Seepage Rehabilitation for Embankment Dams

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Presentation Content

• Seepage control objectives and categories of options
• Seepage collection and control
• Seepage reduction (barriers)
Seepage Control Objectives

- Prevent Piping and Internal Erosion
- Limit Pore Pressures, Uplift, and Seepage Forces
- Prevent Slope Instability and Surface Sloughing
- Prevent “Wet Spots” and Surface Erosion
- Limit loss of stored water (operational concern, not dam safety)
Seepage Rehabilitation Methods

• Two Broad Categories
  – Collection and Control
  – Seepage Reduction (Barriers)

• “Best” Solution
  – Depends on particular dam and foundation
  – Consider full range of alternatives – avoid tunnel vision or bias
  – Sometimes a combination of both are used
Barriers and Collection
Some Collection and Control Considerations

- Can construct remediation where seepage has been observed
- Often can directly observe placement of all elements of construction
- May require reservoir lowering
- May require dewatering
Some Collection and Control Considerations

Seepage does not threaten dam safety, . . . . when it is directed to through a filtered exit.
R.B. Peck, lecture, 1985

Regarding the idea of designing dams for controlled under seepage, without filters –

\[ I \textit{consider we have gotten scared out of doing this.} \]

Effective filters are critical to success of seepage collection and control alternatives.

Filtered Exit

Seepage can be addressed by providing a 'filtered exit' (toe drain)
Nomenclature

Old

“Filter” and “Drain”

Historically used interchangeably as nouns and verbs.

New

Filter = first stage, primary function is to provide filter

Drain = second stage, primary function is to provide flow capacity
Tools to Provide Collection and Control

- Filters to limit piping potential
- Drains to collect and convey seepage
- Berms to resist uplift and provide stability
- Relief wells to reduce uplift

These tools can be used in different combinations as illustrated in the next several slides.
Filter/Drain Blankets

- Generally for limited seepage
- Can also be applicable to abutment seepage
Filter/Drain Blankets and Berm

- Berm provides resistance for pressure and improved stability
Seepage Berm Over Confining Layer

- Not highly effective - seepage will not enter drainage layer because it is blocked by the confining layer
If foundation horizontal permeability is very high, the seepage berm may only ‘push’ the seepage downstream.

A vertical drainage element (toe drain or vertical trench) may be needed to intercept horizontal flow.
Shallow Toe Drain

- For limited, shallow foundation seepage
- Likely requires dewatering
- May require reservoir lowering
Deep Toe Drain

- For more extensive, deep foundation seepage
- Almost certainly requires dewatering
- Likely requires reservoir lowering
Toe Drain and Berm

- Addresses foundation and embankment seepage
- Berm provides resistance for pressure and improved stability
Trench Toe Drain Trench and Berm

- With slurry (biodegradable) methods, trench drain can be constructed without dewatering
- Potential constructability issues
  - Slurry does not revert
  - Trench instability
  - Backfill contamination
Chimney Filter Overlay

- Relatively simple solution for embankment seepage
- May not require reservoir lowering
- Embankment zoning can cause complications (e.g. coarse downstream shells)
Internal Chimney Filter

- Comprehensive embankment solution
- Construction risks need to be addressed
- Reservoir lowering almost certainly required
New Creek 14 - Typical Section

Courtesy of T. Brown, NRCS
Blanket Drain Detail

Filter – ASTM C33 Fine Aggregate
Drain – ASTM # 8
Drain and Filter Placement

Courtesy of T. Brown, NRCS
Welding HDPE Toe Drain

Courtesy of T. Brown, NRCS
Placing Fill on Top of Blanket

Courtesy of T. Brown, NRCS
Placement of Chimney Filter and Berm

Courtesy of T. Brown, NRCS
Benefits of Chimney Filters

- Provide protection against internal erosion through defects
- Lower phreatic surface
  - Preventing breakout of seepage on downstream face
  - Increasing stability of downstream slope
Prevention of Internal Erosion Through Defects

Without Chimney Filter

With Chimney Filter
Lowering of Phreatic Surface

Without Chimney Filter

With Chimney Filter
Top Elevation for Chimney Filter

• Historic practice – top of estimated phreatic surface

• Current practice
  – Top of maximum normal pool as a minimum
  – Often top of maximum flood pool or dam crest
J.L. Sherard, lecture, 1984¹

• P. 5. I believe there is already sufficient evidence from dam behavior, supported by theory, to require the designer to assume that small concentrated leaks can develop through the impervious section of most embankment dams, even those without exceptional differential settlement.

