identify and eliminate any illicit source(s) of cyanide from occurring in storm drain system connections and non-stormwater discharges to the maximum extent practicable. The program developed to satisfy this minimum control measure shall contain elements to field screen storm drain system outfalls, inspect the storm drain system for the purpose of identifying the source of any illicit discharges, eliminate any illegal connection or illicit discharge to the storm drain system, and enforce penalties where appropriate. The illicit discharge program shall also contain components to address illegal dumping and spills. This minimum control measure may be implemented and maintained by the permittee or by another responsible entity. Additionally, the permittee may coordinate its efforts to identify and eliminate non-stormwater discharges with those of the surrounding County performing similar activities. If the responsibilities for complying with this minimum control measure are to be shared between the permittee and another responsible entity, the relationship and specific duties of all participating entities shall be outlined in a notification to the Department (WSA).

At a minimum, a program developed to implement illicit cyanide discharge detection and elimination to satisfy this control measure shall contain the following:

- 1. A map showing the extent of the storm drain system;
- 2. The legal means to provide for entering onto private property to investigate and eliminate illicit storm drain system discharges;
- 3. Procedures to field screen storm drain outfalls on a consistent basis;
- 4. Inspection procedures for identifying the source of any suspected illicit discharges to the storm drain system;
- 5. Enforcement and penalty procedures;
- 6. Procedures to address spills and illegal dumping;
- 7. Means to inform public employees, businesses, and the general public of illegal discharges and improper waste disposal; and

Permit Number: 10-DP-3060 (MD0066818) Page 14 of 24

8. Any other components deemed necessary to ensure that non-stormwater discharges to the municipal separate storm sewer system are either permitted by MDE under NPDES or eliminated.

Regardless of whether the permittee develops its own program or relies on another responsible entity to satisfy this minimum control measure, the permittee shall cooperate regarding discharges entering or leaving its jurisdictional boundaries or Waters of the State. The intent of this program is to control non-stormwater discharges to and from municipal separate storm sewer systems. Therefore, it is essential that the permittee cooperate actively in instances where storm drain systems are interconnected with entities covered under this or any other NPDES stormwater permit.

T. REMOVAL OF MS4 STORMDRAINS FROM COVERAGE UNDER THIS PERMIT

Upon demonstration to the Department (WSA) that the source of chromium from an outfall has been addressed through implementation of the permit's technology based non-numeric effluent limitations and resulting in uncontaminated groundwater infiltration (see definition B.25) for two consecutive years, the outfall may be removed from the permit only after a major permit modification limited to the specific Outfall(s) removal, including public participation. Any permit modification application for removing an outfall shall include submission of a report covering that outfall, which shall include data to support the removal, and a plan which includes any BMPs that may be used to monitor, prevent or reduce the discharge of chromium from the outfall to be implemented and reported pursuant to terms of the current MS4 permit (permit no. 05-SF-5501 or its successor).

U. <u>IMPLEMENTATION OF DRY WEATHER DISCHARGE MONITORING REQUIREMENT (see Special Condition A.2)</u>

The permittee must provide a plan to the Department (WSA), within 30 days of the permit effective date, on how dry weather discharges will be identified and how dry weather discharge samples will be obtained. The plan should ensure that a representative sample may be taken monthly or quarterly as required by Special Condition A.2, or, if there is no discharge, that it is verifiable. If there is no likelihood of a dry weather discharge due to the configuration of the outfall, related details should be provided as part of the plan submittal.

Permit Number: 10-DP-3060 (MD0066818) Page 15 of 24

II. GENERAL CONDITIONS

A. MONITORING AND REPORTING

1. REPRESENTATIVE SAMPLING

Samples and measurements taken as required herein shall be taken at such times as to be representative of the quantity and quality of the discharges during the specified monitoring periods.

2. REPORTING-MONITORING RESULTS SUBMITTED QUARTERLY

Monitoring results obtained during each calendar month shall be summarized and submitted electronically using NetDMR. Results shall be submitted to the Department via NetDMR no later than the 28th of the month following the end of the reporting month. Specific requirements regarding submittal of data and reports in hard copy form and for submittal using NetDMR are described below:

- a. NetDMR is a U.S. EPA tool allowing regulated Clean Water Act permittees to submit monitoring reports electronically via a secure Internet application. The permittee must obtain access to NetDMR at www.epa.gov/netdmr and register for a NetDMR Webinar. Before the permittee can submit official DMRs using NetDMR the permittee must attend a training Webinar and successfully set-up and submit test monitoring results electronically.
- b. Hard copies of monitoring results obtained before the permittee obtains access to NetDMR, but not later than December 21, 2016, may be submitted postmarked no later than the 28th of the month following the end of the reporting month. Signed copies of the results shall be submitted to the Department at the following address:

Attention: Discharge Monitoring Reports
Water and Science Administration
Compliance Program
Maryland Department of the Environment
1800 Washington Boulevard, STE-425
Baltimore, MD 21230-1708

3. <u>SAMPLING AND ANALYSIS METHODS</u>

The analytical and sampling methods used shall conform to procedures for the analysis of pollutants as identified in Title 40 CFR Part 136 - "Guidelines Establishing Test Procedures for the Analysis of Pollutants" unless otherwise specified.

4. <u>DATA RECORDING REQUIREMENTS</u>

For each measurement or sample taken pursuant to the requirements of this permit, the permittee shall record the following information:

- a. the exact place, date, and time of sampling or measurement;
- b. the person(s) who performed the sampling or measurement;
- c. the dates and times the analyses were performed;

Permit Number: 10-DP-3060 (MD0066818) Page 16 of 24

- d. the person(s) who performed the analyses;
- e. the analytical techniques or methods used; and
- f. the results of all required analyses.

5. MONITORING EQUIPMENT MAINTENANCE

The permittee shall periodically calibrate and perform maintenance procedures on all monitoring and analytical instrumentation to insure accuracy of measurements.

6. ADDITIONAL MONITORING BY PERMITTEE

If the permittee monitors any pollutant, using approved analytical methods as specified above, at the locations designated herein more frequently than required by this permit, the results of such monitoring, including the increased frequency, shall be included in the calculation and reporting of the values required in the Discharge Monitoring Report form (EPA No. 3320-1).

7. RECORDS RETENTION

All records and information resulting from the monitoring activities required by this permit, including all records of analyses performed, calibration and maintenance of instrumentation, and original recordings from continuous monitoring instrumentation shall be retained for a minimum of three years. This period shall be automatically extended during the course of litigation, or when requested by the Department (WSA).

B. MANAGEMENT REQUIREMENTS

1. CHANGE IN DISCHARGE

All discharges authorized herein shall be consistent with the terms and conditions of this permit. The discharge of any pollutant identified in this permit at a level in excess of that authorized shall constitute a violation of the terms and conditions of this permit. The permittee shall report any anticipated facility expansions, production increases, or process modifications which will result in new, different or an increased discharge of pollutants by submitting a new application at least 180 days prior to the commencement of the changed discharge except that if the change only affects a listed pollutant and will not violate the effluent limitations specified in this permit, by providing written notice to the Department (WSA). Following such notice, the permit may be modified by the Department (WSA) to include new effluent limitations on those pollutants.

2. NONCOMPLIANCE WITH EFFLUENT LIMITATIONS

If, for any reason, the permittee does not comply with or will be unable to comply with any daily maximum or daily minimum effluent limitation specified in this permit, the permittee shall notify the Inspection and Compliance Program by telephone at (410) 537-3510 within 24 hours of becoming aware of the noncompliance. Within five calendar days, the permittee shall provide the Department (WSA) with the following information in writing:

- a. a description of the non-complying discharge including its impact upon the receiving waters;
- b. cause of noncompliance;

Permit Number: 10-DP-3060 (MD0066818) Page 17 of 24

c. anticipated time the condition of noncompliance is expected to continue or if such condition has been corrected, the duration of the period of noncompliance;

- d. steps taken by the permittee to reduce and eliminate the non-complying discharge;
- e. steps to be taken by the permittee to prevent recurrence of the condition of noncompliance; and
- f. a description of the accelerated or additional monitoring by the permittee to determine the nature and impact of the noncomplying discharge.

3. FACILITIES OPERATION

All treatment, control and monitoring facilities, or systems installed or used by the permittee, are to be maintained in good working order and operated efficiently.

4. ADVERSE IMPACT

The permittee shall take all reasonable steps to minimize or prevent any adverse impact to waters of the State or to human health resulting from noncompliance with any effluent limitations specified in this permit, including such accelerated or additional monitoring as necessary to determine the nature and impact of the noncomplying discharge.

5. BYPASSING

Any bypass of treatment facilities necessary to maintain compliance with the terms and conditions of this permit is prohibited unless:

- a. the bypass is unavoidable to prevent a loss of life, personal injury or substantial physical damage to property, damage to the treatment facilities which would cause them to become inoperable, or substantial and permanent loss of natural resources;
- b. there are no feasible alternatives:
- c. notification is received by the Department (WSA) within 24 hours (if orally notified, then followed by a written submission within five calendar days of the permittee's becoming aware of the bypass). Where the need for a bypass is known (or should have been known) in advance, this notification shall be submitted to the Department (WSA) for approval at least ten calendar days before the date of bypass or at the earliest possible date if the period of advance knowledge is less than ten calendar days; and
- d. the bypass is allowed under conditions determined by the Department (WSA) to be necessary to minimize adverse effects.

6. CONDITIONS NECESSARY FOR DEMONSTRATION OF AN UPSET

An upset shall constitute an affirmative defense to an action brought for noncompliance with technology-based effluent limitations only if the permittee demonstrates, through properly signed, contemporaneous operating logs, or other relevant evidence, that:

a. an upset occurred and that the permittee can identify the specific cause(s) of the upset;

Permit Number: 10-DP-3060 (MD0066818) Page 18 of 24

b. the permitted facility was at the time being operated in a prudent and workman-like manner and in compliance with proper operation and maintenance procedures;

- c. the permittee submitted a 24-hour notification of upset in accordance with the reporting requirements of General Condition II.B.2 above;
- d. the permittee submitted, within five (5) calendar days of becoming aware of the upset, documentation to support and justify the upset; and
- e. the permittee complied with any remedial measures required to minimize adverse impact.

7. <u>REMOVED SUBSTANCES</u>

Wastes such as solids, sludges, or other pollutants removed from or resulting from treatment or control of wastewaters, or facility operations, shall be disposed of in a manner to prevent any removed substances or runoff from such substances from entering or from being placed in a location where they may enter the waters of the State.

8. POWER FAILURE

In order to maintain compliance with the effluent limitations and prohibitions of this permit, the permittee shall either:

- a. provide an alternative power source sufficient to operate the wastewater collection and treatment facilities or,
- b. halt, reduce or otherwise control production and all discharges upon the reduction, loss, or failure of the primary source of power to the wastewater collection and treatment facilities.

C. RESPONSIBILITIES

1. RIGHT OF ENTRY

The permittee shall permit the Secretary of the Department, the Regional Administrator for the Environmental Protection Agency, or their authorized representatives, upon the presentation of credentials to:

- a. enter upon the permittee's premises where an effluent source is located or where any records are required to be kept under the terms and conditions of this permit;
- b. access and copy, at reasonable times, any records required to be kept under the terms and conditions of this permit;
- c. inspect, at reasonable times, any monitoring equipment or monitoring method required in this permit;
- d. inspect, at reasonable times, any collection, treatment, pollution management, or discharge facilities required under this permit; and

Permit Number: 10-DP-3060 (MD0066818) Page 19 of 24

e. sample, at reasonable times, any discharge of pollutants.

2. TRANSFER OF OWNERSHIP OR CONTROL OF FACILITIES

In the event of any change in ownership or control of facilities from which the authorized discharge emanates, the permit may be transferred to another person if:

- a. the permittee notifies the Department in writing, of the proposed transfer;
- b. a written agreement, indicating the specific date of proposed transfer of permit coverage and acknowledging responsibilities of current and new permittees for compliance with the liability for the terms and conditions of this permit, is submitted to the Department; and
- c. neither the current permittee nor the new permittee receive notification from the Department, within 30 calendar days, of intent to modify, revoke, reissue or terminate the existing permit.

3. <u>REAPPLICATION FOR A PERMIT</u> – [Reserved]

4. **AVAILABILITY OF REPORTS**

Except for data determined to be confidential under Section 308 of the Clean Water Act, 33 U.S.C. § 1318, all submitted data shall be available for public inspection at the offices of the Department and the Regional Administrator of the Environmental Protection Agency.

5. PERMIT MODIFICATION

A permit may be modified by the Department (WSA) upon written request of the permittee and after notice and opportunity for a public hearing in accordance with and for the reasons set forth in 40 CFR § 122.62 and 122.63.

6. PERMIT MODIFICATION, SUSPENSION, OR REVOCATION

After notice and opportunity for a hearing, this permit may be modified, suspended, or revoked and reissued in whole or in part during its term, in accordance with the provisions set forth in COMAR 26.08.04.10, for causes including, but not limited to, the following:

- a. violation of any terms or conditions of this permit;
- b. obtaining this permit by misrepresentation or failure to disclose fully all relevant facts;
- c. a change in any condition that requires either a temporary or permanent reduction or elimination of the authorized discharge; or
- d. a determination that the permitted discharge poses a threat to human health or welfare or to the environment and can only be regulated to acceptable levels by permit modification or termination.
- e. upon a final, unreviewable determination that the permittee lacks, or is in violation, of any federal, state, or local approval necessary to conduct the activities by this permit.

Permit Number: 10-DP-3060 (MD0066818) Page 20 of 24

7. TOXIC POLLUTANTS

If a toxic effluent standard or prohibition (including any schedule of compliance specified in such toxic effluent standard or prohibition) is established by the U.S. Environmental Protection Agency, or pursuant to Section 9-314 of the Environment Article, Annotated Code of Maryland, for a toxic pollutant which is present in the discharges authorized herein and such standard is more stringent than any limitation upon such pollutant in this permit, this permit shall be revoked and reissued or modified in accordance with the toxic effluent standard or prohibition and the permittee so notified. Any effluent standard established in this case for a pollutant which is injurious to human health is effective and enforceable by the time set forth in the promulgated standard, even absent permit modification.

8. <u>OIL AND HAZARDOUS SUBSTANCES PROHIBITED</u>

Nothing in this permit shall be construed to preclude the institution of any legal action or relieve the permittee from any responsibility, liability, or penalties to which the permittee may be subject under Section 311 of the Clean Water Act (33. U.S.C. § 1321), or under the Annotated Code of Maryland.

9. CIVIL AND CRIMINAL LIABILITY

Except as provided in permit conditions on "bypassing," "upset," and "power failure," nothing in this permit shall be construed to preclude the institution of any legal action nor relieve the permittee from civil or criminal responsibilities and/or penalties for noncompliance with Title 9 of the Environment Article, Annotated Code of Maryland or any federal, local, or other State law or regulation.

10. PROPERTY RIGHTS/COMPLIANCE WITH OTHER REQUIREMENTS

The issuance of this permit does not convey any property rights in either real or personal property, or any exclusive privileges, nor does it authorize any injury to private property or any invasion of personal rights, nor any infringement of federal, State or local laws or regulations.

11. SEVERABILITY

The provisions of this permit are severable. If any provisions of this permit shall be held invalid for any reason, the remaining provisions shall remain in full force and effect. If the application of any provision of this permit to any circumstances is held invalid, its application to other circumstances shall not be affected.

12. WATER CONSTRUCTION AND OBSTRUCTION

This permit does not authorize the construction or placing of physical structures, facilities, or debris, or the undertaking of related activities in any waters of the State.

13. COMPLIANCE WITH WATER POLLUTION ABATEMENT STATUTES

The permittee shall comply at all times with the provisions of the Environment Article, Title 7, Subtitle 2 and Title 9, Subtitle 3 of the Annotated Code of Maryland and the Clean Water Act, 33 U.S.C. § 1251 et seq.

Permit Number: 10-DP-3060 (MD0066818) Page 21 of 24

14. ACTION ON VIOLATIONS

The issue or reissue of this permit does not constitute a decision by the State not to proceed in administrative, civil, or criminal action for any violations of State law or regulations occurring before the issue or reissue of this permit, nor a waiver of the State's right to do so.

15. CIVIL PENALTIES FOR VIOLATIONS OF PERMIT CONDITIONS

In addition to civil penalties for violations of State water pollution control laws set forth in Section 9-342 of the Environment Article, Annotated Code of Maryland, the Clean Water Act, as adjusted pursuant to 40 C.F.R. § 19.4, provides that any person who violates Section 301, 302, 306, 307, 308, 318 or 405 of the Act, or any permit condition or limitation implementing any of such sections in a permit issued under Section 402 of the Act or in a permit issued under Section 404 of the Act, is subject to a civil penalty not to exceed \$37,500 per day for each violation.

16. <u>CRIMINAL PENALTIES FOR VIOLATIONS OF PERMIT CONDITIONS</u>

In addition to criminal penalties for violations of State water pollution control laws set forth in Section 9-343 of the Environment Article, Annotated Code of Maryland, the Clean Water Act provides that:

- a. any person who negligently violates Section 301, 302, 306, 307, 308, 318, or 405 of the Act, or any permit condition or limitation implementing any of such sections in a permit issued under Section 402 of the Act, or in a permit issued under Section 404 of the Act, is subject to a fine of not less than \$2,500 nor more than \$25,000 per day of violation, or by imprisonment for not more than one (1) year, or by both.
- b. any person who knowingly violates Section 301, 302, 306, 307, 308, 318 or 405 of the Act, or any permit condition or limitation implementing any of such sections in a permit issued under Section 402 of the Act, or in a permit issued under Section 404 of the Act, is subject to a fine of not less than \$5,000 nor more than \$50,000 per day of violation, or by imprisonment for not more than three (3) years, or by both.
- c. any person who knowingly violates Section 301, 302, 306, 307, 318 or 405 of the Act, or any permit condition or limitation implementing any of such sections in a permit issued under Section 402 of the Act, or in a permit issued under Section 404 of the Act, and who knows at that time that he thereby places another person in imminent danger of death or serious bodily injury, is subject to a fine of not more \$250,000 or imprisonment of not more than 15 years, or both.
- d. any person who knowingly makes any false material statement, representation, or certification in any application, record, report, plan, or other document filed or required to be maintained under the Act or who knowingly falsifies, tampers with or renders inaccurate any monitoring device or method required to be maintained under the Act, is subject to a fine of not more than \$10,000 or by imprisonment for not more than two (2) years, or by both.

17. DUTY TO PROVIDE INFORMATION

The permittee shall furnish to the Director, within a reasonable time, any information which the Director may request to determine whether cause exists for modifying, revoking and

reissuing, or terminating this permit, or to determine compliance with this permit. The permittee shall also furnish to the Director, upon request, copies of records required to be kept by this permit.

18. SIGNATORY REQUIREMENTS

All applications, reports, or information submitted to the Director shall be signed and certified as required by 40 CFR 122.22.

19. REOPENER CLAUSE FOR PERMITS

This permit shall be modified, or alternatively, revoked and reissued, to comply with any applicable effluent standard or limitation issued or approved under Sections 301, 304, and 307 of the Clean Water Act [33 USCS §§ 1311, 1314, 1317] if the effluent standard or limitation so issued or approved:

- a. contains different conditions or is otherwise more stringent than any effluent limitation in this permit; or
- b. controls any pollutant not limited in this permit.

This permit, as modified or reissued under this paragraph, shall also contain any other requirements of the Act then applicable.

D. <u>AUTHORITY TO ISSUE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM</u> (NPDES) PERMITS

On September 5, 1974, the Administrator of the U.S. Environmental Protection Agency approved the proposal submitted by the State of Maryland for the operation of a permit program for discharges into navigable waters pursuant to Section 402 of the Clean Water Act, 33 U.S.C. Section 1342.

Pursuant to the aforementioned approval, this discharge permit is both a State of Maryland discharge permit and a NPDES permit.

This permit and the authorization to discharge shall expire at midnight on the expiration date. The permittee shall not discharge after that date unless a new application has been submitted to the Department in accordance with the renewal application provisions of this permit.

D. Lee Currey, Director

Water and Science Administration

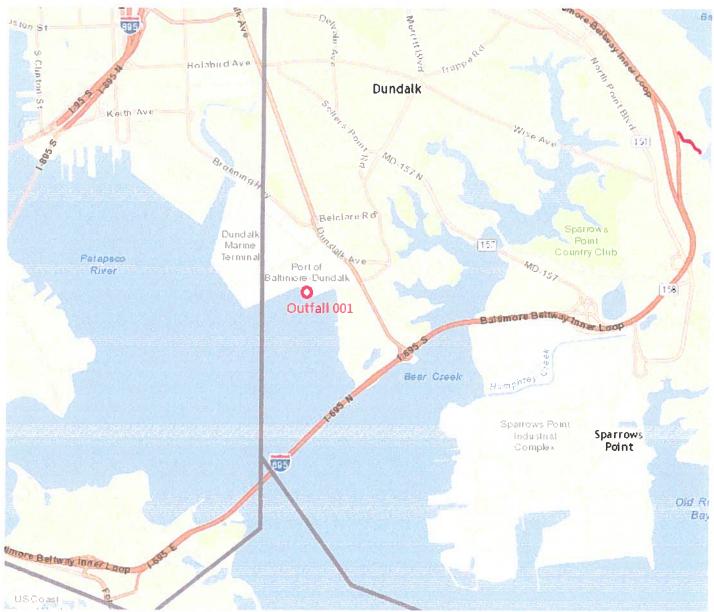


Figure 1 - Area Map showing Outfall Location

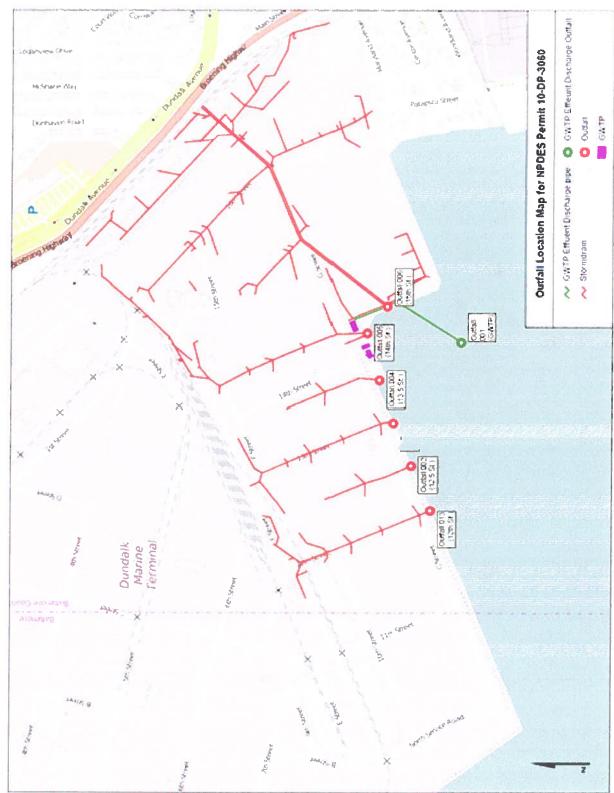
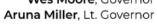


Figure 2 - Drainage Areas and Outfall Locations





Serena McIlwain, Secretary Designate Suzanne E. Dorsey, Deputy Secretary

STATE DISCHARGE PERMIT NUMBER	10-DP-3060B	NPDES PERMIT NUMBER	MD0066818
EFFECTIVE DATE	October 1, 2017	EXPIRATION DATE	September 30, 2022
MODIFICATION A DATE:	January 25, 2023	REAPPLICATION DATE	September 30, 2021

Pursuant to the provisions of Title 9 of the Environment Article, Annotated Code of Maryland, and regulations promulgated thereunder, and the provisions of the Clean Water Act, 33 U.S.C. § 1251 et seq. and implementing regulations 40 CFR Parts 122, 123, 124, and 125, the Department of the Environment, hereinafter referred to as the "Department," hereby authorizes

> Maryland Port Administration 401 E. Pratt Street, Suite 1653 Baltimore, Maryland 21202

TO DISCHARGE FROM

a municipal separate storm sewer system (MS4) containing infiltrated groundwater and a wastewater treatment system at a terminal and cargo handling facility

LOCATED AT

2700 Broening Highway, Baltimore, Maryland

VIA OUTFALLS

001, 005, 006, and 013 as identified and described herein

TO

Patapsco River which is protected for (Use II) water contact recreation, fishing, aquatic life, wildlife in accordance with the following special and general conditions and map(s) made a part hereof.



Larry Hogan, Governor Boyd K. Rutherford, Lt. Governor

Ben Grumbles, Secretary **Horacio Tablada**, Deputy Secretary

Sep 24, 2020

CERTIFIED MAIL

Bill Richardson, Environmental Maryland Port Administration 401 E. Pratt Street, Suite 1653 Baltimore, MD 21202

Re: State Discharge Permit No. 10DP3060A, NPDES Permit MD0066818

Dear Mr. Richardson,

Enclosed are the modified pages from your issued discharge permit, referenced above, with the effective and modified date indicated on the cover page. The permittee is responsible for complying with all permit conditions. You are therefore advised to read the permit carefully and become thoroughly familiar with the requirements.

If you have any other questions, please do not hesitate to call Paul Hlavinka, Industrial Stormwater Permits Division, at 410-537-3323.

Sincerely,

0½ & D Lee Currey (Sep 24, 2020 16:06 EDT)

D. Lee Currey, Director Water and Science Administration

Enclosures (2)

Cc: Arno Laud (delivered electronically)
WMA-Compliance, Central Division Chief



Larry Hogan, Governor Boyd K. Rutherford, Lt. Governor

Ben Grumbles, Secretary **Horacio Tablada**, Deputy Secretary

STATE DISCHARGE 10-DP-3060A PERMIT NUMBER	NPDES PERMIT MD0066818 NUMBER
EFFECTIVE DATE October 1, 2017	EXPIRATION September 30, 2022 DATE
MODIFICATION A DATE: Sep 24, 2020	REAPPLICATION September 30, 2021 DATE

Pursuant to the provisions of Title 9 of the Environment Article, <u>Annotated Code of Maryland</u>, and regulations promulgated thereunder, and the provisions of the Clean Water Act, 33 U.S.C. § 1251 <u>et seq</u>. and implementing regulations 40 CFR Parts 122, 123, 124, and 125, the Department of the Environment, hereinafter referred to as the "Department," hereby authorizes

Maryland Port Administration 401 E. Pratt Street, Suite 1653 Baltimore, Maryland 21202

TO DISCHARGE FROM

a municipal separate storm sewer system (MS4) containing infiltrated groundwater and a wastewater treatment system at a terminal and cargo handling facility

LOCATED AT

2700 Broening Highway, Baltimore, Maryland

VIA OUTFALLS

001, 005, 006, and 013 as identified and described herein

TO

Patapsco River which is protected for (Use II) water contact recreation, fishing, aquatic life, wildlife in accordance with the following special and general conditions and map(s) made a part hereof.

Permit Number: 10-DP-3060A (MD0066818) Page 4A of 24

1. During the effective period of this permit, the permittee must select, design, install, and implement control measures (including best management practices) to minimize pollutant discharges of infiltrated contaminated ground water from the following MS4 stormdrains: Outfall 005 (14 Street Storm Drain), Outfall 006 (15 Street Storm Drain), and Outfall 013 (12 Street Storm Drain). The permittee must minimize exposure of stormwater to infiltrated contaminated groundwater by using available engineering practices as control measures (such as lining of the storm drains, capping the area, etc.).

- 2. The Dundalk Corrective Measures Alternatives Analysis (CMAA) resulted in an approved approach to reline storm drains leading to Outfalls 005, 006 and 013. The permittee shall submit to the Department (WSA) on a quarterly basis the current "Storm Drain Rehabilitation Schedule" (see Definition B.23) via NetDMR.
- 3. If the selected control measures are not achieving the intended effect of minimizing pollutant discharges as indicated by action level results required below, the permittee must modify these control measures per the corrective action requirements in Special Condition Q.
- 4. Beginning on the dates specified in paragraph A.2.5 below, such discharges shall be monitored by the permittee as specified below at the monitoring points established for the outfall pipes to the Patapsco River or, if inaccessible due to tidal influence, at the nearest manhole or tidal exclusion vault upstream of any tidal influence.

PARAMETER	ACTION LEVEL	UNITS	FREQUENCY OF ANALYSIS	SAMPLE	NOTES
Flow	Report	MGD	1/Month	Estimated	
Total Chromium (Wet Weather)	Report	mg/l	1/Month	Composite	(1) (2) (3)
Hexavalent Chromium (Wet Weather)	0.55	mg/l	1/Month	Composite	(1) (2) (3) (4)
Total Chromium (Dry Weather)	Report	mg/l	1/Month	Composite	(1) (4)
Hexavalent Chromium (Dry	0.55	mg/l	1/Month	Composite	(1) (3) (4)

(1) Sampling and reporting requirements apply to discharges from at least one *wet weather* event and one *dry weather* event (refer to Special Condition U) per sampling period for all outfalls except Outfalls 005 and 006. For Outfalls 005 and 006, only *wet weather* monitoring applies, since all *dry weather* flows must be pumped via the wastewater treatment system to Outfall 001 (Special Condition A.1), until such time that the treatment plant is closed and Outfall 001 is removed from the permit after a major permit modification. Sampling results from *dry weather* events shall be reported with the designation "-D" after the Outfall number, such as "Outfall 005-D", and results of *wet weather* events shall be reported with the designation "-W" after the outfall number, such as "Outfall 005-W". If a *dry weather* event or a *wet weather* event discharge does not occur during the reporting period, the permittee shall report "No Discharge" for the applicable parameter(s).

Permit Number: 10-DP-3060A (MD0066818) Page 5A of 24

(2) Beginning on the effective date of the permit, the permittee shall monitor the discharges from Outfalls 005, 006, and 013 at a frequency of twice per year until the effective date of the Action Levels in paragraph A.2.5.

- (3) See "Action Level Implementation Requirements" in Special Condition P. When an action level is exceeded see "Corrective Actions and Deadlines in Special Condition Q."
- (4) Monthly monitoring of an outfall may be reduced to quarterly monitoring after 4 consecutive months of sampling in which all samples have concentrations of Hexavalent Chromium at or below the Action Level, by notifying the Department (WSA). However, if a quarterly sample exceeds the Action Level, monthly monitoring will again be required for a minimum of another 4 months.
- 5. Action Level monitoring shall begin no later than the following dates:

OUTFALL NUMBER	MONITORING START DATE
Outfall 013 (12 Street Storm Drain)	November 1, 2019
Outfall 005 (14 Street Storm Drain)	November 1, 2022
Outfall 006 (15 Street Storm Drain)	November 1, 2022

SPECIAL CONDITIONS

B. DEFINITIONS

- 1. "Action Level": This permit establishes pollutant action level concentrations applicable to discharges from this facility. The action level concentrations are not effluent limitations; and an action level exceedance therefore, is not a permit violation. Action level monitoring data is primarily for the permittee's use to determine the overall effectiveness of control measures, and to indicate when additional corrective actions may be necessary.
- 2. "Bypass" means the intentional diversion of wastes from any portion of a treatment facility.
- 3. "Clean Water Act: means the Federal Water Pollution Control Act, as amended, 33 U.S.C. Section 1251 et seq.
- 4. "CFR" means the Code of Federal Regulations.
- 5. "Composite sample" means a combination of individual samples obtained at least at hourly intervals over a time period. Either the volume of each individual sample is proportional to discharge flow rates or the sampling interval (for constant volume samples) is proportional to the flow rates over the time period used to produce the composite.

Permit Number: 10-DP-3060A (MD0066818) Page 9A of 24

C. TOXIC POLLUTANT REPORTING

The permittee shall notify the Department (WSA) as soon as it is known or suspected that any toxic pollutants at Outfall 001 which are not specifically limited by this permit have been discharged at levels specified in 40 CFR Part 122.42(a).

D. <u>REMOVED SUBSTANCES</u>

- 1. Within 30 days after notification by the Department (WSA), the permittee shall provide information on the disposal of any removed substances, as defined by General Condition B.7, including the following information:
 - a. A suitable map showing all areas used for disposal of removed substances.
 - b. The physical, chemical, and biological characteristics, as appropriate; quantities of any removed substances; and the method of disposal.
 - c. If disposal is handled by persons other than the permittee, identification of the contractor or subcontractor, their mailing address, and the information specified in a and b above.
- 2. The Department's notification may also require the permittee to provide the above information prior to the use of new or additional disposal areas, contractors, or subcontractors.

E. <u>ANALYTICAL LABORATORY</u>

Within 30 days after the effective date of this permit, the permittee shall submit to the Department (WSA) the name and address of the analytical laboratory (including the permittee's own laboratory) which is used to perform the monitoring required by this permit.

If the laboratory changes during the effective period of this permit, the permittee shall notify the Department (WSA) of the new laboratory within 30 days after the change.

F. WASTEWATER OPERATOR CERTIFICATION

As of the effective date of this permit, the permittee's facility shall be operated by an industrial wastewater operator duly certified by the Maryland Board of Waterworks and Waste Systems Operators. Certification shall be for operation of a Class 6 industrial wastewater works, unless the Board determines that a different classification is appropriate. At no time during the effective period of this permit shall the treatment facilities be operated for more than two months without a certified operator.

G. <u>FLOW MONITORING</u>

In lieu of providing measured flow (defined in the Special Conditions Definitions section) at Outfalls 001, 005, 006 and 013, the permittee may estimate flows and submit the following information at the time of submission of the initial discharge monitoring report and/or upon any change in the methodology:

1. a description of the methodology used to estimate flow at each outfall where flow measurement equipment is not present;

Permit Number: 10-DP-3060A (MD0066818) Page 11A of 24

unless the level and the substance were disclosed in writing in the permit application prior to the issuance of the permit. If a discharge regulated by this permit causes or contributes to an exceedance of the water quality standards in COMAR 26.08.02.03, including but not limited to the general water quality standards, or if the discharge includes a pollutant that was not disclosed or addressed in the public record for the permit determination, the Department (WSA) is authorized to modify, suspend or revoke this permit or take enforcement action to address unlawful discharges of pollutants.

O. COMPLIANCE SCHEDULE FOR CYANIDE LIMITS AT OUTFALL 001

- 1. Every six months, the permittee shall submit to the Department (WSA) a status report detailing current plans for meeting cyanide limits in Special Condition A.1. This report is due six months after the effective date of the permit and every six months thereafter until compliance with the permit limits have been achieved or until a permit modification under condition O.2 has been implemented. The plans may include alternative treatment technologies or other discharge options which will result in compliance with the final discharge limitations.
- 2. No later than 24 months after the effective date of this permit, the permittee may apply for a permit modification to revise or remove the permit limit(s) based on any combination of the following options allowed under COMAR 26.08:

Additional Data: As part of the application for a permit modification, the permittee may submit any new or additional data which demonstrates that there is no reasonable potential for violation of applicable water quality standards.

Alternatives for making such a demonstration include, but are not limited to, the following:

- i. collection of effluent data using analytical methods which have lower detection levels;
- ii. collection of data using techniques designed to minimize the effects of contamination on sample results; and
- iii. collection of data utilizing alternate monitoring locations to show compliance with water quality standards.

P. ACTION LEVEL IMPLEMENTATION REQUIREMENTS

- 1. When lining of the storm drain leading to any specific Outfall (005, 006 or 013) is completed, the permittee shall notify the WSA-Compliance Program in writing within 14 days of lining completion for that outfall and shall:
 - a. Include in the notification a copy of the Draft Construction Completion Report:
 - b. Identify and label a discharge monitoring location for the completed outfall and provide a written location description to the Department's WSA-Compliance Program;
 - c. Include updated status details for any remaining Outfalls not yet complete;
 - d. Update the "Stormdrain Inspection and Maintenance Plan" as each outfall is completed: and

Number: 10-DP-3060B (MD0066818) Permit Page 5B of 24

(1) Beginning on the effective date of the permit, the permittee shall monitor the discharges from Outfalls 005, 006, and 013 at a frequency of twice per year until the effective date of the Action Levels in paragraph A.2.5.

- (2) See "Action Level Implementation Requirements" in Special Condition P. When an action level is exceeded see "Corrective Actions and Deadlines in Special Condition Q."
- (3) Monthly monitoring of an outfall may be reduced to quarterly monitoring after 4 consecutive months of sampling in which all samples have concentrations of Hexavalent Chromium at or below the Action Level, by notifying the Department (WSA). However, if a quarterly sample exceeds the Action Level, monthly monitoring will again be required for a minimum of another 4 months.
- 1. Action Level monitoring shall begin no later than the following dates:

OUTFALL NUMBER	MONITORING START DATE
Outfall 013 (12 Street Storm Drain)	November 1, 2019
Outfall 005 (14 Street Storm Drain)	November 1, 2022
Outfall 006 (15 Street Storm Drain)	February 1, 2023

SPECIAL CONDITIONS

B. DEFINITIONS

- 1. "Action Level": This permit establishes pollutant action level concentrations applicable to discharges from this facility. The action level concentrations are not effluent limitations; and an action level exceedance therefore, is not a permit violation. Action level monitoring data is primarily for the permittee's use to determine the overall effectiveness of control measures, and to indicate when additional corrective actions may be necessary.
- 2. "Bypass" means the intentional diversion of wastes from any portion of a treatment facility.
- 3. "Clean Water Act: means the Federal Water Pollution Control Act, as amended, 33 U.S.C. Section 1251 et seq.
- 4. "CFR" means the Code of Federal Regulations.
- 5. "Composite sample" means a combination of individual samples obtained at least at hourly intervals over a time period. Either the volume of each individual sample is proportional to discharge flow rates or the sampling interval (for constant volume samples) is proportional to the flow rates over the time period used to produce the composite.