One-Stage vs. Two-Stage Chimneys

Two-stage:
  • Provides capacity to handle large flow
  • Addresses potential negative effects of filter contamination
Prevent Concentrated Flows from Overwhelming Filter

Diagram showing the flow of concentrated seepage and the effect of a filter and drain on accommodating it.
Negate Effects of Filter Contamination
Relief Wells

- Relieve pore pressures and lower piezometric surface within **confined** pervious foundation strata
- Reduce uplift and improve stability
- Control exit gradients and reduce piping potential
- Maintenance required
- Possible limitation of radius of influence
- Drain trench may be better alternative
Relief Wells

- Low permeability layer
- Confined aquifer with seepage pressure
- Relief Wells
- Piezometric level in confined aquifer without relief wells
- Piezometric level in confined aquifer with relief wells
Piping failure along unprotected conduits is a leading cause of dam failure.
Outlet Conduit Penetrations
Little Wewoda, Site 17, OK

Courtesy: D. McCook
How is compaction achieved here?
Filter Diaphragm or Collar for Outlet Works

For pipes less than 3 ft in diameter

Dimensions recommended by NRCS for filter diaphragms in homogeneous dams with no other internal drainage system such as a chimney drain
# Filter Diaphragm or Collar for Outlet Works

## FEMA Filter Design Around Conduits

<table>
<thead>
<tr>
<th>Inside Diameter</th>
<th>Minimum Dimensions</th>
<th>Thickness (upstream to downstream)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Sides</td>
<td>Top</td>
</tr>
<tr>
<td>&lt; 2.5 ft</td>
<td>3 diameters</td>
<td>3 diameters</td>
</tr>
<tr>
<td>&gt; 2.5 ft</td>
<td>8 ft</td>
<td>8 ft</td>
</tr>
</tbody>
</table>
For a small homogeneous dam with no chimney drain, NRCS recommends a filter diaphragm (collar) around the outlet works conduit approximately 2/3 distance through the dam with outlet to the downstream toe for discharging collected seepage water.
Filter Diaphragm Placement
If the outlet works is located in a trench below the foundation level, the filter diaphragm should extend a short distance into the slopes of the excavation to intercept seepage that may follow the contact between earthfill and the natural foundation soil.
Some Practicalities

- Natural vs. processed materials
- Use of standard gradations
- Drain pipe gravel envelopes
- Geotextiles
Natural vs. Processed Materials

• Rare to find natural soils suitable for filters
  – Not “clean” enough
  – Can be gap graded
  – Gradations can vary significantly within the deposit
  – Can contain excessive coarse particles - segregation

• Readily available ASTM C33 fine aggregate is an excellent filter in almost all cases
ASTM C33 Fine Aggregate

- Suitable for most base soils
- Readily available
- Similar gradations can be used, if available at less cost
- Not suitable for some clays and silts (some Category 1 base soils) – soils with more than 85% finer than about 0.045 mm
ASTM C33 Fine Aggregate as a Filter

Note: Current version of ASTM C33 includes limits for No. 200 size
Fines Contents for Filters and Drains

- Recommend ≤2 to 3% in stockpile and ≤5% in place
- Some breakdown should be expected
- Permeability decreases dramatically with fines contents greater than 5%
Effect of Fines Content

![Graph showing the effect of fines content on permeability](image-url)
Economical for small quantities
Specify locally available sand and gravel materials that fall within the latitude of the filter requirements
Potential sources:
- State DOT specifications
- AASHTO gradations
- ASTM gradations
- Products of local aggregate producers
Verify local availability
Avoid cohesive fines
Gravel Envelopes Around Drain Pipes

- Slotted pipes embedded in filter sand often become plugged
- Full pipe capacity is not realized
Clogging of Slotted Drain Pipe Embedded in Sand Filter
Criteria for Pipe Perforations or Slots

<table>
<thead>
<tr>
<th>Bureau of Reclamation and Corps of Engineers</th>
<th>( D_{50} \geq \text{max. opening size} )</th>
</tr>
</thead>
<tbody>
<tr>
<td>Natural Resources Conservation Service</td>
<td>Non-critical: ( D_{85} \geq \text{max. opening size} )</td>
</tr>
<tr>
<td></td>
<td>Critical: ( D_{15} \geq \text{max. opening size} )</td>
</tr>
</tbody>
</table>

\( D_{85} = 85\% \) size of aggregate surrounding pipe

\( D_{15} = 15\% \) size of aggregate surrounding pipe
Slots or Holes

- Slots are preferred – less chance of clogging
- Not that important – either should work if sized correctly
Gravel Envelope Configurations

Other Configurations

4' min.