Appendix G NPDES MS4 General Permit for Discharges from State and Federal Small Separate Storm Sewer Systems

NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM GENERAL PERMIT FOR DISCHARGES FROM STATE AND FEDERAL SMALL MUNICIPAL SEPARATE STORM SEWER SYSTEMS

GENERAL DISCHARGE PERMIT NO. 13-SF-5501 GENERAL NPDES NO. MDR055501

Final Determination: April 27, 2018
Effective Date: October 31, 2018
Expiration Date: October 30, 2023

This National Pollutant Discharge Elimination System (NPDES) general permit covers State and federal small municipal separate storm sewer systems (MS4s) in certain portions of the State of Maryland. MS4 owners and operators to be regulated under this general permit must submit a Notice of Intent (NOI) to MDE by October 31, 2018. An NOI serves as notification that the MS4 owner or operator intends to comply with the terms and conditions of this general permit.

PART I.	COVERAGE UNDER THIS GENERAL PERMIT	1
A.	Permit Area	1
B.	Eligible Small MS4s	1
C.	Obtaining Coverage	1
D.	Definitions	1
PART II. A.	NOTICE OF INTENT REQUIREMENTS Deadlines for Notification	
B.	Contents	2
C.	Where to Submit	3
PART III.	WATER QUALITY	3
PART IV.	MINIMUM CONTROL MEASURES	
A.	Public or Personnel Education and Outreach	
B.	Public or Personnel Involvement and Participation	5
C.	Illicit Discharge Detection and Elimination (IDDE)	6
D.	Construction Site Stormwater Runoff Control	7
E.	Post Construction Stormwater Management	8
F.	Pollution Prevention and Good Housekeeping	9
PART V.	CHESAPEAKE BAY RESTORATION AND MEETING TOTAL MAXIMUM DAILY LOADS	11
A.	Develop a Baseline Impervious Area Assessment	
B.	Develop and Implement an Impervious Area Restoration Work Plan	12
C.	Develop a Restoration Activity Schedule	13
D.	BMP Database Tracking	14
PART VI.	EVALUATION AND ASSESSMENT, RECORDKEEPING, REPORTI AND PROGRAM REVIEW	
A.	Evaluation and Assessment	
B.	Recordkeeping	15
C.	Reporting	15
D.	Program Review	16

PART VII.	STANDARD PERMIT CONDITIONS	
	Duty to Comply	
B.	Failure to Notify	
C.	Limitations on Coverage	
D.	Penalties Under the CWA - Civil and Criminal	
E.	Penalties Under the State's Environment Article - Civil and Criminal	17
F.	Need to Halt or Reduce Activity not a Defense	18
G.	Continuation of an Expired General Permit	18
H.	Duty to Mitigate	18
I.	Duty to Provide Information	18
J.	Other Information	18
K.	Requiring an Individual Permit	18
L.	Property Rights	19
M.	Severability	19
N.	Permit Actions and Reopener Clause	19
O.	Signature of Authorized Administrator and Permittee	20
P.	Inspection and Entry	20
Q.	Proper Operations and Maintenance	20
R.	Reporting Requirements	20
PART VIII.	AUTHORITY TO ISSUE GENERAL NPDES PERMITS	21
APPENDIX	A Maryland Designation Criteria for Small Municipal Separate Stori Systems	
APPENDIX	B Compliance with General Permit Requirements for Small Municip. Separate Storm Sewer Systems	
APPENDIX	C State and Federal Small MS4 Notice of Intent Form and Waiver Fo	orm C-1
APPENDIX	D State and Federal Small MS4 Progress Report	D-1

PART I. COVERAGE UNDER THIS GENERAL PERMIT

A. Permit Area

This National Pollutant Discharge Elimination System (NPDES) general permit covers small municipal separate storm sewer systems (MS4s) owned or operated by the United States of America (U.S.) or the State of Maryland (State) in certain portions of the State of Maryland as defined under Title 40 of the Code of Federal Regulations (CFR) § 122.26(b)(16) and 122.32(a)(1).

B. Eligible Small MS4s

MS4s eligible for coverage under this general permit include those properties that:

- 1. Are owned or operated by the State of Maryland or the U.S. and located within an urbanized area; and
- 2. Serve developed land area greater than five acres and have at least ten percent impervious area property wide; or
- 3. Are already covered under an NPDES small MS4 Phase II general permit.

C. Obtaining Coverage

Owners or operators of MS4s regulated under this general permit must apply for coverage by submitting a Notice of Intent (NOI) according to requirements in Part II below, using the form provided by Maryland Department of the Environment (MDE) in Appendix C. A list of State and federal agencies eligible for permit coverage is found in Appendix A. Others not listed that meet eligibility criteria described in Appendix A are required to file an NOI as well. An NOI may represent:

- 1. An individual MS4 located on a State or federal property; or
- 2. MS4s located on multiple properties owned or operated by a single government agency.

D. Definitions

Terms used in this permit are defined in relevant chapters of 40 CFR § 122 or the Code of Maryland Regulations (COMAR) 26.08.01, 26.17.01, and 26.17.02. Terms not defined in CFR or COMAR shall have the meanings attributed by common use.

PART II. NOTICE OF INTENT REQUIREMENTS

A. Deadlines for Notification

Small MS4 owners or operators in State of Maryland and U.S. government properties that meet the designation criteria in Appendix A must apply for coverage under this general permit and submit to MDE an NOI that contains the information outlined in PART II.B by October 31, 2018.

B. Contents

An NOI serves as notification that the MS4 owner or operator intends to comply with this general permit. A permittee may file an application for an individual property or file a joint application that includes multiple MS4s owned, operated, or maintained by an individual government agency. The NOI form is provided in Appendix C of this permit. The NOI must contain the following:

- 1. The name and address of each property for which coverage under this general permit is being sought;
- 2. A brief description of each property. This must include the approximate size, land uses, a description of the stormwater conveyance system, and a list of properties owned or operated by the permittee covered under the Maryland General Permit for Stormwater Discharges Associated with Industrial Activity or an individual industrial surface water discharge permit;
- 3. The contact name, address, telephone number, and e-mail address of responsible personnel for the required MS4 programs listed in Parts IV and V of this general permit;
- 4. A brief description of any agreements with another entity when responsibilities for permit compliance are shared between the permittee and other entity. The relationship and specific duties of all parties must be provided;
- 5. An estimate of the anticipated expenditures to implement the required programs specified in this general permit; and
- 6. An authorized signature according to Part VII.O of this general permit.

C. Where to Submit

State of Maryland and U.S. government agencies applying for coverage under this general permit must submit NOIs to the following:

Maryland Department of the Environment Water and Science Administration Sediment, Stormwater, and Dam Safety Program 1800 Washington Boulevard Suite 440 Baltimore, Maryland 21230-1708

PART III. WATER QUALITY

State and federal government agencies covered under this general permit must manage, implement, and enforce management programs for controlling all stormwater discharges in accordance with the CWA and corresponding stormwater NPDES regulations, 40 CFR § 122, to meet the following requirements:

- 1. Effectively prohibit pollutants in stormwater discharges or other unauthorized discharges into the MS4 as necessary to comply with Maryland's receiving water quality standards;
- 2. Attain applicable wasteload allocations (WLAs) for each established or approved Total Maximum Daily Load (TMDL) for each receiving water body, consistent with Title 33 of the U.S. Code (USC) 1342(p)(3)(B)(iii); 40 CFR § 122.44(k)(2) and (3); and
- 3. Comply with all other provisions and requirements contained in this general permit, and in plans and schedules developed in fulfillment of this permit.

Compliance with the conditions contained in Parts IV and V of this permit shall constitute compliance with Section 402(p)(3)(B)(iii) of the CWA and adequate progress toward compliance with Maryland's receiving water quality standards and any stormwater WLA approved by the United States (U.S.) Environmental Protection Agency (EPA) for this permit term.

PART IV. MINIMUM CONTROL MEASURES

Permittees must ensure that the following minimum control measures (MCMs) are implemented in the property(ies) served by the small MS4 covered under this permit. The six MCMs described below include Personnel Education and Outreach, Public or Personnel Involvement and Participation, Illicit Discharge Detection and Elimination, Construction Site Stormwater Runoff Control, Post Construction Stormwater Management, and Pollution Prevention and Good Housekeeping. Specific requirements for compliance with this general permit are outlined for

each MCM below. Permittees must report on the status of implementation of these required programs in accordance with the MS4 Progress Report (Appendix D).

Any permittee renewing coverage under the general permit must continue to make progress on permit requirements and report information as described below. All new permittees must begin development of programs described below within the first year of permit issuance and initiate implementation of programs thereafter. Annual MS4 Progress Reports must document program development and demonstrate full implementation of all permit requirements by the end of the five-year permit term.

Permittees can choose to utilize partnerships or share responsibilities with other entities for compliance with any requirement of this general permit. This may entail establishing partnerships with the surrounding county or municipality performing similar activities under the requirements of an NPDES MS4 permit. If responsibilities for permit compliance are shared between the permittee and another entity, the relationship and specific duties of all participating entities must be described in the NOI and updated information provided in the MS4 Progress Report. However, the permittee shall remain responsible for compliance with all conditions of this general permit. For this reason, a legally binding contract, memorandum of understanding (MOU), or other similar means must be executed between the permittee and all other entities to avoid conflicts resulting from noncompliance with this general permit.

A. Public or Personnel Education and Outreach

Permittees are required to implement and maintain a personnel education and outreach program, and distribute education materials to the community and employees to help reduce the discharge of pollutants caused by stormwater runoff. This entails developing brochures, booklets, and training programs to educate personnel about the impacts of stormwater discharges on receiving waters, why controlling these discharges is important, and what personnel and the public and/or staff can do to reduce pollutants in stormwater runoff. These activities may be coordinated with other portions of the permittee's MS4 program or developed independent of other pollution control efforts.

Renewal permittees must update and continue to maintain their personnel education and outreach program. New permittees must begin development of this program within the first year of permit issuance and initiate implementation thereafter. All permittees must provide program updates in accordance with the MS4 Progress Report specified for this MCM. MS4 Progress Reports must document program development and demonstrate full implementation of all permit requirements by the end of the five-year permit term.

- 1. Develop a process by which the public and/or staff can report water quality complaints that must include a phone number, within one year of permit issuance;
- 2. Determine the target audience and develop materials to educate the audience on the impact of stormwater. These topics may include water conservation, chemical

application on lawns and landscaping, proper car wash procedures, proper disposal of paint and other household hazardous waste, recycling and trash pick-up, and proper pet waste disposal;

- 3. Distribute stormwater educational materials through newsletters, website, or other appropriate methods. Submit examples of educational material to MDE in accordance with reporting requirements;
- 4. Develop and implement an annual employee training program that addresses appropriate topics to prevent or reduce the discharge of stormwater pollution into the MS4. Submit example training materials and attendee list to MDE in accordance with reporting requirements; and
- 5. Briefly describe in reports to MDE how the education programs complement and strengthen other programs of the MS4 permit.

B. Public or Personnel Involvement and Participation

Permittees are required to create and foster opportunities for public and/or staff participation in the MS4 management program for controlling stormwater discharges. Recommended activities include adopt-a-stream programs, public and/or staff surveys, storm drain stenciling, stream cleanups, tree plantings, and Earth Day events. These activities may be coordinated with other portions of the permittee's MS4 program or developed independent of other pollution control efforts.

Renewal permittees must update and continue to maintain their public or personnel involvement and participation program. New permittees must begin development of this program within the first year of permit issuance and initiate implementation thereafter. All permittees must provide program updates in accordance with the MS4 Progress Report specified for this MCM. MS4 Progress Reports must document program development and demonstrate full implementation of all permit requirements by the end of the five-year permit term.

- 1. Determine the target audience to promote public and/or staff involvement and participation activities;
- 2. Specify activities appropriate for the target audience and promote participation;
- 3. Perform at least five public and/or staff participation events during the permit term and report to MDE in accordance with reporting requirements;
- 4. Provide public and staff access to the permittee's MS4 Progress Reports via website or other method and consider any substantive public and/or staff comments received concerning the permittee's MS4 program (a permittee may

reserve from public and staff review any information considered confidential or information that may compromise the security of an agency); and

5. Comply with all State and federal public notice requirements for any regulated activity associated with this general permit.

C. Illicit Discharge Detection and Elimination (IDDE)

Permittees are required to develop, implement, and enforce a program to detect and eliminate illicit discharges into the MS4 in accordance with 40 CFR § 122.34(b)(3). A permittee will satisfy this MCM by field screening outfalls, inspecting the MS4 to identify sources of illicit discharges, eliminating illegal connections or illicit discharges, and enforcing penalties where appropriate. The illicit discharge program must also address illegal dumping and spills. Additional guidance is provided in Appendix B, Section II to assist permittees with the development of an acceptable IDDE program.

Renewal permittees must update and continue to maintain their IDDE program. New permittees must begin development of this program within the first year of permit issuance and initiate implementation thereafter. All permittees must provide program updates in accordance with the MS4 Progress Report specified for this MCM. MS4 Progress Reports must document program development and demonstrate full implementation of all permit requirements by the end of the five-year permit term.

- 1. Develop and maintain an updated map of the MS4 that identifies all stormwater conveyances, outfalls, stormwater best management practices (BMPs), and waters of the U.S. receiving stormwater discharges;
- 2. Establish a policy or other agency directive that prohibits illicit discharges into the MS4;
- 3. Maintain the capability to access the storm sewer system across the entire property(ies) to investigate and eliminate illicit discharges (e.g., physical access, proper internal permissions);
- 4. Develop and implement written standard operating procedures (SOPs) that specify the following:
 - a. An inspection checklist describing how outfalls are screened for dry weather flows (see Appendix B, Figure B.2 for an example of an outfall screening checklist);
 - b. Frequency of outfall inspections; Screening efforts for State and federal properties may be tiered based on property size. For small properties (i.e., less than 100 acres), all outfalls must be screened each year. Medium size properties (i.e., 100 2,000 acres) must screen 50% of total outfalls.

- Large properties (i.e., more than 2,000 acres) must screen 20% per year, up to 100 outfalls;
- c. Procedures for identifying the source, and eliminating spills, illegal dumping, and other suspected illicit discharges;
- d. Identification of priority areas for illicit discharge screening based on pollution potential;
- e. Permittee policy to ensure illicit discharges are eliminated;
- f. Procedures to inform employees, businesses, and the general public of the issues relating to illegal discharges and improper waste disposal; and
- g. Coordination with adjacent MS4 operator(s).
- 5. Submit SOPs to MDE for review and approval within two years of permit issuance. MDE will review for consistency with guidance in Appendix B, Section II;
- 6. Document results of illicit discharge screening efforts, including a description of how screening locations were prioritized and any necessary follow-up investigations and remediation measures implemented to address any suspected discharge. Submit to MDE in accordance with reporting requirements; and
- 7. Maintain complete records of IDDE program investigations and make available to MDE during field reviews of the permittee's MS4 program.

D. Construction Site Stormwater Runoff Control

Permittees are required to comply with Environment Article, Title 4, Subtitle 1, Annotated Code of Maryland and State erosion and sediment control regulations under COMAR 26.17.01. The statute and COMAR specify the requirements for any construction activity that disturbs 5,000 square feet of land area or 100 cubic yards or more of earth movement. MDE considers compliance with the State statute to be compliance with this MCM of this general permit, and 40 CFR § 122.34(b)(4).

All permittees must provide program updates in accordance with the MS4 Progress Report specified for this MCM. MS4 Progress Reports must document program development and demonstrate full implementation of all permit requirements by the end of the five-year permit term. In order to comply with State and federal laws and regulations pertaining to an acceptable erosion and sediment control program, all permittees must:

1. Submit erosion and sediment control plans to MDE (or other authority when applicable) for review and approval in accordance with COMAR and with the *Maryland Stormwater Management and Erosion and Sediment Control Guidelines for State and Federal Projects* (February 2015);

- 2. Ensure compliance with requirements under MDE's 2011 Maryland Standards and Specifications for Soil Erosion and Sediment Control or most recent revision and COMAR 26.17.01;
- 3. Ensure all necessary permits have been obtained, including MDE's General Permit for Stormwater Associated with Construction Activity for projects disturbing one acre or more, and local sediment and erosion control plan approval;
- 4. Develop a process for receiving, investigating, and resolving complaints from any interested party related to construction activities within the property(ies). Notify the complainant of the investigation and findings within seven days;
- 5. Track all active grading permits within each property covered under this general permit and report disturbed areas for all active grading permits to MDE in accordance with reporting requirements;
- 6. Ensure that construction site inspections and enforcement procedures are performed in accordance with COMAR. This will require ongoing communication and collaboration with MDE to ensure that any violations are properly addressed;
- 7. Incorporate procedures within property operations to effectively abate sediment pollution and comply with all applicable State and federal laws pertaining to erosion and sediment control practices; and
- 8. Ensure staff is adequately trained on proper procedures and actions to address potential discharge of pollutants into the MS4 as a result of any construction activity. The Responsible Personnel Certification on-line training course through MDE must be made available to appropriate staff.

E. Post Construction Stormwater Management

Permittees are required to maintain an acceptable stormwater management program in accordance with Environment Article, Title 4, Subtitle 2, Annotated Code of Maryland and State stormwater management regulations under COMAR 26.17.02. The statute and COMAR require that stormwater management must be addressed for new development and redevelopment for any proposed project that disturbs 5,000 square feet or more of land area. MDE considers compliance with the State statute to be compliance with this MCM of this general permit, and 40 CFR § 122.34(b)(5).

All permittees must provide program updates in accordance with the MS4 Progress Report specified for this MCM. MS4 Progress Reports must document program development and demonstrate full implementation of all permit requirements by the end of the five-year permit term. In order to comply with State and federal laws and

regulations pertaining to an acceptable stormwater management program, all permittees must:

- 1. Submit stormwater management plans to MDE (or other authority when applicable) for review and approval in accordance with COMAR and with the *Maryland Stormwater Management and Erosion and Sediment Control Guidelines for State and Federal Projects* (February 2015) for compliance with State stormwater management requirements;
- 2. Implement the principles, methods, and practices found in the latest version of the 2000 Maryland Stormwater Design Manual, Volumes I & II (Manual), and the latest version of MDE's Maryland Stormwater Management Guidelines for State and Federal Projects. This includes that environmental site design (ESD) be implemented to the maximum extent practicable (MEP);
- 3. Maintain stormwater program implementation information and provide updates in accordance with the MS4 Progress Report that include:
 - a. Total number of plans submitted to MDE for review and approval;
 - b. Total number of as-built plans submitted to MDE and approved;
 - c. Verification that BMPs are maintained in accordance with MDE requirements outlined on approved plans.
- 4. Provide training to stormwater program staff and to staff responsible for proper BMP design, performance, inspection, and routine maintenance. Report the number of trainings offered, topics covered, and number of attendees; and
- 5. Maintain and submit an Urban BMP database in accordance with the database structure in Appendix B, Tables B.1.a, b, and c. This information shall be submitted to MDE with annual reports.

F. Pollution Prevention and Good Housekeeping

Permittees are required to develop and implement an operation and maintenance program that includes a training component, to prevent and reduce pollutant runoff from municipal operations in accordance with 40 CFR § 122.34(b)(6). A permittee will satisfy this MCM by developing, implementing, and maintaining procedures for pollution prevention and good housekeeping on permittee owned or operated property(ies) and roads as outlined below.

Renewal permittees must update and continue to maintain their pollution prevention and good housekeeping program. New permittees must begin development of this program within the first year of permit issuance and initiate implementation thereafter. All permittees must provide program updates in accordance with the MS4 Progress Report. MS4 Progress Reports must document program development and demonstrate full implementation of all permit requirements by the end of the five-year permit term.

- 1. Ensure that appropriate staff and contractors receive training at least annually. The training must be designed to reduce or eliminate the discharge of pollutants during property operations. Training may include in-person, online, toolbox talks, on-the-job, or other formats, and permittees may build on existing training activities to fulfill this requirement. Topics must include spill prevention and response, proper disposal of waste, and periodic visual inspections to detect and correct potential discharges at properties owned or operated by the permittee;
- 2. Develop, implement, and maintain a good housekeeping plan for permittee owned or operated properties where any of the following activities is performed: maintenance of vehicles or heavy equipment, and handling of any of the following materials: deicers, anti-icers, fertilizers, pesticides, road maintenance materials such as gravel and sand, or hazardous materials. A standard plan may be created to address multiple properties where similar activities are conducted, provided the below items are addressed. The plan must include:
 - a. A description of site activities;
 - b. A list of potential pollutants including their sources and locations on the site. The plan must consider conveyance of stormwater entering, flowing across, and leaving the site;
 - c. Written good housekeeping procedures designed to prevent discharge of pollutants off site that include regular visual inspections to detect potential discharges;
 - d. Written procedures for corrective actions to address any release, spill, or leak on site; and
 - e. Documentation of any discharge, release, leak, or spill, including date, findings, and response actions.
- 3. Quantify and report pollution prevention efforts related to the following activities:
 - a. Number of miles swept and pounds of material collected from street sweeping and inlet cleaning programs, as applicable;
 - b. Good housekeeping methods for pesticide application such as integrated pest management plans or alternative techniques;
 - c. Good housekeeping methods for fertilizer application such as chemical storage, landscaping with low maintenance/native species, and application procedures;
 - d. Good housekeeping methods for snow and ice control such as use of pretreatement, truck calibration and storage, and salt dome storage and containment; and
 - e. Other good housekeeping methods performed by the permittee not listed above.

4. Submit in the NOI a list of properties owned or operated by the permittee where the activities listed in this MCM are performed, and indicate which are covered under the Maryland General Permit for Stormwater Discharges Associated with Industrial Activity. Provide an update in annual reports if the status of industrial activity permit coverage changes for any property.

PART V. CHESAPEAKE BAY RESTORATION AND MEETING TOTAL MAXIMUM DAILY LOADS

Maryland's Watershed Implementation Plan (WIP) specifies the nutrient and sediment load reductions required to address the Chesapeake Bay TMDL by 2025. This general permit will make progress toward that strategy by requiring small MS4s to commence restoration efforts for twenty percent of existing developed lands that have little or no stormwater management. This five-year permit term requires permittees to develop planning strategies and work toward implementing water quality improvement projects. Restoration planning strategies and implementation schedules required under this general permit are consistent with addressing the water quality goals of the Chesapeake Bay TMDL by 2025. The conditions established below require permittees to perform watershed assessments, identify water quality improvement opportunities, secure appropriate funding, and develop an implementation schedule to show the twenty percent impervious area restoration requirement will be achieved by 2025. This constitutes adequate progress toward compliance with Maryland's receiving water quality standards and any stormwater WLA established or approved by the EPA for small MS4s regulated under this permit.

Restoration efforts may include the use of ESD practices, structural stormwater BMPs, retrofitting, stream restoration, or other alternative restoration practices. Trading with other sectors may also be considered as another method to achieve pollutant reductions, once a program has been established, regulations are adopted, public participation requirements are satisfied, and its use approved by EPA. Acceptable design criteria for stormwater BMPs are outlined in the Manual and the most recent version of the *Accounting for Stormwater Wasteload Allocations and Impervious Acres Treated*, referred to hereafter as the Accounting Guidance. Appendix B of this permit provides relevant guidance from the Accounting Guidance for small MS4 permittees to comply with these requirements. A permittee will demonstrate compliance with restoration requirements by performing the following:

A. Develop a Baseline Impervious Area Assessment

Permittees must determine the total impervious surface area within their property(ies) and delineate the portions that are treated with acceptable water quality BMPs. This analysis will provide the baseline used to calculate the twenty percent restoration requirement. This must be done in accordance with the guidance outlined in Appendix B, Section III of this permit (which is consistent with the Accounting Guidance). The impervious area baseline assessment must be submitted with the first year Progress Report for MDE review and approval. The following information must be submitted with this assessment:

- 1. Total impervious acres in accordance with the guidance in Appendix B, Section III of this general permit;
- 2. Total impervious acres treated by stormwater water quality BMPs;
- 3. Total impervious acres treated by BMPs providing partial water quality treatment;
- 4. Total impervious acres treated by nonstructural practices (i.e., rooftop disconnections, non-rooftop disconnections, or vegetated swales);
- 5. Verification that any impervious area draining to BMPs with missing inspection records are not considered treated; and
- 6. Total impervious acres untreated and twenty percent of this total area (i.e., the restoration requirement).

B. Develop and Implement an Impervious Area Restoration Work Plan

Permittees must submit a work plan with the first year MS4 Progress Report to describe the activities and milestones that will be performed over the permit term to show progress toward the twenty percent impervious area restoration requirement. This will form the basis of a long term plan; however, the plan may be adjusted and refined as part of the adaptive management process over the course of the permit term. A work plan, recommended in the format of Table 1 below, must be submitted to MDE annually to describe progress and any modifications necessary to remain on track with restoration requirements. A suggested work plan is provided in Table 1. Permittees may use the work plan or develop a custom plan that addresses the unique circumstances of individual permittees for MDE review and approval.

Table 1. Impervious Area Restoration Work Plan

	Table 1. Impervious Area Restoration Work Plan				
Timeline	Management Strategies and Goals				
Year 1	Develop impervious area baseline assessment.				
	Develop restoration work plan for MDE review and approval.				
	• Assess opportunities and timelines for implementing water quality BMPs.				
	• Assess opportunities to develop partnerships with other NPDES permittees.				
	Determine funding needs and develop a long term budget.				
Year 2	Update and submit Urban BMP database.				
	Maintain inspection records for all BMPs.				
	Perform watershed assessments and identify water quality problems and				
	opportunities for restoration.				
	• Develop list of specific projects to be implemented for restoration and identify				
	on the Restoration Activity Schedule (Table 2).				
	• Incorporate future growth agency-wide/jurisdiction-wide master plans into				
	restoration planning efforts.				
	Evaluate and refine budget needs for project implementation.				

Timeline	Management Strategies and Goals					
Year 3	• Update and submit Urban BMP database and documented maintenance and inspection status for all BMPs.					
	• Develop adaptive management strategies for BMP implementation that identify opportunities for improved processes and procedures.					
	• Continue to identify opportunities for water quality improvement projects and collaborative partnerships to meet restoration requirements.					
Year 4	 Update and submit project implementation status in Table 2. Update and submit Urban BMP database and documented maintenance and inspection status for all BMPs. 					
	• Submit narrative describing progress and updated adaptive management strategies toward implementing restoration projects.					
Year 5	 Update and submit project implementation status in Table 2. Provide complete list of specific projects needed to meet the twenty percent restoration requirement in Table 2 and include the projected implementation year (no later than 2025). 					

C. Develop a Restoration Activity Schedule

Permittees are required to develop a Restoration Activity Schedule (Table 2) and provide annual updates on the status of projects in the planning, construction, and final phase of implementation. A brief narrative must accompany Table 2 and describe progress of planned restoration activities. Table 2 below provides an example of how to submit the required information. The table outlines a schedule for various BMPs under different stages of implementation during the permit term. The impervious acre baseline is indicated as 100 acres and noted in year one. With the implementation of each BMP, the balance toward achieving the restoration requirement is recalculated in the Impervious Acre Restoration Target and Balance ("Imperv Acre Target and Balance") column. This plan must be continuously refined and updated over the duration of the permit term. By the end of the permit term, a complete list of projects required to meet the twenty percent restoration requirement must be provided. The projected implementation year must be no later than 2025.

Permittees may take credit for retrofit and redevelopment that has been implemented between January 1, 2006, and the beginning of the permit term. When the impervious area baseline analysis considers the drainage areas to these practices as untreated, then these projects may be credited toward impervious area restoration requirements. Credits may be reported using the Restoration Activity Schedule (Table 2) discussed below.

Impervious acre credits are based on the level of water quality treatment provided. When water quality BMPs treat one inch of rainfall, the impervious acres draining to the BMP will be considered restored. When the rainfall treated is less than one inch, a proportional acreage will be calculated for impervious acres treated based on the percentage of one inch of rainfall treated. When the rainfall treated is greater than one inch, credit is granted according to the Accounting Guidance. When alternative BMPs are

implemented, acreage may be calculated based on an impervious acre equivalent identified in Appendix B, Table B.4. Additional information on BMP implementation and impervious acre credits may be found in the Accounting Guidance.

Table 2. Restoration Activity Schedule (Example)

	abic 2.	Nesto	Tauvii A	ctivity Sche	dule (L	xampie)		
				Imperv		Year Complete or Projected	MD Coord	
			Imperv	Acre		Implementation		
Type of Restoration	BMP^1	Cost	Acres	Target and	Project	-		
Project	Code	$(\$K)^2$	Treated	Balance	Status ³		Northing	Easting
J		()		100		,	S	<u> </u>
Dry pond retrofit to			36	64				
wet	PWET	1,500			UC			
Bioretention	FBIO	260	6	58	P			
Bioswale	MSWB	100	2	56	P			
Dry pond retrofit to			10	46				
wet	PWET	800			P			
BMP retrofit	PWET	500	8	38	P			
Redevelopment	REDE	300	5	33	P			
Rain Gardens (4)	MRNG	20	2	31	P			
Disconn rooftop r/o	NDRR	200	10	21	P			
Stream restoration								
(1,000 linear feet)	STRE	500	10	11	P			
Outfall Stabilization	OUT	200	2	9	P			
Shallow marsh	WSHW	150	4	5	P			
Reforestation on								
Imperv	IMPF	100	3	2	P			
Green Roof, extensive	AGRE	100	0.5	1.5	P			
Perm pavement on								
existing pavement	APRP	150	2	-0.5	P			

¹ See Appendix B, Tables B.1.a, b, and c, Urban BMP database. BMP codes are identified under "MDE BMP Classification"

D. BMP Database Tracking

Permittees are required to develop a BMP inventory consistent with the required fields outlined in the BMP Database provided in Appendix B, Tables B.1.a, b, and c. A brief narrative must accompany the BMP database and provide verification that routine inspection and maintenance activities are up to date. The database fields for inspection and maintenance need to be completed and show that BMPs are inspected every three years and routinely maintained. If the required inspection and maintenance data are missing or incomplete then any credit previously applied must be removed.

² Provide cost at project completion

³ Project Status: Enter P for planning and design, UC for under construction, and C for complete

PART VI. EVALUATION AND ASSESSMENT, RECORDKEEPING, REPORTING, AND PROGRAM REVIEW

A. Evaluation and Assessment

The permittee must evaluate progress toward achieving compliance with all permit requirements, and the appropriateness of implemented BMPs. This must be achieved through reporting to MDE as specified in Part VI.C below.

B. Recordkeeping

The permittee must keep records for at least three years after the termination of this general permit. In addition to the information required in MS4 Progress Reports specified below, permittees must submit any additional supporting documentation at the request of MDE. The permittee must make its MS4 program information, including records, available to the public during regular business hours.

C. Reporting

- 1. The required information specified in the MS4 Progress Report in Appendix D must be completed as described in this section. The reporting period must be based on State fiscal year, i.e., July 1 June 30. MS4 Progress Reports are due no later than October 31 of each year with the first report due October 31, 2019.
- 2. Annually, the permittee must submit a report to MDE that evaluates progress toward meeting the twenty percent impervious area restoration requirement specified in Part V above. Restoration activity described in the MS4 Progress Report must be completed and include:
 - a. An impervious area baseline analysis in accordance with Part V.A and the guidance in Appendix B, Section III. This analysis must be submitted with the first year MS4 Progress Report for MDE review and approval;
 - b. The Impervious Area Restoration Work Plan (Table 1 or other format) must be submitted with the first year MS4 Progress Report and in annual updates. The work plan must include a narrative discussing progress made toward restoration efforts and a description of adaptive management strategies necessary to keep proposed implementation efforts on track;
 - c. An updated Restoration Activity Schedule in accordance with Table 2 must be submitted annually. By the end of the permit term, a complete list of projects required to meet the twenty percent restoration requirement must be specified in Table 2. The projected implementation year must be no later than 2025; and
 - d. An updated Urban BMP database in accordance with Appendix B, Tables B.1.a, b, and c in electronic format and a brief narrative discussing progress made toward completing the database and performing routine maintenance and inspections.

3. Reporting for the six MCMs specified in Part IV must be submitted in years 2 and 4 of the permit term and include all information requested in the MS4 Progress Report in Appendix D.

D. Program Review

In order to assess the effectiveness of the permittee's NPDES program for eliminating non-stormwater discharges and reducing the discharge of stormwater pollutants to the MEP, MDE will review program implementation as described in MS4 Progress Reports. Procedures for the review of local erosion and sediment control and stormwater management programs exist in Maryland's sediment control and stormwater management laws. Additional reviews of MCM implementation and the twenty percent restoration requirement may be conducted at any time to determine compliance with permit conditions.

PART VII. STANDARD PERMIT CONDITIONS

A. Duty to Comply

The permittee must comply with all conditions of this general permit. Any permit noncompliance constitutes a violation of the CWA and is grounds for enforcement action, permit coverage termination, revocation, or modification. The permittee must comply at all times with the provisions of the Environment Article, Title 4, Subtitles 1, 2, and 4; Title 7, Subtitle 2; and Title 9, Subtitle 3, Annotated Code of Maryland.

B. Failure to Notify

Agencies engaging in an activity under this general permit that fail to notify MDE of their intent to be covered under this general permit as described in PART II and who discharge to waters of the State without submitting an NOI application are in violation of the Environment Article, Annotated Code of Maryland and may be subject to penalties.

C. Limitations on Coverage

- 1. The following categories of non-stormwater discharges or flows must be addressed where such discharges are identified by the permittee as sources of pollutants to waters of the U.S.: landscape irrigation, diverted stream flows, rising groundwater, uncontaminated groundwater infiltration, uncontaminated pumped groundwater, foundation drains, air conditioning condensate, irrigation water, springs, water from crawl space pumps, footing drains, lawn watering runoff, flows from riparian habitats and wetlands, residual street wash water, and discharges or flows from fire fighting activities.
- 2. Non-stormwater sources, stormwater associated with industrial activity, or discharges associated with construction activities may be authorized to discharge

via the municipal separate storm sewer system if such discharges are specifically authorized under an applicable NPDES discharge permit.

3. Only stormwater discharges from municipal separate storm sewer systems are authorized to discharge under this general permit.

D. Penalties Under the CWA - Civil and Criminal

For violations of this permit, the permittee is subject to civil and criminal penalties as set forth in 33 U.S.C. 1319(c) and (d) of the Clean Water Act, as adjusted for inflation according to 40 CFR § 19.4.

E. Penalties Under the State's Environment Article - Civil and Criminal

Nothing in this permit shall be construed to preclude the institution of any legal action or relieve permittee from civil or criminal responsibilities and/or penalties for a violation of Title 4, Title 7, and Title 9 of the Environment Article, Annotated Code of Maryland, or any federal, local, or other State law or regulation. Section 9-342 of the Environment Article provides that a person who violates any condition of this permit is liable to a civil penalty of up to \$10,000 per violation, to be collected in a civil action brought by MDE, and with each day a violation continues being a separate violation. Section 9-342 further authorizes MDE to impose upon any person who violates a permit condition, administrative civil penalties of up to \$10,000 per violation, up to \$100,000.

Section 9-343 of the Environment Article provides that any person who violates a permit condition is subject to a criminal penalty not exceeding \$25,000 or imprisonment not exceeding one year, or both for a first offense. For a second offense, Section 9-343 provides for a fine not exceeding \$50,000 and up to two years imprisonment.

The Environment Article, Section 9-343, Annotated Code of Maryland, provides that any person who tampers with, or knowingly renders inaccurate any monitoring device or method required to be maintained under this permit shall, upon conviction, be punished by a fine of not more than \$50,000 per violation, or by imprisonment for not more than two years per violation, or both.

The Environment Article, Section 9-343, Annotated Code of Maryland, provides that any person who knowingly makes any false statement, representation, or certification in any records or other document submitted or required to be maintained under this permit, including monitoring reports or reports of compliance or noncompliance shall, upon conviction, be punished by a fine of not more than \$50,000 per violation, or by imprisonment for not more than two years per violation, or both.

F. Need to Halt or Reduce Activity not a Defense

It shall not be a defense for a permittee in an enforcement action that it would have been necessary to halt or reduce the permitted activity in order to maintain compliance with the conditions of this permit.

G. Continuation of an Expired General Permit

An expired general permit continues in force and effect for all permittees covered under this general permit until a new general permit is issued or the general permit is revoked or withdrawn. Coverage for new permittees may not be granted under an expired general permit.

H. Duty to Mitigate

The permittee shall take all reasonable steps to minimize or prevent any discharge that has a reasonable likelihood of adversely affecting human health or the environment and is in violation of this general permit.

I. Duty to Provide Information

The permittee shall furnish to MDE any information that may be requested to determine compliance with this general permit. The permittee shall also furnish to MDE, upon request, copies of records required to be maintained in compliance with the conditions of this general permit.

J. Other Information

When a permittee becomes aware that it failed to submit any relevant facts or submitted incorrect information in the NOI or in any other report to MDE, it shall promptly notify MDE of the facts or information.

K. Requiring an Individual Permit

- 1. MDE may require any agency to apply for and/or obtain an individual NPDES permit. When MDE requires a permittee to apply for an individual NPDES permit, MDE will provide notification in writing that an application is required. This notification shall include a brief statement of the reasons for the decision, an application form, and a deadline for filing the application. Applications must be submitted to MDE. MDE may grant additional time to submit an application upon request of the applicant.
- 2. Any agency eligible for coverage under this general permit may request to be excluded from the coverage of this general permit by applying for an individual permit. In such cases, the agency must submit an individual application in

- accordance with the requirements of 40 CFR § 122.26(c)(1)(ii), with reasons supporting the request, to MDE.
- 3. When an individual NPDES permit is issued to an agency eligible for coverage under this general permit, the applicability of this general permit to the individual NPDES permittee is automatically terminated on the effective date of the individual permit. When an individual NPDES permit is denied to an agency otherwise subject to this general permit, then coverage under this general permit may be terminated by MDE.