10' min.
Pipe Size

Pipe size is controlled by the most stringent of the following criteria:

• Minimum inner diameter
  – This requirement is for access for future camera inspection

• Estimated maximum flow rate should not exceed a flow depth of 75% of the pipe height.
  – This requirement is to account for post construction sags in the pipe alignment due to differential settlement.
  – Pipes should not flow full or be pressurized.
Geotextiles

• Susceptible to installation damage
• May clog or deteriorate
• Use in critical locations not allowed by USACE and Reclamation
• Published position of the NDSRB:
  – “It is the policy of the National Dam Safety Review Board that geotextiles should not be used in locations that are critical to the safety of the dam.” ¹

¹ Geotextiles in Embankment Dams, Status Report on the Use of Geotextiles in Embankment Dam Construction and Rehabilitation, FEMA, 2008
Seepage “Cut Off” Methods

- Grouting
- Low Permeability Blankets
- Barrier Walls

Note: Cut off is in quotes, because it is very difficult to truly cut off seepage – seepage reduction or seepage barriers may be better terms.
Some Cut Off (Barrier) Considerations

- Based on estimated seepage flow paths – how well are these known?
- Often involves underground construction, which cannot be directly observed.
- May create new seepage issues (e.g. high gradients at the bottom or edges of a barrier wall).
- May not require significant reservoir lowering.
- May not require dewatering.
Note: For existing dams, grouting in soils is NOT generally recommended due to potential hydraulic fracturing issues; caution needs to be exercised near top of rock and in weathered rock.
Grouting Cautions

- Limited soils for which grouting is effective.
- Soils (and soft rock) can be hydrofractured.
- Current practice is multiple lines.
- In rock, grout only penetrates water- and air-filled features – potential future erosion of remaining infilling.
- Grout may deteriorate over time.
- Grouting is sometimes considered to be a temporary solution.
Low Permeability Blankets

- Soil Blankets
- Geomembranes

Possible need for connection to existing water barrier in the embankment.
Low Permeability Blankets: Example 2

Ochoco Dam: Geomembrane – Surface Preparation
Placing the Membrane
Placing Protective Cover
Seepage Barrier Walls: Foundation and Embankment

- Embankment
- Pervious Foundation
- Impervious Foundation
Seepage Barrier Walls: Foundation and Partial Embankment

Wall needs to connect to water barrier in the embankment.
Seepage Barrier Walls: Foundation Only

Wall needs to connect to water barrier in the embankment.
90% penetration allows almost 40% of original flow.
Seepage Barrier Walls

- Continuous Trench Walls
- Soil Mix Walls
- Single Pass Walls
- Element Walls (Panels and Secant Piles)
- Jet Grouting Walls
- Sheet Pile Walls
Continuous Trench Wall Construction

Soil-Bentonite Barrier Wall

- Emplaced Backfill
- Slurry-Supported Excavation
- Unexcavated Soil
- Aquiclude
Continuous Trench Barrier Walls

**Salient Features**
- Low Cost/Rapid Const.
- Slurry Supported Excavation
- No Backfill Joints
- Non-Structural
- Low Permeability
- Depth up to ~85 feet with Backhoe

**Typical Backfills**
- Soil-Bentonite (SB)
- Cement-Bentonite (CB)
- Soil-Cement-Bentonite (SCB)
Excavators

- Long Boom/Long Stick Excavator
- Clamshell / Grab
Backfill Equipment

- Bulldozer and Excavator
- Mixing Box
- CB Mix Plant
- No Tremie Placement
  (except for unusual depths)
Keechelus Dam

- Modified in 2002 for seepage and internal erosion deficiencies.
- Soil-bentonite wall installed near right end of dam as part of larger modifications.
- Foundation (alluvial fan) sand and gravel deposits.
- Soil amended with fines for 'soil' part of S-B Wall.
Soil Bentonite Wall

Keechelus Dam

Creek
Keechelus Dam

Cross Section - Left

Soil Bentonite Wall
Keechelus Dam

S-B Wall

Section

Dives Away
Keechelus Dam – Excavator
Keechelus Dam: Delivery of 'Fines' Soil
Keechelus Dam: Fines Stock Pile
Keechelus Dam: Mixing
A.V. Watkins Dam – CB Wall

- Impounds Willard Reservoir.
- Dam is 14 miles long.
- Earthfill Structure.
- Maximum Ht. = 36 ft (approx. 20 ft in incident area).
- November 13, 2006; active piping was noticed at approx. sta. 639+00.
- Intervention was successful in preventing failure.
Cement Deep Soil Mixing (CDSM*)

Mixing in situ soils with cement grout or other slurries

- Multiple shaft mixing tools with cutting heads and mixing paddles, or
- Wheels on horizontal axis or trenching techniques
- Depths currently somewhat more than 100 feet