L. Property Rights

The issuance of this general permit does not convey any property rights of any sort, nor any exclusive privileges, nor does it authorize any injury to private property nor any invasion of personal rights, nor any infringement of federal, State, or local laws or regulations.

M. Severability

The provisions of this general permit are severable. If any provision of this general permit shall be held invalid for any reason, the remaining provisions shall remain in full force and effect. If the application of any provision of this general permit to any circumstances is held invalid, its application to other circumstances shall not be affected.

N. Permit Actions and Reopener Clause

This permit may be modified, revoked and reissued, or terminated for cause. The filing of a request by the Permittee for a permit modification, revocation and reissuance, or termination or a notification of planned changes or anticipated noncompliance does not stay any permit condition. The Environment Article, Section 9-330, Annotated Code of Maryland, provides that MDE may revoke coverage under this permit if it finds that:

- 1. False or inaccurate information was contained in the application;
- 2. Conditions or requirements of the discharge permit have been or are about to be violated;
- 3. Substantial deviation from the requirements has occurred;
- 4. MDE has been refused access for the purpose of inspecting to ensure compliance with the conditions of the discharge permit;
- 5. A change in conditions exists that requires temporary or permanent reduction or elimination of the permitted discharge;

- 6. Any State or federal water quality stream standard or effluent standard has been or is threatened to be violated; or
- 7. Any other good cause exists for revoking the discharge permit.
- 8. If there is evidence indicating that the stormwater discharges authorized by this general permit cause, or have the reasonable potential to cause or contribute to, a violation of a water quality standard, the permittee may be required to obtain an individual permit or the general permit may be modified to include specific limitations and/or requirements. Permit modification or revocation will be conducted according to 40 CFR § 122.62, 122.63, 122.64, and 124.5.

O. Signature of Authorized Administrator and Permittee

All NOIs, annual reports, and information submitted to MDE shall be signed as required by COMAR 26.08.04.01-1 and 40 CFR § 122.22. As in the case of municipal or other public properties, signatories shall be a principal executive officer, ranking elected official, or other duly authorized employee.

P. Inspection and Entry

The permittee shall allow representatives of MDE and EPA access at reasonable times to conduct an inspection of a regulated property or activity, or to review records that must be kept as a condition of this permit.

Q. Proper Operations and Maintenance

The permittee shall properly operate and maintain all BMPs and controls which are used to achieve compliance with the conditions of this permit.

R. Reporting Requirements

The permittee shall report any non-compliance which may endanger human health or the environment. Any information shall be provided orally within 24 hours from the time when the permittee becomes aware of the circumstances. A written submission shall also be provided within five days of the time the permittee becomes aware of the circumstances. The written submission shall contain a description of the non-compliance and its cause; the period of non-compliance, including exact dates and times; if the non-compliance has not been corrected, the anticipated time that it is expected to continue; and steps taken or planned to reduce, eliminate, and prevent reoccurrence of the non-compliance.

PART VIII. AUTHORITY TO ISSUE GENERAL NPDES PERMITS

In compliance with the provisions of the CWA, as amended (33 USC 1251 et seq. the Act), agencies that are defined in Parts I.B.1 and 2 of this general permit and that submit an NOI in accordance with Part II of this general permit are authorized to discharge in accordance with the conditions and requirements set forth herein.

1/pn/27, 2018 Date

D. Lee Currey

Director

Water and Science Administration

APPENDIX A

Maryland Designation Criteria for Small Municipal Separate Storm Sewer Systems

Appendix A

Maryland Designation Criteria for Small Municipal Separate Storm Sewer Systems

Phase I of the U.S. Environmental Protection Agency's (EPA) stormwater program was promulgated in 1990 under the Clean Water Act (CWA). This program relies on National Pollutant Discharge Elimination System (NPDES) permit coverage to address polluted discharges from stormwater runoff from medium and large municipal separate storm sewer systems (MS4s) that serve populations of 100,000 or more. The Phase II program expands Phase I by requiring owners and operators of "small" MS4s in urbanized areas to implement programs to control stormwater runoff through the use of an NPDES permit. A small MS4 can be a municipally owned separate storm sewer system, but can also apply to State and federal agencies, and include transportation, universities, local sewer districts, hospitals, military bases, and prisons. This appendix describes the criteria for regulating small MS4 municipalities and State and federal properties.

Small Municipal Separate Storm Sewer Systems Permit Area

Parts I.A and I.B of the General Permits for Discharges From Small Municipal Separate Storm Sewer Systems for municipalities and for State and federal properties specify that small MS4s in the State of Maryland are regulated if located within the following geographical areas:

- 1. Urbanized areas as determined by the latest Decennial Census by the U.S. Census Bureau. Coverage is required for owners or operators of small MS4s located within the boundaries of an "urbanized area" (UA) based on the 2010 Decennial Census in accordance with 40 CFR § 122.32(a)(1). A map of designated urbanized areas is located at the following website: www.epa.gov/npdes/urbanized-area-maps-npdes-ms4-phase-ii-stormwater-permits
- 2. Other areas determined by MDE to be eligible for coverage. MDE has developed a set of designation criteria for small municipalities located outside of urbanized areas in accordance with 40 CFR § 122.26(a)(9) and 123.35(b)(2).

MS4 General Permit Waiver Criteria

The Code of Federal Regulations specifies that certain municipalities may be waived from permit coverage under the following conditions:

- 1. An MS4 serves a population of less than 1,000 within the urbanized area and does not contribute substantially to the pollutant loadings of a physically interconnected regulated MS4 and stormwater controls are not needed based on wasteload allocations (WLAs) in an EPA approved or established total maximum daily load (TMDL); or
- 2. An MS4 serves a population of less than 10,000 and the permitting authority has evaluated receiving waters and determined that additional stormwater controls are not

needed based on WLAs associated with an EPA approved TMDL or, if a TMDL has not been approved, an equivalent analysis that determines sources and allocations for the pollutants of concern; and has determined that future discharges from the MS4 do not have the potential to result in exceedances of water quality standards or other significant water quality impacts.

In addition to the above waiver criteria, municipalities that discharge stormwater runoff combined with municipal sewage (i.e., combined sewer systems (CSS)) are point sources that are not subject to MS4 requirements (40 CFR § 122.26(a)(7)).

Table A.1 below provides a list of all Maryland counties and their municipalities that are required to be regulated under the MS4 program. The municipalities designated for Phase II MS4 general permit coverage are identified in the table based on the criteria herein. A municipality may request co-permittee status with its respective Phase I or Phase II county. Approximately 40 small municipalities are currently regulated through the MS4 NPDES program as co-permittees within Carroll, Montgomery, and Prince George's Counties.

Table A.1. Phase II MS4 General Permit Designation by County

		mit Designation by County		
Counties and Baltimore City	Jurisdictions Designated for Phase II MS4 Coverage	Justification		
Allegany	N/A	County has CSS		
Anne Arundel	Annapolis	City is located w/in UA		
Baltimore	N/A	Phase I permit covers entire county		
Baltimore City	N/A	Phase I permit covers entire city		
Calvert	Calvert County*	County is located w/in UA and meets MDE		
Caroline	N/A	designation criteria Not located w/in UA		
Carroll	N/A			
		Phase I permit covers all municipalities		
Cecil	Cecil County, Elkton, North East*, Perryville*, and Rising Sun*	County and municipalities are located w/in UA; County also meets MDE designation criteria		
Charles	Indian Head* and La Plata*	Towns are located w/in UA		
Dorchester	N/A	Not located w/in UA		
Frederick	Brunswick, Emmitsburg, Frederick, Middletown, Mount Airy, Myersville, Thurmont, and	Middletown, Mount Airy, and Walkersville are located w/in UA; Brunswick, Emmitsburg, Thurmont, and		
Garrett	Walkersville N/A	Myersville meet MDE designation criteria Not located w/in UA		
Harford	Aberdeen, Bel Air, and Havre de Grace	Towns and city are located w/in UA		
Howard	N/A	Phase I permit covers entire county		
Kent	N/A	Not located w/in UA		
Montgomery	Gaithersburg, Rockville, and Takoma Park	Cities are located w/in UA; Phase I permit covers all other municipalities		
Prince George's	Bowie	City is located w/in UA; Phase I permit covers all other municipalities		
Queen Anne's	Queen Anne's County*	County is located w/in UA and meets MDE designation criteria		
St. Mary's	St. Mary's County*	County is located w/in UA and meets MDE designation criteria		
Somerset	N/A	Not located w/in UA		
Talbot	Easton*	Town meets MDE designation criteria		
Washington	Washington County, Boonsboro*, Hagerstown, Smithsburg, and Williamsport*	County and municipalities are located w/in UA; County also meets MDE designation criteria		
Wicomico	Wicomico County*, Fruitland*, and Salisbury	County and cities are located w/in UA; County also meets MDE designation criteria		
Worcester	N/A	Not located w/in UA		

^{*} Indicates a county or municipality newly designated for coverage as a Phase II small MS4

Eligible State and Federal Properties for MS4 Permit Coverage

The definition of a small MS4 is noted under 40 CFR § 122.26(b)(16)(iii), and specifies these are: "[o]wned or operated by the United States, a State, city, town, borough, county, parish district, association, or other public body" and are "systems similar to separate storm sewer systems in municipalities, such as systems at military bases, large hospitals or prison complexes, and highways or other thoroughfares". Therefore, the CFR definition of a small MS4 indicates that regulated State and federal properties are similar to municipal systems. EPA clarifies that regulated small MS4s should be those that provide stormwater drainage service to human populations, and not to individual buildings (64 Federal Register 68749).

Other available documentation such as federal guidance defining urban areas and literature describing water resource impacts from developed lands are also an important consideration when determining eligibility criteria. For example, the U.S. Census Bureau defines "Nonresidential Urban Territory" in the Federal Register (volume 76, no. 164, August 24, 2011) as those areas that contain a "high degree of impervious surface", or twenty percent impervious area, and are within 0.25 miles of an urban area. Furthermore, documentation that evaluates the potential for properties to contribute pollutants to the MS4 is also considered. For example, *Impacts of Impervious Cover on Aquatic Systems* (Center for Watershed Protection, 2003) indicates that in-stream water quality declines when watershed impervious cover exceeds ten percent.

Based on this information, MDE has determined that an impervious area threshold is appropriate for establishing eligibility criteria for government properties for which agencies are required to obtain MS4 general permit coverage. Eligible properties will be those that have greater than ten percent impervious area. This is a conservative threshold when compared to the U.S. Census Bureau's urban area definition for non-residential urban territory, and considers water quality and natural resource protection. This threshold will allow the focus of the small MS4 program to concentrate on the most developed properties, such as military bases, hospitals, prison complexes, and highways, and is consistent with the intent of federal regulations.

MS4s eligible for coverage under this general permit include those properties that:

- 1. Are owned or operated by the State of Maryland or the U.S. and located within an urbanized area; and
- 2. Serve developed land area greater than five acres and have at least ten percent impervious area property wide; or
- 3. Are those properties already covered under an NPDES small MS4 Phase II general permit.

State and Federal MS4 General Permit Waiver Criteria

MDE may grant a waiver from permit coverage if a State or federal agency does not own or operate a system of conveyances on a property, consistent with the intent of EPA guidelines described above. The owner or operator must demonstrate that the property:

- Is comprised of very discrete areas, such as individual buildings. For example, a small property containing few buildings that have associated parking and driveways with storm drains directly connected to a surrounding MS4 may be eligible for a waiver. On the other hand, properties with numerous buildings, interior roads, and interior storm sewer infrastructure would not qualify for a waiver; and
- 2. Does not discharge a significant amount of pollutants from its MS4; or
- 3. Is not a military base, large hospital complex, prison complex, highway, or thoroughfare, and meets MDE's waiver criteria one or two above.

A State or federal agency that owns or operates any property that meets the eligibility criteria above and is not eligible for a waiver must file an NOI and obtain coverage under the NPDES program and comply with all terms and conditions of this MS4 permit. A list of potential State and federal agencies that may be affected by the eligibility criteria is available in the general permit. Permittees may file joint applications and share responsibilities in an effort to efficiently comply with permit requirements.

Summary

In accordance with the CWA, the criteria described above will require general permit coverage for the small municipalities and State and federal properties that have the greatest likelihood of causing discharge of polluted stormwater runoff. Regulating these small MS4s under the NPDES program will allow implementation of stormwater programs to protect water quality. MDE will consider additional information from municipal, State, or federal MS4 owners or operators regarding eligibility of permit coverage, such as high population and growth areas, as well as whether a system discharges to sensitive waters, is contiguous to other regulated systems, or is a significant contributor of pollutant loadings to a physically interconnected MS4 that is regulated by the NPDES program.

Table A.2. Federal Agencies Potentially Eligible for Permit Coverage

	gencies I otentiany Engine for I erinit Coverage
Federal Agency	Property Name
Amtrak	Multiple properties
Architect of the Capitol	Library of Congress*
	1SG Adam S Brandt Memorial (Curtis Bay)*, Jachman
	USARC*, Jecelin USARC #1*, Prince George's County
Army Reserves	Memorial USARC*
	Beltsville Agricultural Research Center* and National Plant
Dept of Agriculture	Germplasm & Biotechnology Lab*
Dept of Defense, Air Force	Joint Base Andrews*
	Aberdeen Proving Grounds*, Fort Detrick*, Adelphi Lab*,
	Fort George G. Meade*, Washington Aqueduct*, and multiple
Dept of Defense, Army	properties
	Indian Head*, Bethesda*, Carderock*, Naval Academy*, and
Dept of Defense, Navy	multiple properties
Federal Bureau of Prisons	Multiple properties
National Security Agency	Fort Meade* and Friendship Annex
Dept of Homeland Security	FLETC Cheltenham Training Center* and multiple properties
National Park Service	Multiple properties
Dept of Veterans Affairs (VA)	Multiple properties (VA hospitals)
General Services Administration	Multiple properties
National Aeronautics and Space	
Administration	Goddard Space Flight Center*
National Institutes of Health	Bethesda Campus* and multiple properties
National Institute of Standards &	
Technology	Gaithersburg Campus*
Smithsonian Support Center	Suitland property
U.S. Coast Guard	Multiple properties
U.S. Postal Service	William F. Bolger Center* and multiple properties

^{*} Indicates a federal property or agency currently regulated under the Phase II small MS4 program

Table A.3. State Agencies Potentially Eligible for Permit Coverage

State Agency	Property Name
MD Air National Guard	Multiple properties*
MD Army National Guard	Multiple properties*
MD Aviation Authority	Martin State Airport* and multiple properties
MD Dept of General Services	Ellicott City District Court* and multiple
	properties
MD Dept of Health	Multiple properties
MD Dept of Juvenile Services	Multiple properties
MD Dept of Public Safety & Correctional	
Services	Multiple properties
MD Dept of Transportation, Motor Vehicle	
Administration	Multiple properties* including Glen Burnie*
MD Dept of Transportation, Port	
Administration	Multiple properties*
MD Dept of Transportation, Transit	
Administration	Multiple properties*
MD Dept of Transportation, Transportation	
Authority	Multiple properties*
MD Food Center Authority	Multiple properties
MD National Capital Parks & Planning	Montgomery* and Prince George's Parks
MD School for the Deaf	Columbia and Frederick campuses
MD Stadium Authority	Camden Yards Sports Complex*
MD State Police	Multiple properties
Universities	Towson University*, University of Maryland -
	College Park*, and numerous additional
	campuses
Washington Metropolitan Area Transit	Multiple Metro stations*
Washington Suburban Sanitary Commission	Multiple properties*

^{*} Indicates a State property or agency currently regulated under the Phase II small MS4 program

APPENDIX B

Compliance with General Permit Requirements for Small Municipal Separate Storm Sewer Systems

Appendix B

Compliance with General Permit Requirements for Small Municipal Separate Storm Sewer Systems

The Maryland Department of the Environment (MDE) has issued two general discharge permits for small Municipal Separate Storm Sewer Systems (MS4s): one for small municipalities and another for State and federal agencies. These two permits require that management programs be developed to effectively control the discharge of pollutants from stormwater runoff and improve water quality. These small MS4 general permits are issued in accordance with the Clean Water Act (CWA) and corresponding National Pollutant Discharge Elimination System (NPDES) regulations, 40 Code of Federal Regulations (CFR) § 122.26. The permits establish the minimum requirements for municipal and State and federal agencies eligible for coverage under the NPDES program. This appendix provides guidance and additional information related to compliance with permit requirements. The guidance is organized into three sections as follows:

Section I: Describes management options for permit compliance;

Section II: Provides guidance for developing an illicit discharge detection and elimination

program; and

Section III: Provides guidance for developing and implementing a restoration program to meet Chesapeake Bay water quality goals by 2025.

Section I. Management Options for Permit Compliance

According to 40 CFR § 122.30, the U.S. Environmental Protection Agency (EPA) strongly encourages partnerships and the watershed approach as the management framework for efficiently, effectively, and consistently protecting water quality and restoring aquatic ecosystems. This regulation offers flexibility to regulated owners and operators for complying with permit requirements. Therefore, the following options may be considered by small MS4s during planning and implementation efforts. This will allow government agencies and small municipalities to combine resources and collaborate with other NPDES programs to most effectively and efficiently achieve the water quality goals intended in the CWA.

Α. Options for filing a Notice of Intent (NOI) Application.

MDE will allow multiple options for filing an NOI to receive permit coverage. An NOI application may represent an individual government property or multiple properties owned or operated by a single agency. If an NOI represents all storm sewers owned, operated, or maintained by a single agency, the application must specify each individual property to be covered under the permit.

B. Qualifying Local Programs (State or local).

An applicant may develop programs to comply with all minimum control measures independently, or rely on another responsible entity, or rely on a qualifying local program to comply with permit requirements. Maryland has existing State statutes and local ordinances in place that already require implementation of specific management measures that are more stringent than the conditions in 40 CFR § 122. Therefore, the statewide regulatory requirements under the Environment Article, Title 4, Subtitle 1, Annotated Code of Maryland for erosion and sediment control and Title 4, Subtitle 2 for stormwater management are considered to be "qualifying local programs." Compliance with these laws will meet the "Construction Site Stormwater Runoff Control" and "Post Construction Stormwater Management" permit requirements. The permittee remains responsible for the implementation of these measures through compliance with Maryland's erosion and sediment control and stormwater management laws.

C. Sharing Responsibility.

A permittee may rely on another entity such as a State, federal, or municipal partner to satisfy one or more of the permit obligations. All permit obligations of each entity must be noted in the NOI submitted to MDE according to Part II of this general permit and 40 CFR § 122.35. Other responsible entities must implement control measures that are at least as stringent as the corresponding requirements found in this NPDES general permit. Additionally, the other entity must agree to implement the minimum control measures on the permittee's behalf. However, the permittee remains responsible for all regulatory obligations. Therefore, MDE encourages the permittee to enter into a legally binding agreement such as a memorandum of understanding with the other entity to minimize uncertainty about compliance with the permit. This information must be specified in the NOI (Appendix C).

Section II. Illicit Discharge Detection and Elimination (IDDE) Program Guidance

Small municipalities and State and federal agencies covered under this NPDES MS4 permit are required to implement an IDDE program. The goal of this program is to find and eliminate pollutants entering the MS4. IDDE program activities include mapping the stormwater conveyance system, inspecting outfalls to discover polluted discharges, investigating the source of pollution, and taking steps to eliminate the discharge, which may include enforcement actions. Permittees are required to develop standard operating procedures (SOPs) that detail the steps to implement these activities. This section provides guidance that permittees may use as a starting point to develop and implement their programs.

A discharge to an MS4 is illicit if it is not composed entirely of stormwater (40 CFR § 122.26(b)(2)). Illicit discharges can originate from a number of different types of sources, including incorrect plumbing, broken infrastructure, inappropriate business practices, and illegal dumping. For example, sanitary sewer lines or car wash drains may be connected to the MS4 instead of the sanitary sewer system. Drinking water lines or sanitary sewer pipes may be

broken and leaking effluent into the MS4. Businesses may be inappropriately washing vehicles, allowing wash water to drain into stormwater inlets. Illicit discharges may also result from purposeful dumping of pollutants into an MS4.

A. Mapping

As part of their IDDE programs, permittees must develop a map of the MS4 that they own or operate. Map features must include stormwater conveyances, outfalls, stormwater best management practices (BMPs), and waters of the U.S. receiving stormwater

discharges. As defined in 40 CFR § 122.26(b)(9), an outfall is a point source "at the point where a municipal separate storm sewer discharges to waters of the United States" (see Figure B.1). Mapping outfalls, stormwater conveyances, and stormwater BMPs will assist the permittee with tracking the source of a suspected illicit discharge. In this permit term, permittees may prioritize their initial mapping efforts to areas with a higher potential to pollute, such as areas that are urbanized, commercial, or rapidly developing.

If submitting a map would compromise the operational security of a State or federal agency, the agency may indicate that the map is available for MDE review on site.

B. Standard Operating Procedures

Permittees must develop SOPs that outline methods to conduct dry weather outfall inspections, locate



Figure B.1. The above outfalls are examples of different types of outfalls that must be identified on MS4 maps and included in the permittee's screening program. Areas with highly developed land uses (e.g., commercial business complexes, aging infrastructure) have a greater potential to pollute and must be prioritized. Structural stability and erosion concerns should also be identified as part of an effective IDDE program.

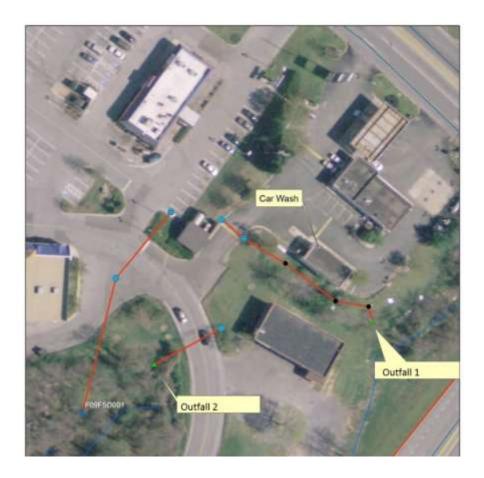
the source of a suspected illicit discharge, and address illicit discharges. Program implementation as detailed in the SOPs can be prioritized in the areas that have a higher potential to pollute (e.g., urbanized, commercial, or areas with older stormwater infrastructure), and must include a long-term schedule for completing a property(ies)-wide map. The SOPs must identify the number of outfalls to be investigated per year and include an inspection checklist to document the outfall screening. A good resource for developing the IDDE program and field checklist is found in the 2004 *Illicit Discharge Detection and Elimination: A Guidance Manual for Program Development and Technical Assessments*, authored by the Center for Watershed Protection and Dr. Robert Pitt. Figure B.2, the "Outfall Reconnaissance Inventory/Sample Collection Field Sheet", is one of several tools permittees may choose to use in their own programs. This checklist will assist a permittee in identifying any potential illicit discharge, determining the need for a more in-depth investigation, and noting any other outfall maintenance needs (e.g., cracks, erosion, excessive vegetation).

A Phase II MS4 municipality must screen 20% of total outfalls per year, up to 100 outfalls. Screening efforts for State and federal properties are tiered based on property size. For small properties (i.e., less than 100 acres), all outfalls must be screened each year. Medium size properties (i.e., 100 - 2,000 acres) must screen 50% of total outfalls. Large properties (i.e., more than 2,000 acres) must screen 20% per year, up to 100 outfalls. A tiered approach takes into consideration the scale of each State or federal property. For example, a small property with a total of five outfalls is expected to screen all five outfalls per year. Likewise, larger properties may screen a smaller percentage per year to account for the increased effort a greater number of outfalls would require.

C. Illicit Discharge Investigation

A dry weather screening is an outfall inspection conducted at a time when rain has not occurred recently (e.g., within the past 48 hours). During a period of dry weather, it is expected that any observed flow would be the result of some type of discharge other than precipitation. In some cases, the permittee may find that an outfall is not a useful inspection point to detect an illicit discharge (e.g., outfall is submerged, significant groundwater flow is present, the outfall serves a large drainage area). In these cases, the permittee has the discretion to pick an inspection point further up the system (e.g., a manhole or inlet, inflow to a stormwater BMP, or point source discharge in a commercial or industrial area) and document the adjustment in the inspection report. MDE encourages approaches where the permittee conducts screenings closer to the source of potential illicit discharges. When a dry weather flow is observed, a permittee must initiate an investigation to discover the source. If the source is determined to be illicit, the permittee is required to take corrective measures to eliminate the discharge and initiate enforcement actions when necessary. Two examples of illicit discharge investigations are provided below to illustrate outfall identification, mapping, and discharge source tracking. These examples are taken from a Phase I MS4 annual report.

Example 1: Illicit Discharge Investigation for Discovered Wash Water



During a dry weather screening of Outfall 1, a flow was observed dripping into green sudsy water that had an oily odor. A chemical test indicated a high level of detergents. In the process of tracking the source, a high level of detergents was detected at Outfall 2, as well. The source was traced to a car wash that was believed to be discharging wash water into the MS4.



Example 2: Illicit Discharge Investigation for Detergents

A dry weather flow was discovered at the outfall of a BMP. A chemical test revealed the presence of chlorine and a high pH. A chemical test at the pond inflow indicated a high level of detergents. Upslope manholes were inspected to determine the path of the discharge. Starting at the point of discharge and inspecting contributing segments of stormwater conveyance pipes (sometimes called a trunk investigation), a single point of flow that exceeded the acceptable level of detergents was isolated. The investigation revealed that the source of the discharge was located within the segment connected to inlets protected by berms on a private commercial business property yard.

D. Illicit Discharge Elimination and Enforcement

After identifying the source of an illicit discharge, a municipal permittee is required to provide notice to the property owner and require that the responsible party takes appropriate action to eliminate the source of the illicit discharge. The permittee may exercise its legal authority to access the property and utilize enforcement. State and federal permittees are required to take appropriate action to eliminate the source of the illicit discharge. These IDDE investigation procedures and enforcement actions must be specified in the permittee's SOPs.

Figure B.2. Outfall Reconnaissance Inventory/Sample Collection Field Sheet

(from Center for Watershed Protection and Pitt, 2004)

Section 1: Backgr Subwatershed:			Outfull ID:	Outfall ID:			
Today's date:			Time (Military):				
Investigators:			Form completed by:				
Temperature (°F):		Rainfall (in.): Last 24 ho	ours: Last 48 hour	S.			
Latitude:	Longitu	de:	GPS Unit:	GPS LMK	H;		
Camera:			Photo #s:	10			
Land Use in Drainag	e Area (Check all that apply):		111				
☐ Indestrial			Open Space				
Ultra-Urban Resi	dential		☐ Institutional				
Suburban Resider	atial		Other:		20		
☐ Commercial			Known Industries	x			
Notes (e.g., origin of	8.5 01 70						
LOCATION	MATERIAL		SHAPE	DIMENSIONS (IN.)	SUBMERGED		
☐ Closed Pipe	PVC HE	PE Eliptical Box	Single Double Triple Other:	Diameter/Dimensions:	In Water: No Partially Fully With Sediment: No Partially Partially		
□ Open drainage	Concrete Earthen rip-rap Other:	☐ Trapezoid ☐ Parabolic ☐ Other:		Depth: Top Width: Bottom Width:			
] In-Stream	(applicable when colle	cting samples)					
low Present?	☐ Yes [] No If N	o, Skip to Section 5				
Flow Description If present)	☐ Trickle ☐ Mo	derate Substantial					
ection 3: Quanti	tative Characterizatio		OR FLOWING OUTFALE	LS			
PAR	AMETER	RESULT		UNIT	EQUIPMENT		
Oslaw #1	Volume			Liter	Bottle		
Flow #1	Time to fill			Sec			
	Flow depth		()	In	Tape measure		
Flow #2	Flow width			Pt, In	Tape measure		
	Measured length			Pt, In	Tape measure		
	Time of travel			S	Stop watch		
Ten	perature			°F	Thermometer		
	pH			pH Units	est strip/Probe		
.00	nmonis			med	Test drin		

Illicit Discharge Detection and Elimination: Technical Appendices

Illicit Discharge Detection and Elimination: Technical Appendices

INDICATOR	CHECK III Present			DESCRIPTION	z		REI	RELATIVE SEVERITY INDEX (1-3)	(1-3)
Odar	0	Sewage	Rancidis	Rancid'sour Petroleum'gas	m/gas	ō	1 - Faint	2 - Easily detected	3 - Noticeable from a distance
Color	0	Clear	Brown Orange	O Gray	D'Yellow	ō	☐ 1 – Faint colors in sample bottle	2 - Clearly visible in sample bottle	3 - Clearly visible in outfall flow
Turbidity	0			See severity		ō	1 - Slight cloudiness	2 - Cloudy	3 - Opaque
Floatables -Does Not Include Trash!!	_	Sewage (Todet Paper	Sewage (Toilet Paper, etc.)	:) Suds		not o	☐ 1 – Few/slight, origin not obvious	2 – Some, indications of origin (e.g., possible sade or oil sheen)	3 - Some, origin clear (e.g., obvious oil sheen, suds, or floating sanitary materials)
Are physical indicators that are not related to flow present? INDICATOR CHECK if Present.	S that are not rela	Present		Tes No	ESCR	(J/No, Skip to Section 6) IPTION	- 16	COMMEN	13
INDICATOR	CHECK if Present	Present			DESCRIPTION			COMMENTS	IS
Outfall Damage		-22	Spalling, (Spalling, Cracking or Chipping Corrosion		Peeling Paint			
Deposits/Stains			Oolty	Plow Line	Paint Other:	ther:			
Absormal Vegetation			□ Excessive	□ Inhibited					
Poor pool quality		<u> </u>	Odors Suds	Colors Colors Excessive Algae	Floatables	Oil Sheen			
Pipe benthic growth		1	Brown	Orange	Oreen	Other			
Section 6: Overall Outfall Characterization	utfall Character	ization							
☐ Unlikely	Potential (presence of two or more indicators)	sence of two c	и more indic		Suspect (one	or more indicat	☐ Suspect (one or more indicators with a severity of 3)	of 3)	
Section 7: Data Collection	ction								
1. Sample for the lab?		-	□ Yes	oN 🗆					
If yes, collected from	ш.		□ How	Dood					
3. Intermittent flow trap set?	sp set?	С	□ Yes	oN C	If Vos. tyme	voe: GOBM	Caulk dam		

Outfall Reconnaissance Inventory Field Sheet

Section 8: Any Non-Illicit Discharge Concerns (e.g., trash or needed infrastructure repairs)?

Section III. Guidance for Impervious Area Restoration Program Development

Small MS4 owners and operators covered under this NPDES general permit are required to commence impervious area restoration for twenty percent of existing developed lands that have little or no stormwater management by the end of the permit term. This requirement supports the Maryland Watershed Implementation Plan (WIP) strategy for achieving nutrient and sediment load reductions on small MS4 properties to address Chesapeake Bay and local total maximum daily loads (TMDLs). Guidance for implementing restoration activities is available in the MDE document *Accounting for Stormwater Wasteload Allocations and Impervious Acres Treated* hereafter referred to as the Accounting Guidance. While the most recent version of the Accounting Guidance should be referenced by all stormwater permittees, the method below highlights the most relevant information from that document for small MS4 owners and operators. This provides a clear outline for compliance with impervious area restoration for small MS4s.

A. Establishing Baselines: Impervious Surface Area Assessment

Permittees must develop an impervious surface area baseline assessment and delineate the areas that are treated with acceptable water quality BMPs to the maximum extent practicable (MEP). This analysis will provide the baseline used to calculate the twenty percent restoration requirement. The following information is needed for this assessment:

- 1. Land Use and Impervious Surface Area Analysis: Evaluate the total impervious surface within a permittee's regulated permit area using the best available land use data that can be generated from the same source from year to year. BMPs designed in compliance with the water quality volume (WQ_v) treatment criteria found in the 2000 Maryland Stormwater Design Manual, Volumes I & II (Manual) are considered to provide water quality treatment to the MEP. Therefore, the impervious area draining to BMPs designed and approved in accordance with the Manual does not need to be counted toward impervious area restoration requirements.
- 2. Urban BMPs: All municipalities and State and federal agencies are required to develop and maintain an Urban BMP database in accordance with Tables B.1.a, b, and c. The database identifies all existing stormwater BMPs along with design, construction, and inspection information. This database and accompanying field inspections must be used to verify the level of water quality treatment provided for an existing BMP. The following guidelines can be used to determine the level of water quality treatment provided by existing stormwater BMPs:
 - BMPs constructed according to the Manual for new development after the baseline year of 2002 provide acceptable water quality treatment. The impervious areas draining to these BMPs do not need to be counted in the impervious area required to be restored.

- BMPs implemented for new development after 2002 may not be used for credit toward impervious area restoration.
- BMPs implemented prior to 2002 may provide some water quality treatment. These include wet ponds, wetlands, and infiltration BMPs. In these cases, the original design parameters for each BMP are needed to verify the level of treatment provided. The impervious area treated is based on the volume provided in relation to the WQ_v (i.e., 1 inch of rainfall). For example, if a BMP was designed to treat a half inch of rainfall, the amount of impervious area treated is 50% of the actual impervious area draining to the BMP.
- BMPs designed for flood control do not provide water quality treatment. The impervious area draining to these BMPs must count toward the baseline.
- Where plans, design specifications, and complete inspection and maintenance records are not available, BMPs are not considered to provide acceptable water quality treatment. Impervious areas draining to these structures must count toward the baseline.
- The impervious area treated by BMPs implemented for retrofitting or redevelopment between January 1, 2002, and December 31, 2005, may be subtracted from the baseline number.

A useful tool for an initial assessment is the Stormwater Management by Era approach documented by MDE in 2009. The approach considers four distinct regulatory eras where stormwater management requirements correlate with a certain level of BMP performance. These eras are as follows:

- Prior to 1985. Stormwater management regulations came into effect after this era. Any development constructed in this time period is most likely untreated (unless retrofits were constructed in later years).
- Between 1985 and 2002. BMPs implemented during this time addressed flood control; however, individual BMP design criteria must be used to verify whether water quality is provided.
- Between 2002 and 2010. The Manual was fully implemented during this era. New development that meets the water quality requirements of the Manual is considered to have acceptable treatment.
- Post-2010. ESD to the MEP is required. Any development project that complied with State regulations in the third and fourth eras is considered to have acceptable water quality treatment.

This approach was used in the development of Maryland's WIP for meeting Chesapeake Bay TMDLs. It can be used for identifying BMPs that provide water quality so that the treated impervious areas may be deducted from the baseline assessment. The stormwater management by era approach can also be valuable for long term planning and for targeting potential areas suitable for retrofitting.

3. Impervious Surfaces in Rural Areas: Many rural roads and residential subdivisions have open vegetated drainage systems, impervious area disconnections, and sheetflow to conservation areas that filter and infiltrate

stormwater runoff. Each permittee must conduct a systematic review of existing rural areas to determine the extent of water quality treatment already provided. This review will also aid in identifying opportunities for retrofitting.

Land use designation can help in selecting areas that are already adequately managed. For example, public roads and residential subdivisions in predominantly rural areas with low population densities (e.g., one or fewer dwelling unit per three acres) may have water quality design features equivalent to those defined in the Manual. Typically, areas that are less than fifteen percent impervious may meet ESD requirements according to the criteria for nonstructural practices in the Manual. These practices include rooftop disconnect, non-rooftop disconnect, and sheetflow to conservation areas. These practices promote sheetflow or treatment through vegetative filtering of runoff. If a permittee documents where conditions meet the Manual's criteria and adequate treatment is provided, then the impervious acres in these areas may be excluded from the baseline. Acceptable documentation can include a comprehensive GIS desktop analysis of land use and zoning conditions and local runoff patterns. Sufficient evidence to justify assumptions in the analysis must be included for MDE review and approval.

4. Total Impervious Acres Not Treated to the MEP: Subtract total impervious areas draining to water quality BMPs and nonstructural practices (determined above) from the total impervious land area owned or operated by the permittee as of the baseline year selected. Restoration requirements will apply to twenty percent of the remaining untreated impervious area at the start of the permit term.

B. Criteria for Impervious Area Restoration Crediting

The water quality objective for impervious area restoration is based on treating the WQ_v (i.e., 1 inch of rainfall) using BMPs defined in the Manual. Because of numerous constraints inherent in the urban environment, meeting the design standards specified in the Manual may not always be achievable. In these cases, retrofit opportunities that currently achieve less than the WQ_v must be pursued where they make sense. Applying impervious area treatment credit for these projects will be based on the proportion of the full WQ_v treated.

Where stormwater retrofits provide water quality treatment for existing unmanaged urban areas, impervious area restoration credit may be applied according to the following criteria:

- An acre for acre impervious credit will be given when a BMP is designed to provide treatment for the full WQ_v (i.e., 1 inch of rainfall); or
- A proportional acreage of credit will be given when less than the WQ_v is provided: (percent of the WQ_v achieved) x (drainage area impervious acres).
- When a BMP is designed to treat greater than one inch of rainfall, additional credit may be granted in accordance with the Accounting Guidance.

C. Acceptable Restoration Strategies

The following are acceptable restoration strategies for receiving impervious area restoration credit. Restoration BMPs may be implemented anywhere within the boundary of the property(ies). Permittees may submit alternative actions to comply with impervious area restoration requirements, subject to MDE approval.