* aka DSM, DMM – some names are trademarked
RSW (Triple Auger)
Cutter Soil Mixing (CSM)
Cutter Soil Mixing (CSM) – Herbert Hoover Dike
Single Pass Methods

- Trench Cutting Remixing Deep (TRD) Wall Method
- DeWind OnePass Method

Depths currently somewhat more than 100 feet; working on machines for up to 150 feet.
TRD Method

Dike  Fill
Peat
Sands
Limestone Layers
Sands

Soil-Cement-Bentonite
TRD Wall Construction - Herbert Hoover Dike

Grout Batch Plant

TRD
Top of Exposed TRD Wall - Herbert Hoover Dike
Side of Exposed TRD Wall - Herbert Hoover Dike
Bentonite and cement are injected and mixed with the native soils to create SCB backfill.
DeWind OnePass
DeWind OnePass
Element Barrier Walls

- Installed in primary / secondary sequence.
- Panels and secant piles are common elements.
- Can extend to great depths, but alignment control, joint integrity, and backfill quality can be challenging.
  - Pilot holes and guided equipment have been used to address alignment.
Primary / Secondary Sequence

6 ft

P S P

9'-2"

PLAN

5” min overlap

Embankment

Alluvium

Rock

Cavity

PROFILE
Element Wall Configurations

Primary Elements
Installed First

Secondary Elements
Fill Between
Panel Element Equipment

Grab or Clam
Shell
Panel Element Equipment

Cable-Clam Bucket
Panel Element Equipment

Hydromill (Hydrofraise)

1 Hydromill
2 Suction pump
3 Mud processing plant
4 Slurry tank
5 Slurry pump
6 Screened cuttings
7 Slurry pump
8 Bentonite mixing plant
Secant Pile Element Equipment
Secant Pile Element Equipment

Wirth Drill
Secant Pile Element Equipment

Wirth Drill
Some Notable Element Barrier Walls

- Wolf Creek Dam – Kentucky (270’)
- Beaver Dam – Arkansas
- Mud Mountain Dam – Washington (420’)
- Walter F. George Dam – Alabama (210’)
- Navajo Dam – New Mexico (400’)
- Fontenelle Dam (180’)
- Center Hill Dam

About 400’ deepest to date, but technology is advancing.
Types of Solution Features at Wolf Creek Dam Addressed with an Element Barrier Wall
Jet Grouting - Features

- Suitable in a wide range of soils and applications.
- Columns with diameters ranging from 60 cm up to 250 cm (and perhaps more), by using small size drilled holes.
- Capability to overpass pre-existing masonry, boulders, rocky layers and obstructions.
- Use of light weight and small-sized drilling rigs able to operate in limited working areas.
Jet Grouting - Commentary

• Jet grouting is a soil improvement method.
• Jet grouted soil can work in compression and shear, not in tension.
• Jet grouted soil can be reinforced, but it is not a concrete structure.
• Depth limited only by drilling capability, but alignment and continuity will be a concern at large depths.
• Expensive on a per volume basis.
Jet Grout Working Sequence

1. Drilling
2. Jetting from bottom-up
3. Columns completed
Three Categories of Jet Grouting

- **SINGLE FLUID System**: cement grout is used as disaggregating and consolidating fluid (T1) – standard diameter achievable 40-100 cm.

- **DOUBLE FLUID System**: cement grout plus air are used as disaggregating and consolidating fluid (T1/S) – standard diameter achievable 80-250 cm.

- **TRIPLE FLUID System**: water plus air are used as disaggregating fluid, while cement grout is used as consolidating fluid (T2) – standard diameter achievable 120-300 cm.
Jet Grouting Shapes

Elements are generally either columns or panels, obtained by retrieving the jetting monitor with simultaneous rotation or with no rotation, respectively.
Sheetpiles

Conventional sheetpiles can be used for temporary or permanent seepage barriers.

- Sheetpiles can be either steel or plastic (vinyl).
- Issues include:
  - corrosion of steel sheetpile.
  - buckling of steel or plastic sheetpile.
  - lack of advancement.
  - interlock separation.
  - interlock leakage.
- Realistic depths of 100 feet or less.
Sheetpiles
Sheetpiles

Can be treated to reduce leakage.

Interlock
Vinyl Piles
Vinyl Piles
Deer Flat Dam
Deer Flat Dam

Steel sheetpiles used as a temporary water barrier in a sand and gravel cofferdam.
Deer Flat Dam
Closing

• Dam engineers have a wide range of tools available for seepage rehabilitation.
• The challenge is to consider, with an open mind, the range of options and select the “best” choice for a particular dam.
• Robustness, redundancy, and resiliency should be duly considered in the selection.
Questions?