- 1. New Retrofit BMPs: This includes new stormwater BMPs installed to provide water quality treatment for existing developed lands with no controls. Acceptable water quality BMPs and design criteria are provided in the Manual. When a BMP from this list is used and the full WQ_v is provided, the total impervious surface within the drainage area may be credited toward restoration.
- **2. Existing BMP Retrofits:** These are existing BMPs that were not originally designed to provide water quality treatment (e.g., detention pond). As discussed previously, the impervious area draining to these BMPs may not be counted as treated. However, when retrofitted to an acceptable water quality BMP, such as converting a dry pond to a wetland, or providing additional WQ_v storage; the impervious acres draining to the BMP may be credited as restored.
- 3. BMP Enhancement and Restoration: Routine inspection and maintenance is essential to ensure optimal water quality treatment of any BMP. When BMP maintenance has not been performed, substantial structural problems will occur over time, undermining any water quality benefit intended from the practice. Therefore, when BMPs are not properly maintained they may not be considered to provide effective treatment for impervious surfaces. If credit was originally taken for water quality treatment, then future MS4 Progress Reports must remove that credit until the BMP is restored.

MDE has published maintenance guidance for each BMP and specified time periods for inspection and corrective action. This guidance is posted on the MDE stormwater webpage. In addition, the Natural Resources Conservation Service of Maryland has published *Pond Code 378*, which includes an inspection checklist for ponds. Code 378 identifies areas that will cause significant problems if left unaddressed. When inspections and repairs are performed according to these guidelines (or others required by local review authorities), then the BMP is considered properly maintained.

When a BMP has failed and significant structural problems exist, the BMP must be restored to receive proper restoration credit. Restoring a failed BMP must include providing the full WQ_{ν} , and may entail increasing storage capacity, providing forebays, increasing the flow path by installing berms or other design enhancements, re-planting with desirable wetland and native vegetation, or significant sediment clean outs. This restoration credit may apply to failed structures that need water quality enhancements in accordance with Chapter 3 of

MDE's Manual. This is intended to ensure that BMPs are functioning as designed and that routine maintenance is addressed in order for the permittee to keep the credit.

4. Alternative Stormwater BMPs: The Accounting Guidance recognizes that new and innovative approaches to stormwater management are being developed on a continuous basis. Therefore, several alternative BMPs are documented that may be used for the purpose of impervious area restoration. Some of these alternative BMPs include street sweeping, buffer planting, reforestation, stream restoration, inlet cleaning, shoreline stabilization, and others. A list of these alternative BMPs is provided in Table B.3, below. The Accounting Guidance references acceptable criteria for BMP implementation and provides a method for translating pollutant load reductions from alternative BMPs into an impervious acre equivalent in order to credit these practices toward restoration requirements. When innovative practices are approved through Chesapeake Bay Program (CBP) expert panels or by MDE, the associated credits and design criteria may also be used for restoration credit.

Impervious acres treated must be reported according to the "impervious acre equivalent" identified in Table B.4 for each alternative practice. As an example, where stream restoration is proposed, the impervious acre equivalent is equal to 0.01 acre per linear foot. This means that when 1,000 linear feet of stream are restored, then 10 acres of credit may be granted toward impervious area restoration.

- **Trading**: MDE supports trading as a cost effective means for achieving pollutant load reductions. Adoption of new trading regulations in Maryland will include public participation and approval by EPA. Therefore, trading with other source sectors may be an option after formal regulatory procedures are satisfied.
- 6. Redevelopment: Maryland's stormwater management regulations for redeveloped lands are intended to gain water quality treatment on existing developed lands while supporting initiatives to improve urban areas. Therefore, when water quality treatment practices are provided to address State redevelopment regulations, the existing impervious area treated may be credited toward restoration requirements. In most cases the credit will be equivalent to 50% of the existing impervious area for the project. When additional volume above the regulatory requirements is provided, additional credit will be accepted on a proportional basis as described in Appendix B, Section III.A, above.
- 7. Establishing Partnerships and Master Planning: As discussed above, redevelopment activities may be credited toward restoration requirements. This presents an opportunity to develop future growth master plans to provide water quality treatment beyond regulatory requirements. This can be a cost effective solution for addressing Maryland's stormwater management regulations while

incorporating impervious area restoration initiatives into long-range planning efforts.

In addition, government agencies have the opportunity to collaborate with other watershed groups, and State, federal, or local entities to combine resources and facilitate implementation of restoration activities. As discussed in Section I of Appendix B, this could be a formal agreement with another entity and outlined in the NOI application, or this may be a partnership established for an individual project. Because the intent of the small MS4 general permit is to encourage partnerships to achieve the water quality goals of the CWA, MDE will remain flexible when any permittee pursues this option.

D. Urban Best Management Practice (BMP) Database and Codes

The data tables below provide a tracking system for all BMPs. BMP reporting requires populating data from three related tables as follows:

- 1. Table B.1.a: Information in this table must be completed for all structural, ESD, and alternative BMPs.
- 2. Table B.1.b: This table provides more specific information related to structural and ESD practices. The table is linked to Table B.1.a using the common field BMP_ID.
- 3. Table B.1.c: This table provides more specific information related to alternative BMPs. The table is linked to Table B.1.a using the common field BMP_ID.

Data must be submitted in Microsoft Excel spreadsheet format. A map using geographic information system (GIS) software is optional. An Excel spreadsheet template is provided on MDE's Phase II webpage to assist permittees in developing the database.

Some data for older BMPs may not be available, as the information was not required at the time of BMP construction. In these cases, an explanation must be provided. MDE expects that data development and verification will be an ongoing process throughout the permit term and baselines may be adjusted accordingly. Permittees may submit an adjusted impervious area baseline in MS4 Progress Reports to reflect updated information.

Reporting for ESD Practices

ESD practices may be entered as a single structure or as a system of practices. When numerous ESD practices are installed to collectively address stormwater requirements for a project, permittees may choose to enter these data as a system of ESD practices. Data for ESD systems may be captured by specifying:

- The common BMP_ID field will link ESD data in Table B.1.a to Table B.1.b.
- Table B.1.a requires Maryland grid coordinates for each BMP. For ESD systems this location must represent the most downstream point or practice.

- Table B.1.a requires the BMP type (BMP_Type). This is the most predominant BMP type in the ESD system.
- Table B.1.b requires the total number of BMPs (NUM_BMPS) implemented to address stormwater requirements for the ESD system of practices.
- Table B.1.b requires the total rainfall treated (PE_ADR). This represents the total rainfall treated for the collective number of BMPs in the ESD system.

Inspections for ESD Systems

Projects that meet the ESD to MEP requirement may be inspected as a collection of practices. Inspection and maintenance data in Table B.1.a. for ESD systems will represent the performance of the system of practices versus each individual practice. This is consistent with Code of Maryland Regulations 26.17.02.

Table B.1.a BMP Reporting Requirements

Description: This table is to be completed for all structural, ESD, and alternative BMPs.

Column Name	Data Type	Size	Description
BMP_ID	TEXT	13	Unique MDE BMP ID. (Ex: RO12BMP000001, Table
			B.2.a) (Ex: AOC12BMP00001, Table B.2.b)
REPORTING_YEAR	TEXT	4	State fiscal year (YYYY)
MD_NORTH	NUMERIC	8	Maryland grid coordinate Northing (NAD 83 meters)
MD_EAST	NUMERIC	8	Maryland grid coordinate Easting (NAD 83 meters)
PERMIT_NUM	TEXT	10	General Discharge Permit Number (municipal permittees
			use: 13-IM-5500. State and federal permittees use 13-SF-
			5501)
LOCAL_BMP_ID	TEXT	25	Local or State/federal project approval number (optional
			info)
BMP_NAME	TEXT	100	Use BMP names (e.g., Glendale Pond)
BMP_CLASS	TEXT	1	Use BMP classification noted in Table B.3 below (E, S, or
			(A)
BMP_TYPE	TEXT	4	Use BMP Type or most predominant type in Table B.3
			below
CON_PURPOSE	TEXT	4	Enter code for New Development (NEWD),
			Redevelopment (REDE), or Restoration (REST),
			Conversion (CONV)
LAST_INSP_DATE	DATE	8	Last inspection date (MM/DD/YYYY)
BMP_STATUS	TEXT	1	Enter $P = Pass$ or $F = Fail$ for BMP inspection status
MAIN_DATE	DATE	8	Last date maintenance was performed (MM/DD/YYYY);
			field is conditional on the BMP failing an inspection
REINSP_DATE	DATE	8	Next planned inspection date (MM/DD/YYYY)
REINSP_STATUS	TEXT	1	Re-inspection status (i.e., Pass/Fail); This is a follow-up
			inspection after a failed BMP has undergone maintenance
GEN_COMMENTS	TEXT	255	General comments - optional information

Table B.1.b Reporting Requirements for ESD and Structural Practices

Description: More specific data related to ESD and structural BMPs is populated in this table.

Column Name	Data Type	Size	Description
BMP_ID	TEXT	13	BMP_ID linking record to BMP_ID in Table B.1.a
NUM_BMPS	NUMERIC	2	Sum total of BMPs used to meet P _E (enter 1 for a single BMP)
ON_OFF_SITE	TEXT	10	Is the BMP located on the project site or off site
CONVERTED_FROM	TEXT	13	If conversion of existing BMP then prior BMP_ID must be
			entered here. Conditional on Con_Purpose = CONV
BMP_STATUS	TEXT	10	Enter "ACT" for active or "REM" for removed
BMP_DRAIN_AREA	NUMERIC	6	Total drainage area (acres) to a single BMP or ESD system
IMP_ACRES	NUMERIC	8	Total impervious area (acres) to a single BMP or ESD system
PE_ADR	NUMERIC	8	P _E addressed: Water quality treatment reported as rainfall
			(inches) treated for a single BMP or system of ESD practices
			within the drainage area
APPR_DATE	DATE	8	Permit approval date (MM/DD/YYYY)
BUILT_DATE	DATE	8	Construction completion date (MM/DD/YYYY)
GEN_COMMENTS	TEXT	255	General comments - optional information

Table B.1.c Reporting Requirements for Alternative BMPs

Description: More specific data related to alternative BMPs is populated in this table.

Column Name	Data Type	Size	Description
BMP_ID	TEXT	13	BMP_ID linking record to BMP_ID in Table B.1.a
PROJECT_DESC	TEXT	75	Description of project
PROJECT_LENGTH	NUMERIC	8	Length of stream restoration, shoreline or outfall stabilization in feet; Field is conditional on BMP_TYPE = OUT, SHST, or STRE
ACRES_SWEPT	NUMERIC	6	Acres swept for street sweeping (one pass); Field is conditional on BMP_TYPE = MSS or VSS
TIMES_SWEPT	NUMERIC	2	Number of times per year area is swept; Field is conditional on BMP_TYPE = MSS or VSS
ACRES_PLANTED	NUMERIC	6	Acres of trees planted; Field is conditional on BMP_TYPE = FPU or IMPF
IMP_ACR_ELIM	NUMERIC	6	Impervious acres removed to pervious land (IMPP); Field is conditional on BMP_TYPE = IMPP
EQU_IMP_ACR	NUMERIC	6	Equivalent impervious acres treated by alternative BMP (total acres of credit for the alt BMP)
INSTALL_DATE	DATE	8	BMP completion date (MM/DD/YYYY); Field is conditional on BMP_TYPE = OUT, SHST, STRE, SEPC, SEPD, or SEPP
IMPL_COMP_YR	TEXT	4	Year (calendar) of completed Project (YYYY); Field is conditional on BMP_TYPE = MSS, VSS, CBC, SDV, IMPF, IMPP, or FPU
GEN_COMMENTS	TEXT	255	General comments - optional information

BMP ID Field

The BMP_ID is a unique identifier assigned to each BMP or system of BMPs. An example of how to populate the BMP_ID field for a municipality using the required 13 characters is provided:

County or Municipal code + 2 digit year + BMP identifying code + 6 digit sequential number = 13 character BMP_ID code.

Table B.2.a

Year feature/record was captured: 2012	12
Identifying code: BMP	$\stackrel{ op}{BMP}$
Record number: 1	000001
BMP ID	= RO12BMP000001

County or Municipal Codes for Phase II Reporting:

Jurisdiction	Code
Aberdeen	AB
Annapolis	AN
Bel Air	BE
Bowie	ВО
Calvert County	CV
Cecil County (includes North East, Perryville, and Rising Sun)	CE
Easton	EA
Elkton	EL
Frederick County (includes Brunswick, Emmitsburg, Middletown, Myersville,	FR
Thurmont, and Walkersville)	
City of Frederick	FC
Gaithersburg	GA
Hagerstown	HG
Havre de Grace	HV
Indian Head	IH
La Plata	LP
Queen Anne's County	QA
Rockville	RO
Takoma Park	TP
Salisbury	SI
St. Mary's County	SM
Wicomico County (includes Fruitland)	WI
Washington County (includes Boonsboro, Smithsburg, and Williamsport)	WA

State and federal permittees are also required to use a 13 character BMP_ID. Suggested agency codes are listed in the Excel spreadsheet template. If a permittee would like to use a different agency code than found in the template, MDE must approve that alternative agency code to ensure that it is not already in use.

Examples of how to populate the BMP_ID field for a State or federal permittee using the required 13 characters is provided:

Table B.2.b

Agency: Architect of the Capitol	AOC
Year feature/record was captured: 2012	12
Identifying code: BMP	$\overset{+}{BMP}$
Record number: 1	00001
BMP_ID	= AOC12BMP00001

Agency: Maryland Army National Guard	MARNG
Year feature/record was captured: 2012	12
Identifying code: BMP	$\overset{+}{BMP}$
Record number: 1	001
BMP_ID	= MARNG12BMP001

Table B.3 BMP Database Codes: BMP Class and BMP Type

	BMP			
BMP Class	Туре	BMP Type		
	Code	21		
		Alternative Surfaces (A)		
Е	AGRE	Green Roof – Extensive		
Е	AGRI	Green Roof – Intensive		
Е	APRP	Permeable Pavements		
Е	ARTF	Reinforced Turf		
		Nonstructural Techniques (N)		
Е	NDRR	Disconnection of Rooftop Runoff		
Е	NDNR	Disconnection of Non-Rooftop Runoff		
Е	NSCA	Sheetflow to Conservation Areas		
		Micro-Scale Practices (M)		
Е	MRWH	Rainwater Harvesting		
Е	MSGW	Submerged Gravel Wetlands		
Е	MILS	Landscape Infiltration		
Е	MIBR	Infiltration Berms		
Е	MIDW	Dry Wells		
Е	MMBR	Micro-Bioretention		
Е	MRNG	Rain Gardens		
Е	MSWG	Grass Swale		
Е	MSWW	Wet Swale		
Е	MSWB	Bio-Swale		
E	MENF	Enhanced Filters		
		Ponds (P)		
S	PWED	Extended Detention Structure, Wet		
S	PWET	Retention Pond (Wet Pond)		
S	PMPS	Multiple Pond System		
S	PPKT	Pocket Pond		
S	PMED	Micropool Extended Detention Pond		
		Wetlands (W)		
S	WSHW	Shallow Marsh		
S	WEDW	Extended Detention – Wetland		
S	WPWS	Wet Pond – Wetland		
S	WPKT	Pocket Wetland		
	Infiltration (I)			
S	IBAS	Infiltration Basin		
S	ITRN	Infiltration Trench		
-		Filtering Systems (F)		
S	FBIO	Bioretention		
S	FSND	Sand Filter		
S	FUND	Underground Filter		
S	FPER	Perimeter (Sand) Filter		

BMP Class	BMP Type Code	BMP Type		
S	FORG	Organic Filter (Peat Filter)		
S	FBIO	Bioretention		
	Open Channels (O)			
S	ODSW	Dry Swale		
S	OWSW	Wet Swale		
	Other Practices (X)			
S	XDPD	Detention Structure (Dry Pond)		
S	XDED	Extended Detention Structure, Dry		
S	XFLD	Flood Management Area		
S	XOGS	Oil Grit Separator		
S	XOTH	Other		

Alternative BMP Classification, Alternative BMP Type, and Alternative BMP Name

Alt. BMP	BMP	BMP Name	
Class	Type		
	Code		
A	MSS	Mechanical Street Sweeping	
A	VSS	Regenerative/Vacuum Street Sweeping	
A	IMPP	Impervious Surface Elimination (to pervious)	
A	IMPF	Impervious Surface Elimination (to forest)	
A	FPU	Planting Trees or Forestation on Pervious Urban	
A	CBC	Catch Basin Cleaning	
A	SDV	Storm Drain Vacuuming	
A	STRE	Stream Restoration	
A	OUT	Outfall Stabilization	
A	SPSC	Regenerative Step Pool Storm Conveyance	
A	SHST	Shoreline Management	
A	SEPP	Septic Pumping	
A	SEPD	Septic Denitrification	
A	SEPC	Septic Connections to WWTP	
A	NNET	Nutrient Net (Agriculture Trading)	
A	POTW	Publicly Owned Treatment Works (WWTP Trading)	

Table B.4. Alternative Urban BMPs and Impervious Acre Credit

Alternative BMP	Calculating Impervious Acre Credit ¹	Impervious Acre Equivalent
Mechanical Street Sweeping	Acres swept multiplied by $0.07 = acres of credit$	0.07
Regen/Vacuum Street Sweeping	Acres swept multiplied by 0.13 = acres of credit	0.13
Reforestation on Pervious Urban	Acres of reforested land multiplied by 0.38 = acres of credit	
Impervious Urban to Pervious	Acres of reforested land multiplied by 0.75 = acres of credit	0.75
Impervious Urban to Forest	Acres of reforested land multiplied by 1.00 = acres of credit	1.00
Regenerative Step Pool Storm Conveyance (SPSC) ²	Located in dry or ephemeral channels; credit is based on rainfall depth treated	Varies ²
Catch Basin Cleaning	Tons of dry material collected multiplied by 0.40 = acres of credit	0.40
Storm Drain Vacuuming	Tons of dry material collected multiplied by 0.40 = acres of credit	0.40
Mechanical Street Sweeping	Tons of dry material collected multiplied by 0.40 = acres of credit	0.40
Regen/Vacuum Street Sweeping	Tons of dry material collected multiplied by 0.40 = acres of credit	0.40
Stream Restoration	Linear feet of stream restored multiplied by $0.01 = acres$ of credit	0.01
Outfall Stabilization	Linear feet of outfall stabilized multiplied by 0.01 = acres of credit; max credit is 2 acres per project	0.01
Shoreline Management	Linear feet of shoreline restored multiplied by 0.04 = acres of credit	0.04
Septic Pumping	Units pumped (annually) multiplied by 0.03 = acres of credit	0.03
Septic Denitrification	Units upgraded (w/denitrification) multiplied by 0.26 = acres of credit	0.26
Septic Connections to WWTP	Units connected to a WWTP multiplied by 0.39 = acres of credit	0.39

^{1.} For more information on calculating credits for alternative BMPs, see *Accounting for Stormwater Wasteload Allocations and Impervious Acres Treated*.

^{2.} Full impervious area credit is granted when practice treats 1 inch of rainfall. If the full WQ_v is not provided, then the impervious area credit is based on the percentage of 1 inch that is treated. Described in Appendix B, Section III.B.

APPENDIX C

State and Federal Small MS4 Notice of Intent Form and Waiver Form

State and Federal Small MS4 Notice of Intent

Maryland Department of the Environment (MDE)

National Pollutant Discharge Elimination System (NPDES) Small Municipal Separate Storm Sewer Systems (MS4) General Permit

This Notice of Intent (NOI) is intended for State and federal agencies applying for coverage under the General Discharge Permit (No. 13-SF-5501) for Small MS4s. Submitting this application constitutes notice that the agency below agrees to comply with all terms and conditions of the general permit. The information required in this NOI must be submitted to:

Maryland Department of the Environment, Water and Science Administration Sediment, Stormwater, and Dam Safety Program 1800 Washington Boulevard, Baltimore, MD 21230-1708 Phone: 410-537-3543 FAX: 410-537-3553

Web Site: www.mde.maryland.gov

Contact Information

Permittee Name:

Responsible Personnel:					
Mailing Address:					
ı					
Phone Number(s):					
Email address:					
l					
Additional Contact(s):					
Mailing Address:					
Phone Number(s):					
Email address:					
Signature of Responsible Personnel I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.					
Printed Name	Signature	Date			

State and Federal Small MS4 Notice of Intent

Due D	ate:		Date of Submiss	ion:			
Permittee Information:							
	Renew	Renewal Permittee:					
	New P	New Permittee: □					
	Check	if sharing respo	onsibilities with another ent	ity:	□ Yes	□ No	
	Check	if this NOI app	lies to multiple properties:		□ Yes	□ No	
<u>Requi</u>	Required Information:						
1.	multip	le properties are	property(ies) for which covered under this general the specific information re	l permit, _l	provide a	separate)
2.	The ap	pproximate size	of property(ies) in acres:				
3.	Popula	ntion (or number	of employees):				
4.	Maryla	and General Per	rties owned or operated by mit for Stormwater Dischanal industrial surface water	rges Asso	ciated w		
5.	compli	iance with anoth	s that the applicant will sha her entity. Describe the role of agreement when appli	e of all pa			a
6.	. Antic	ipated expendit	ares to implement the terms	s and con	ditions o	f the perr	nit:

State and Federal Small MS4 Waiver Application

Maryland Department of the Environment (MDE)

National Pollutant Discharge Elimination System (NPDES) Small Municipal Separate Storm Sewer Systems (MS4) General Permit

This Waiver Application is intended for State and federal agencies applying for a waiver of coverage under the General Discharge Permit (No. 13-SF-5501) for Small MS4s. The information required in this Waiver Application must be submitted to:

Maryland Department of the Environment, Water and Science Administration Sediment, Stormwater, and Dam Safety Program 1800 Washington Boulevard, Baltimore, MD 21230-1708 Phone: 410-537-3543 FAX: 410-537-3553

Web Site: www.mde.maryland.gov

Contact Information	
Agency Name and Property Name:	
Responsible Personnel:	
Mailing Address:	
Phone Number(s):	
Email address:	
Additional Contact(s):	
Mailing Address:	
Phone Number(s):	
Email address:	
Signature of Responsible	e Personnel
direction or supervision in personnel properly gather person or persons who ma	aw that this document and all attachments were prepared under my accordance with a system designed to assure that qualified and evaluate the information submitted. Based on my inquiry of the mage the system, or those persons directly responsible for gathering mation submitted is, to the best of my knowledge and belief, true,

Date

accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Signature

Printed Name

State and Federal Small MS4 Waiver Application

Due D	Date:	Date of Submission:		
<u>Permi</u>	ittee Information			
Proper	rty(ies) for which the a	gency is requesting a waiver:		
Size a	nd Description of each	property:		
If requ		ore than one property, answer all r each additional property.	l of the following questions on	
1.	. Attach a map of the property showing all directions of stormwater flow (indicate using arrows).			
2.	. Does the site have interior roads? \square Yes \square No			
3.	Does the site discharg	ge a significant amount of pollutar	nts from its MS4? Yes	
4.		r Question 2 OR 3 is Yes, explain a description of land use, site acti pollution sources:		
5.	Describe any stormw the property:	rater controls or pollution control p	programs implemented on	
6.	Explain why the site justify the waiver req	will not contribute substantially to	the downstream MS4, to	

APPENDIX D

State and Federal Small MS4 Progress Report

Maryland Department of the Environment (MDE)

National Pollutant Discharge Elimination System (NPDES) Small Municipal Separate Storm Sewer Systems (MS4) General Permit

This Progress Report is required for those State and federal agencies covered under General Discharge Permit No. 13-SF-5501. Progress Reports must be submitted to:

Maryland Department of the Environment, Water and Science Administration Sediment, Stormwater, and Dam Safety Program 1800 Washington Boulevard, Suite 440, Baltimore, MD 21230-1708 Phone: 410-537-3543 FAX: 410-537-3553

Web Site: www.mde.maryland.gov

Contact Information Permittee Name: Responsible Personnel: Mailing Address: Phone Number(s): Email address: Additional Contact(s): Mailing Address: Phone Number(s): Email address: **Signature of Responsible Personnel** I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. Printed Name Signature Date

Reporting Period (State Fiscal Year):		
Due Date:	Date of Submission:	
Type of Report Submitted:		
Imperv	rious Area Restoration Progress Report (Annual):	
Six Mi	nimum Control Measures Progress (Years 2 and 4):	
Both:		
Permittee Information:		
Renew	al Permittee:	
New Po	ermittee:	

Compliance with Reporting Requirements

Part VI of the Small MS4 General Discharge Permit (No. 13-SF-5501) specifies the reporting information that must be submitted to MDE to demonstrate compliance with permit conditions. The specific information required in this MS4 Progress Report includes:

- 1. Annual: Progress toward compliance with impervious area restoration requirements in accordance with Part V of the general permit. All requested information and supporting documentation must be submitted as specified in Section I of the Progress Report.
- 2. Years 2 and 4: Progress toward compliance with the six minimum control measures in accordance with Part IV of the general permit. All requested information and supporting documentation shall be reported as specified in Section II of the Progress Report. MDE may request more frequent reporting and/or a final report in year 5 if additional information is needed to demonstrate compliance with the permit.

Instructions for Completing Appendix D Reporting Forms

The reporting forms provided in Appendix D allow the user to electronically fill in answers to questions. Users may enter quantifiable information (e.g., number of outfalls inspected) in text boxes. When a more descriptive explanation is requested, the reporting forms will expand as the user types to allow as much information needed to fully answer the question. The permittee must indicate in the forms when attachments are included to provide sufficient information required in the MS4 Progress Report.

Section I: Impervious Area Restoration Reporting Form

Section I: Impervious Area Restoration Reporting

1.	a. Was the impervious area baseline assessment submitted in year 1? $\square_{\mbox{\sc Yes}}$ $\square_{\mbox{\sc No}}$
	b. If No, describe the status of completing the required information and provide a date at which all information required by MDE will be submitted:
2.	 c. Has the baseline been adjusted since the previous reporting year? ☐ Yes ☐ No Complete the information below based on the most recent data:
	Total impervious acres of area covered under this permit:
	Total impervious acres treated by stormwater water quality best management practices (BMPs):
	Total impervious acres treated by BMPs providing partial water quality treatment
	(multiply acres treated by percent of water quality provided):
	Total impervious acres treated by nonstructural practices (i.e., rooftop disconnections,
	non-rooftop disconnections, or vegetated swales):
	Total impervious acres untreated:
	Twenty percent of this total area (this is the restoration requirement):
	Verify that all impervious area draining to BMPs with missing inspection records is not considered treated. Describe how this information was incorporated into the overall analysis:
3.	Has an Impervious Area Restoration Work Plan been developed and submitted to MDE in accordance with Part V.B, Table 1 of the permit or other format? ☐ Yes ☐ No
	Has MDE approved the work plan? ☐ Yes ☐ No

Section I: Impervious Area Restoration Reporting

	Describe progress made toward restoration planning, design, and construction efforts and describe adaptive management strategies necessary to meet restoration requirements by the end of the permit term:
4.	Has a Restoration Schedule been completed and submitted to MDE in accordance with Part V.B, Table 2 of the permit? \square Yes \square No
	In year 5, has a complete restoration schedule been submitted including a complete list of projects and implementation dates for all BMPs needed to meet the twenty percent restoration requirement? \square Yes \square No
	Are the projected implementation years for completion of all BMPs no later than 2025? $\hfill Tes $
	Describe actions planned to provide a complete list of projects in order to achieve compliance by the end of the permit term:
	Describe the progress of restoration efforts (attach examples and photos of proposed or completed projects when available):
5.	Has the BMP database been submitted to MDE in Microsoft Excel format in accordance with Appendix B, Tables B.1.a, b, and c? ☐ Yes ☐ No
	Is the database complete? ☐ Yes ☐ No
	If either answer is No, describe efforts underway to complete all data fields, and a date that MDE will receive the required information:

Section I: Impervious Area Restoration Reporting

- 6. Provide a summary of impervious area restoration activities planned for the next reporting cycle (attach additional information if necessary):
 7. Describe coordination efforts with other agencies regarding the implementation of impervious area restoration activities:
- 8. List the total cost of developing and implementing impervious area restoration program during the permit term:

Section II: Minimum Control Measures Reporting Forms

MCM #1: Personnel Education and Outreach

1.	Does the permittee maintain a process and phone number for the public and/or staff to report water quality complaints? Yes No Number of complaints received: Describe the actions taken to address the complaints:
2.	Describe training to employees to reduce pollutants to the MS4:
3.	Describe the target audience(s):
4.	Are examples of educational/training materials attached with this report? Yes No
	Provide the number and type of educational materials distributed:
	Describe how the personnel education program is appropriate for the target audience(s):
5.	Describe how stormwater education materials were distributed to the public and/or staff (e.g., newsletters, website):
6.	Describe how educational programs facilitated efforts to reduce pollutants in stormwater runoff:
7.	Provide a summary of activities planned for the next reporting cycle:
8.	List the total cost of implementing this MCM over the permit term:

MCM #2: Public or Personnel Involvement and Participation

1.	Describe how the public or personnel involvement and participation program is appropriate for the target audience(s):
2.	Quantify and report public and/or staff involvement and participation efforts as shown below where applicable.
	Number of participants at public and/or staff events:
	Quantity of trash and debris removed at clean up events:
	Number of employee volunteers participating in sponsored events:
	Number of trees planted:
	Length of stream cleaned (feet):
	Number of storm drains stenciled:
	Number of public notices published to facilitate public and/or staff participation:
	Number of public and/or staff meetings organized:
	Total number of attendees at all public and/or staff meetings:
	Describe the agenda, items discussed, and collaboration efforts with interested parties for public and/or staff meetings:
	Describe how public and/or staff comments have been incorporated into the permittee's MS4 program, including water quality improvement projects to address impervious area restoration requirements:
	Describe any additional events and activities if applicable:

MCM #2: Public or Personnel Involvement and Participation
3. Provide a summary of activities planned for the next reporting cycle:
4. List the total cost of implementing this MCM for the permit term:

MCM #3: Illicit Discharge Detection and Elimination (IDDE)

1.	Does the permittee maintain a map of the MS4 owned or operated by the permittee, including stormwater conveyances, outfalls, stormwater best management practices (BMPs), and waters of the U.S. receiving stormwater discharges? Yes No If Yes, attach the map to this report and provide a progress update on any features that are still being mapped. (If submitting a map would compromise the operational security of an agency, indicate that the map is available for MDE review on site.) If No, detail the current status of map development and provide an estimated date of submission to MDE:
2.	Does the permittee have a policy, or other agency directive, that prohibits illicit discharges? Yes No
	If Yes, describe the policy utilized for enforcement by the permittee (alternatively, a link may be provided to the permittee's webpage where this information is available). If No, describe the permittee's plan, including approximate time frame, to establish a policy that prohibits illicit discharges into the storm sewer system:
3.	Did the permittee submit to MDE standard operating procedures (SOPs) in accordance with Part IV.C of the permit? Yes No
	If No, provide a proposed date that SOPs will be submitted to MDE. MDE may require more frequent reports for delays in program development:
	Did MDE approve the submitted SOPs? ☐ Yes ☐ No
	If No, describe the status of requested SOP revisions and approximate date of resubmission for MDE approval:
4.	Describe how the permittee prioritized screening locations in areas of high pollutant potential and identify the areas within which screenings were conducted during this reporting period:

MCM #3: Illicit Discharge Detection and Elimination (IDDE)

5.	Answers to the following questions must reflect this two-year reporting period.
	How many outfalls were identified on the map?
	How many outfalls were required to be screened for dry weather flows to meet the minimum numeric requirement based on property size?
	How many outfalls were screened for dry weather flows?
	Per the permittee's SOP, how frequently were outfalls required to be screened?
	At what frequency were outfalls screened during the reporting period?
	How many dry weather flows were observed?
	If dry weather flows were observed, how many were determined to be illicit discharges?
	Describe the investigation process to track and eliminate each suspected illicit discharge and report the status of resolution:
6.	Describe maintenance or corrective actions undertaken during this reporting period to address erosion, debris buildup, sediment accumulation, or blockage problems:
7.	Is the permittee maintaining all IDDE inspection records and are they available to MDE during site inspections? \square Yes \square No
8.	If spills, illicit discharges, and illegal dumping occurred during this reporting period, describe the corrective actions taken, including enforcement activities, and indicate the status of resolution:
9.	Attach to this report specific examples of educational materials distributed to the public and/or staff related to illicit discharge reporting, illegal dumping, and spill prevention. If these are not available, describe plans to develop public and/or staff education

MCM #3: Illicit Discharge Detection and Elimination (IDDE)

materials and submit examples with the next Progress Report:
10. Specify the number of employees trained in illicit discharge detection and spill prevention:
11. Provide examples of training materials. If not available, describe plans to develop employee training and submit examples with the next Progress Report:
12. List the cost of implementing this MCM during this permit term:

MCM #4: Construction Site Stormwater Runoff Control

1.	Does the permittee have a process for receiving, investigating, and resolving complaints from interested parties related to construction activities and erosion and sediment control? \square Yes \square No
	Describe the process:
	Provide a list of all complaints and a summary of actions taken to resolve them:
2.	Total number of active construction projects within the reporting period:
	Provide a list of all construction projects and tabulate the total disturbed area:
3.	Total number of violation notices issued by MDE related to this MCM on the agency's property:
	Describe the status of enforcement activities:
	Describe how the permittee communicates and collaborates with MDE to maintain compliance with this MCM for all active construction projects on the agency's property:
	Are erosion and sediment control inspection records retained and available to MDE during field review of the agency MS4 program? \[\subseteq \text{Yes} \subseteq \text{No} \]
	If No, explain:
4.	Number of staff trained in MDE's Responsible Personnel Certification:
5.	Describe the coordination with other entities regarding implementation of this MCM:

MCM #4: Construction Site Stormwater Runoff Control

6. List the total cost of implementing this MCM over the permit term:

MCM #5: Post Construction Stormwater Management

1.	Has an Urban BMP database been submitted in accordance with the database structure in Appendix B, Tables B.1.a, b, and c as a Microsoft Excel file? Yes No Describe the status of the database, efforts to complete all data fields, and provide a date as to when the required information will be submitted to MDE:
2.	Total number of plans submitted to MDE for review and approval:
	Total number of as-built plans submitted to MDE:
	Number of submitted as-built plans approved by MDE:
3.	Total number of BMPs located on each property covered under the general permit (list individual property, and total BMPs for that property – provide separate attachment if necessary):
	Does the permittee perform inspections for all structural BMPs in accordance with the Dam Inspection Checklist in Maryland Pond Code 378 at least once every three years? Yes No
	If No, describe efforts to train staff and develop a program to perform these required inspections on a triennial basis:
	Are BMP inspection records retained and available to MDE during field review of local programs? \square Yes \square No
4.	Provide a summary of routine maintenance activities for all BMPs:
	Are BMP maintenance procedures consistent with maintenance requirements on MDE approved plans? Yes No

MCM #5: Post Construction Stormwater Management

	Are completed BMP maintenance checklists available to MDE during field review of local programs? \[\subseteq \text{Yes} \text{No} \]
	If either answer is No, describe planned actions to implement maintenance checklists and procedures and provide formal documentation of these activities:
	Describe all problems discovered during routine maintenance operations and repair work performed to restore the function of the BMP(s) (attach photos and additional documentation as needed):
5.	Number of staff trained in proper BMP design, performance, inspection, and routine maintenance:
6.	Provide a summary of activities planned for the next reporting cycle:
7.	List the total cost of implementing this MCM over the permit term:

MCM #6: Pollution Prevention and Good Housekeeping

	1.	Provide a list of topics covered during the last training session related to pollution prevention and good housekeeping, and attach to this report specific examples of training materials:
		List all training dates within this two-year reporting period:
		Number of staff attended:
	2.	Are the good housekeeping plan and inspection records at each property retained and available to MDE during field review of the local program? Yes No
		If No, explain:
		Provide details of all discharges, releases, leaks, or spills that occurred in the past reporting period using the following format (attach additional sheets if necessary).
		Property Name: Date:
		Describe observations:
		Describe permittee's response:
:	3.	Quantify and report property management efforts as shown below, where applicable (attach additional sheets if necessary).
		Number of miles swept:
		Amount of debris collected from sweeping (indicate units):
		If roads and streets are swept, describe the strategy the permittee has implemented to maximize efficiency and target high priority areas:
		Number of inlets cleaned:
		Amount of debris collected from inlet cleaning (indicate units):

MCM #6: Pollution Prevention and Good Housekeeping

	Describe how trash and hazardous waste materials are disposed of at permittee owned and operated property(ies), including debris collected from street sweeping and inlet cleaning:
	Does the permittee have a current State of Maryland public agency permit to apply pesticides? \square Yes \square No
	If No, explain (e.g., contractor applies pesticides):
	Does the permittee employ at least one individual certified in pesticide application? \square Yes \square No
	If Yes, list name(s):
	If the permittee applied pesticides during the reporting year, describe good housekeeping methods (e.g., integrated pest management, alternative materials/techniques):
	If the permittee applied fertilizer during the reporting year, describe good housekeeping methods (e.g., application methods, chemical storage, native or low maintenance species, training):
	If the permittee applied materials for snow and ice control during the reporting year, describe good housekeeping methods (e.g., pre-treatment, truck calibration and storage, salt domes):
	Describe good housekeeping BMP alternatives not listed above:
4.	If applicable, provide a status update for permittee owned or operated properties regarding coverage under the Maryland General Permit for Stormwater Discharges Associated with Industrial Activity or an individual industrial surface water discharge permit:
5.	List the total cost of implementing this MCM over the permit term: